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VILLAGE OF DOWNERS GROVE Report for the Village 2/6/2018

SUBJECT:	SUBMITTED BY:
2410 Ogden Avenue - Special Use	Stan Popovich, AICP Director of Community Development

SYNOPSIS

The petitioner is requesting Special Use approval to operate an automobile dealership at 2410 Ogden Avenue.

STRATEGIC PLAN ALIGNMENT

The goals for 2017-2019 include Strong and Diverse Local Economy.

FISCAL IMPACT

n/a

RECOMMENDATION

Approval on the February 13, 2018 active agenda per the Plan Commission's unanimous 6:0 positive recommendation. The Plan Commission found that the proposal is an appropriate use in the district, compatible with the Comprehensive Plan and meets all standards for approval of a Special Use per Section 28.12.050.

BACKGROUND

Property Information & Zoning Request

The petitioner is proposing to redevelop the site of an existing one-story vacant building (formerly a restaurant) with a new two-story, 8,500 square-foot automobile dealership. The proposed building will be near the center of the lot and parking will wrap around the building on all sides with automobile display concentrated in the rear. The eastern curb cut onto Ogden Avenue will be removed, while the western curb cut will be improved.

The new building's modern design will consist primarily of steel, masonry and concrete. The street facing elevation includes tinted window panes, framing, metal clad panels and upwards slanted eaves. The southern half of the side elevations maintain the window panes, and the north half incorporates two different variations of metal clad panels. The rear elevation is mostly brown colored metal clad paneling with horizontal lines and glazed overhead doors leading to the service area.

The interior will consist of a front sales area, interior vehicle display, service bays, customer waiting area, and reception. The smaller second floor will also have offices and a common area. The rear service area has five vehicle bays for maintenance, washing/detailing, and a photographic marketing area.

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Compliance with the Comprehensive Plan

The Comprehensive Plan's Future Land Use Map designates this property as Corridor Commercial. The plan calls for the concentration of auto dealerships within this western section of the corridor and to promote uses that have a regional draw. The proposal also achieves the Comprehensive Plan's aesthetic, site and screening improvement goals. The proposed use at this property is consistent with the Comprehensive Plan.

Compliance with the Zoning Ordinance

The property is zoned B-3, General Services and Highway Business. An automobile dealership use is an allowable Special Use in the B-3 zoning district. The new building and redeveloped site will be compliant with the required B-3 bulk standards. The trash enclosure will be screened, and the photometric plan demonstrates that the light trespass will not exceed code requirements. The proposed use and site is consistent with the Zoning Ordinance.

Engineering\Public Improvements

Post Construction Best Management Practices (PCBMPs) are not required since the proposal results in a decrease in impervious area. The existing detention area and utility connections will remain the same. The traffic study concluded that the single full movement access drive will be adequate in accommodating the projected traffic and onsite vehicle deliveries. The impact on the existing road network will be minimal. The owner will dedicate land to be incorporated into the Ogden Avenue right-of-way. A public sidewalk and an internal pedestrian connection is proposed.

Public Comment

No member of the public spoke at the public hearing. After the Plan Commission meeting, two members of the public contacted staff and expressed concerns regarding traffic, light glare, and the general number of automobile dealerships in the Village. Staff explained and reviewed the proposal including the site plan, traffic study, landscaping/screening and photometric plans with the residents.

ATTACHMENTS

Ordinance Aerial Map Staff Report with attachments dated January 8, 2018 Draft Minutes of the Plan Commission Hearing dated January 8, 2018

2410 Ogden Special Use – 17-PLC-0039

ORDINAN	CE NO.	•

AN ORDINANCE AUTHORIZING A SPECIAL USE FOR 2410 OGDEN AVENUE TO PERMIT AN AUTOMOBILE DEALERSHIP

WHEREAS, the following described property, to wit:

LOTS 18, 19, 20, 21 AND 22 IN BLOCK 1 IN ARTHUR T. MCINTOSH AND CO'S FOURTH OGDEN AVENUE SUBDIVISION, BEING A SUBDIVISION IN THE SOUTH HALF OF SECTION 1, TOWNSHIP 38 NORTH, RANGE 10, EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO THE PLAT THEREOF RECORDED APRIL 9, 1925 AS DOCUMENT 190962, IN DUPAGE COUNTY, ILLINOIS.

Commonly known as: 2410 Ogden Avenue, Downers Grove, IL 60515

PINs: 08-01-303-014; -015; -016; -017

(hereinafter referred to as the "Property") is presently zoned in the "B-3, General Services and Highway Business District" under the Comprehensive Zoning Ordinance of the Village of Downers Grove; and

WHEREAS, such petition was referred to the Plan Commission of the Village of Downers Grove, and said Plan Commission has given the required public notice, has conducted a public hearing for the petition on January 8, 2018 and has made its findings and recommendations, all in accordance with the statutes of the State of Illinois and the ordinances of the Village of Downers Grove; and,

WHEREAS, the Plan Commission has recommended approval of the Special Use, subject to certain conditions; and,

WHEREAS, the Village Council finds that the evidence presented in support of said petition, as stated in the aforesaid findings and recommendations of the Plan Commission, is such as to establish the following:

- 1. That the proposed use is expressly authorized as a Special Use in the district in which it is to be located;
 - 2. That the proposed use at the proposed location is necessary or desirable to provide a service or a facility that is in the interest of public convenience and will contribute to the general welfare of the neighborhood or community.
 - 3. That the proposed use will not, in the particular case, be detrimental to the health, safety or general welfare of persons residing or working in the vicinity or be injurious to property values or improvements in the vicinity.

NOW, THEREFORE, BE IT ORDAINED by the Council of the Village of Downers Grove, in DuPage County, Illinois, as follows:

<u>SECTION 1</u>. That Special Use of the Property is hereby granted to allow construction of an automobile dealership.

<u>SECTION 2.</u> This approval is subject to the following conditions:

- 1. The Special Use shall substantially conform to the staff report dated January 8, 2018; engineering drawings prepared by Damas Consulting Group dated December 1, 2017 and last revised on December 20, 2017 and architectural drawings prepared by Phorma Designs, Inc. dated December 1, 2017 and last revised December 14, 2017, except as such plans may be modified to conform to the Village codes and ordinances.
- 2. All test drives are limited to arterial streets as defined in the Comprehensive Plan.
- 3. All vehicle deliveries must be completed on private property. Vehicles may not be dropped off or picked up on Ogden Avenue.
- 4. A pedestrian connection must be provided from the public right-of-way to the main building entrance in conformance with the Zoning Ordinance.
- 5. The building shall be equipped with an automatic fire suppression system and an automatic and manual fire alarm system.
- 6. Prior to the issuance of a building permit, an administrative lot consolidation shall be completed that dedicates additional IDOT right-of-way.
- 7. A sidewalk easement shall be granted to the Village.

SECTION 3. The above conditions are hereby made part of the terms under which the Special Use is granted. Violation of any or all of such conditions shall be deemed a violation of the Village of Downers Grove Zoning Ordinance, the penalty for which may include, but is not limited to, a fine and/or revocation of the Special Use granted herein.

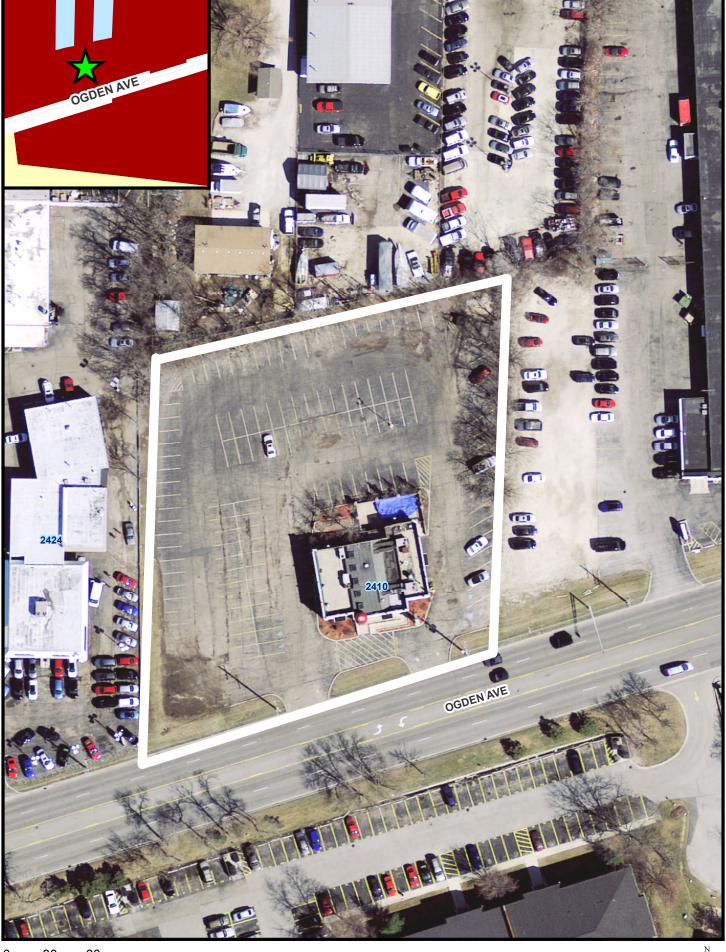
<u>SECTION 4</u>. It is the Petitioner's obligation to maintain compliance with all applicable Federal, State, County and Village laws, ordinances, regulations, and policies.

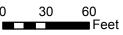
<u>SECTION 5</u>. That all ordinances or parts of ordinances in conflict with the provisions of this ordinance are hereby repealed.

		Mayor
Passed:		-
Published:		
Attest:		
	Village Clerk	

1\mw\ord.18\SU-2410-Ogden-17-PLC-0039

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VILLAGE OF DOWNERS GROVE REPORT FOR THE PLAN COMMISSION JANUARY 8, 2018 AGENDA

SUBJECT:	TYPE:	SUBMITTED BY:
17-PLC-0039		Scott Williams, AICP
2410 Ogden Avenue	Special Use	Planner

REQUEST

The petitioner is requesting approval of a Special Use to construct a personal vehicle sales business at 2410 Ogden Avenue.

NOTICE

The application has been filed in conformance with applicable procedural and public notice requirements.

GENERAL INFORMATION

OWNER: Agri-Pes, LLC

> 857 Willow Lane Willowbrook, IL 60527

Agent: Anas Alkhatib APPLICANT:

> Agri-Pes, LLC 857 Willow Lane Willowbrook, IL 60527

PROPERTY INFORMATION

EXISTING ZONING: B-3, General Services and Highway Business

EXISTING LAND USE: Vacant Restaurant PROPERTY SIZE: 65,030 sq. ft. (1.5 acres)

PINS: 08-01-303-014, -015, -016, -017

SURROUNDING ZONING AND LAND USES

	ZONING	FUTURE LAND USE
NORTH:	B-3, General Services and Highway Business	Corridor Commercial
	M-1, Light Manufacturing	Corridor Commercial
South:	B-3, General Services and Highway Business	Corridor Commercial
WEST:	B-3, General Services and Highway Business	Corridor Commercial
EAST:	B-3, General Services and Highway Business	Corridor Commercial

ANALYSIS

SUBMITTALS

This report is based on the following documents, which are on file with the Department of Community Development:

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- 1. Project Narrative
- 2. Plat of Survey
- 3. Plat of Consolidation
- 4. Engineering Plans
- 5. Architectural Plans
- 6. Landscape Plan
- 7. Photometric Plan
- 8. Traffic Study

PROJECT DESCRIPTION

The owner of the property is proposing to construct a personal vehicle sales business at the property located on the north side of Ogden Avenue, approximately 385 feet east of Cross Street, commonly known as 2410 Ogden Avenue. The property is zoned B-3, General Services and Highway Business. Personal vehicle sales business is an allowable Special Use in the B-3 zoning district.

Currently, the subject property is improved with a one-story vacant building (formerly a restaurant) and a surface parking lot. The property consists of five lots of record. The site is accessed currently by two existing Ogden Avenue curb-cuts.

Proposed Development

Site Design: The petitioner is proposing to construct a two-story, 8,500 square foot building. The proposed building will be sited in the center of the lot, and the parking will wrap around the building on all sides. Multiple rows of parking are shown in the rear yard. A trash enclosure is located to the side of the primary structure at the northeast corner.

The existing eastern Ogden Avenue curb-cut has been eliminated. Turning exhibits demonstrate that the curb-cut and drive aisle widths are sufficient for onsite vehicle unloading/loading. An internal pedestrian connection leads from the main entrance to the proposed public sidewalk.

Parking: The parking area will consist of 92 parking spaces including display vehicles, employee parking, service parking and customer parking. The outdoor automobile display and service parking is shown in the rear and western side yards. Employee parking is concentrated in the eastern side yard. Guest or customer parking is located in the street yard.

Elevations: The new building's modern design will consist primarily of steel, masonry and concrete. The south elevation includes tinted window panes, framing, metal clad panels and upwards slanted eaves. The southern half of the side elevations maintain the window panes, and the north half incorporates two different variations of metal clad panels. The rear elevation is mostly a brown colored metal clad paneling with vertical lines and glazed overhead doors leading to the service area. Parapet walls fully screen the rooftop mechanical units.

Floor Plans: The interior will consist of a front sales area, interior vehicle display, service bays, customer waiting area, and reception. The smaller second floor will also have offices and a common area. The rear service area has five vehicle bays for maintenance, washing/detailing, and a photographic marketing area.

Landscaping/Screening: The petitioner is proposing landscaping around the perimeter of the property and interior islands, in conformance with the Village requirements. The new trash area will be screened with a metal clad panel gate and walls over concrete masonry unit (CMU) block walls. Additional landscaping will be planted between the enclosure and Ogden Avenue. The submitted photometric plan indicates that the new light fixtures will not adversely spill over on the adjacent properties.

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COMPLIANCE WITH THE COMPREHENSIVE PLAN

The Comprehensive Plan's Future Land Use Map designates this property as Corridor Commercial. The plan calls for the concentration of auto dealerships within this western section of the corridor and to promote uses that have a regional draw. The petitioner's redesigned site plan reduces the number of curb cuts, includes a dumpster enclosure, new parking lot landscape islands and the beautification of Ogden Avenue with dense landscaping, all of which are Comprehensive Plan goals.

The proposed personal vehicle sales business at this property is consistent with the Comprehensive Plan.

COMPLIANCE WITH ZONING ORDINANCE

The property is zoned B-3, General Services and Highway Business. The proposed personal vehicle sales business is an allowable Special Use in the B-3 District per Section 5.010 of the Zoning Ordinance. The new building and redeveloped site will be compliant with the required B-3 bulk standards. The table below identifies the required regulations and what is proposed:

2410 Ogden Avenue	Required	Proposed
South Setback (Street Yard –		
Ogden Avenue) - Building	75 ft.	99 ft.
East Setback (Side Yard) -		
Building	0 ft.	75 ft.
West Setback (Side Yard) -		
Building	30 ft.	63 ft.
North Setback (Rear Yard) -		
Building	0 ft.	92 ft.
South Setback (Street Yard		
Ogden Avenue) - Parking	50 ft.	50.5 ft.
Landscaped Open Space	6,503 sf. (10%)	12,648 sf. (20%)
Street yard landscaped open space	3,251 sf	6,261 sf
Floor Area Ratio	0.75 (max)	0.13
Building Height	60 ft. (max)	27.75 ft.
Parking Spaces	21	92

The proposed use and site is consistent with the Zoning Ordinance.

ENGINEERING/PUBLIC IMPROVEMENTS

Post Construction Best Management Practices (PCBMPs) are not required since the proposal results in a decrease in impervious area. There is no existing floodplain or wetlands on the property. The existing detention in the southwest corner of the site is to be maintained. Storm sewers will connect to the existing detention area and outlet to an existing storm sewer along Ogden Avenue.

The existing water service and sanitary services will be replaced. The Sanitary District provided conceptual approval for the proposed redevelopment.

Vehicle deliveries will be made on site by vehicle carriers. No vehicle deliveries or other business activities will be permitted to take place on Ogden Avenue. The traffic study concluded that the single full movement access drive will be adequate in accommodating the traffic projected to be generated by the proposed development. The study also stated there will be a low volume of traffic generated during

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peak travel hours, and the impact on adjacent levels of service will be minimal. Staff concurs with the findings of the traffic study. The proposed personal vehicle sales business and site plan has received conceptual approval from IDOT.

The petitioner is proposing a sidewalk along Ogden Avenue. Because it would be constructed on private property, enacting a sidewalk easement is a condition of approval. This easement will be conveyed through the required plat of consolidation.

PUBLIC SAFETY REQUIREMENTS

The Fire Prevention Division has reviewed the proposed plans and will require the building include a fire alarm and sprinkler system that meet the Village's code requirements. The proper fire department connection and hydrant are shown on the plans.

The proposed development provides sufficient access for emergency vehicles. The site layout permits Fire Department apparatus the opportunity to enter and exit the site from the Ogden Avenue curb cut. The loop around the building provides good access around the building and property as needed.

NEIGHBORHOOD COMMENT

Notice was provided to all property owners 250 feet or less from the property in addition to posting public hearing notice signs and publishing the legal notice in the *Downers Grove Suburban Life*. Staff has received one informational inquiry.

FINDINGS OF FACT

The petitioner is requesting a Special Use to construct a personal vehicle sales business at 2410 Ogden Avenue. Staff finds that the proposal meets the standards for granting a Special Use as outlined below:

Section 28.12.050.H Approval Criteria

No special use may be recommended for approval or approved unless the respective review or decision-making body determines that the proposed special use is constituent with and in substantial compliance with all Village Council policies and plans and that the applicant has presented evidence to support each of the following conclusions:

1. That the proposed use is expressly authorized as a Special Use in the district in which it is to be located;

The property is located in the B-3, General Service and Highway Business zoning district. Under Section 5.010 of the Zoning Ordinance, personal vehicle sales business is listed as an allowable Special Use in the B-3 zoning district. This standard has been met.

2. That the proposed use at the proposed location is necessary or desirable to provide a service or a facility that is in the interest of public convenience and will contribute to the general welfare of the neighborhood or community.

The proposed plan will allow the petitioner to redevelop a vacant site which in turn will enhance the Ogden Avenue corridor and provide vehicle sales and services to the local residents, businesses and the larger region. The proposed use is in the interest of the public convenience and will contribute to the general welfare of the area by providing growth and employment opportunities. The petitioner's proposed use will meet various Comprehensive Plan goals. This standard has been met.

3. That the proposed use will not, in the particular case, be detrimental to the health, safety or general welfare of persons residing or working in the vicinity or be injurious to property values or improvements in the vicinity.

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The proposed use will not be detrimental to the health, safety or general welfare of persons residing in or working in the vicinity and will not be injurious to property values or improvements in the vicinity. The petitioner will be redeveloping a vacant property. The proposed development will meet all applicable Village regulations including the Stormwater Ordinance. Additionally, specific conditions will be placed on the subject property to ensure that there will be no or minimal secondary impacts to the surrounding properties, including loading/unloading and test driving restrictions. This standard has been met.

RECOMMENDATIONS

The proposed Special Use for personal vehicle sales business at 2410 Ogden Avenue is consistent with the Comprehensive Plan, the Zoning Ordinance and surrounding zoning and land use classifications. Based on the findings listed above, staff recommends the Plan Commission recommend the Village Council **approve** the Special Use as requested in case 17-PLC-0039 subject to the following conditions:

- The Special Use shall substantially conform to the staff report; engineering drawings prepared by Damas Consulting Group dated December 1, 2017 and last revised on December 20, 2017 and architectural drawings prepared by Phorma Designs, Inc. dated December 1, 2017 and last revised December 14, 2017, except as such plans may be modified to conform to the Village codes and ordinances.
- 2. All test drives are limited to arterial streets as defined in the Comprehensive Plan.
- 3. All vehicle deliveries must be completed on private property. Vehicles may not be dropped off or picked up in Ogden Avenue.
- 4. A pedestrian connection must be provided from the public right-of-way to the main building entrance in conformance with the Zoning Ordinance.
- 5. The building shall be equipped with an automatic suppression system and an automatic and manual fire alarm system.
- 6. An administrative lot consolidation shall be completed prior to the issuance of a building permit.
- 7. A sidewalk easement shall be granted to the Village.

Staff Report Approved By:

Stanley J. Popovich, AICP

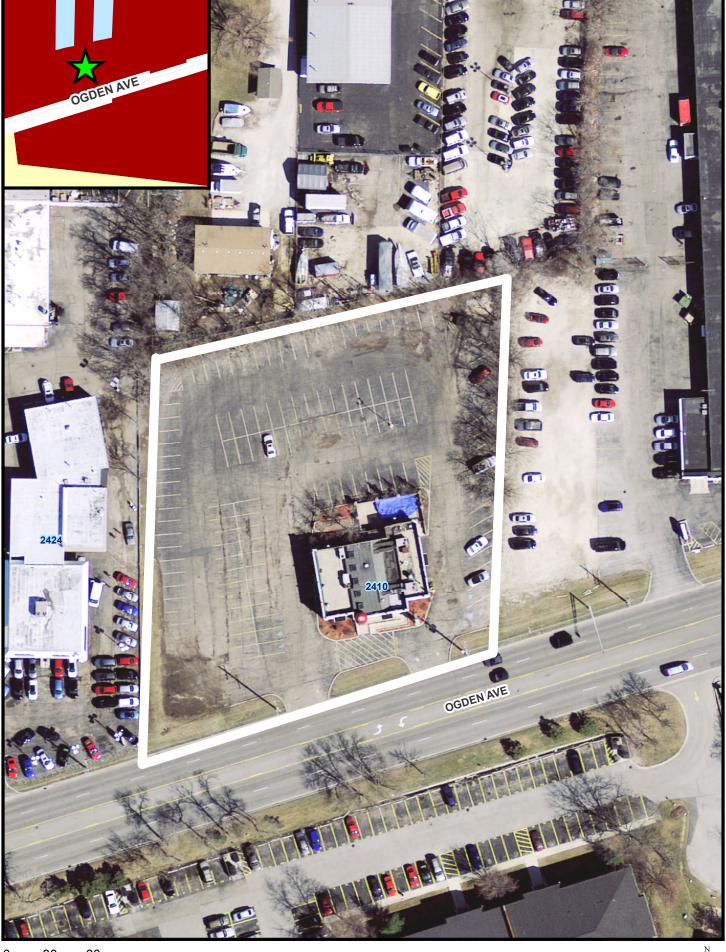
Director of Community Development

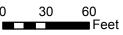
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Phorma Designs, Inc.

Architectural & Planning

2092 Gardner Circle E. Suite 1 Aurora, IL. 60503 Tel: (630)229-6498 Fax: (630)982-3795

BRIEF PROJECT DESCRIPTION

The proposed building project located at 2410 Ogden Ave. in Downers Grove, IL property is designated to be a new Auto Dealership in the business of selling and buying vehicles along with minor maintenance towards vehicles. The said property is currently zoned as a B3 business district with the applicant applying for special use of the property. The site of approximately 46,000 square feet shall be remodeled with new curb and parking pavement appeal with additional required new landscaping and parking lot lighting.

The proposed Auto Dealership gross building of approximately 8,500 square feet shall be constructed of but not limited to concrete, masonry, steel, metal clad panels and drywall materials. All finishes shall consist of commercial grade designation. The structure shall be of steel, concrete and masonry construction along with new mechanical HVAC systems of RTU's placed on roof and interior space heaters hung from roof joists in auto bay area. New plumbing domestic water and separate fire suppression water lines shall be put in place for the new building along with new electrical panels and transformer feed if required.

The proposed Auto Dealership shall consist of a sales area, interior vehicle display, customer waiting area, reception and conference/office areas along with employee break room space. The other portion of the proposed building shall consist of a 5 vehicle bays for maintenance, washing/detailing and photo marketing area.

The dealership will operate from 10 AM to 8 AM (Monday through Friday) and 11 AM to 6 PM (Saturday and Sunday).

The building will include 5 employees in sales and management and 2 in the service area. The delivery of vehicles will be scheduled on a weekly bases and all loading and unloading of the inventory shall take place within the dealership lot.

All employees are to park east of the building only. And the day to day operations shall include an Auto retail (selling and buying) and some minor service to the dealership own inventory (no Auto service will be open for the public). Detailing of cars and photographing of new Auto to be conducted on the site.

Listed below is additional site and building information as follows –

Project Description

This project consists of 1.50-acres. This site is located in Dupage County North Along Ogden Ave . The address of the property is 2400 Ogden Ave Downers Grove, IL. The proposed development will be a new Auto Dealership with parking lot.



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Phorma Designs, Inc.

Architectural & Planning

2092 Gardner Circle E. Suite 1 Aurora, IL. 60503 Tel: (630)229-6498 Fax: (630)982-3795

Existing Conditions

The subject property existing lot is used for the exiting restaurant building and parking lot, The existing building and parking lot to be removed There is mo existing flood plain or wetland on the lot.

Proposed Development

The proposed development is to include new building with foot print of 7000 SF and proposed parking lot with total of 101 parking spaces. The development is not going to have any negative impact to the existing storm water runoff.

Floodplains and Wetlands

There is no jurisdictional wetland or floodplain on the lot. There is not LPDA on the site.

Storm Water Management

SITE AREA = 1.50 ACRE

The Existing Site Condition Impervious area= 54,628 SF

Building=3737 SF

Concrete Sidewalk/Concrete patio=1583 SF

Asphalt Parking lot=49,308 SF

Green area=10,760 SF

-Proposed site condition decreases impervious area from the existing condition, PCBMP is not required for this site

<u>Proposed Site condition:</u>

Total Site Area = 1.5 ACRE

Area breakdown:

Building = 7000 SF Concrete Sidewalk/Concrete pad = 870 SF Asphalt Parking lot = 42,710 SF Green area = 14,808 SF

Total Impervious area = 50,580 SF

The site out flow to the culvert located at the southeast corner of the lot. The existing detention area southwest of the site to be maintained (no change) the site out flow will be connected to the existing storm manhole.



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Narrative

Storm Management Submittal

Project Description

This project consists of 1.50-acres. This site is located in Dupage County North Along Ogden Ave . The address of the property is 2400 Ogden Ave Downers Grove, IL. The proposed development will be a new Auto Dealership with parking lot.

Existing Conditions

The subject property existing lot is used for the exiting restaurant building and parking lot, The existing building and parking lot to be removed There is mo existing flood plain or wetland on the lot.

Proposed Development

The proposed development is to include new building with foot print of 7000 SF and proposed parking lot with total of 101 parking spaces. the development is not going to have any negative impact to the existing storm water runoff.

Floodplains and Wetlands

There is no jurisdictional wetland or floodplain on the lot. There is not LPDA on the site.

Storm Water Management

SITE AREA = 1.50 ACRE
The Existing Site Condition Impervious area= 54,628 SF
Building=3737 SF
Concrete Sidewalk/Concrete patio=1583 SF
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Green area=10.760 SF

-Proposed site condition decreases impervious area from the existing condition, PCBMP is not required for this site

Proposed Site condition:

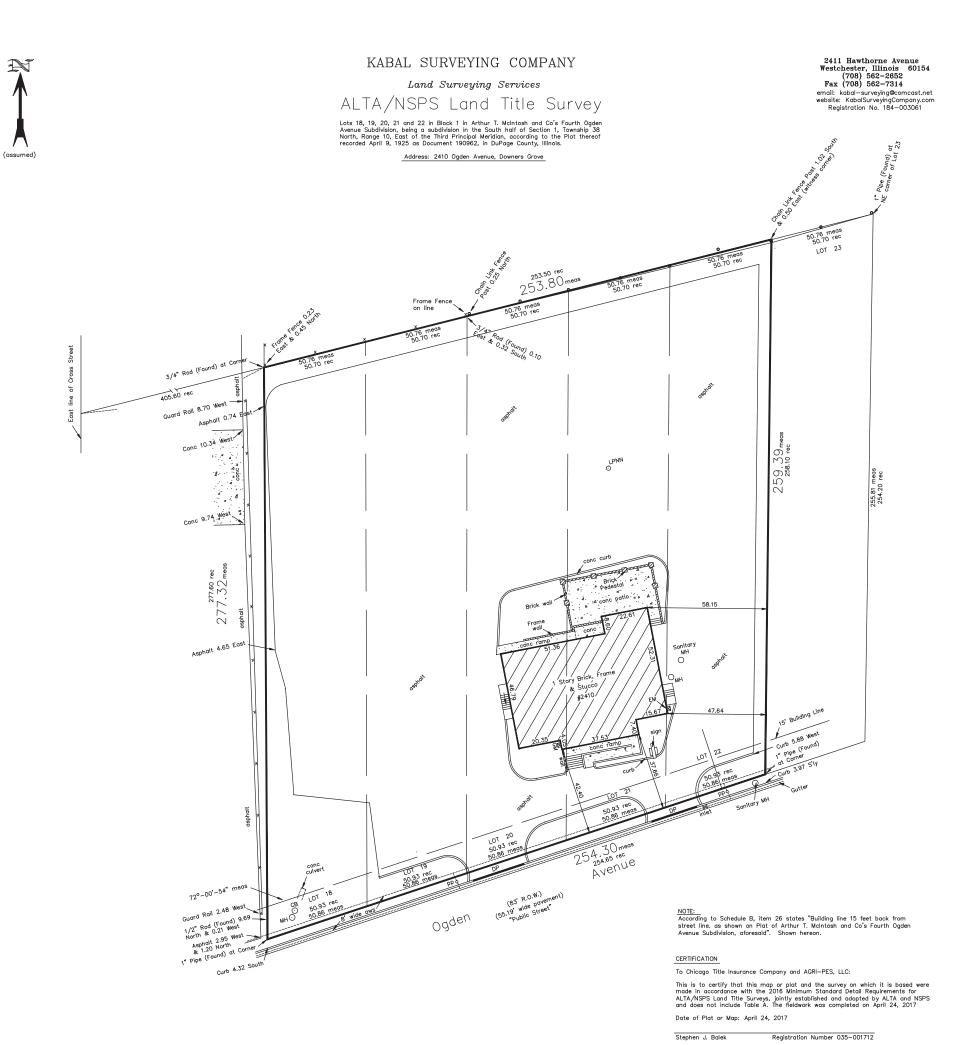
Total Site Area = 1,5 ACRE

Area breakdown:
Building=7000 SF
Concrete Sidewalk/Concrete pad=870 SF
Asphalt Parking lot=42,710 SF
Green area=14,808 SF

Total Impervious area=50,580 SF

The site out flow to the culvert located at the southeast corner of the lot. The existing detention area southwest of the site to be maintained (no change) the site out flow will be connected to the existing storm manhole

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LEGEND

meas = measured, S = South, pp = power pole
rec = record, E = East, W = West, LP = light pole
R.O.W. = right-of-way, BS = bumper stop
conc = concrete, wv = water valve, MH = manhole
pch = porch, N = North, EM = electric meter
DP = depressed curb, GM = gas meter
aw = aerial wire, CB = catch basin Area of property is approximately 65,360 square feet

X" in box indicates that hereon drawn plat was ordered as a non-monumented survey

Please check Legal Description with Deed and report any discrepancy immediately.

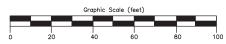
Surveyed _____ April 24

170405 Order No. Ordered By: Wade Joyner, Attorney at Law



Chicago Title Insurance Company Order No. 17SA3716035AU Effective Date: February 16, 2017 Proposed Insured: AGRI-PES, LLC

ORIGINAL SEAL IN RED



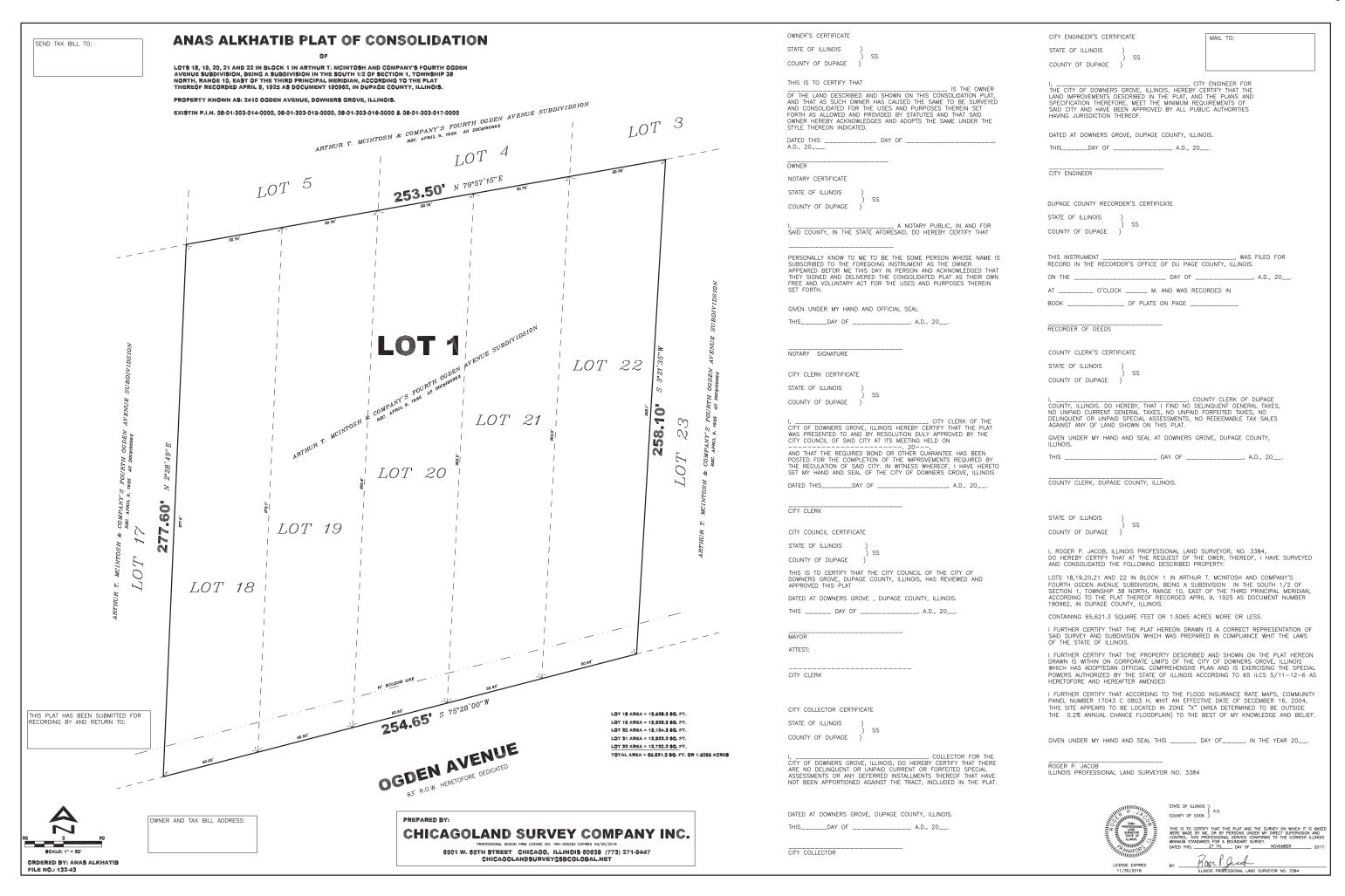
This professional service conforms to the current Illinois minimum standards for an ALTA/NSPS survey

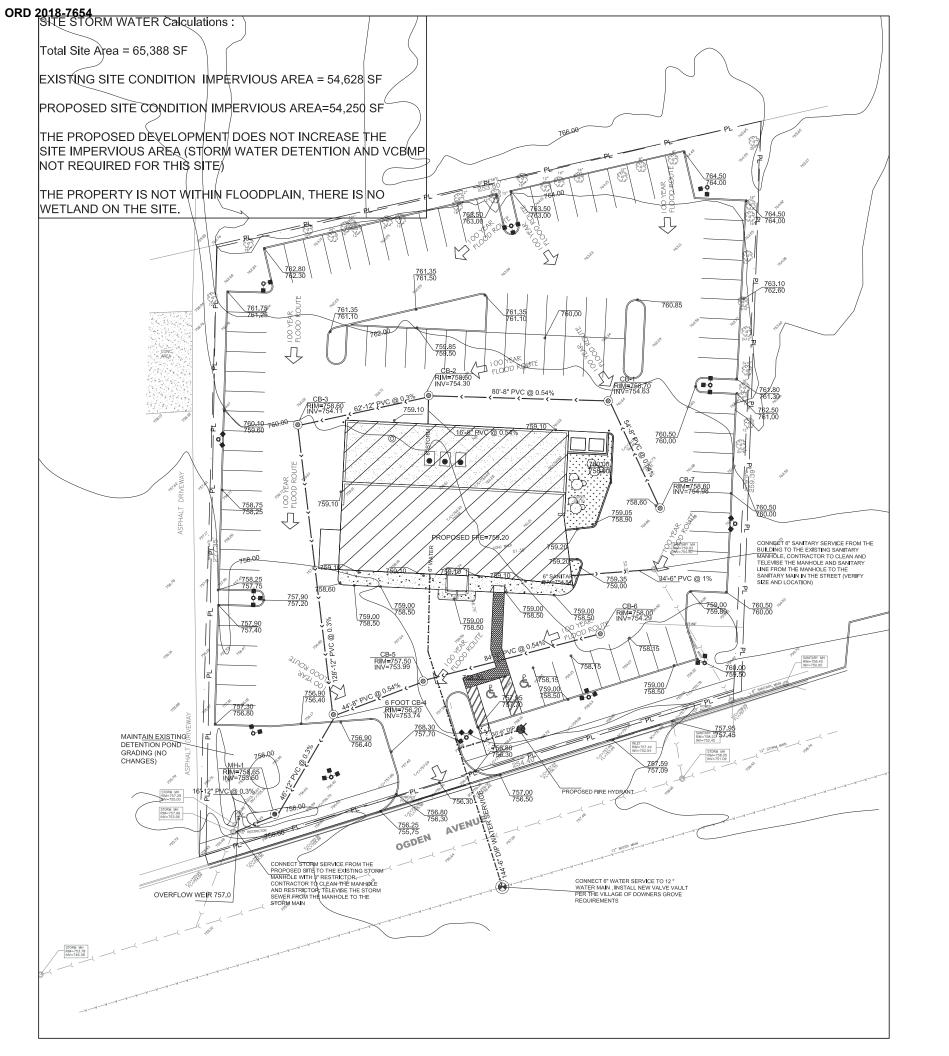
l, STEPHEN J. BALEK, an Illinois Professional Land Surveyor, hereby certify that I have surveyed the property described above and the plat hereon drawn is a correct representation of said survey.

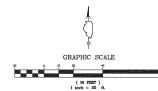
Dimensions are in feet and decimal parts thereof and are corrected to a temperature of 62 degrees Fahrenheit.

Illinois Professional Land Surveyor No. 035-001712 My license expires on November 30, 2018

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LEGEND AND ABBREVIATIONS:

—— PL ——	PROPERTY LINE
	LIMITS OF FENCE LINE
	STORM LINE
—	SANITARY LINE
_v _v _v _v _v _v _v _	WATER LINE
FFE	FINISHED FLOOR ELEVATION
ıн-x ⊚	CATCH BASIN (CB)
IM 733.00 NV 730.00	FLARED END SECTION (FE)
1	MANHOLE (MH)
⊕	VALVE VAULT
€(FIRE HYDRANT
733.50 733.00	TOP OF CURB BOTTOM OF CURB
732.00	SPOT ELEVATION
722 FO TO	TOP OF CURB

UTILITY NOTES:

- I. THE EXISTENCE AND LOCATION OF UNDERGROUND UTILITIES SHALL BE OBTAINED FROM ALL UTILITY COMPANIES, INVESTIGATED AND VERIFIED IN THE FIELD BY THE CONTRACTOR BEFORE STARTING WORK IN THE CONSTRUCTION AREA. EXCAVATION IN THE VICINITY OF EXISTING STRUCTURES SHALL BE PERFORMED BY HAND. THE CONTRACTOR SHALL BE HELD RESPONSIBLE FOR ANY AND ALL DAMAGES TO EXISTING FACILITIES, MAINTENANCE AND PROTECTION OF EXISTING UTILITIES AND STRUCTURES.
- 2. THE CONTRACTOR IS TO UNCOVER ALL LINES BEING TIED INTO AND VERIFY GRADES BEFORE ANY CONSTRUCTION.
- 3. CALL JULIE (800)892-0123 PRIOR TO DIGGING FOR ANY UTILITY CONSTRUCTION.
- 4. ALL DISTURBED AREAS SHALL BE RESTORED BY THE CONTRACTOR TO THE ORIGINAL CONDITION.
- 5. THE CONTRACTOR MUST COMPLY WITH ALL FEDERAL, STATE AND LOCAL CODES.
- 6. THE CONTRACTOR SHALL OBTAIN ALL NECESSARY PERMITS AND PAY THE REQUISITE FEES TO THE CITY OF LOMBARD PRIOR TO COMMENCING WORK
- 7. ALL WORK AND MATERIALS WHICH DO NOT CONFORM TO THE SPECIFICATIONS ARE SUBJECT TO REMOVAL AND REPLACEMENT AT THE CONTRACTOR'S EXPENSE.
- 8. WATERMAIN SHALL BE DUCTILE IRON PIPE, CLASS 52, CEMENT LINED.
- 9. SEWER SHALL CONFORM TO ASTM SPECIFICATIONS FOR MATERIALS AND JOINTS, PVC USED TO BE SDR 26 CONFORMING TO ASTM-3034, JOINT TO CONFIRM TO ANSI A21.11
- 10. DO NOT SCALE PLANS FOR CONSTRUCTION DIMENSIONS.
- 11. A WATER MAIN SHALL BE SEPARATED FROM A SEWER SO THAT ITS INVERT IS A MINIMUM OF 18 INCHES ABOVE THE CROWN OF THE SEWER WHENEVER WATER MAINS CROSS STORM SEWERS, SANITARY SEWERS OR SEWER SERVICE CONNECTIONS. THE VERTICAL SEPARATION SHALL BE MAINTAINED FOR THAT PORTION OF THE WATER MAIN LOCATED WITHIN TEN FEET HORIZONTALLY OF ANY SEWER OR DRAIN CROSSED. A LENGTH OF WATER MAIN PIPE SHALL BE CENTERED OVER THE SEWER TO BE CROSSED WITH JOINTS EQUIDISTANT FROM THE SEWER OR DRAIN.
- 2. BOTH THE WATER MAIN AND SEWER SHALL BE CONSTRUCTED OF SLIP-ON OR MECHANICAL JOINT CAST OR DUCTILE IRON PIPE EQUIVALENT TO WATER MAIN STANDARDS OR CONSTRUCTION WHEN IT IS IMPOSSIBLE TO OBTAIN THE PROPER VERTICAL SEPARATION AS DESCRIBED ABOVE, OR THE WATER MAIN PASSES UNDER A SEWER OR DRAIN.
- 13. A VERTICAL SEPARATION OF 18 INCHES BETWEEN THE INVERT OF THE SEWER OR DRAIN AND THE CROWN OF THE WATER MAIN SHALL BE MAINTAINED WHERE A WATER MAIN CROSSES UNDER A SEWER. SUPPORT THE SEWER OR DRAIN LINES TO PREVENT SETTLING AND BREAKING THE WATER MAIN.
- 14. CONSTRUCTION SHALL EXTEND ON EACH SIDE OF THE CROSSING UNTIL THE PERPENDICULAR DISTANCE FROM THE WATER MAIN TO THE SEWER OR DRAIN OR SEWER SERVICE CONNECTION SHALL BE THE SAME AS WATER MAIN SEPARATION DESCRIBED ABOVE.
- 15. CONTRACTOR IS TO RESTORE ALL STREET PAVEMENT PER VILLAGE OF DOWNERS GROVE REQUIREMENTS.



07-15-2017	AA
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Page 17 of 98

Damas Consulting Group 5625 MIDDAUGH AVE Downers Grove, IL. 60516 h 630-991-3299 FAX 630-541-2382

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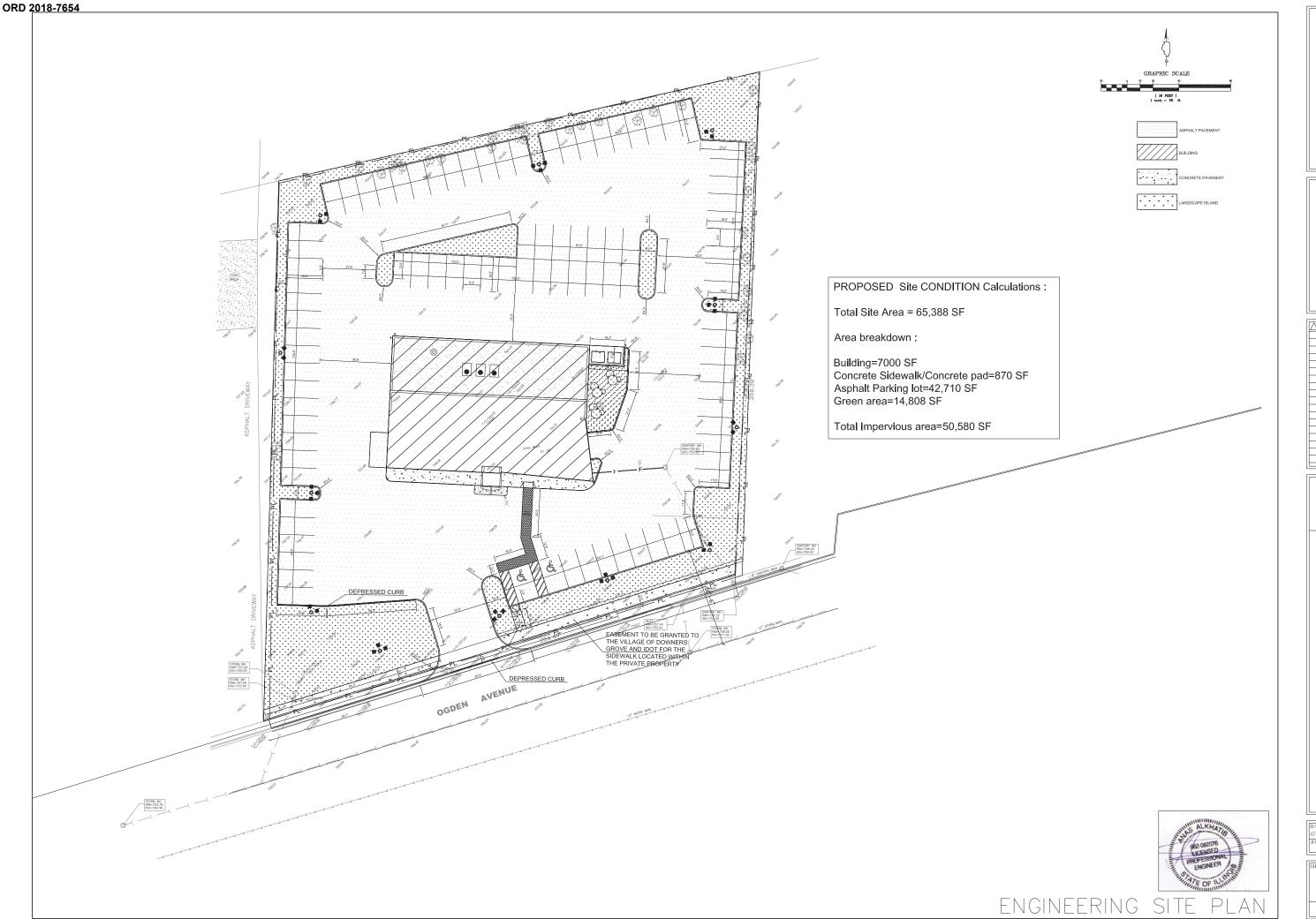
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GRADING AND UTILITIES



Damas Consulting Group
5625 MIDDAUGH AVE
Downers Grove, IL. 60516
Ph 630-991-3299 FAX 630-541-2382

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DATE	DRAWING ISSUE
12-1-17	ISSUED FOR ZONING
12-20-17	ISSUED FOR ZONING
	12-1-17

DEALERSHIP AVE. 60515 OGDEN A GROVE, CAR 2410 W. DOWNERS G PROPOSED

DATE STARTED:	DRAWN BY:
07-15-2017	AA
JOB NO:	FILE NO:

ORD 2018-7654

2410 W. OGDEN AVE, DOWNERS GROVE

PROPOSED SCOPE: DEMOLISH EXISTING BUILDING, DESIGN AND BUILD NEW CAR DEALERSHIP PER APPLICABLE CODES ADOPTED BY THE CITY OF DOWNERS GROVE

DRAWINGS INDEX

- A1 PROPOSED SITE PLAN, APPLICABLE CODES, DRAWINGS INDEX
 F1 PROPOSED VEHICLE TURNING EXHIBIT
 L1 PROPOSED LANDSCAPE PLAN
 A2 PROPOSED FLOOR PLANS
 A3 PROPOSED EXTERIOR ELEVATIONS
 A4 PROPOSED BUILDING SECTIONS
 A5 PROPOSED UGHTING PHOTOMETRICS
 A6 PROPOSED BUILDING PERSPECTIVES

CODES ADOPTED BY THE CITY OF DOWNERS GROVE

- CODES ADOPTED BY THE CITY OF DOWNERS GROVE

 1. CURRENT DOWNERS GROVE ZONING ORDINANCE
 2. CURRENT DOWNERS GROVE STORMWATER AND FLOOD PLAIN ORDINANCE
 3. 2015 INTERNATIONAL BUILDING CODE WITH D.G. AMENDMENTS
 4. 2014 NATIONAL ELECTRICAL CODE WITH D.G. AMENDMENTS
 5. CURRENT STATE OF ILLINOINS PLUMBING CODE WITH D.G. AMENDMENTS
 6. 2015 INTERNATIONAL MECHANICAL CODE WITH D.G. AMENDMENTS
 7. 2015 INTERNATIONAL FUEL GAS CODE WITH D.G. AMENDMENTS
 8. 2015 INTERNATIONAL FUEL GAS CODE WITH D.G. AMENDMENTS
 8. 2015 INTERNATIONAL ENERGY CONSERVATION CODE WITH D.G. AMENDMENTS
 9. 2015 INTERNATIONAL PROPERTY MAINTENANCE CODE WITH D.G. AMENDMENTS
 10. 2015 INTERNATIONAL FIRE CODE WITH D.G. AMENDMENTS
 11. 2015 INTERNATIONAL FIRE CODE WITH D.G. AMENDMENTS
 12. 2003 LITERNATIONAL SWIMMING POOL AND SPA CODE WITH D.G. AMENDMENTS
 12. 2003 LITE SAFETY CODE (MFPA 101)
 13. CURRENT STATE OF ILLINOIS ACCESSIBILITY CODE

SITE A	ND BUILDING DA	ATA	
OCCUPANCY USE	(B) BUSINESS	CONSTRUCTION TYPE	TYPE 2B
LAND USE, ZONING	COMMERCIAL, B3		
LOT SIZE	65,030 SF	1.49 ACRES	
PROPOSED BUILDING GROSS AREA	7,000 SF		
TOTAL PROPOSED FIRST FLOOR AREA	6,376 SF		
PROPOSED SHOWROOM AREA	3,523 SF	2 SPACES PER 1,000 SQ. FT.	7 SPACES
PROPOSED SECOND FLOOR	1,300 SF		
LOT COVERAGE	11%		
LANDSCAPE AREA & PERCENTAGE	12,648 SF (20%)		
STREET YARD OPEN SPACE	6,261 SF (9.6%)		

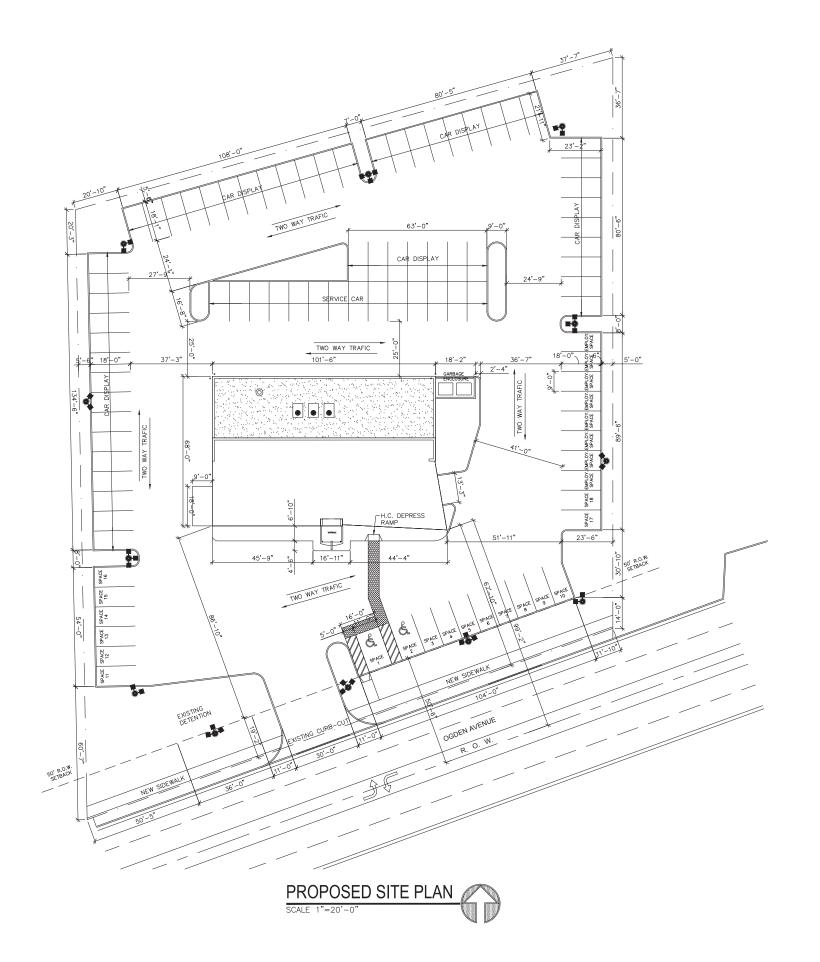
COMMERCIAL VEHICLE SALES PARKING REQUIREMENTS				
DESCRIPTION	AREA SF	CODE	REQUIRED	PROVIDED
SHOWROOM AREA	3,523	2 SPACES PER 1,000 SF OF SHOWROOM AREA	7	5
OUTDOOR DISPLAY AREA	8,424	0.4 SPACES PER 1,000 SF OF OUTDOOR AREA	4	52
SERVICE AREA	5 BAYS	2 SPACES PER SERVICE BAY.	10	14
GUEST PARKING		NOT INCLUDED IN OVERALL SUBTOTAL	-	26
EMPLOYEE PARKING		NOT INCLUDED IN OVERALL SUBTOTAL	-	8
		TOTA	21	71

* MINIMUM MOTOR VEHICLE PARKING REQUIREMENT AS FOLLOWS;

- 2 SPACES PER 1,000 SQ. FT. OF SHOWROOM AREA, PLUS 0.4 SPACES
1,000 SQ.FT. OF OUTDOOR DISPLAY SPACE, PLUS 2 CARS PER SERVICE
BAY

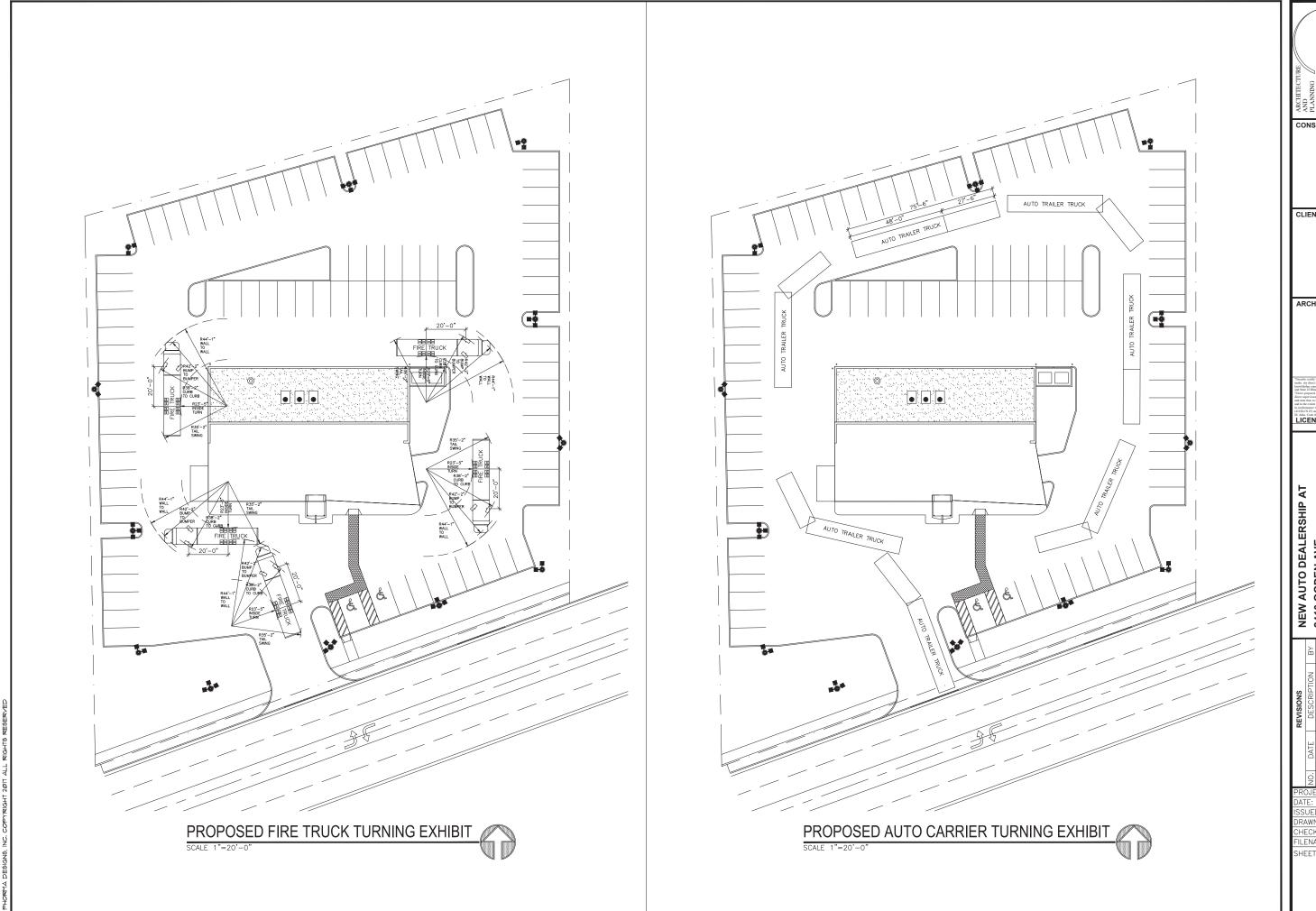
GENERAL NOTES

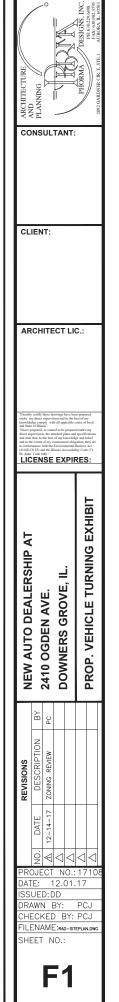
- ALL WORK SHALL CONFORM TO THE APPLICABLE CODES AND ORDINANCES OF THE VILLAGE OF DOWNERS GROVE & THE STATE OF ILLINOIS ACCESSIBILITY CODE
- 2. ALL WORK SHALL CONFORM TO THE SPECIFICATIONS AND QUALITY STANDARDS AS EXPRESSED IN THE DRAWINGS WHICH FORM A PART OF THE CONTRACT DOCUMENTS
- START OF THE WORK BY THE CONTRACTORS SHALL SIGNIFY THE ACCEPTANCE OF THE EXISTING SITE CONDITIONS
- ALL DEBRIS AND WASTE MATERIALS AND EQUIPMENT SHALL BE TRANSPORTED OFF OF THE PREMISES AND LEGALLY DISPOSED OF. ALL ITEMS SHALL BECOME THE PROPERTY OF THE CONTRACTOR AND DISPOSAL OF SUCH ITEMS SHALL BECOME THEIR RESPONSIBILITY EXCEPT FOR EXISTING MATERIALS PROPOSED FOR RE—USE
- 5. THE ARCHITECT SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES OR FOR ANY SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THIS PROJECT
- 6. THE ARCHITECT SHALL NOT BE RESPONSIBLE FOR ANY ADDITIONAL EXPENDITURES THAT MIGHT BE NECESSARY AFTER CONSTRUCTION BEGINS



Page 19 of 98 CONSULTANT: CLIENT: ARCHITECT LIC.: LICENSE EXPIRES: NEW AUTO DEALERSHIP A 2410 OGDEN AVE. DOWNERS GROVE, IL. DATE: 12.01.17 SSUED: DD RAWN BY: PCJ HECKED BY: PCJ TLENAME:rad-siteplan.di SHEET NO.:

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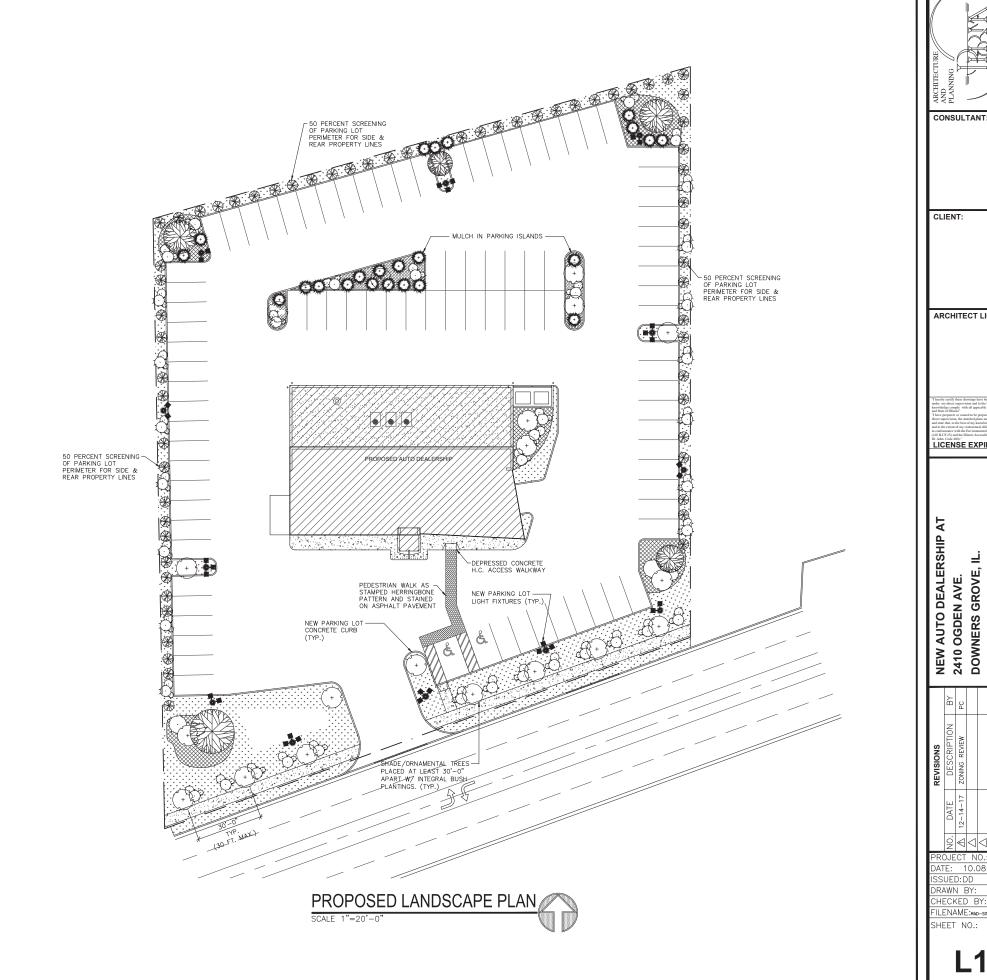
ORD 2018-7654

LEGEND OF SYMBOLS									
SYMBOL	NAME	ACTUAL NAME	MATURE HEIGHT						
	CANOPY TREE	AUTUMN BLAZE RED MAPLE	30-40 FT.						
	EVERGREEN TREE	EMERALD GREEN ARBOVITAE	12-14 FT.						
+	+ UNDERSTORY TREE		10-15 FT.						
	EVERGREEN SHRUB	GREEN VELVET BOXWOOD	4-6 FT.						
8	DECIDUOUS SHRUB	DIABLO NINE BARK	6-8 FT.						
	MULCH	HARDWOOD MU TO 3 INCHES SOIL PLANTING	DEEP WITHIN						
	PERENNIALS		3 INCHES						
	GRASS	CLASS 1 LAWN PER IDOT SPE FOR SEED W/ SOD PLANTING	CIFICATION STRAW OR						
	PROPERTY LINE								
	SETBACK LINE								
	CENTER RIGHT OF WAY								
	BUILDINGS								
	CONCRETE SIDEWALK								

NOTE:
THE ABOVE LANDSCAPE BLOCKS DEPICT THE WIDTH OF
THE CANOPY AT MATURITY. LANDSCAPE PLANS
SUBMITTED TO THE CITY OF AURORA FOR APPROVAL
MUST BE SCALED TO DEPICT THE WIDTH OF CANOPY
AT THE TIME OF INSTALLATION.

LANDSCAPE NOTES:

- 1) ALL ORNAMENTAL TREES SHALL BE AT LEAST 4
 FEET IN HEIGHT AT THE TIME OF INSTALL.
 2) ALL SHADED TREES SHALL BE AT LEAST 2.5 INCH
 CALIPER AT THE TIME OF INSTALL.
 3) ALL PLANTING SCHRUBS SHALL BE AT LEAST 8
 INCHES IN HEIGHT AT THE TIME OF INSTALL.
 4) ALL ORNAMENTAL PLANTINGS (GRASSES AND
 PERENNIALS SHALL BE AT LEAST 12 INCHES IN
 HEIGHT AT THE TIME OF INSTALL.



DATE: 10.08.17 SSUED:DD DRAWN BY: PCJ

HECKED BY: PCJ FILENAME:rad-siteplan.du SHEET NO .:

Page 21 of 98

CONSULTANT:

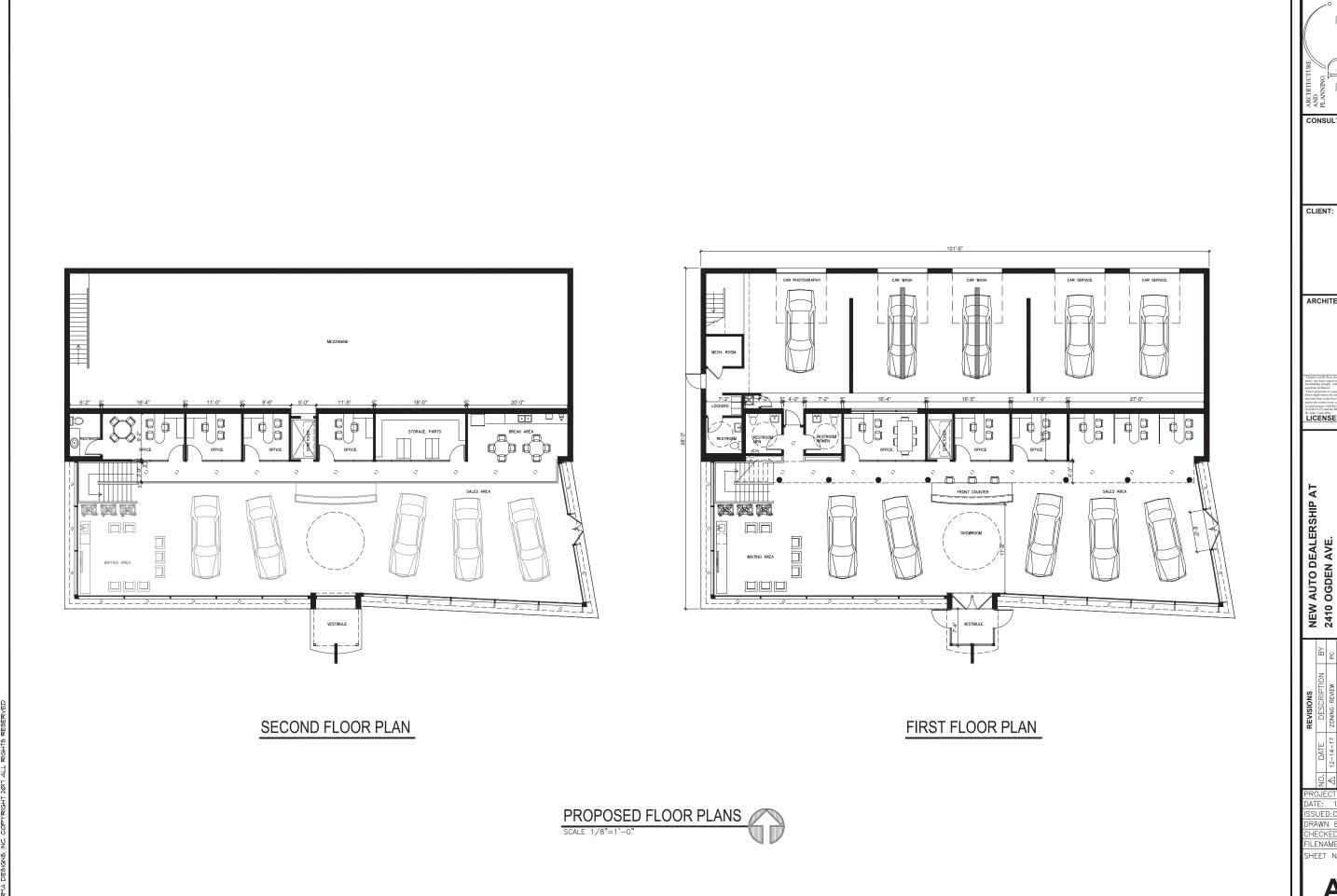
CLIENT:

ARCHITECT LIC.:

LICENSE EXPIRES:

PROPOSED LANDSCAPE PLAN

ORD 2018-7654



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ARCHITECT LIC.:

LICENSE EXPIRES:

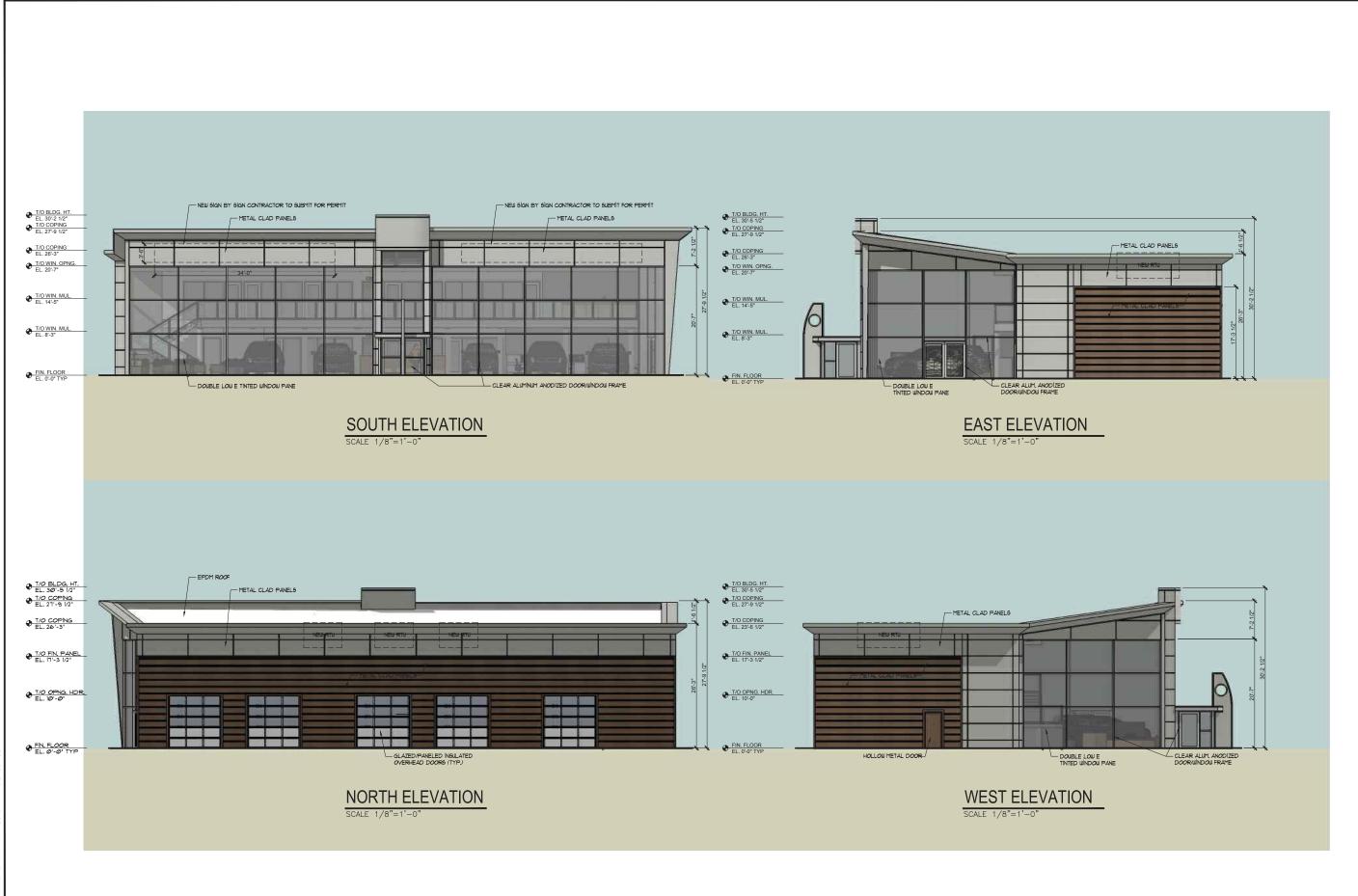
NEW AUTO DEALERSHIP AT 2410 OGDEN AVE. DOWNERS GROVE, IL. PROPOSED FLOOR PLANS

HECKED BY: PCJ

TLENAME:rad-xbasepl

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ORD 2018-76<u>5</u>4 Page <u>2</u>3 of



Page 23 of 98

ARCHITECTURE
AND
PLANNING
PLANNING
PROBLEM OF IL STE. 1

CONSULTANT:

CLIENT:

ARCHITECT LIC.:

Throby certify these duratings have been prepared sakes my fixest supervision and to the best of say knowledge comply with all apprisable codes of local and State of Illimas.¹

These prepared, or caused to be prepared under my decet supervision, the attacked plans and specifications and the contract of the prepared to the contract of the and to the center of my contractent adoption, they are in customuses with the Erricommontal Baniers. Act (4) 01.18.25 years and the Illimas Accordable Yorke (7)

and to the extent of my contentual obligation, they are in conformance with the Eminemental Businst and (410 ILCS 25) and the Illinois Accessibility Code (71 Ill. Adm. Code 400).**

LICENSE EXPIRES:

NEW AUTO DEALERSHIP AT
2410 OGDEN AVE.
DOWNERS GROVE, IL.
PROPOSED EXTERIOR ELEVATIONS

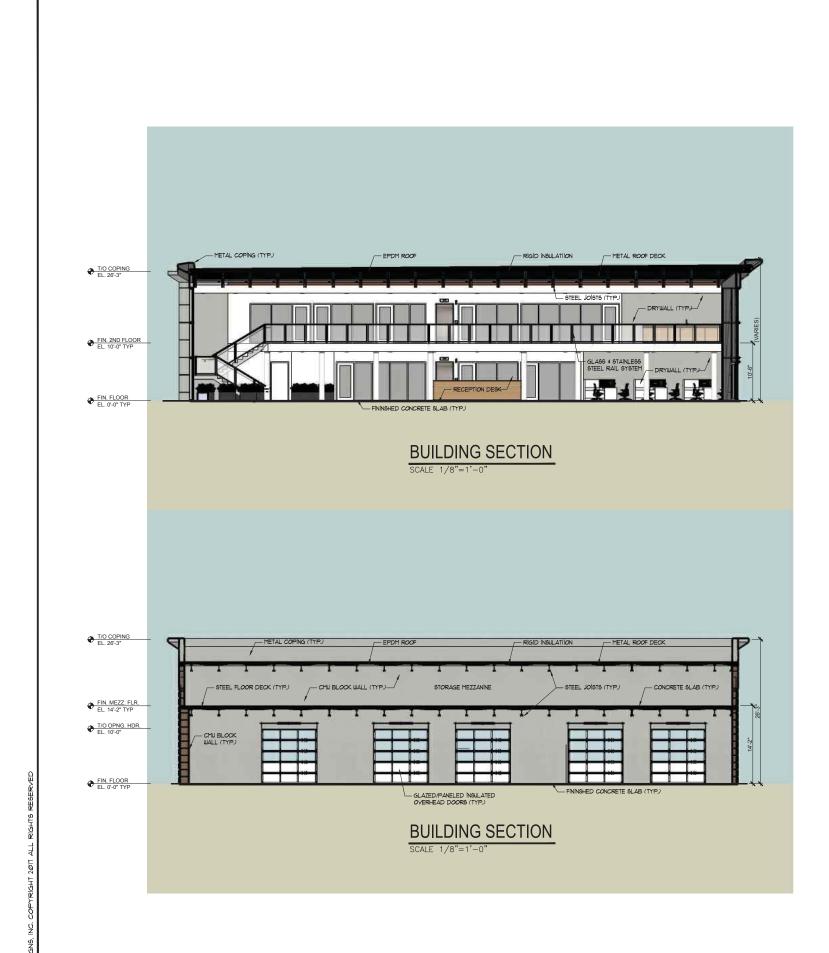
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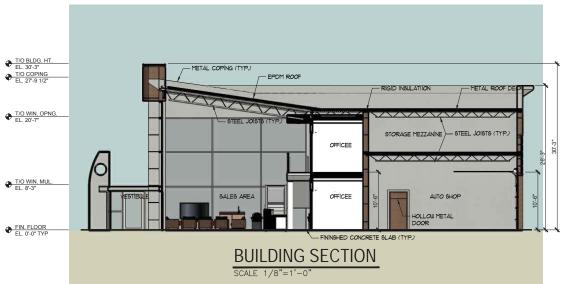
PROJECT NO.:1710 DATE: 12.01.17 ISSUED: DD DRAWN BY: PCJ CHECKED BY: PCJ

FILENAME:rad-xbase elevi.d\u00e4c SHEET NO.:

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DUMPSTER ENCLOSURE ELEVATION

SCALE 1/8"=1'-0"

CONSULTANT: CLIENT: ARCHITECT LIC.: LICENSE EXPIRES: PROPOSED BUILDING SECTIONS NEW AUTO DEALERSHIP AT 2410 OGDEN AVE. DOWNERS GROVE, IL.

DATE: 12.01.17
ISSUED: DD
DRAWN BY: PCJ
CHECKED BY: PCJ
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SHEET NO.:

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HIGHLINE S2

PRECISION SERIES WALL PANELS

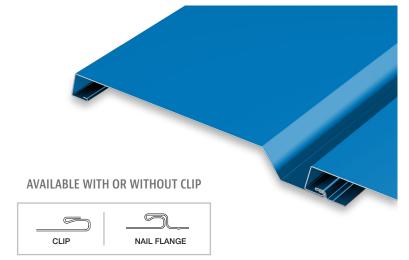
MATERIALS

.050 aluminum 22 gauge steel

SPECS

15.356" Wide 1-3/8" High





PRODUCT FEATURES

- No-clip panel or clip installation for expansion/contraction
- Multiple rib patterns provide a variety of looks and design options
- Panel depth of 1-3/8"
- ▶ Cost-effective installation
- ▶ Horizontal or vertical installation

Panel lengths: 30' maximum for steel; 22' maximum for aluminum; longer lengths available on clip panels; 4' min. steel and aluminum

MATERIAL

- ▶ 15 stocked colors (22 gauge steel)
- ▶ 29 stocked colors (.050 aluminum)
- ▶ Galvalume Plus available

TESTS

- ASTM E283
- ASTM E331

FLORIDA BUILDING PRODUCT APPROVALS

Please refer to pac-clad.com or your local factory for specific product approval numbers for Precision Series panels.

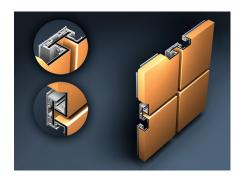
Note: Line drawings may not be to scale.

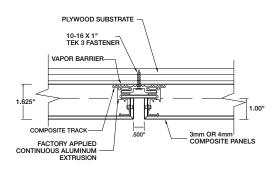


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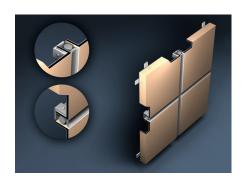
COMPOSITE WALL PANELS

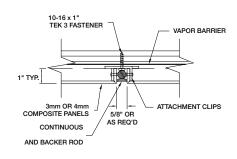
PAC-3000 RS





PAC-3000 CS





PRODUCT FEATURES

- Available in a wide variety of non-PAC-CLAD colors and finishes
- ► Consult Petersen rep for color options (extra fee applies for PAC-CLAD colors)
- Precise fabrication to meet exacting tolerances
- ▶ Rout-and-return fabrication
- ▶ Welded corners available

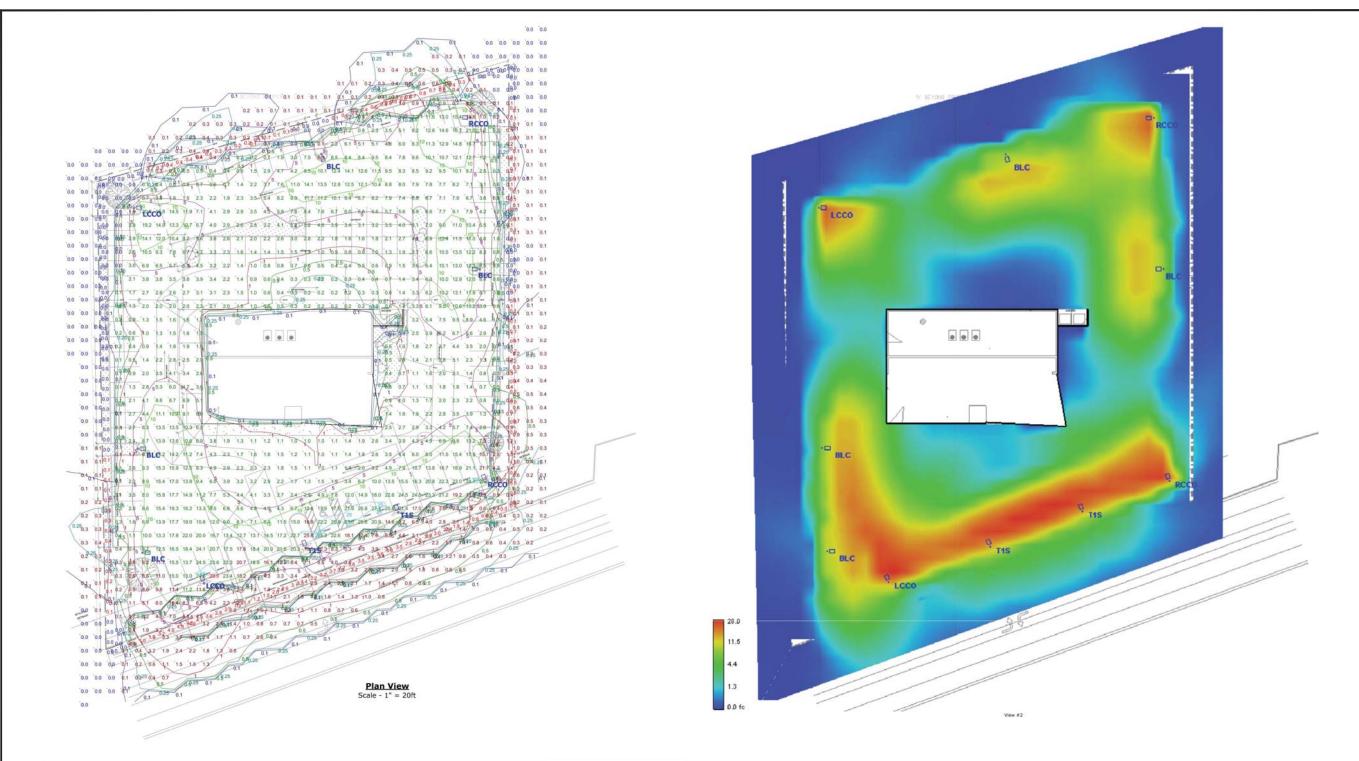
MATERIALS

- 3mm, 4mm, 6mm Composite
- ▶ .063 .125 Mill Finish Aluminum
- Zinc
- ▶ Stainless steel
- Anodized aluminum

TESTS

- ▶ ASTM E283*
- ▶ ASTM E330*
- ASTM E331*
- * Composite material only

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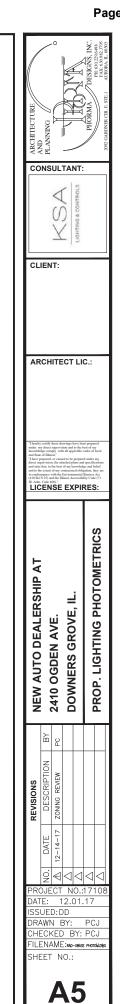
Schedule											
Symbol	Label	Quantity	y Manufacturer	Catalog Number	Description	Lamp	Number Lamps	Filename	Lumens Per Lamp	Light Loss Factor	Wattage
	T1S	2	Lithonia Lighting	DSX2 LED P8 40K T1S MVOLT	DSX2 LED P8 40K T1S MVOLT	LED	1	DSX2_LED_P8_40K_T1S_MVOLT.ies	49133	0.95	431
	BLC	4	Lithonia Lighting	DSX2 LED P8 40K BLC MVOLT	DSX2 LED P8 40K BLC MVOLT	LED	1	DSX2_LED_P8_40K_BLC_MVOLT.ies	40324	0.95	431
	RCCO	2	Lithonia Lighting	DSX2 LED P8 40K RCCO MVOLT	DSX2 LED P8 40K RCCO MVOLT	LED	1	DSX2_LED_P8_40K_RCCO_MVOLT.ies	30005	0.95	431
	LCCO	2	Lithonia Lighting	DSX2 LED P8 40K LCCO MVOLT	DSX2 LED P8 40K LCCO MVOLT	LED	1	DSX2_LED_P8_40K_LCCO_MVOLT.ies	30005	0.95	431

Statistics						
Description	Symbol	Avg	Max	Min	Max/Min	Avg/Min
1. PAVED AREAS ONLY - GRADE LEVEL	ж	6.1 fc	28.0 fc	0.0 fc	N/A	N/A
2. WHOLE PARCEL TO PROPERTY LINES - GRADE LEVEL	+	5.7 fc	28.0 fc	0.0 fc	N/A	N/A
3. BEYOND PROPERTY LINE - 15' ZONE	\Q	0.5 fc	4.2 fc	0.0 fc	N/A	N/A
4. WEST PROPERTY LINE		0.1 fc	0.4 fc	0.0 fc	N/A	N/A
5. NORTH PROPERTY LINE		0.3 fc	0.8 fc	0.0 fc	N/A	N/A
6. EAST PROPERTY LINE		0.3 fc	1.2 fc	0.0 fc	N/A	N/A
7. SOUTH PROPERTY LINE (OGDEN AVE.)		2.4 fc	4.9 fc	0.0 fc	N/A	N/A

NOTES:

- CALCULATION POINTS ARE AT GRADE LEVEL.
 FIXTURES ARE MOUNTED AT 23'-0" ABOVE GRADE (20' POLES ON 3' CONCRETE BASES).
 CACULATIONS PROVIDED ARE NOT A GUARANTEE OF PERFORMANCE. ACTUAL LIGHT LEVELS

**This document contains confidential and proprietary information of KSA Lighting & Controls. This is document may only be used by or for the benefit of KSA Lighting & Controls representatives and customers. FOR LIGHTING DESIGNS This lighting design is not a professional engineering drawing and is provided for informational purposes only, without warranty as to accuracy, completeness, reliability or otherwise. KSA Lighting & Controls is not responsible for specifying the lighting or eligination and interest in a professional engineering advisor to determine whether this lighting design meets the applicable project requirements. It is the obligation of the end-user to consult with a professional engineering advisor to determine whether this lighting design extra lighting design and in a custometric performance project requirements. But is the obligation of the end-user or custometric performance in a calculated photometric performance to differ from the calculated photometric performance to differ from the calculated by our controls and office and in a calculated photometric performance to differ from the calcul





BLC Optics – (From Above)





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CONSULTANT

CLIENT:

ARCHITECT LIC.:

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NEW AUTO DEALERSHIP AT 2410 OGDEN AVE. DOWNERS GROVE, IL. PROPOSED BUILDING PERSPECTIVES

PROJECT NO.:17
DATE: 12.01.17
ISSUED: DD

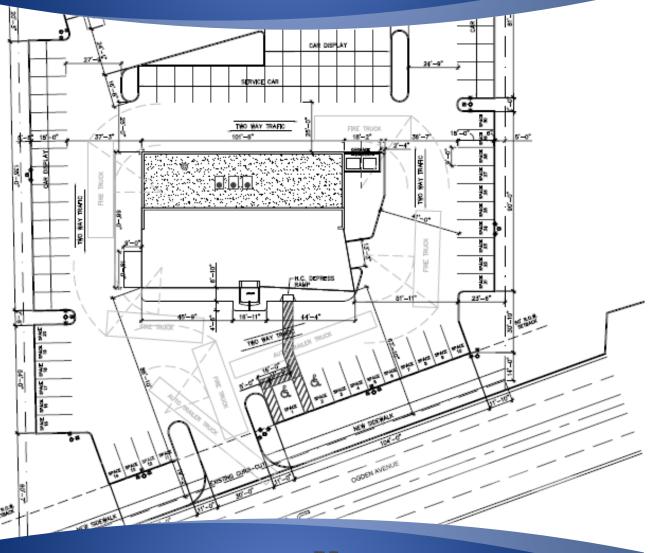
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December 12, 2017

1. Introduction

This report summarizes the methodologies, results, and findings of a traffic impact study conducted by Kenig, Lindgren, O'Hara, Aboona, Inc. (KLOA, Inc.) for the proposed used car dealership to be located on the north side of Ogden Avenue approximately 600 feet east of Cross Street in Downers Grove, Illinois. As proposed, the site will be developed with an approximately 7,000 square-foot building with an approximately 3,500 square-foot showroom and a five-bay service center. Access to the development will be provided via the existing westerly full movement curb cut serving the site. A total of 101 parking spaces will be provided with 30 spaces reserved for guest parking.

The purpose of this study was to examine background traffic conditions, assess the impact that the proposed development will have on traffic conditions in the area, and determine if any roadway or access improvements are necessary to accommodate traffic generated by the proposed development.

Figure 1 shows the location of the site in relation to the area roadway system. **Figure 2** shows an aerial view of the site area.

The sections of this report present the following:

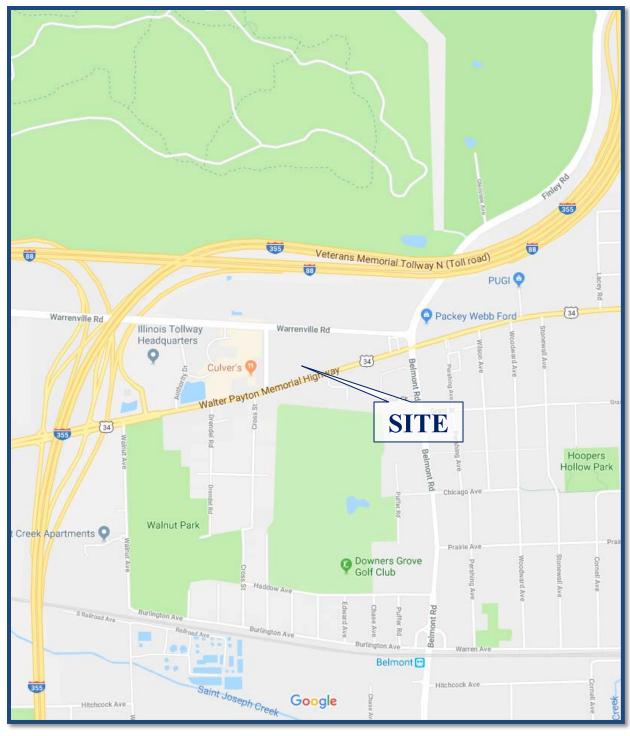
- Existing roadway conditions
- A description of the proposed development
- Directional distribution of the development traffic
- Vehicle trip generation for the development
- Future traffic conditions including access to the development
- Traffic analyses for the weekday morning and weekday evening peak hours
- Recommendations with respect to adequacy of the site access and adjacent roadway system

Traffic capacity analyses were conducted for the weekday morning and weekday evening peak hours for the following conditions:

- 1. Existing Condition Analyzes the capacity of the existing roadway system using existing peak hour traffic volumes in the surrounding area.
- 2. Future Condition The future projected traffic volumes include the existing traffic volumes increased by an ambient area growth factor (growth not attributable to any particular development) and the traffic estimated to be generated by the proposed subject development.



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Site Location Figure 1



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Aerial View of Site Location

Figure 2

2. Existing Conditions

Existing transportation conditions in the vicinity of the site were documented based on a field visit conducted by KLOA, Inc. in order to obtain a database for projecting future conditions. The following provides a description of the geographical location of the site, physical characteristics of the area roadway system including lane usage and traffic control devices, and existing peak hour traffic volumes.

Site Location

The site, which is currently occupied by a vacant building, is located on the north side of Ogden Avenue approximately 600 feet east of Cross Street. Land uses in the vicinity of the site are primarily commercial to the west, north, and east and residential to the south and include Gerber Collision and Glass to the east, Premier Auto Auctions, Riggs Brothers Tops and Interiors, R & D Fence, and the Downers Grove Park District to the north, Max Madsen Mitsubishi and Culvers to the west, and Auto Extreme, Inc. and Fairway Grove Condominiums to the south.

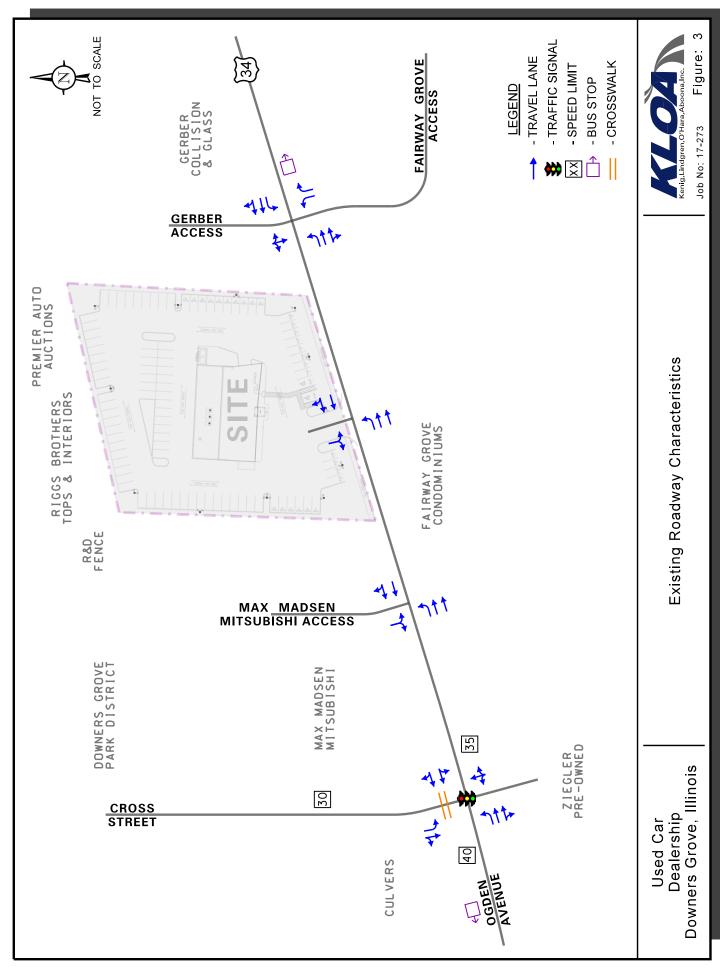
Existing Roadway System Characteristics

The characteristics of the existing roadways near the development are described below. **Figure 3** illustrates the existing roadway characteristics.

Ogden Avenue is generally an east-west arterial roadway that in the vicinity of the site provides two through lanes in each direction separated by a center, two-way left-turn lane. At its signalized intersection with Cross Street, Ogden Avenue provides an exclusive left-turn lane and two exclusive through lanes on the eastbound approach and an exclusive through lane and a shared through/right-turn lane on the westbound approach. At its unsignalized intersection with the Max Madsen Mitsubishi access drive, Ogden Avenue provides two exclusive through lanes on the eastbound approach and an exclusive through lane and a shared through/right-turn lane on the westbound approach with eastbound left-turn movements accommodated via the center, two-way left-turn lane. At its unsignalized intersection with the Gerber Collision & Glass and Fairway Grove access drives, Ogden Avenue provides an exclusive through lane and a shared through/right-turn lane on both approaches with left turns onto the access drives accommodated via the existing center, two-way left-turn lane. Ogden Avenue is under the jurisdiction of the Illinois Department of Transportation (IDOT), carries an annual average daily traffic volume (AADT) volume of 30,600 vehicles (IDOT AADT 2016), and has a posted speed limit of 35 miles per hour east of Cross Street and a posted speed limit of 40 miles per hour west of Cross Street.

Cross Street is a north-south local roadway that has an offset intersection with Ogden Avenue. The north leg of the intersection extends from Ogden Avenue to Warrenville Road, provides access to the commercial developments along Cross Street, and has signalized intersections with both roadways. At its signalized intersection with Ogden Avenue, the north leg of Cross Street provides an exclusive left-turn lane, an exclusive right-turn lane, and a standard style crosswalk. The south leg of this intersection is the full movement access drive serving Auto Extreme, Inc. This access drive provides one inbound lane and one outbound lane. Cross Street is under the jurisdiction of the DuPage County Division of Transportation, carries an AADT volume of 7,442 vehicles (DuDOT AADT 2010), and has a posted speed limit of 30 miles per hour.





Existing Traffic Volumes

In order to determine current traffic conditions in the vicinity of the site, KLOA, Inc. conducted peak period traffic counts using Miovision Scout Video Collection Units on Wednesday, November 15, 2017 during the weekday morning (7:00 A.M. to 9:00 A.M.) and weekday evening (4:00 P.M. to 6:00 P.M.) peak periods at the following intersections:

- Ogden Avenue with Cross Street/Auto Extreme, Inc. Access Drive
- Ogden Avenue with the Max Madsen Mitsubishi Access Drive
- Ogden Avenue with Fairway Grove/Gerber Collision and Glass Access Drives

The results of the traffic counts showed that the weekday morning peak hour of traffic occurs from 7:15 A.M. to 8:15 A.M. and the weekday evening peak hour of traffic occurs from 4:30 P.M. to 5:30 P.M. **Figure 4** illustrates the existing peak hour traffic volumes. Copies of the traffic count summary sheets are included in the Appendix.

Crash Analysis

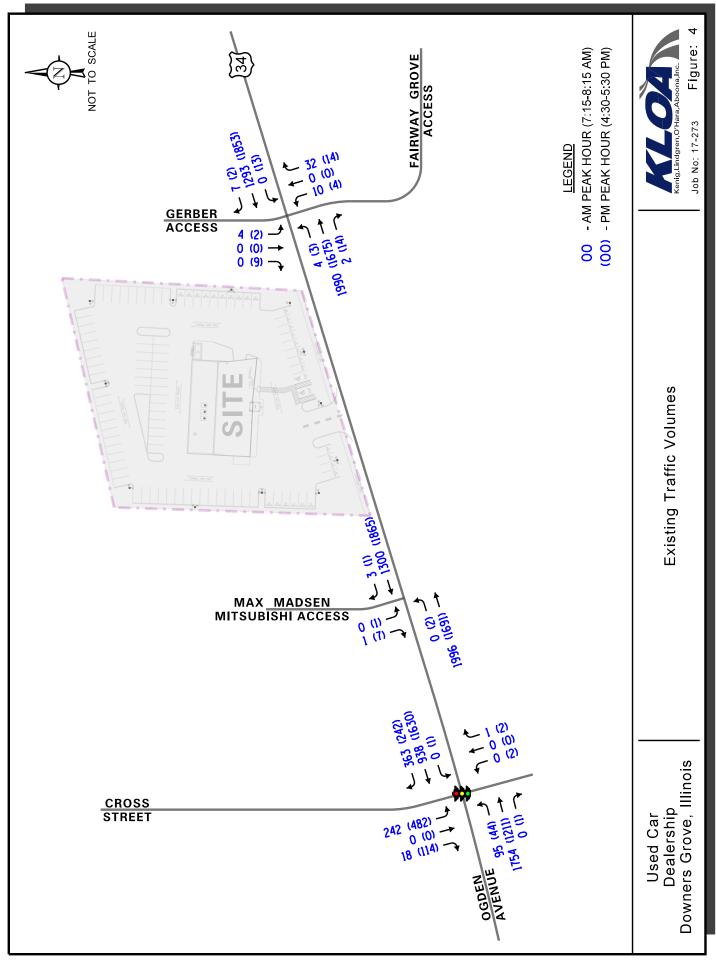
KLOA, Inc. obtained crash data for the past five years (2010 to 2014) for the intersections of Ogden Avenue with Cross Street, Ogden Avenue with the Max Madsen Mitsubishi access drive, and Ogden Avenue with the Gerber/Fairway Grove access drives. The crash data for the intersection of Ogden Avenue with Cross Street is summarized in **Table 1**. A review of the crash data indicated that the intersection of Ogden Avenue with the Max Madsen Mitsubishi access drive experienced only one crash in 2011, 2012, and 2015 and zero crashes in 2013 and 2014. Only one crash involved a turning vehicle at this intersection. Furthermore, the intersection of Ogden Avenue with the Gerber/Fairway Grove access drives experienced zero crashes in 2011 and 2015, one crash in 2012 and 2014, and two crashes in 2013. None of the crashes at this intersection involved a turning vehicle. Additionally, the crash data indicated there were no fatalities reported at any of the intersections.

Table 1
OGDEN AVENUE WITH CROSS STREET – CRASH SUMMARY

	Type of Crash Frequency								
Year	Angle	Object	Rear End	Sideswipe	Turning	Other	Total		
2011	0	0	2	0	2	0	4		
2012	0	0	1	0	0	0	1		
2013	0	0	1	0	3	0	4		
2014	0	0	3	0	2	0	5		
2015	<u>0</u>	<u>0</u>	<u>0</u>	<u>0</u>	<u>4</u>	<u>0</u>	<u>4</u>		
Total	0	0	7	0	11	0	18		
Average	0	0	1.4	0	2.2	0	3.6		

DISCLAIMER: The motor vehicle crash data referenced herein was provided by the Illinois Department of Transportation. The author is responsible for any data analyses and conclusions drawn.





3. Traffic Characteristics of the Proposed Development

In order to properly evaluate future traffic conditions in the surrounding area, it was necessary to determine the traffic characteristics of the proposed development, including the directional distribution and volumes of traffic that it will generate.

Proposed Site and Development Plan

As proposed, the plans call for developing the site with an approximately 7,000 square-foot used car dealership that will provide an approximately 3,500 square-foot showroom. Additionally, a five-bay service center will be provided. Access to the used car dealership will be provided via the existing full movement curb cut that is located approximately 250 feet east of the Max Madsen Mitsubishi full movement access drive. This access drive provides one inbound lane and one outbound lane and outbound movements should be under stop sign control. Left turns onto the access drive will be accommodated via the existing center, two-way left-turn lane on Ogden Avenue. It should be noted that the proposed access system will result in the elimination of the existing easterly full movement curb cut on Ogden Avenue along the site frontage. A total of 101 parking spaces will be provided with 30 spaces reserved for guest parking. A copy of the preliminary site plan depicting the proposed development and access is included in the Appendix.

Directional Distribution

The directions from which employees and patrons of the used car dealership will approach and depart the site were estimated based on existing travel patterns, as determined from the traffic counts. **Figure 5** illustrates the directional distribution of the development-generated traffic.

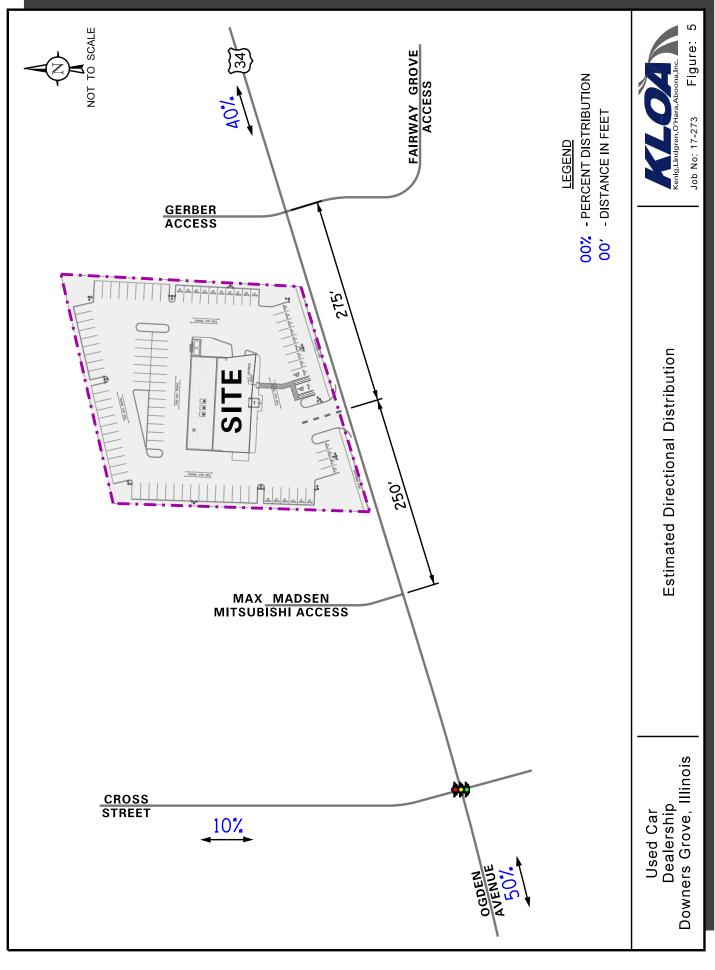
Estimated Site Traffic Generation

The estimates of traffic to be generated by the development are based upon the proposed land use type and size. The volume of traffic generated for the auto dealership was estimated using data published in the Institute of Transportation Engineers (ITE) *Trip Generation Manual*, 9th Edition. The ITE rates and equations used are included in the Appendix. **Table 2** tabulates the vehicle trips anticipated for this development for the weekday morning and weekday evening peak hours.

Table 2
ESTIMATED SITE-GENERATED TRAFFIC VOLUMES

ITE Land			kday M Peak Ho	orning our		kday E Peak H	Evening our	Daily Two-Way
Use Code	Type/Size	In	Out	Total	In	Out	Total	Trips
841	Auto Dealership (7,000 s.f.)	10	3	13	7	11	18	226





4. Projected Traffic Conditions

The total projected traffic volumes include the existing traffic volumes, increase in background traffic due to growth, and the traffic estimated to be generated by the proposed subject development.

Development Traffic Assignment

The estimated weekday morning and evening peak hour traffic volumes that will be generated by the proposed development were assigned to the roadway system in accordance with the previously described directional distribution (Figure 5). The total new traffic assignment for the commercial development is illustrated in **Figure 6**.

Background Traffic Conditions

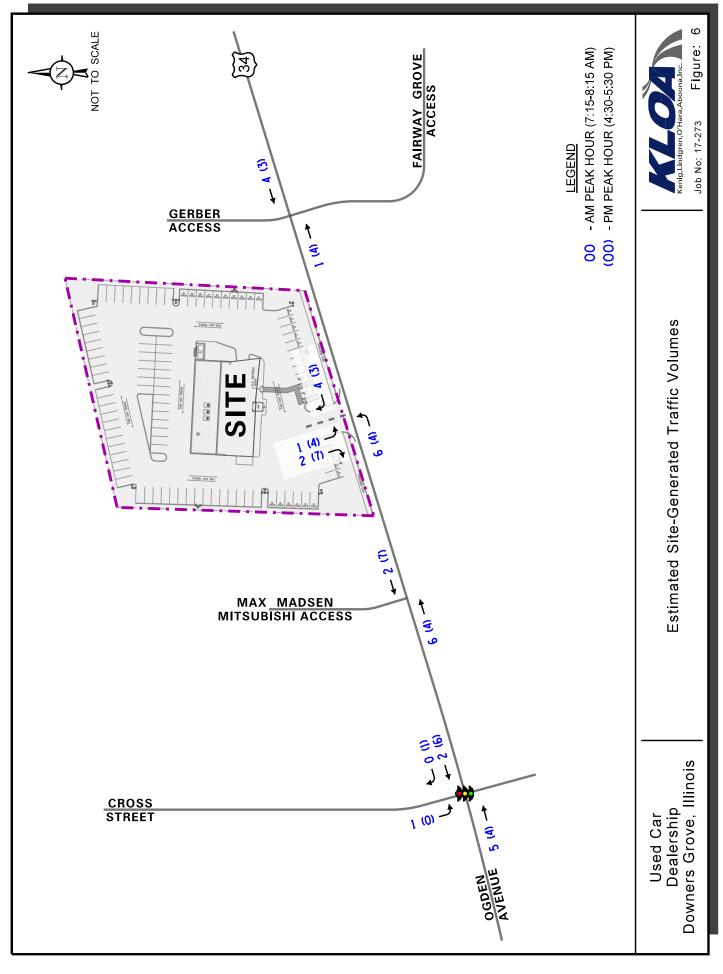
The existing traffic volumes (Figure 4) were increased by a regional growth factor to account for the increase in existing traffic related to regional growth in the area (i.e., not attributable to any particular planned development). Based on ADT projections provided by the Chicago Metropolitan Agency for Planning (CMAP) in a letter dated December 11, 2017, an increase of approximately four-tenths of a percent per year for six years (buildout year plus five years) was applied to project Year 2023 conditions. A copy of the CMAP 2040 projections letter is included in the Appendix.

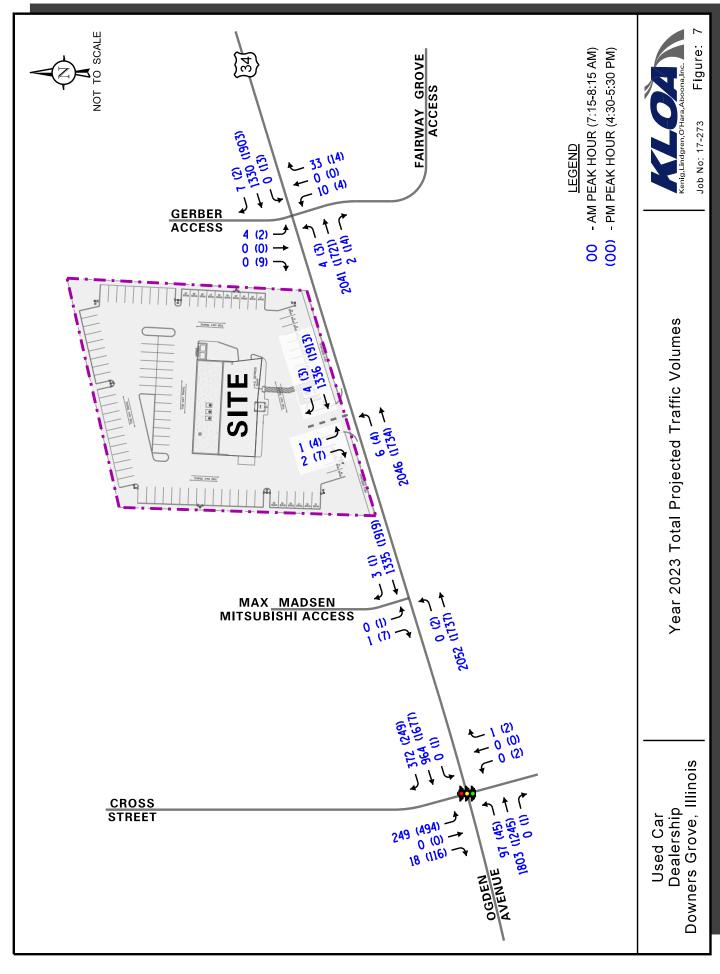
Total Projected Traffic Volumes

The development-generated traffic was added to the existing traffic volumes accounting for background growth to determine the Year 2023 total projected traffic volumes, shown in **Figure** 7.



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5. Traffic Analysis and Recommendations

The following provides an evaluation conducted for the weekday morning and weekday evening peak hours. The analysis includes conducting capacity analyses to determine how well the roadway system and access drives are projected to operate and whether any roadway improvements or modifications are required.

Traffic Analyses

Roadway and adjacent or nearby intersection analyses were performed for the weekday morning and weekday evening peak hours for the existing (Year 2017) and future projected (Year 2023) traffic volumes.

The traffic analyses were performed using the methodologies outlined in the Transportation Research Board's *Highway Capacity Manual (HCM)*, 2010 and analyzed using the HCS 7 computer software. The analyses for the intersection of Ogden Avenue with Cross Street were completed utilizing actual cycle lengths and phasings.

The analyses for the unsignalized intersections determine the average control delay to vehicles at an intersection. Control delay is the elapsed time from a vehicle joining the queue at a stop sign (includes the time required to decelerate to a stop) until its departure from the stop sign and resumption of free flow speed. The methodology analyzes each intersection approach controlled by a stop sign and considers traffic volumes on all approaches and lane characteristics.

The ability of an intersection to accommodate traffic flow is expressed in terms of level of service, which is assigned a letter from A to F based on the average control delay experienced by vehicles passing through the intersection. The *Highway Capacity Manual* definitions for levels of service and the corresponding control delay for signalized intersections and unsignalized intersections are included in the Appendix of this report.

Summaries of the traffic analysis results showing the level of service and overall intersection delay (measured in seconds) for the existing and Year 2023 total projected conditions are presented in **Tables 3** and **4**, respectively. A discussion of the intersections follows. Summary sheets for the capacity analyses are included in the Appendix.



Table 3 CAPACITY ANALYSIS RESULTS – EXISTING TRAFFIC CONDITIONS

		Morning Hour	•	Evening Hour
Intersection	LOS	Delay	LOS	Delay
Ogden Avenue with Cross Street/Auto Extre	eme, Inc. Acc	cess Drive ¹		
• Overall	A	9.5	Е	64.4
Eastbound Approach	A	1.5	A	4.2
Westbound Approach	A	5.9	В	16.0
Northbound Approach	D	48.9	D	41.9
Southbound Approach	F	84.7	F	99+
Ogden Avenue with Max Madsen Mitsubish	ni Access Dri	ve ²		
Southbound Approach	В	14.2	C	24.7
Westbound Left Turns	В	12.2	С	17.3
Ogden Avenue with Gerber/Fairway Grove	Access Drive	es^2		
Northbound Approach	F	55.7	D	30.7
Southbound Approach	Е	47.7	D	32.0
Eastbound Left Turns	В	12.1	С	17.2
Westbound Left Turns	С	18.5	C	15.8
LOS = Level of Service Delay is measured in seconds. 1 – Signalized Access Drive 2 – Unsignalized Access Drive				

Table 4 CAPACITY ANALYSIS RESULTS - YEAR 2023 PROJECTED TRAFFIC CONDITIONS

	•	Morning Hour		Evening Hour
Intersection	LOS	Delay	LOS	Delay
Ogden Avenue with Cross Street/Auto Extre	me, Inc. Ac	cess Drive ¹		
• Overall	В	10.0	Е	68.3
Eastbound Approach	A	1.6	A	4.3
Westbound Approach	A	6.1	В	17.4
Northbound Approach	D	48.9	D	42.0
Southbound Approach	F	89.5	F	99+
Ogden Avenue with Max Madsen Mitsubishi	Access Dri	ive ²		
Southbound Approach	В	14.5	С	25.9
Westbound Left Turns	В	12.4	С	17.9
Ogden Avenue with Gerber/Fairway Grove A	Access Driv	es ²		
Northbound Approach	F	60.7	D	32.4
Southbound Approach	Е	50.4	D	33.8
Eastbound Left Turns	В	12.4	С	17.8
Westbound Left Turns	C	19.2	C	16.3
Ogden Avenue with Proposed Full Movemen	t Access D	rive ²		
Southbound Approach	C	21.8	Е	36.0
Westbound Left-Turns	В	12.4	С	17.8
LOS = Level of Service Delay is measured in seconds. 1 – Signalized Access Drive 2 – Unsignalized Access Drive				

Discussion and Recommendations

The following summarizes how the intersections are projected to operate and identify any roadway and traffic control improvements to accommodate the development traffic.

Ogden Avenue with Cross Street

The results of the capacity analysis indicate that overall this intersection currently operates at level of service (LOS) A during the weekday morning peak hour and at LOS E during the weekday evening peak hour. It should be noted that the delays experienced during the weekday evening peak hour are a result of the southbound approach which operates at LOS F during the weekday evening peak hour due to the limited amount of greentime allocated to this approach. Under Year 2023 projected conditions, this intersection overall is projected to operate at LOS B during the weekday morning peak hour with increases in delay of less than one second and is projected to continue operating at LOS E during the weekday evening peak hours with increases in delay of approximately four seconds. It should be noted that the increases in delay at this intersection are primarily due to the increase in background growth as the proposed development is projected to increase the volume of traffic traversing this intersection by less than one-half percent. As such, the proposed development traffic will have a limited impact on the operations of this intersection.

Ogden Avenue with Max Madsen Mitsubishi Access Drive

The results of the capacity analysis indicate that the southbound approach currently operates at LOS B during the weekday morning peak hour and at LOS C during the weekday evening peak hour. Under Year 2023 conditions, the southbound approach will continue to operate at existing levels of service during the peak hours with increases in delay of approximately one second or less. Additionally, eastbound left-turns onto the access drive are projected to continue operating at LOS B during the weekday morning peak hour and at LOS C during the weekday evening peak hour with increases in delay of less than one second and 95th percentile queues of one to two vehicles. As such, the proposed development traffic will have a limited impact on the operations of this intersection and no roadway or traffic control improvements will be required.

Ogden Avenue with Gerber/Fairway Grove Access Drives

The results of the capacity analysis indicate that the northbound approach currently operates at LOS F during the weekday morning peak hour and at LOS D during the weekday evening peak hour. Additionally, the southbound approach currently operates at LOS E during the weekday morning peak hour and at LOS D during the weekday evening peak hour. However, this level of service is expected for access driveways that have unsignalized intersections with major roadways such as Ogden Avenue. Under Year 2023 projected conditions, the northbound and southbound approaches are projected to continue operating at existing levels of service during the peak hours with increase in delay of approximately five seconds or less. Additionally, eastbound and westbound left-turns onto the access drives are projected to continue operating at LOS C or better during the peak hours with increases in delay of less than one second and 95th percentile queues of one to two vehicles. As such, the proposed development traffic will have a limited impact on the operations of this intersection and no roadway or traffic control improvements will be required.



Ogden Avenue with Proposed Full Movement Access Drive

The results of the capacity analysis indicate that outbound movements from the proposed access drive are projected to operate at LOS C during the weekday morning peak hour and at LOS E during the weekday evening peak hour with 95th percentile queues of one to two vehicles. As previously indicated, this LOS is expected for access driveways that have an unsignalized intersection with a major roadway such as Ogden Avenue. Additionally, eastbound left-turns onto the access drive are projected to operate at LOS B during the weekday morning peak hour and at LOS C during the weekday evening peak hour with 95th percentile queues of one to two vehicles which can be accommodated within the existing center, two-way left-turn lane. As such, the proposed access driveway will be adequate in accommodating the traffic projected to be generated by the proposed development



6. Conclusion

Based on the preceding analyses and recommendations, the following conclusions have been made:

- The proposed development will generate a low volume of traffic, approximately 13 trips during the weekday morning peak hour and 18 trips during the weekday evening peak hour.
- The development-generated traffic will not have a significant impact on area roadways.
- Providing a single full movement access drive off Ogden Avenue will be adequate in accommodating the traffic projected to be generated by the proposed development and will eliminate an existing curb cut on Ogden Avenue along the site frontage.



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Appendix

Traffic Count Summary Sheets
ITE Rates and Equations
Site Plan
CMAP 2040 Projections Letter
Level of Service Criteria
Capacity Analysis Summary Sheets

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Traffic Count Summary Sheets



9575 W. Higgins Rd., Suite 400 Co.
Rosemont, Illinois, United States 60018 States (847)518-9990 Pac

Count Name: Ogden Avenue with Cross Street Site Code: Start Date: 11/15/2017 Page No: 1

		App. Int. Total	54 759	698 09	62 855	91 870	267 3353	47 760	61 762	58 799	48 774	214 3095		166 907	128 862	170 902	133 927	597 3598	149 900	144 976	125 888	126 825	544 3589	1622 13635		11.9	1600 13419	98.6 98.4	9 20	0.4 0.5	13 100	0.8	3 46	0.2 0.3	0 0	0.0 0.0	
		Peds	0	0	0	0	0	0	0	0	0	0		0	0	1	0	1	0	0	0	0	0	_	1	1				-	-	,	-	,		-	1
Cross Street	Southbound	Right	8	5	3	5	21	5	8	11	8	32		47	24	44	24	139	23	23	17	21	84	276	17.0	2.0	271	98.2	0	0.0	5	1.8	0	0.0	0	0.0	
Cros	Sout	Thru	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0	'	0	-	0	•	0	٠	0	•	
		n Left	46	22	29	98	246	42	53	47	40	182		119	104	126	109	458	126	121	108	105	460	1346	83.0	9.9	1329	98.7	9	0.4	8	9.0	3	0.2	0	0.0	
		U-Turn	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0	-	0	-	0	'	0	•	0	•	
		s App. Total	0	0	0	2	2	0	0	0	-	-	•	3	0	1	1	5	0	2	-	-	4	12	'	0.1	12	100.0	0	0.0	0	0.0	0	0.0	0	0.0	•
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Access Drive	Northbound	u Right	0	0	0	1	1	0	0	0	1	_	•	2	0	0	1	3	0	1	_	_	3	80	66.7	0.1	80	100.0	0	0.0	0	0.0	0	0.0	0	0.0	
¥	z	ft Thru	0	0	0	0	0	0	0	0	0	0	•	0	0	0	0	0	0	0	0	0	0	0	0.0 0.0	0.0	0	- 0.0	0	- 0	0	- 0	0		0	- 0	•
		U-Tum Left	0 0	0 0	0 0	1 0	1 0	0 0	0 0	0 0	0 0	0 0		0 1	0 0	0 1	0 0		0 0	0 1	0 0	0 0	0 1	1 3	8.3 25.0	0.0 0.0	ł	100.0 100.0	0 0	0.0 0.0	0 0	0.0 0.0	0 0	0.0 0.0	0 0	0.0 0.0	
		App. U-1 Total	300	308	305	348	1261	323	328	359	359	1369	-	461	461	427	499	1848	460	474	489	423	1846	6324		46.4 0	6222	98.4 10	35	0.6 0.0	41	0.6	56	0.4	0	0.0	
		Peds A	0 3	0 3	0 3	0 3	0 12	0 3	0 3	0 3	0 3	0 13	-	0 4	0 4	0	0 4	0 18	0 4	4	0	0	0 18	0	1	- 4	- 62	-	1) -	7	,	-	,) -	0
anne	pu	Right F	61	89	06	107	326	98	98	104	89	389		53	71	62	61	247	99	63	71	44	234	1196	18.9	8.8	1188	99.3	4	0.3	4	0.3	0	0.0	0	0.0	
Ogden Avenue	Westbound	Thru F	239	240	215	241	935	225	230	255	569	979		406	389	365	438	1598	403	411	418	379	1611	5123	81.0	37.6	ł	98.2	31	9.0	37	0.7	26	0.5	0	0.0	
		. Feft	0	0	0		0	0	0	0	1	_		2	1	0	0		1	0	0	0	_	5	0.1	0.0	-	100.0	0	0.0	0	0.0	0	0.0	0	0.0	
		U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0		0	-	0	,	0		0	-	
	-	App. Total	405	501	488	429	1823	390	373	382	366	1511	-	277	273	304	294	1148	291	356	273	275	1195	2677	,	41.6	5885	98.4	59	0.5	46	8.0	17	0.3	0	0.0	
		Peds	0	0	1	0	1	0	0	0	0	0	-	0	0	1	0	1	0	0	0	0	0	2	1	1				-	-	,		,		-	2
Sil cer	puno	Right	0	0	0	0	0	0	0	1	-	2		2	0	0	0	2	0	1	0	0	-	5	0.1	0.0	5	100.0	0	0.0	0	0:0	0	0.0	0	0.0	,
Cross Street	Eastbound	Thru	391	486	464	403	1744	359	344	344	337	1384		269	264	295	281	1109	279	345	261	258	1143	5380	94.8	39.5	5290	98.3	29	0.5	45	8.0	16	0.3	0	0.0	,
		Left	14	15	23	26	78	31	29	37	28	125		9	6	6	13	37	12	10	12	17	51	291	5.1	2.1	289	99.3	0	0.0	-	0.3	_	0.3	0	0.0	
		U-Turn	0	0	1	0	1	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	-	0:0	0.0	-	100.0	0	0.0	0	0:0	0	0:0	0	0.0	,
		Start Time	7:00 AM	7:15 AM	7:30 AM	7:45 AM	Hourly Total	8:00 AM	8:15 AM	8:30 AM	8:45 AM	Hourly Total	*** BREAK ***	4:00 PM	4:15 PM	4:30 PM	4:45 PM	Hourly Total	5:00 PM	5:15 PM	5:30 PM	5:45 PM	Hourly Total	Grand Total	Approach %	Total %	Lights	% Lights	Buses	% Buses	Single-Unit Trucks	% Single-Unit Trucks	Articulated Trucks	% Articulated Trucks	Bicycles on Road	% Bicycles on Road	Pedestrians



Count Name: Ogden Avenue with Cross Street Site Code: Start Date: 11/15/2017 Page No: 3

Rosemont, Illinois, United States 60018 (847)518-9990

	Cross Street	Peds App. U-Turn Left Thru Right Peds App. Int. Total	0 0 0 55 0 5 0 60 869	0 0 0 59 0 3 0 62 855	0 2 0 86 0 5 0 91 870	0 0 0 42 0 5 0 47 760	0 2 0 242 0 18 0 260 3354	0.0 93.1 0.0 6.9	- 0.1 0.0 7.2 0.0 0.5 - 7.8 -	- 0.250 0.000 0.703 0.000 0.900 - 0.714 0.964	- 2 0 237 0 16 - 253 3280	- 100.0 - 97.9 - 88.9 - 97.3 97.8	. 0 0 2 0 0 . 2 30	- 0.0 - 0.8 - 0.0 - 0.8 0.9	0 0 1 0 2 - 3 31	- 0.0 - 0.4 - 11.1 - 1.2 0.9	- 0 0 2 0 0 - 2 13	- 0.0 - 0.8 - 0.0 - 0.8 0.4	0 0 - 0 0 0 0 0 -	0.0 - 0.0 - 0.0 - 0.0	0 0	
:15 A	Access Drive	Thru Right	0	0	0	0	0	0.0 50.0	0.0	0.000 0.250	0	- 100.0	0		0		0		0			,
ata (7		Left	0	0	0	0	0	0.0	0.0	0.000	0		0		0		0		0			
Turning Movement Peak Hour Data (7:15 AM)		U-Tum	0	0	1	0	1	20.0	0.0	0.250	1	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
eak H		App. Total	308	305	348	323	1284	,	38.3	0.922	1250	97.4	12	6.0	15	1.2	7	0.5	0	0.0	-	
ent P		Peds	0	0	0	0	0	,		,		,	,						1		0	
ovem	venue	Right	89	06	107	86	363	28.3	10.8	0.848	361	99.4	0	0.0	2	9.0	0	0.0	0	0.0		
ing Mc	Ogden Avenue Westbound	Thru	240	215	241	225	921	71.7	27.5	0.955	889	96.5	12	1.3	13	1.4	7	8.0	0	0.0		
Turn		Left	0	0	0	0	0	0.0	0.0	0.000	0		0		0		0		0		-	
		U-Turn	0	0	0	0	0	0.0	0.0	0.000	0		0		0	-	0		0	-	-	
•	,	App. Total	501	488	429	390	1808		53.9	0.902	1775	98.2	16	6.0	13	0.7	4	0.2	0	0.0		
		Peds	0	1	0	0	1			-	-		-			-			-	-	1	100.0
	Street	Right	0	0	0	0	0	0.0	0.0	0.000	0		0	,	0	,	0		0			
	Cross Street	Thru	486	464	403	359	1712	94.7	51.0	0.881	1679	98.1	16	6.0	13	0.8	4	0.2	0	0.0		
		Left	15	23	56	31	92	5.3	2.8	0.766	92	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
		U-Tum	0	1	0	0	1	0.1	0.0	0.250	1	100.0	0	0.0	0	0.0	0	0.0	0	0.0		'
		Start Time	7:15 AM	7:30 AM	7:45 AM	8:00 AM	Total	Approach %	Total %	PHF	Lights	% Lights	Buses	% Buses	Single-Unit Trucks	% Single-Unit Trucks	Articulated Trucks	% Articulated Trucks	Bicycles on Road	% Bicycles on Road	Pedestrians	% Pedestrians



Count Name: Ogden Avenue with Cross Street Site Code: Start Date: 11/15/2017 Page No: 4

								Tur	√ guir	Joven	nent F	Peak !	Hour [Turning Movement Peak Hour Data (4:30 PM	4:30	PM)									
			Cross	Cross Street					Ogden	Ogden Avenue				•	Access Drive	Drive					Cross Street	eet o			
Start Time	U-Tum	Left	Thr	Right	Peds	App. Total	U-Turn	Left	Thr	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App.	U-Tum	Left	Thru	ŧ	Peds /	App. Total	Int. Total
4:30 PM	0	6	295	0	_	304	0	0	365	62	0	427	0	-	0	0	0	-	0	126	0	44	_	170	902
4:45 PM	0	13	281	0	0	294	0	0	438	61	0	499	0	0	0	-	0	-	0	109	0	24	0	133	927
5:00 PM	0	12	279	0	0	291	0	1	403	56	0	460	0	0	0	0	1	0	0	126	0	23	0	149	006
5:15 PM	0	10	345	-	0	356	0	0	411	63	0	474	0	1	0	-	1	2	0	121	0	23	0	144	926
Total	0	44	1200	1	_	1245	0	1	1617	242	0	1860	0	2	0	2	2	4	0	482	0	114	1	296	3705
Approach %	0.0	3.5	96.4	0.1		1	0.0	0.1	86.9	13.0	ı	-	0.0	50.0	0.0	50.0			0.0	80.9	0.0	19.1	-		
Total %	0.0	1.2	32.4	0.0		33.6	0.0	0.0	43.6	6.5		50.2	0.0	0.1	0.0	0.1		0.1	0.0	13.0	0.0	3.1		16.1	
PHF	0.000	0.846	0.870	0.250	,	0.874	0.000	0.250	0.923	0.960	,	0.932	0.000	0.500	0.000	0.500	-	0.500	0.000	0.956	0.000	0.648	- 0	0.876 0	0.949
Lights	0	43	1183	1		1227	0	1	1606	242	1	1849	0	2	0	2		4	0	478	0	114	-	592	3672
% Lights	-	7.76	98.6	100.0		98.6	-	100.0	99.3	100.0	,	99.4		100.0		100.0		100.0		99.2		100.0		99.3	99.1
Buses	0	0	2	0		2	0	0	3	0		3	0	0	0	0	-	0	0	1	0	0		1	9
% Buses	-	0.0	0.2	0.0		0.2	1	0.0	0.2	0.0	1	0.2		0.0		0.0		0.0		0.2		0.0	-	0.2	0.2
Single-Unit Trucks	0	0	12	0		12	0	0	4	0		4	0	0	0	0		0	0	3	0	0		3	19
% Single-Unit Trucks		0.0	1.0	0.0	,	1.0	'	0.0	0.2	0.0		0.2	,	0.0		0.0	,	0.0		9.0		0.0		0.5	9.0
Articulated Trucks	0	1	3	0		4	0	0	4	0	1	4	0	0	0	0		0	0	0	0	0	-	0	8
% Articulated Trucks		2.3	0.3	0.0	,	0.3	,	0.0	0.2	0.0	,	0.2		0:0	,	0.0	,	0.0		0.0		0.0	,	0:0	0.2
Bicycles on Road	0	0	0	0	٠	0	0	0	0	0	,	0	0	0	0	0	-	0	0	0	0	0		0	0
% Bicycles on Road		0.0	0.0	0.0	-	0.0	-	0.0	0.0	0.0		0.0		0.0		0.0		0.0	-	0.0		0.0		0.0	0.0
Pedestrians					1						0	-					2						1	-	
% Pedestrians					100.0												100.0					-	100.0	,	



Count Name: Ogden Avenue with Max Madsen Access Site Code: Start Date: 11/15/2017 Page No: 1

	_)									
			Ogden Avenue Eastbound					Ogden Avenue Westbound				Max	Max Madsen Access Drive Southbound	Orive		
Start Time	U-Tum	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	U-Tum	Left	Right	Peds	App. Total	Int. Total
7:00 AM	0	0	441	0	441	0	302	0	0	302	0	0	0	0	0	743
7:15 AM	0	0	530	0	530	0	312	1	0	313	0	0	0	0	0	843
7:30 AM	0	0	532	0	532	0	305	0	0	305	0	0	1	0	1	838
7:45 AM	0	0	515	0	515	0	358	2	0	360	0	0	0	0	0	875
Hourly Total	0	0	2018	0	2018	0	1277	8	0	1280	0	0	-	0	-	3299
8:00 AM	0	0	419	0	419	0	311	0	0	311	0	0	0	0	0	730
8:15 AM	-	-	417	0	419	0	340	г	0	343	0	0	0	0	0	762
8:30 AM	0	0	417	0	417	0	350	0	0	350	0	2	0	0	2	692
8:45 AM	0	2	400	0	402	0	362	-	0	363	0	-	0	0	-	992
Hourly Total	-	8	1653	0	1657	0	1363	4	0	1367	0	3	0	0	3	3027
*** BREAK ***	•	1	1	1		,	ı	,	1	-			•	1	-	
4:00 PM	0	0	396	0	396	0	461	0	0	461	0	-	0	0	-	828
4:15 PM	0	0	378	0	378	0	469	0	0	469	0	0	0	0	0	847
4:30 PM	0	1	411	0	412	0	430	1	0	431	0	1	2	1	3	846
4:45 PM	0	0	401	0	401	0	488	0	0	488	0	0	2	0	2	891
Hourly Total	0	1	1586	0	1587	0	1848	1	0	1849	0	2	4	1	9	3442
5:00 PM	0	0	401	0	401	0	476	0	0	476	0	0	2	0	2	879
5:15 PM	0	1	478	0	479	0	471	0	0	471	0	0	1	1	1	951
5:30 PM	0	-	368	0	369	0	511	_	0	512	0	0	0	0	0	881
5:45 PM	0	0	375	0	375	0	414	0	0	414	0	0	0	0	0	789
Hourly Total	0	2	1622	0	1624	0	1872	_	0	1873	0	0	က	_	3	3500
Grand Total	_	9	6829	0	6886	0	6360	6	0	6369	0	5	8	2	13	13268
Approach %	0.0	0.1	99.9		,	0.0	6.66	0.1	1	•	0.0	38.5	61.5	1		
Total %	0.0	0.0	51.8		51.9	0.0	47.9	0.1	-	48.0	0.0	0.0	0.1	-	0.1	
Lights	_	9	6767		6774	0	6256	6	-	6265	0	5	8	'	13	13052
% Lights	100.0	100.0	98.4	,	98.4		98.4	100.0	1	98.4	'	100.0	100.0	1	100.0	98.4
Buses	0	0	38		38	0	35	0	-	35	0	0	0	-	0	73
% Buses	0.0	0.0	9.0	-	9.0		9.0	0.0	-	0.5	-	0.0	0.0	_	0.0	9.0
Single-Unit Trucks	0	0	49		49	0	42	0	1	42	0	0	0	-	0	91
% Single-Unit Trucks	0.0	0.0	0.7	-	7.0		0.7	0.0	-	0.7	-	0.0	0.0	-	0.0	0.7
Articulated Trucks	0	0	25		25	0	27	0	-	27	0	0	0		0	52
% Articulated Trucks	0.0	0.0	0.4		0.4		4:0	0.0		0.4		0.0	0.0		0.0	4.0
Bicycles on Road	0	0	0		0	0	0	0		0	0	0	0	1	0	0
% Bicycles on Road	0.0	0.0	0.0		0.0		0.0	0.0		0.0		0.0	0.0		0.0	0.0
Pedestrians	'	'		0	,		1	,	0	'	-		-	2	-	
0/ Dodostrions																



Count Name: Ogden Avenue with Max Madsen Access Site Code: Start Date: 11/15/2017 Page No: 2

		Turning	⁻ urning Movement Peak Hour Data (7:15 AM)	ent Pea	ak Hour I)ata (7:	15 AM)		
Ogden Avenue					Ogden Avenue				Max N
Eastbound					Westbound				
Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left
530	0	530	0	312	1	0	313	0	0
532	0	532	0	305	0	0	305	0	0
515	0	515	0	358	2	0	360	0	0
419	0	419	0	311	0	0	311	0	0
1996	0	1996	0	1286	က	0	1289	0	0

3286

100.0

0.0 0.0

0

0

843 838 875 730

App. Total

Peds

Right

Madsen Access Drive Southbound 0.939 3210 7.76

100.0 0.250 0.0

100.0

0.250 0.0

0.000

0.000 0.0 0.0

> 39.2 0.895 1254 97.3 6.0

0.1

8.66

100.0

0.0

Approach %

Total %

PHF

7:45 AM

7:30 AM

8:00 AM

Total

0 0

0

3 100.0 0.375 0.2

39.1 0.898 1251 97.3

0.000 0.0 0.0

60.7 0.938 1955 97.9

0.938 1955 97.9 2.09

0.00

0.000 0.0 0.0

0

0

% Lights

Buses

Lights

0.0 0.0

12 0.9 1.1

0

4

0.7

0.5 0.0

0.5 0.0

6 0

0

0

0

0

18

6.0

6.0 18

4 0.7

0

% Single-Unit Trucks

Single-Unit Trucks

% Buses

% Articulated Trucks % Bicycles on Road

% Pedestrians Pedestrians

Articulated Trucks Bicycles on Road

6.0 30

0.0

0.0

0

0

28 6.0 8 0.5

0.0

0.0 0.0 0.0

0

0

0 0 0

4

-

0.7 0.0

0.0

6 0.7 0 0.0

0 0

0.0

0.0 0.0

0



Count Name: Ogden Avenue with Max Madsen Access Site Code: Start Date: 11/15/2017 Page No: 3

					Turning	Turning Movement Peak Hour Data (4:30 PM	ent Pea	k Hour [)ata (4:	30 PM)						
			Ogden Avenue					Ogden Avenue		•		Max N	Max Madsen Access Drive	rive		
Start Limo			Eastbound					Westbound					Southbound			
Start Hille	U-Tum	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	Int. Total
4:30 PM	0	1	411	0	412	0	430	1	0	431	0	1	2	1	3	846
4:45 PM	0	0	401	0	401	0	488	0	0	488	0	0	2	0	2	891
5:00 PM	0	0	401	0	401	0	476	0	0	476	0	0	2	0	2	879
5:15 PM	0	1	478	0	479	0	471	0	0	471	0	0	1	1	1	951
Total	0	2	1691	0	1693	0	1865	1	0	1866	0	1	7	2	8	3567
Approach %	0.0	0.1	6.66	-	-	0.0	6.66	0.1		-	0.0	12.5	87.5		-	
Total %	0.0	0.1	47.4	-	47.5	0.0	52.3	0.0		52.3	0.0	0.0	0.2		0.2	
PHF	0.000	0.500	0.884	-	0.884	0.000	0.955	0.250	-	0.956	0.000	0.250	0.875		0.667	0.938
Lights	0	2	1669	-	1671	0	1853	1		1854	0	1	7		8	3533
% Lights	-	100.0	98.7		98.7	-	99.4	100.0		99.4	-	100.0	100.0		100.0	99.0
Buses	0	0	5		5	0	3	0	-	3	0	0	0	-	0	8
% Buses	-	0.0	0.3	-	0.3	-	0.2	0.0		0.2	-	0.0	0.0		0.0	0.2
Single-Unit Trucks	0	0	12		12	0	5	0		5	0	0	0		0	17
% Single-Unit Trucks	-	0.0	0.7		0.7	-	0.3	0.0		0.3	-	0.0	0.0		0.0	0.5
Articulated Trucks	0	0	5		5	0	4	0		4	0	0	0		0	6
% Articulated Trucks	-	0.0	0.3		0.3		0.2	0.0		0.2	'	0.0	0.0		0.0	0.3
Bicycles on Road	0	0	0		0	0	0	0	-	0	0	0	0	_	0	0
% Bicycles on Road	-	0.0	0.0		0.0	-	0.0	0.0		0.0	-	0.0	0.0		0.0	0.0
Pedestrians	-		·	0	'				0	-	'	,		2		
% Pedestrians	,	,	,	i		,	,	,	1	,	,	,	,	100.0	,	,



Count Name: Ogden Avenue with Access Drives Site Code: Start Date: 11/15/2017 Page No: 1

-						-				Turn	ing M	loven	Turning Movement Data	ata											
			Ogden Avenue Eastbound	enne					Ogden Avenue Westbound	venue					Fairway Grove Access Northbound	ve Access		•			Gerber Access Southbound	Access			
Start Time U-Tı	U-Tum Le	. Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	Int. Total
7:00 AM 0		0	439	0	0	439	0	0	301	2	0	303	0	0	0	9	0	9	0	0	0	0	0	0	748
7:15 AM 0	, ,	_	521	0	0	522	0	0	310	0	0	310	0	3	0	7	0	10	0	-	0	0	0	-	843
7:30 AM 0		0	533	1	0	534	0	0	315	3	0	318	0	0	0	8	0	8	0	0	0	0	0	0	860
7:45 AM 0		1	202	0	0	508	0	0	358	3	0	361	0	4	0	8	0	12	0	0	0	0	0	0	881
Hourly Total 0		2	2000	1	0	2003	0	0	1284	8	0	1292	0	7	0	29	0	36	0	-	0	0	0	-	3332
8:00 AM 0		2	420	-	0	423	0	0	310	-	0	311	0	3	0	6	0	12	0	3	0	0	0	3	749
8:15 AM 0		2	408	4	0	414	0	2	337	1	0	343	0	1	0	2	0	9	0	1	0	1	0	2	765
8:30 AM 0		2	412	0	0	414	0	1	335	3	0	339	0	2	0	2	0	7	0	1	0	3	0	4	764
8:45 AM 0			401	-	0	405	0	0	367	0	0	367	0	2	0	2	0	7	0	0	0	2	0	2	781
Hourly Total 0		6	1641	9	0	1656	0	9	1349	2	0	1360	0	8	0	24	0	32	0	2	0	9	0	11	3059
*** BREAK ***					1												-					•	-		
4:00 PM 0		_	395	2	0	398	-	2	453	2	0	458	0	0	0	2	0	2	0	-	0	-	0	2	860
4:15 PM 0	, C	1	374	2	0	377	0	2	454	2	0	458	0	1	0	8	0	6	0	1	0	2	0	3	847
4:30 PM 0		1	406	2	0	409	0	4	438	2	0	444	0	0	0	3	0	3	0	0	0	1	0	1	857
4:45 PM 0		2	396	4	0	402	0	4	488	0	0	492	0	2	0	9	0	8	0	-	0	4	0	2	206
Hourly Total 0		. 2	1571	10	0	1586	1	12	1833	9	0	1852	0	3	0	19	0	22	0	3	0	8	0	11	3471
5:00 PM 0		0	395	2	0	397	0	3	458	0	0	461	0	1	0	2	0	3	0	0	0	4	0	4	865
5:15 PM 0		0	468	9	0	474	0	2	469	0	0	471	0	1	0	3	0	4	0	1	0	0	0	1	950
5:30 PM 0		0	370	2	0	372	0	2	496	0	0	501	0	0	0	1	0	1	0	1	0	0	0	1	875
5:45 PM 0		_	372	4	0	377	0	4	420	-	0	425	0	0	0	3	0	3	0	_	0	_	0	2	807
Hourly Total 0	0	_	1605	4	0	1620	0	4	1843	_	0	1858	0	2	0	6	0	7	0	3	0	2	0	8	3497
Grand Total 0		17 (6817	31	0	6865	1	32	6309	20	0	6362	0	20	0	81	0	101	0	12	0	19	0	31	13359
Approach % 0.0		0.2	99.3	0.5	1	,	0.0	0.5	99.2	0.3	1	'	0.0	19.8	0.0	80.2	1	,	0.0	38.7	0.0	61.3	1	,	
Total % 0.0		0.1	51.0	0.2	1	51.4	0.0	0.2	47.2	0.1	1	47.6	0.0	0.1	0.0	9.0		0.8	0.0	0.1	0.0	0.1		0.2	
Lights 0		16	6703	31	,	6750	-	32	6205	20		6258	0	20	0	81		101	0	10	0	19		29	13138
% Lights	- 94	94.1	98.3	100.0	1	98.3	100.0	100.0	98.4	100.0	-	98.4		100.0		100.0		100.0	'	83.3		100.0	,	93.5	98.3
Buses 0		0	38	0	,	38	0	0	36	0	,	36	0	0	0	0		0	0	0	0	0		0	74
- Ruses		0.0	9.0	0.0	-	9.0	0.0	0.0	9.0	0.0		9.0		0.0	-	0.0	-	0.0		0.0	-	0.0	-	0.0	9.0
Single-Unit Trucks 0	, C	1	51	0		52	0	0	44	0		44	0	0	0	0	-	0	0	2	0	0	-	2	98
% Single-Unit Trucks	. 5	5.9	0.7	0.0	,	8.0	0.0	0.0	7.0	0.0	,	0.7		0.0	,	0.0	,	0.0	,	16.7	,	0:0	,	6.5	0.7
Articulated Trucks 0		0	25	0		25	0	0	24	0		24	0	0	0	0		0	0	0	0	0	-	0	49
% Articulated Trucks	- 0	0.0	0.4	0.0		0.4	0.0	0.0	4.0	0.0		0.4		0.0		0.0		0.0		0.0		0.0		0.0	0.4
Bicycles on Road 0		0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles on Road	- 0.	0.0	0.0	0.0	,	0.0	0.0	0.0	0.0	0.0	,	0.0		0.0	,	0.0	,	0.0	,	0.0	,	0:0	,	0.0	0.0
Pedestrians -					0						0	,					0						0		



Count Name: Ogden Avenue with Access Drives Site Code: Start Date: 11/15/2017 Page No: 3

								Turn	ing M	ovem	ent P	eak F	our D	Turning Movement Peak Hour Data (7:15 AM)	7:15	(M)									
			Ogden Avenue	Avenue					Ogden Avenue	.venue				ъ	Fairway Grove Access	'e Access		-			Gerber Access	sess			
Start Time			Eastb	Eastbound					Westbound	punc					Northbound	punc					Southbound	pun			
Olait IIIIe	U-Tum	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total In	Int. Total
7:15 AM	0	1	521	0	0	522	0	0	310	0	0	310	0	3	0	7	0	10	0	1	0	0	0	1	843
7:30 AM	0	0	533	1	0	534	0	0	315	3	0	318	0	0	0	8	0	8	0	0	0	0	0	0	860
7:45 AM	0	-	202	0	0	208	0	0	358	3	0	361	0	4	0	8	0	12	0	0	0	0	0	0	881
8:00 AM	0	2	420	1	0	423	0	0	310	1	0	311	0	3	0	6	0	12	0	3	0	0	0	3	749
Total	0	4	1981	2	0	1987	0	0	1293	7	0	1300	0	10	0	32	0	42	0	4	0	0	0	4	3333
Approach %	0.0	0.2	99.7	0.1		-	0.0	0.0	99.5	0.5		-	0.0	23.8	0.0	76.2			0.0	100.0	0.0	0.0	-	-	
Total %	0.0	0.1	59.4	0.1		59.6	0.0	0.0	38.8	0.2		39.0	0.0	0.3	0.0	1.0		1.3	0.0	0.1	0.0	0.0		0.1	
PHF	0.000	0.500	0.929	0.500	,	0.930	0.000	0.000	0.903	0.583		0.900	0.000	0.625	0.000	0.889		0.875	0.000	0.333	0.000	0.000) -	0.333	0.946
Lights	0	4	1936	2		1942	0	0	1258	7		1265	0	10	0	32		42	0	3	0	0		3	3252
% Lights		100.0	7.76	100.0		97.7			97.3	100.0		97.3		100.0		100.0		100.0		75.0				75.0	97.6
Buses	0	0	20	0	,	20	0	0	13	0	,	13	0	0	0	0	,	0	0	0	0	0		0	33
% Buses		0.0	1.0	0.0	,	1.0	,		1.0	0.0	,	1.0	,	0.0	,	0.0	,	0.0		0.0		,		0.0	1.0
Single-Unit Trucks	0	0	17	0	'	17	0	0	15	0	'	15	0	0	0	0	1	0	0	-	0	0		-	33
% Single-Unit Trucks		0.0	6.0	0.0	,	0.9	,	,	1.2	0.0	,	1.2	,	0.0		0.0		0.0		25.0			1	25.0	1.0
Articulated Trucks	0	0	8	0	,	8	0	0	7	0	,	7	0	0	0	0	,	0	0	0	0	0		0	15
% Articulated Trucks		0.0	9.0	0.0		0.4			0.5	0.0		0.5		0.0		0.0		0:0		0.0				0.0	0.5
Bicycles on Road	0	0	0	0	,	0	0	0	0	0	,	0	0	0	0	0	1	0	0	0	0	0		0	0
% Bicycles on Road		0.0	0.0	0.0		0.0			0.0	0.0		0.0		0.0		0.0		0.0		0.0			-	0.0	0.0
Pedestrians					0	-	-				0	-					0	-					0	-	
% Pedestrians							,				,				,		,	,						,	,



Count Name: Ogden Avenue with Access Drives Site Code: Start Date: 11/15/2017 Page No: 4

Rosemont, Illinois, United States 60018 (847)518-9990

								Turn	ing M	ovem	ent P	eak h	dour E	Turning Movement Peak Hour Data (4:30 PM)	4:30	DM)									
			Ogden Avenue Eastbound	Avenue					Ogden Avenue Westbound	Avenue				, щ	airway Grove Ac Northbound	Fairway Grove Access Northbound		•			Gerber Access Southbound	cess			
Start Time	U-Tum	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. In Total	Int. Total
4:30 PM	0	_	406	2	0	409	0	4	438	2	0	444	0	0	0	3	0	3	0	0	0	-	0	-	857
4:45 PM	0	2	396	4	0	402	0	4	488	0	0	492	0	2	0	9	0	8	0	-	0	4	0	5	206
5:00 PM	0	0	395	2	0	397	0	3	458	0	0	461	0	1	0	2	0	3	0	0	0	4	0	4	865
5:15 PM	0	0	468	9	0	474	0	2	469	0	0	471	0	1	0	3	0	4	0	1	0	0	0	1	950
Total	0	3	1665	14	0	1682	0	13	1853	2	0	1868	0	4	0	14	0	18	0	2	0	6	0	11	3579
Approach %	0.0	0.2	0.66	8.0			0.0	0.7	99.2	0.1		-	0.0	22.2	0.0	77.8			0.0	18.2	0.0	81.8	-	-	
Total %	0.0	0.1	46.5	0.4		47.0	0.0	0.4	51.8	0.1		52.2	0.0	0.1	0.0	0.4		0.5	0.0	0.1	0.0	0.3	-	0.3	
PHF	0.000	0.375	0.889	0.583	,	0.887	0.000	0.813	0.949	0.250		0.949	0.000	0.500	0.000	0.583	,	0.563	0.000	0.500	0.000	0.563		0.550	0.942
Lights	0	3	1644	14		1661	0	13	1841	2		1856	0	4	0	14		18	0	2	0	6	-	11	3546
% Lights		100.0	98.7	100.0		98.8		100.0	99.4	100.0		99.4		100.0		100.0		100.0		100.0		100.0	-	100.0	99.1
Buses	0	0	4	0	,	4	0	0	3	0		3	0	0	0	0	,	0	0	0	0	0	-	0	7
% Buses	,	0.0	0.2	0.0	,	0.2	,	0.0	0.2	0.0	,	0.2		0.0		0.0	,	0.0		0.0		0.0	,	0.0	0.2
Single-Unit Trucks	0	0	13	0	,	13	0	0	4	0	,	4	0	0	0	0	,	0	0	0	0	0		0	17
% Single-Unit Trucks		0.0	0.8	0.0	,	0.8		0.0	0.2	0.0	,	0.2		0:0	,	0.0	,	0.0		0.0		0:0	,	0.0	9.0
Articulated Trucks	0	0	4	0	1	4	0	0	5	0	1	5	0	0	0	0		0	0	0	0	0	-	0	6
% Articulated Trucks	,	0.0	0.2	0.0	,	0.2		0.0	0.3	0.0	,	0.3		0:0	,	0.0	,	0.0		0.0		0.0	,	0.0	0.3
Bicycles on Road	0	0	0	0	,	0	0	0	0	0	,	0	0	0	0	0	,	0	0	0	0	0	-	0	0
% Bicycles on Road		0.0	0.0	0.0	-	0.0		0.0	0.0	0.0		0:0		0.0		0.0		0.0		0.0		0.0	-	0.0	0.0
Pedestrians					0		-				0						0	-					0	-	
% Pedestrians	,											,					,				,				

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ITE Rates and Equations

Automobile Sales

(841)

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area

On a: Weekday

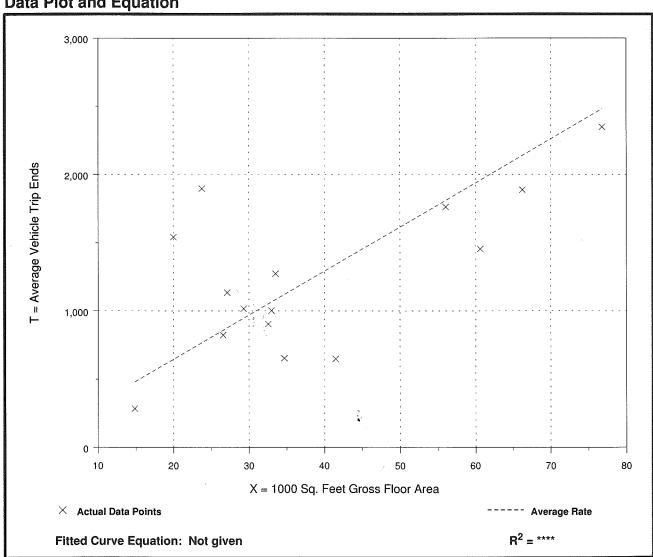
Number of Studies: 15 Average 1000 Sq. Feet GFA: 38

Directional Distribution: 50% entering, 50% exiting

Trip Generation per 1000 Sq. Feet Gross Floor Area

Average Rate	Range of Rates	Standard Deviation
32.30	15.64 - 79.66	15.70

Data Plot and Equation



Automobile Sales

(841)

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

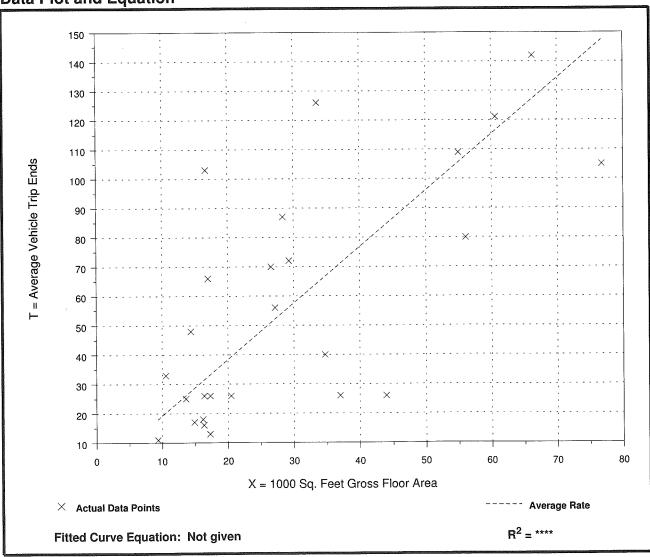
Number of Studies: 26 Average 1000 Sq. Feet GFA: , 30

Directional Distribution: 75% entering, 25% exiting

Trip Generation per 1000 Sq. Feet Gross Floor Area

Average Rate	Range of Rates	Standard Deviation
1.92	0.59 - 6.17	1.72

Data Plot and Equation



Automobile Sales

(841)

Average Vehicle Trip Ends vs: 1000 Sq. Feet Gross Floor Area

On a: Weekday,

Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

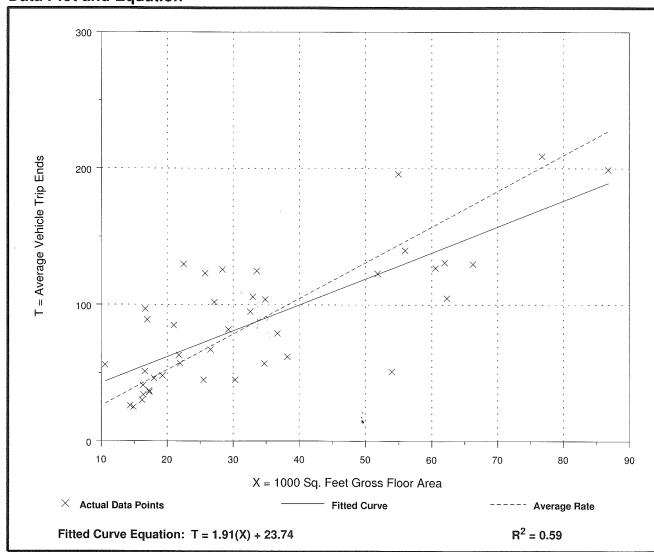
Number of Studies: 41 Average 1000 Sq. Feet GFA: 33

Directional Distribution: 40% entering, 60% exiting

Trip Generation per 1000 Sq. Feet Gross Floor Area

Average Rate	Range of Rates	Standard Deviation
2.62	0.94 - 5.81	1.90

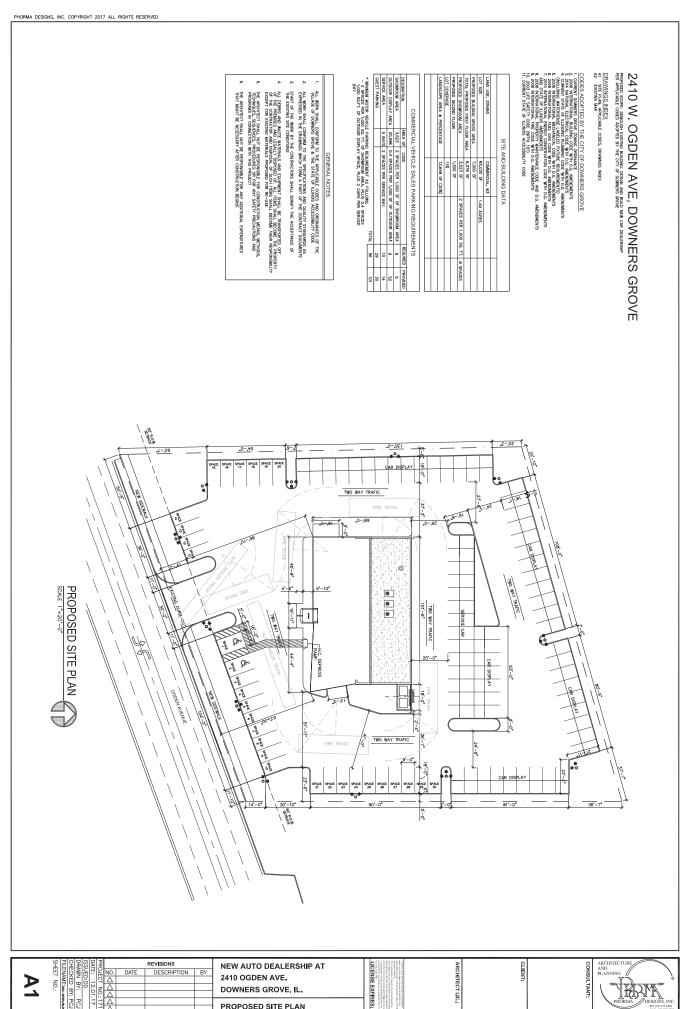
Data Plot and Equation



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Site Plan

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PROPOSED SITE PLAN

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CMAP 2040 Projections Letter



233 South Wacker Drive Suite 800 Chicago, Illinois 60606

312 454 0400 www.cmap.illinois.gov

December 11, 2017

Brendan S. May Consultant Kenig, Lindgren, O'Hara and Aboona, Inc. 9575 West Higgins Road Suite 400 Rosemont, IL 60018

Subject: Ogden Avenue (US 34) @ Cross Street

IDOT

Dear Mr. May:

In response to a request made on your behalf and dated December 11, 2017, we have developed year 2040 average daily traffic (ADT) projections for the subject location.

ROAD SEGMENT	Current ADT	Year 2040 ADT
Ogden Ave, @ Cross St	30,600	34,100
Ogden Ave east of Cross St	36,609	40,700
Ogden Ave west of Cross St	34,195	38,100
Cross St north of Ogden Ave	7,442	8,300

Traffic projections are developed using existing ADT data provided in the request letter and the results from the October 2017 CMAP Travel Demand Analysis. The regional travel model uses CMAP 2040 socioeconomic projections and assumes the implementation of the GO TO 2040 Comprehensive Regional Plan for the Northeastern Illinois area.

If you have any questions, please call me at (312) 386-8806.

Sincerely,

Jose Rodriguez, PTP, AICP

Senior Planner, Research & Analysis

cc: Quigley (IDOT)

S.\AdminGroups\ResearchAnalysis\TrafficForecasts_CY2017\DownersGrove\du-69-17\du-69-17\docx

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Level of Service Criteria

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LEVEL OF SERVICE CRITERIA

SE VEE OF SE	Signalized Intersections	
Level of Service	Interpretation	Average Control Delay (seconds per vehicle)
A	Favorable progression. Most vehicles arrive during the green indication and travel through the intersection without stopping.	≤10
В	Good progression, with more vehicles stopping than for Level of Service A.	>10 - 20
С	Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear. Number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping.	>20 - 35
D	The volume-to-capacity ratio is high and either progression is ineffective or the cycle length is too long. Many vehicles stop and individual cycle failures are noticeable.	>35 - 55
E	Progression is unfavorable. The volume-to-capacity ratio is high and the cycle length is long. Individual cycle failures are frequent.	>55 - 80
F	The volume-to-capacity ratio is very high, progression is very poor, and the cycle length is long. Most cycles fail to clear the queue.	>80.0
	Unsignalized Intersections	
	Level of Service Average Total De	elay (SEC/VEH)
	A 0	- 10
	B > 10	- 15
	C > 15	- 25
	D > 25	- 35
	E > 35	- 50
	F > 5	0
Source: Highwa	ty Capacity Manual, 2010.	

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Capacity Analysis Summary Sheets

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HCS7	Signa	alized	Inter	sectio	on Ir	put Da	ata				1 45	je 71 Ol
General Information						Intersec	tion Inf	ormatic	nn	<u></u>		Įs L
Agency KLOA, Inc.					_	Duration,		0.25	711		47	
Analyst BSM	Δnalve	eis Data	Dec 1	2 2017	_	Area Typ		Other	•			<u>*</u> _ &
Jurisdiction IDOT	Time F		-	eak Hou	_	PHF	<u> </u>	0.96			wŶE	<u>~</u> ← &
Urban Street Ogden Avenue		sis Year		ak i iou	_	Analysis	Poriod	1> 7:0	20	_ _ ₹ - ₹		√ _ ÷
	File Na			Ανοριι		Cross St						
Intersection Ogden Avenue with Cro Project Description AM Existing Peak Hour	File IN	ame	Ogder	1 Avenu	e with	Cross St	reet - A	MEX.XU	ıs		কু বিক্স	7 1
)r			ır					
Demand Information		EB		 	WI	_	-	NB		 	SB	
Approach Movement	L	T	R	L	T		L	T	R	L	T	R
Demand (v), veh/h	95	1754	0	0	93	8 363	0	0	1	242	0	18
Signal Information			,									1
Cycle, s 140.0 Reference Phase 2	1	\vdash	12 2	E 43	- A					Z	1	4
Offset, s 0 Reference Point Begin	1	3	3						1	2	3	4
Uncoordinated No Simult. Gap E/W On	Green Yellow		97.3 4.5	23.0 4.5	0.0		0.0		A	→		-4-
Force Mode Fixed Simult. Gap N/S On	Red	0.0	1.5	1.5	0.0		0.0		5	6	7	Y
Traffic Information		EB			WB			NB			SB	
Approach Movement	L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh/h	95	1754	0	0	938	363	0	0	1	242	0	18
Initial Queue (Q _b), veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Base Saturation Flow Rate (s₀), veh/h	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Parking (<i>N_m</i>), man/h		None			None	€		None			None	
Heavy Vehicles (<i>Phv</i>), %	0	2		0	3			0		2	0	
Ped / Bike / RTOR, /h	0	0		0	0	0	0	0	0	0	0	0
Buses (N _b), buses/h	0	0	0	0	0	0	0	0	0	0	0	0
Arrival Type (AT)	3	4	3	3	4	3	3	3	3	3	3	3
Upstream Filtering (/)	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Lane Width (W), ft	12.0	12.0		12.0	12.0			12.0		12.0	12.0	
Turn Bay Length, ft	100	0		50	0			0		300	0	
Grade (<i>Pg</i>), %		0			0			0			0	
Speed Limit, mi/h	40	40	40	35	35	35	15	15	15	30	30	30
Phase Information	EBL		EBT	WBI		WBT	NBL	_	NBT	SBL		SBT
Maximum Green (Gmax) or Phase Split, s	14.0		111.0			97.0		_	29.0			29.0
Yellow Change Interval (Y), s	3.5		4.5			4.5			4.5			4.5
Red Clearance Interval (Rc), s	0.0		1.5			1.5			1.5		\neg	1.5
Minimum Green (Gmin), s	3		15	6		15	6		8	3		8
Start-Up Lost Time (It), s	2.0		2.0	2.0		2.0	2.0		2.0	2.0		2.0
Extension of Effective Green (e), s	2.0		2.0	2.0		2.0	2.0		2.0	2.0		2.0
Passage (PT), s	3.0		7.0	2.0		7.0	2.0		4.0	2.0		4.0
Recall Mode	Off		Min	Off		Min	Off		Off	Off		Off
Dual Entry	Yes		Yes	No		Yes	No		Yes	No		Yes
Walk (<i>Walk</i>), s	0.0		0.0	0.0		0.0	0.0		0.0	0.0		0.0
Pedestrian Clearance Time (PC), s	0.0		0.0	0.0		0.0	0.0		0.0	0.0		0.0
Multimodal Information		EB			WB			NB			SB	
85th % Speed / Rest in Walk / Corner Radius	0	No	25	0	No	25	0	No	25	0	No	25
Walkway / Crosswalk Width / Length, ft	9.0	12	0	9.0	12	0	9.0	12	0	9.0	12	0
Street Width / Island / Curb	0	0	No	0	0	No	0	0	No	0	0	No
Width Outside / Bike Lane / Shoulder, ft	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0
Pedestrian Signal / Occupied Parking	No		0.50	No		0.50	No		0.50	No		0.50

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	HCS7 Si	gnalize	ed Inte	ersec	tion F	Resul	ts Sur	nmar	у				
General Information						l I	ntersect	ion Inf	ormatio	nn .		기록 Yi 4pi f	ba l <u>u</u>
Agency	KLOA, Inc.					_	Duration,		0.25	711	┪ᆜ	17	
Analyst	BSM	Analy	sis Date	Dec 1	2, 2017		Area Typ		Other				R. 2
Jurisdiction	IDOT	Time			eak Hou		HF	-	0.96			w↑E	<i>≛</i>
Urban Street	Ogden Avenue		sis Year		eak i iot		Analysis	Doriod	1> 7:0	20	- ₹¬₹		<u>-</u>
Intersection	-				n Avanu						- F		,
	Ogden Avenue with Cro.	. File N	ame	Ugae	n Avenu	ie with t	Cross St	reet - A	IVIEX.XU	ıs	- 4	† বৰ্ণকাপ	to 7
Project Description	AM Existing Peak Hour												rı
Demand Information		Т	EB			WB	<u> </u>	T	NB			SB	
Approach Movement		L	Т	R	L	Т	R	L	Т	R	L	Т	R
Demand (v), veh/h		95	1754	0	0	938	363	0	0	1	242	0	18
											"		
Signal Information			1 2	1.							_		.
Cycle, s 140.0	Reference Phase 2	_	Ħ	 	T 54	2					Θ	3	s † x
Offset, s 0	Reference Point Begi	n Green	4.2	97.3	23.0	0.0	0.0	0.0			K		
Uncoordinated No	Simult. Gap E/W On	Yellow		4.5	4.5	0.0	0.0	0.0		>	→		N)
Force Mode Fixed	Simult. Gap N/S On	Red	0.0	1.5	1.5	0.0	0.0	0.0		5	6	7	8
Timer Results		EB	L	EBT	WB	L	WBT	NB	L L	NBT	SBI	L	SBT
Assigned Phase		5		2			6			8		$-\!$	4
Case Number		1.0	_	4.0			6.3			8.0			6.0
Phase Duration, s		7.7		111.0			103.3			29.0			29.0
Change Period, (Y+R	? c), S	3.5		6.0			6.0			6.0			6.0
Max Allow Headway (·	4.0		0.0			0.0			5.1			5.1
Queue Clearance Tim	, - ,	4.1								2.1			25.0
Green Extension Time	e (g _e), s	0.1		0.0			0.0			1.4			0.0
Phase Call Probability	1	1.00)							1.00			1.00
Max Out Probability		0.09	9							0.00			1.00
Movement Group Re	sults		EB			WB			NB			SB	
Approach Movement			T	R		T	R	L	Т	R	L	T	R
Assigned Movement		5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow Rate (v) veh/h	99	1827	0	0	706	649		0	10	252	19	
•	low Rate (s), veh/h/ln	1810	1870	0	259	1856	1679		0		1416	1610	_
Queue Service Time (· · · · · · · · · · · · · · · · · · ·	2.1	0.0	0.0	0.0	8.0	20.1		0.0		22.9	1.4	
Cycle Queue Clearan		2.1	0.0	0.0	0.0	8.0	20.1		0.0		23.0	1.4	
Green Ratio (g/C)	00 mile (g c), 3	0.74	0.75	0.0	0.69	0.69	0.69		0.0		0.16	0.16	
Capacity (c), veh/h		331	2806		51	1289	1167				283	265	
Volume-to-Capacity R	atio (X)	0.299	0.651	0.000		0.548	_		0.000		0.890	0.071	
Back of Queue (Q), f		34	21.2	0.000	0.000	103.3			0.000		426	25.8	
, ,	/eh/ln (95 th percentile)	1.4	0.8	0.0	0.0	4.0	9.9		0.0		16.8	1.0	
, ,	(RQ) (95 th percentile)	0.34	0.00	0.00	0.00	0.00	0.00		0.00		1.42	0.00	+
Uniform Delay (d 1),	. ,, ,	7.3	0.00	0.00	0.00	1.9	6.6		0.00		59.5	49.5	
Incremental Delay (d		0.5	1.2	0.0	0.0	1.7	1.9		0.0		27.8	0.2	
Initial Queue Delay (, .	0.0	0.0	0.0	0.0	0.0	0.0		0.0		0.0	0.0	
Control Delay (d), s/v	·	7.8	1.2	5.0	0.0	3.5	8.5		5.0		87.3	49.6	
Level of Service (LOS		A	A		0.0	A	A				F	D	
Approach Delay, s/veh	,	1.5		Α	5.9		Α	48.9	9	D	84.7		F
Intersection Delay, s/v		1			.5						A		
Multimodal Results			EB			WB			NB			SB	
Pedestrian LOS Score	e/LOS	2.0		В	2.2		В	2.9		С	2.9		С
Bicycle LOS Score / L	OS	2.1		В	1.6		В	0.5		Α	0.9		Α

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		HCS7	Sig	nal	ized	Inter	sectio	n In	teri	media	ite Va	lues					
Concret Inform	otion									Intore	a a a ti a m	Inform	otion			1 4 JJ 45 J	N.L.
General Inform	ation	KI OA Jaa								-	section	- 1			- 1	ĮĮ.	
Agency		KLOA, Inc.			<u> </u>	D 1	D 40	004	•	Durat		_	.25				~
Analyst		BSM		\rightarrow			Dec 12,			Area	Туре	_	ther			w∱E	- E
Jurisdiction		IDOT		-	me Pe		AM Pea	к но	ur	PHF		$\overline{}$.96		*	8	**************************************
Urban Street		Ogden Avenue		\rightarrow	nalysis		2017				sis Peri		> 7:00				<u>r</u>
Intersection		Ogden Avenue with (F	le Nan	ne	Ogden /	Avenu	ie Mi	th Cross	s Street	- AME	X.xus		_	*	
Project Descripti	ion	AM Existing Peak Ho	ur		_											ነላ ሰቀን	171
Demand Inform	nation			Т		EB			٧	VB			NB			SB	
Approach Mover	ment				L	Т	R	L		Т	R	L	Т	R	L	Т	R
Demand (v), ve	eh/h				95	1754	0	0	ć	38 3	363	0	0	1	242	0	18
Signal Informat					L	2	, <u>F</u>	ولله							"		\downarrow
	140.0	Reference Phase	2	_	F	₹		- 54	2					1	↔ ₂	3	4
Offset, s	0		3egi	— ს	reen	4.2	97.3	23.0	0	.0 0	0.0	0.0			Ā		
Uncoordinated	No	Simult. Gap E/W	On		ellow	3.5 0.0	4.5	4.5				0.0			7		V
Force Mode	rce Mode Fixed Simult. Gap N/S On						1.5	1.5	0	.0 0	0.0	0.0		5	6	7	8
Ostanstian Flan	Detunation Floor / Balan							_	-					,			
	Saturation Flow / Delay ane Width Adjustment Factor (f _w)					R	L 2 4 000		Г	R	L 4.000	T	F	_	L 1 000	T	R 4 000
	_ane Width Adjustment Factor (f _w)					1.000			000	1.000	1.000	1.00	_	-	1.000	1.000	1.000
	Heavy Vehicles and Grade Factor (fHVg) Parking Activity Adjustment Factor (fp)				0.984			\rightarrow	000	1.000	1.000	1.00		-	0.984 1.000	1.000	1.000
Bus Blockage Activity		,	-	.000	1.000	_			000	1.000	1.000	1.00	_	_	1.000	1.000	1.000
Area Type Adjus	-	· , ,	-	.000	1.000	_		_	000	1.000	1.000	1.00	_	-	1.000	1.000	1.000
Lane Utilization		· ,	_	.000	1.000				000	1.000	1.000	1.00		-	1.000	1.000	1.000
		· ,	-	.952	0.000		0.136	_	\rightarrow	1.000		0.84	_	00	0.745	0.000	1.000
Left-Turn Adjusti			10	.952		+		_	\rightarrow	0.905	1.000	_		17	0.745	0.847	0.847
Right-Turn Adjus		djustment Factor (f _{Lpb})	1	.000	1.000	1.000	1.000	_	905	0.905	1.000	0.00	0 0.0	41	1.000	0.047	0.047
		djustment Factor (<i>f_{Rpb})</i>	-	.000		1.000		'		1.000	1.000		1.0	00	1.000		1.000
Work Zone Adjus		• , ,	_	.000	1.000			1.0	000	1.000	1.000	1.00		_	1.000	1.000	1.000
		Flow Rate (s), veh/h	-	810	3741	0	259		56	979	0	0	16	-	1416	0	1610
		Arriving on Green (P)	-	0.03	1.00	0.00		_	93	0.69	0.00	0.00		-	0.16	0.00	0.16
Incremental Dela			_).11	0.50	0.00	0.00		50	0.50	0.00	0.00	0.	10	0.42	0.00	0.10
moromental Bolt	ay r aoi	ioi (ii)		, , , ,	0.00			0.	00	0.00					0.12	0.10	
Signal Timing /	Mover	ment Groups	т	EBI	_	EBT/R	WI	BL	W	/BT/R	NB	L	NBT/	R	SBL	\top	SBT/R
Lost Time (t _L)			Т	3.5		6.0				6.0			6.0				6.0
Green Ratio (g/0	C)		Т	0.74		0.75			(0.69			0.16				0.16
Permitted Satura	ation Fl	ow Rate (<i>s₀</i>), veh/h/ln		408		0			2	259			1416				1416
Shared Saturation	on Flow	v Rate (ssh), veh/h/ln											0				
Permitted Effecti		(3.).		99.3	3	0.0				0.0			0.0				23.0
Permitted Service		,- ,	L	77.1		0.0				0.0			0.0				22.9
Permitted Queue		, , , , , , , , , , , , , , , , , , ,	L	7.1	_				_	0.0							22.9
Time to First Blo		1- /	L	0.0		0.0				0.0			23.0				0.0
		efore Blockage (<i>g</i> fs), s	_														
	Protected Right Saturation Flow (s _R), veh/h/ln																
	Protected Right Effective Green Time (gR), s						_										
Multimodal									VB			NB				SB	
Pedestrian F _w / I						0.00	1.5		_	0.00	2.10	-	0.00	$\overline{}$	2.10		0.00
	edestrian Fs / F _{delay}					0.059	0.0	00	0	.075	0.00	0	0.156	3	0.000)	0.156
Pedestrian Mcorn	ner / M cw	<u>'</u>	\perp				_										
Bicycle c _b / d _b			1	-3.6	-	4.38	1389			6.52	328.	-	48.89	_	328.5		48.89
Bicycle F _w / F _v	ycle c _b / d _b					1.59	-3.0	64	1	1.12	-3.6	4	0.00		-3.64	<u> </u>	0.45

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--- Messages ---

WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

--- Comments ---

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HCS7™ Streets Version 7.1

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		ı	HCS7	Signa	alizec	l Inter	sectio	on In	put Da	ata					
General Inform	ation								Intersec	tion Inf	ormatic	on.		الماليات	پا لي
Agency	iation	KLOA, Inc.						_	Duration.		0.25		┨	41	
Analyst		BSM		Analys	is Date	e Dec 1	2 2017	_	Area Typ		Other				,
Jurisdiction		IDOT		Time F			eak Hou	\rightarrow	PHF		0.95		- → - [*]	w∮E	÷
Urban Street		Ogden Avenue			sis Year		July 1100		Analysis	Period	1> 7:0	00	<u>₹</u> ₹		~
Intersection		Ogden Avenue with	ı Cro	File Na		_	n Avenu		Cross St					ب.	,
Project Descript	tion	PM Existing Peak H		1 110 140	11110	_ Oguci	17 (VOITA	O WILL	01000 01		IVIE X X c			শু বিক্ল	ት ľ
Demand Inform	nation				EB			WE	3	1	NB			SB	
Approach Move	ment			L	Т	R	L	Т	R	L	Т	R		Т	R
Demand (v), ve				44	1211	_	1	163	_	2	0	2	482	0	114
2011.011.0 (17), 11					1211				2.0				102		
Signal Informa	tion				\Box				\neg	\top					I
Cycle, s	140.0	Reference Phase	2		K		5 to	2					$\boldsymbol{\leftrightarrow}$		4
Offset, s	0	Reference Point	Begin	Green	3 3	89.3	32.0	0.0	0.0	0.0		1	2	3	
Uncoordinated	No	Simult. Gap E/W	On	Yellow		4.5	4.5	0.0		0.0		7	\rightarrow		KŤ2
Force Mode	Fixed	Simult. Gap N/S	On	Red	0.0	1.5	1.5	0.0		0.0		5	6	7	Y
T (C) 1 (4.							\A/D			ND			0.0	
Traffic Informa				H	EB		.	WB	_		NB		<u> </u>	SB	
Approach Move				L	T	R	L	T	R	L	T	R	L 100	T	R
Demand (v), vel		n		44	1211	1	1	1630		2	0	2	482	0	114
Initial Queue (Q				0	0	0	0	0	0	0	0	0	0	0	0
Base Saturation		Rate (s₀), veh/h		1900	1900	1900	1900	1900		1900	1900	1900	1900	1900	1900
Parking (Nm), m		01			None	\vdash		None)		None			None	
Heavy Vehicles	<u> </u>	%		2	1		0	1			0		1	0	
Ped / Bike / RT0				0	0	0	0	0	0	0	0	0	0	0	0
Buses (N _b), bus				0	0	0	0	0	0	0	0	0	0	0	0
Arrival Type (A7				3	4	3	3	4	3	3	3	3	3	3	3
Upstream Filteri				1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Lane Width (W)				12.0 100	12.0	\vdash	12.0	12.0			12.0		12.0 300	12.0	
Turn Bay Length	11, 11		-	100	0	\vdash	0	0			0		300	0	_
Grade (<i>Pg</i>), % Speed Limit, mi	/h			40	40	40	35	35	35	15	15	15	30	30	30
Speed Lillin, Illi	/11			40	40	40	33	33	33	13	10	10	30	30	30
Phase Information				EBL		EBT	WBL	-	WBT	NBL	- '	NBT	SBL	-	SBT
) or Phase Split, s		11.0	_	102.0	<u> </u>	\perp	91.0		_	38.0	<u> </u>	\rightarrow	38.0
Yellow Change		· ·		3.5	_	4.5			4.5			4.5			4.5
Red Clearance				0.0	\perp	1.5			1.5		\perp	1.5		\perp	1.5
Minimum Green				3		15	6		15	6		8	3		8
Start-Up Lost Ti				2.0		2.0	2.0	_	2.0	2.0		2.0	2.0		2.0
Extension of Eff		Green (e), s		2.0	_	2.0	2.0	-	2.0	2.0	-	2.0	2.0		2.0
Passage (PT), s	\$			3.0		7.0	2.0	-	7.0	2.0	_	4.0	2.0		4.0
Recall Mode				Off		Min	Off	-	Min	Off	-	Off	Off	_	Off
Dual Entry				Yes	_	Yes	No	_	Yes	No	_	Yes	No		Yes
Walk (Walk), s				0.0	_	0.0	0.0	_	0.0	0.0	_	0.0	0.0		0.0
Pedestrian Clea	rance T	⊓me (<i>PC</i>), s		0.0		0.0	0.0		0.0	0.0		0.0	0.0		0.0
Multimodal Info	ultimodal Information				EB			WB			NB			SB	
85th % Speed /	th % Speed / Rest in Walk / Corner Radius				No	25	0	No	25	0	No	25	0	No	25
Walkway / Cros	alkway / Crosswalk Width / Length, ft				12	0	9.0	12	0	9.0	12	0	9.0	12	0
Street Width / Is	eet Width / Island / Curb				0	No	0	0	No	0	0	No	0	0	No
Width Outside /	lth Outside / Bike Lane / Shoulder, ft					2.0	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0
	Ith Outside / Bike Lane / Shoulder, ft lestrian Signal / Occupied Parking					0.50	41	-	0.50	No		0.50	4		0.50

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	HCS7	Jigi	14.1120											
General Information								Intersect	ion Info	ormatio	n	4	14 ¹ 1 ⁴ 1	Ja L
Agency	KLOA, Inc.							Duration,	h	0.25			47	
Analyst	BSM		Analys	sis Date	Dec 1	2, 2017		Area Typ	e	Other				
Jurisdiction	IDOT		Time F		_	eak Hou	_	PHF		0.95		→	w ∓ E	÷
Urban Street	Ogden Avenue		Analys	sis Year				Analysis	Period	1> 7:0	00	<u> </u>		•
Intersection	Ogden Avenue with C	ro	File Na			n Avenu		Cross St		MEX.xu	IS		₩.	
Project Description	PM Existing Peak Hou												I 숙 ↑ 숙 🌱	ትሮ
Demand Information				EB			WE	3	Т	NB			SB	
Approach Movement		$\overline{}$	L	Т	R	L	Т	R	L	T	R		Т	R
Demand (v), veh/h			44	1211	1	1	163	_	2	0	2	482	0	114
Ciamal Information					Ţ.									
Signal Information	Deference Dhase				17	= 24/2						,		人
Cycle, s 140.0		2		\bowtie	₹.	T:1	7				1	♦ 2	3	
Offset, s 0		egin	Green		89.3	32.0	0.0	0.0	0.0			<u> </u>		
				3.5	4.5	4.5	0.0	0.0	0.0		~			V
Force Mode Fixed	Simult. Gap N/S	On	Red	0.0	1.5	1.5	0.0	0.0	0.0		5	6	7	
Timer Results			EBI	_	EBT	WBI	L	WBT	NBI	-	NBT	SBI	_	SBT
Assigned Phase			5		2			6			8			4
Case Number		1.0		4.0			6.3			8.0			6.0	
Phase Duration, s			6.7	1	02.0			95.3			38.0			38.0
Change Period, (Y+R	c), S		3.5		6.0			6.0			6.0			6.0
Max Allow Headway (<i>MAH</i>), s		4.0		0.0			0.0			5.1			5.1
Queue Clearance Tim	e (g s), s		3.2								10.7			34.0
Green Extension Time	(g _e), s		0.0		0.0			0.0			3.8			0.0
Phase Call Probability			1.00)							1.00			1.00
Max Out Probability			0.69)							0.03			1.00
Movement Group Re	sults			EB			WB			NB			SB	
Approach Movement	June	_	L	T	R	L	Т	R	L	T	R	1	T	R
Assigned Movement		-	5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow Rate (/) veh/h	-	46	638	638	1	989	989		4	10	507	120	···
<u> </u>	ow Rate (s), veh/h/ln	-	1781	1885	1885	441	1885	_		1153		1426	1610	
Queue Service Time (· ,		1.2	7.4	7.4	0.1	36.6	50.3		0.0		23.3	8.7	
Cycle Queue Clearand	- /		1.2	7.4	7.4	0.1	36.6	50.3		8.7		32.0	8.7	
Green Ratio (g/C)	, , , , , , , , , , , , , , , , , , ,		0.67	0.69	0.69	0.64	0.64			0.23		0.23	0.23	
Capacity (c), veh/h			154	1293	1292	330	1202	-		302		289	368	
	atio (X)		0.301	0.494		0.003	0.823			0.014		1.757	0.326	
VOILIME-TO-Canacity R			32.1	99.1	98.7	0.6	314.8			5.4		1553.1	161.4	
	olume-to-Capacity Ratio (X) ack of Queue (Q), ft/ln (95 th percentile)			JU. 1			U 1 T.U	555.0				61.6	6.5	
Back of Queue (Q), f	· · · · · ·			3 0			12.5	223		(17)		_ 01.0		
Back of Queue (Q), f Back of Queue (Q), v	eh/ln (95 th percentile)	-	1.3	3.9	3.9	0.0	12.5	22.3		0.2			0.00	
Back of Queue (Q), f Back of Queue (Q), v Queue Storage Ratio	reh/ln (95 th percentile) (RQ) (95 th percentile	-	1.3 0.32	0.00	3.9 0.00	0.00	0.00	0.00		0.00		5.18	0.00 45.0	
Back of Queue (Q), f Back of Queue (Q), v Queue Storage Ratio Uniform Delay (d 1), s	reh/ln (95 th percentile) (RQ) (95 th percentile s/veh	-	1.3 0.32 20.0	0.00	3.9 0.00 2.2	0.0 0.00 9.5	0.00 6.5	0.00		0.00 41.9		5.18 60.4	45.0	
Back of Queue (Q), f Back of Queue (Q), v Queue Storage Ratio Uniform Delay (d 1), s Incremental Delay (d	reh/ln (95 th percentile) (RQ) (95 th percentile s/veh ₂), s/veh	-	1.3 0.32 20.0 1.1	0.00 2.2 1.3	3.9 0.00 2.2 1.3	0.0 0.00 9.5 0.0	0.00 6.5 6.4	0.00 10.4 8.6		0.00 41.9 0.0		5.18 60.4 354.5	45.0 0.7	
Back of Queue (Q), f Back of Queue (Q), v Queue Storage Ratio Uniform Delay (d 1), s Incremental Delay (d Initial Queue Delay (d	reh/In (95 th percentile) (RQ) (95 th percentile s/veh 2), s/veh / 3), s/veh	-	1.3 0.32 20.0 1.1 0.0	0.00 2.2 1.3 0.0	3.9 0.00 2.2 1.3 0.0	0.0 0.00 9.5 0.0	0.00 6.5 6.4 0.0	0.00 10.4 8.6 0.0		0.00 41.9 0.0 0.0		5.18 60.4 354.5 0.0	45.0 0.7 0.0	
Back of Queue (Q), f Back of Queue (Q), v Queue Storage Ratio Uniform Delay (d 1), s Incremental Delay (d Initial Queue Delay (d Control Delay (d), s/v	reh/ln (95 th percentile) (RQ) (95 th percentile s/veh 2), s/veh (3), s/veh reh	-	1.3 0.32 20.0 1.1 0.0 21.0	0.00 2.2 1.3 0.0 3.6	3.9 0.00 2.2 1.3 0.0 3.6	0.0 0.00 9.5 0.0 0.0 9.5	0.00 6.5 6.4 0.0 13.0	0.00 10.4 8.6 0.0 19.0		0.00 41.9 0.0 0.0 41.9		5.18 60.4 354.5 0.0 415.0	45.0 0.7 0.0 45.7	
Back of Queue (Q), f Back of Queue (Q), v Queue Storage Ratio Uniform Delay (d 1), s Incremental Delay (d Initial Queue Delay (d Control Delay (d), s/v Level of Service (LOS	reh/In (95 th percentile) (RQ) (95 th percentile s/veh 2), s/veh (3), s/veh reh	-	1.3 0.32 20.0 1.1 0.0 21.0 C	0.00 2.2 1.3 0.0 3.6 A	3.9 0.00 2.2 1.3 0.0 3.6 A	0.0 0.00 9.5 0.0 0.0 9.5 A	0.00 6.5 6.4 0.0 13.0 B	0.00 10.4 8.6 0.0 19.0 B	41.0	0.00 41.9 0.0 0.0 41.9 D	D	5.18 60.4 354.5 0.0 415.0	45.0 0.7 0.0 45.7 D	F
Back of Queue (Q), f Back of Queue (Q), v	reh/In (95 th percentile) (RQ) (95 th percentile s/veh 2), s/veh / 3), s/veh reh)	-	1.3 0.32 20.0 1.1 0.0 21.0	0.00 2.2 1.3 0.0 3.6 A	3.9 0.00 2.2 1.3 0.0 3.6 A	0.0 0.00 9.5 0.0 0.0 9.5	0.00 6.5 6.4 0.0 13.0 B	0.00 10.4 8.6 0.0 19.0	41.9	0.00 41.9 0.0 0.0 41.9 D	D	5.18 60.4 354.5 0.0 415.0	45.0 0.7 0.0 45.7 D	F
Back of Queue (Q), f Back of Queue (Q), v Queue Storage Ratio Uniform Delay (d 1), s Incremental Delay (d Initial Queue Delay (d Control Delay (d), s/v Level of Service (LOS Approach Delay, s/vel Intersection Delay, s/v	reh/In (95 th percentile) (RQ) (95 th percentile s/veh 2), s/veh / 3), s/veh reh)	-	1.3 0.32 20.0 1.1 0.0 21.0 C	0.00 2.2 1.3 0.0 3.6 A	3.9 0.00 2.2 1.3 0.0 3.6 A	0.0 0.00 9.5 0.0 0.0 9.5 A	0.00 6.5 6.4 0.0 13.0 B	0.00 10.4 8.6 0.0 19.0 B	41.9	0.00 41.9 0.0 0.0 41.9 D		5.18 60.4 354.5 0.0 415.0 F	45.0 0.7 0.0 45.7 D	F
Back of Queue (Q), f Back of Queue (Q), v Queue Storage Ratio Uniform Delay (d 1), s Incremental Delay (d Initial Queue Delay (d Control Delay (d), s/v Level of Service (LOS Approach Delay, s/veh	reh/In (95 th percentile) (RQ) (95 th percentile s/veh 2), s/veh / 3), s/veh reh) n / LOS eh / LOS	-	1.3 0.32 20.0 1.1 0.0 21.0 C	0.00 2.2 1.3 0.0 3.6 A	3.9 0.00 2.2 1.3 0.0 3.6 A	0.0 0.00 9.5 0.0 0.0 9.5 A	0.00 6.5 6.4 0.0 13.0 B	0.00 10.4 8.6 0.0 19.0 B	41.9	0.00 41.9 0.0 0.0 41.9 D		5.18 60.4 354.5 0.0 415.0 F	45.0 0.7 0.0 45.7 D	F

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		HCS7	Si	ignal	ized	Inters	sectio	n In	teri	media	te Va	lues				
General Inform	nation									Intore	section	Inform	ation		يار إنه ار	41 P C
	iation	KI OA Ina								Durat		- W	25	-		Ţ
Agency		KLOA, Inc. BSM			ما درام ما	Data	D 10	2047								R_
Analyst				-			Dec 12,			Area	туре	_	ther			Z
Jurisdiction		IDOT			ime Pe		PM Pea	к нос	ır	PHF		$\overline{}$	95			\$ - \
Urban Street		Ogden Avenue	_	_	nalysis		2017				sis Peri		> 7:00			<u></u>
Intersection		Ogden Avenue with			ile Nan	ne	Ogden <i>A</i>	venu	ie wi	tn Cros	s Street	- PME	X.xus	\dashv	K _A A	†
Project Descrip	tion	PM Existing Peak H	our												141	ሳ የ የ
Demand Inform	nation			Т		EB			٧	ΝB			NB		S	BB
Approach Move	ement				L	Т	R	L		T	R	L	Т	₹ L		T R
Demand (v), v	eh/h				44	1211	1	1	1	630 2	249	2	0 :	2 482	2	0 114
Ciamal Inform	4!						_	111:			-					
Signal Informa	r	D-f Db			L	2		W.						7		人
Cycle, s	140.0	Reference Phase		2	F	₹	₹	_ <u>"</u> :1	7				1	→ 2		3 4
Offset, s	0	Reference Point			reen		89.3	32.0				0.0		Δ		
Uncoordinated	No	Simult. Gap E/W			ellow ed		4.5	4.5				0.0	_/ .	Z		Ψ
Force Mode	Force Mode Fixed Simult. Gap N/S C					0.0	1.5	1.5	10	.0 0).0 (0.0	5	6	-	7 8
Saturation Flo	Saturation Flow / Delay					R	L	7	- 1	R	L	Т	R	L	Т	R
Lane Width Adj		•	7	1.000	1.000		1.000		\rightarrow	1.000	1.000	1.00	_	1.000	1.0	
	Heavy Vehicles and Grade Factor (f _{HVg})				0.992				\rightarrow	1.000	1.000	1.00	_		1.0	
	Parking Activity Adjustment Factor (f_P)			0.984 1.000	1.000				-	1.000	1.000	1.00			1.0	
Bus Blockage A				1.000	1.000				\rightarrow	1.000	1.000	1.00	_	_	1.0	
Area Type Adju	-	· , ,	7	1.000	1.000			_	_	1.000	1.000	1.00		_	1.0	
		ment Factor (fLU)		1.000	1.000		_		\rightarrow	1.000	1.000	1.00			1.0	
Left-Turn Adjus		. ,	7	0.952	0.000	+	0.232	_	\rightarrow		0.652	0.60	_	0.751	0.8	
Right-Turn Adju			1	0.002	1.000	+	_	0.9	\rightarrow	0.954	0.002	0.00			0.8	
		djustment Factor (fեր	b)	1.000			1.000	_			1.000			1.000	-	
		djustment Factor (f _{Rp}	_			1.000				1.000			1.000			1.000
Work Zone Adju			_	1.000	1.000			1.0	00	1.000	1.000	1.00			1.0	
		Flow Rate (s), veh/h		1781	3767	3	441	32	08	477	576	0	576	1426	0	1610
		Arriving on Green (P)	0.02	0.91	0.69	0.64	0.6	35	0.64	0.23	0.00	0.23	0.23	0.0	0 0.23
Incremental De				0.11	0.50	0.50	0.50	0.5	50	0.50		0.15	5	0.50	0.1	5
Signal Timing	/ Move	ment Groups	4	EB		EBT/R	WE	3L		/BT/R	NB	L	NBT/R	SB	L	SBT/R
Lost Time (t _L)	(2)		4	3.5		6.0	-			6.0	_	_	6.0	-		6.0
Green Ratio (g/			-	0.6		0.69	-			0.64	-	-	0.23	-		0.23
		low Rate (s_p) , veh/h/	n	220)	0	+		4	441	_	_	1292	+	_	1426
		v Rate (ssh), veh/h/ln	-	04.4	\leftarrow	0.0	-			20.0	_	-	0	+		20.0
Permitted Effect		(3.).	-	91.3	_	0.0				39.3 38.6			32.0 23.3			32.0
Permitted Servi		1- 1	+	39.0		0.0	-	_		0.1		-	0.0	+		23.3
Time to First Bl		(0.).	\dashv	0.0	_	0.0				0.0			2.0			0.0
		efore Blockage (<i>gt</i> s),	s	0.0		0.0				5.5			0.1			0.0
		<u> </u>	_										3.1			
	rotected Right Saturation Flow (s_R), veh/h/lr rotected Right Effective Green Time (g_R), s															
Multimodal					EB			V	VB			NB			S	В
Pedestrian F _w /	Fv		7	1.38		0.00	1.5			0.00	2.10		0.00	2.10		0.00
Pedestrian F _s /			7	0.00	_	0.078	0.0			.089	0.00	-	0.150	0.00	_	0.150
Pedestrian Mcor	ner / Mcv	/	7													
Bicycle c _b / d _b				1371.	43	6.91	1275	5.16	Ś	9.19	457.	15	41.66	457.	15	41.66
Bicycle Fw / Fv				-3.6	4	1.09	-3.6	64	1	1.63	-3.6	4	0.01	-3.6	64	1.04
													_			

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--- Messages ---

WARNING: If demand exceeds capacity, a multiple-period analysis should be conducted.

WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

--- Comments ---

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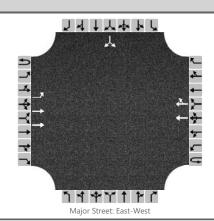
HCS7™ Streets Version 7.1

Generated: 12/12/2017 12:40:27 PM

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	HCS7 Two-Way Sto	p-Control Report	
General Information		Site Information	
Analyst	BSM	Intersection	Ogden with Max Madsen
Agency/Co.	KLOA, Inc.	Jurisdiction	IDOT
Date Performed	12/12/2017	East/West Street	Ogden Avenue
Analysis Year	2017	North/South Street	Max Madsen Access
Time Analyzed	AM Peak Hour	Peak Hour Factor	0.94
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	17-273 - Downers Grove	·	·

Lanes



Vehicle Volumes	and A	Adjus	tments
-----------------	-------	-------	--------

Approach	1	Eastbound				West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		0	0	0
Configuration		L	Т				Т	TR							LR	
Volume, V (veh/h)		0	1996				1300	3						0		1
Percent Heavy Vehicles (%)		0												0		0
Proportion Time Blocked																
Percent Grade (%)														(0	
Right Turn Channelized		No				١	10			N	lo			N	lo	
Median Type/Storage		Left O											1			

Critical and Follow-up Headways

Base Critical Headway (sec)	4.1						7.5	6.9
Critical Headway (sec)	4.10						6.80	6.90
Base Follow-Up Headway (sec)	2.2						3.5	3.3
Follow-Up Headway (sec)	2.20						3.50	3.30

Delay, Queue Length, and Level of Service

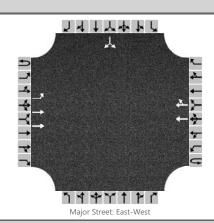
Flow Rate, v (veh/h)	0								1	
Capacity, c (veh/h)	501								391	
v/c Ratio	0.00								0.00	
95% Queue Length, Q ₉₅ (veh)	0.0								0.0	
Control Delay (s/veh)	12.2								14.2	
Level of Service, LOS	В								В	
Approach Delay (s/veh)	0.0						14	1.2		

Approach LOS

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	HCS7 Two-Way Sto	p-Control Report	
General Information		Site Information	
Analyst	BSM	Intersection	Ogden with Max Madsen
Agency/Co.	KLOA, Inc.	Jurisdiction	IDOT
Date Performed	12/12/2017	East/West Street	Ogden Avenue
Analysis Year	2017	North/South Street	Max Madsen Access
Time Analyzed	PM Peak Hour	Peak Hour Factor	0.94
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	17-273 - Downers Grove	·	·

Lanes



Vehicle V	/olumes	and A	djustments
-----------	---------	-------	------------

Approach		Eastk	oound			Westi	oound			North	bound			Southbound		
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		0	0	0
Configuration		L	Т				Т	TR							LR	
Volume, V (veh/h)		2	1691				1865	1						1		7
Percent Heavy Vehicles (%)		0												0		0
Proportion Time Blocked																
Percent Grade (%)														(0	
Right Turn Channelized	No					Ν	lo			Ν	lo			N	lo	
Median Type/Storage				Left	Only								1			

Critical and Follow-up Headways

Base Critical Headway (sec)	4.1						7.5	6.9
Critical Headway (sec)	4.10						6.80	6.90
Base Follow-Up Headway (sec)	2.2						3.5	3.3
Follow-Up Headway (sec)	2.20						3.50	3.30

Delay, Queue Length, and Level of Service

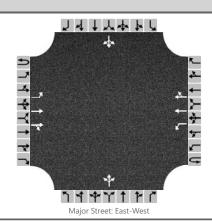
Flow Rate, v (veh/h)	2								8	
Capacity, c (veh/h)	295								190	
v/c Ratio	0.01								0.04	
95% Queue Length, Q ₉₅ (veh)	0.0								0.1	
Control Delay (s/veh)	17.3								24.7	
Level of Service, LOS	С								С	
Approach Delay (s/veh)	0.0							24	.7	

Approach LOS

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	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	BSM	Intersection	Ogden with Gerber/Fairway
Agency/Co.	KLOA, Inc.	Jurisdiction	IDOT
Date Performed	12/12/2017	East/West Street	Ogden Avenue
Analysis Year	2017	North/South Street	Gerber/Fairway Access
Time Analyzed	AM Peak Hour	Peak Hour Factor	0.95
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	17-273 - Downers Grove		

Lanes



Vehicle Volumes	and	Adjus	tments
-----------------	-----	-------	--------

Approach		Eastb	ound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration		L	Т	TR		L	Т	TR			LTR				LTR	
Volume, V (veh/h)		4	1990	2		0	1293	7		10	0	32		4	0	0
Percent Heavy Vehicles (%)		0				0				0	0	0		0	0	0
Proportion Time Blocked																
Percent Grade (%)										(0				0	
Right Turn Channelized		No				١	10			Ν	lo			Ν	10	
Median Type/Storage		No Left O											1			

Critical and Follow-up Headways

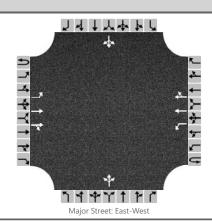
base Critical Headway (sec)								
Critical Headway (sec)								
Base Follow-Up Headway (sec)								
Follow-Up Headway (sec)								

Flow Rate, v (veh/h)		4			0				45			4	
Capacity, c (veh/h)		509			267				114			88	
v/c Ratio		0.01			0.00				0.39			0.05	
95% Queue Length, Q ₉₅ (veh)		0.0			0.0				1.6			0.1	
Control Delay (s/veh)		12.1			18.5				55.7			47.7	
Level of Service, LOS		В			С				F			Е	
Approach Delay (s/veh)		0.0			0	.0		55	.7		47	7.7	
Approach LOS								F				E	

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	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	BSM	Intersection	Ogden with Gerber/Fairway
Agency/Co.	KLOA, Inc.	Jurisdiction	IDOT
Date Performed	12/12/2017	East/West Street	Ogden Avenue
Analysis Year	2017	North/South Street	Gerber/Fairway Access
Time Analyzed	PM Peak Hour	Peak Hour Factor	0.94
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	17-273 - Downers Grove		

Lanes



Vehic	le Vo	olumes	and	Adj	justments
-------	-------	--------	-----	-----	-----------

Approach		Eastbound R				West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration		L	Т	TR		L	Т	TR			LTR				LTR	
Volume, V (veh/h)		3	1675	14		13	1853	2		4	0	14		2	0	9
Percent Heavy Vehicles (%)		0				0				0	0	0		0	0	0
Proportion Time Blocked																
Percent Grade (%)										()				0	
Right Turn Channelized		No				Ν	10			N	lo			Ν	lo	
Median Type/Storage		No Left On											1			

Critical and Follow-up Headways

base Chilical Headway (sec)								
Critical Headway (sec)								
Base Follow-Up Headway (sec)								
Follow-Up Headway (sec)								

Flow Rate, v (veh/h)	3				14				19			12	
Capacity, c (veh/h)	298				348				159			145	
v/c Ratio	0.01				0.04				0.12			0.08	
95% Queue Length, Q ₉₅ (veh)	0.0				0.1				0.4			0.3	
Control Delay (s/veh)	17.2				15.8				30.7			32.0	
Level of Service, LOS	С				С				D			D	
Approach Delay (s/veh)	0.0				0.	1		30	.7		32	0	
Approach LOS))	

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		ı	HCS7	Signa	alized	l Inter	sectio	on In	put Da	ata					
General Inform	ation								Intersec	tion Inf	ormatio	nn .	<u></u>	الماليات	إمالي
Agency	ation	KLOA, Inc.						_	Duration.		0.25	/ 11		41	
Analyst		BSM		Analys	sis Date	Dec 1	2 2017	_	Area Typ		Other				,
Jurisdiction		IDOT		Time F			eak Hou	-	PHF		0.96		_ ^	w∮E	÷
Urban Street		Ogden Avenue			sis Year		July 110u		Analysis	Period	1> 7:0	20	_ \		~
Intersection		Ogden Avenue with	ı Cro	File Na			Avenu		Cross St						<u> </u>
Project Descript	ion	AM Projected Peak		T IIC TVC		Toguci	TYVOTIO	C With	01033 01	1001-71	IVII TX.XC	15	18	শ বিশ্বপ	r r
Demand Inform	nation				EB			WE	3	1	NB			SB	
Approach Move	ment			L	Т	R	L	Т	R		Т	R	L	Т	R
Demand (v), ve				97	1803		0	96		0	0	1	249	0	18
20 (7), 11															
Signal Informat	tion					5			\neg	\top					I
Cycle, s	140.0	Reference Phase	2		B		5 A	2				_	4		4
Offset, s	0	Reference Point	Begin	Green	A 3	97.2	23.0	0.0	0.0	0.0		1	¥ 2	3	
Uncoordinated	No	Simult. Gap E/W	On	Yellow		4.5	4.5	0.0		0.0		7	}		KŤ:
Force Mode	Fixed	Simult. Gap N/S	On	Red	0.0	1.5	1.5	0.0		0.0		5	6	7	Y
T (C) C								\A/D			ND			0.0	
Traffic Informat					EB		H.	WB		-	NB			SB	
Approach Move				L	T	R	L	T	R	L	T	R	L	T	R
Demand (v), veh		1		97	1803	0	0	964	372	0	0	1	249	0	18
Initial Queue (Q				0	0	0	0	0	0	0	0	0	0	0	0
Base Saturation		Rate (s₀), veh/h		1900	1900	1900	1900	1900		1900	1900	1900	1900	1900	1900
Parking (N _m), ma		24	-		None	\blacksquare		None)		None			None	
Heavy Vehicles	<u> </u>	<u>%</u>		0	2		0	3			0		2	0	
Ped / Bike / RTC	·			0	0		0	0	0	0	0	0	0	0	0
Buses (Nb), buse				0	0	0	0	0	0	0	0	0	0	0	0
Arrival Type (AT				3	4	3	3	4	3	3	3	3	3	3	3
Upstream Filteri				1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Lane Width (W),				12.0 100	12.0		12.0	12.0			12.0		12.0 300	12.0	
Turn Bay Length	1, 11		-	100	0		0	0			0		300	0	_
Grade (<i>Pg</i>), % Speed Limit, mi/	/h			40	40	40	35	35	35	15	15	15	30	30	30
Speed Limit, mir	11			40	40	40	33	33	33	10	10	13	30	30	30
Phase Informat				EBL	-	EBT	WBL	-	WBT	NBL	-	NBT	SBL	-	SBT
) or Phase Split, s		14.0		111.0	<u> </u>	\perp	97.0			29.0		\rightarrow	29.0
Yellow Change I		· ·		3.5	_	4.5			4.5			4.5			4.5
Red Clearance I				0.0	\perp	1.5			1.5			1.5		\perp	1.5
Minimum Green				3		15	6	\perp	15	6		8	3		8
Start-Up Lost Tir				2.0		2.0	2.0	_	2.0	2.0		2.0	2.0		2.0
Extension of Effe		Green (e), s		2.0		2.0	2.0	-	2.0	2.0	_	2.0	2.0		2.0
Passage (PT), s	•			3.0		7.0	2.0	-	7.0	2.0		4.0	2.0		4.0
Recall Mode				Off	_	Min	Off	-	Min	Off	_	Off	Off	_	Off
Dual Entry				Yes	_	Yes	No	_	Yes	No	_	Yes	No		Yes
Walk (Walk), s		(5.5)		0.0	_	0.0	0.0	_	0.0	0.0		0.0	0.0		0.0
Pedestrian Clea	rance 7	īme (<i>PC</i>), s		0.0		0.0	0.0		0.0	0.0		0.0	0.0		0.0
Multimodal Info	ormatio	on			EB			WB			NB			SB	
85th % Speed /	Rest in	Walk / Corner Radi	us	0	No	25	0	No	25	0	No	25	0	No	25
Walkway / Cross	swalk V	Vidth / Length, ft		9.0	12	0	9.0	12	0	9.0	12	0	9.0	12	0
Street Width / Is	land / 0	Curb		0	0	No	0	0	No	0	0	No	0	0	No
Width Outside /	Bike La	ane / Shoulder, ft		12	5.0	2.0	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0
		cupied Parking		No		0.50	No		0.50	No		0.50	No		0.50

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		HCS	7 Sig	nalize	d Int	ersec	tion F	Resu	Its Sur	nmar	У				
General Inform		V.							Intersect			on		1 1 7 4 1	Pr /
Agency		KLOA, Inc.							Duration,	h	0.25			* *	_
Analyst		BSM		Analys	is Date	e Dec 1	2, 2017		Area Typ	е	Other		≯≯		ح.
Jurisdiction		IDOT		Time F	Period	AM P	eak Hou	ır	PHF		0.96		♦ →	w ‡ E 8	←
Urban Street		Ogden Avenue		Analys	is Yea	r 2023			Analysis	Period	1> 7:0	00	₹		
Intersection		Ogden Avenue with	n Cro	File Na	ame	Ogde	n Avenu	e with	Cross St	reet - A	MPR.xu	ıs		*	
Project Descrip	tion	AM Projected Peak	Hour										*	14147	7 1
Demand Inforr	nation				EB			W	В		NB			SB	
Approach Move				L	Т	R	L	T			Т	R		T	R
Demand (v), v				97	1803	_	0	96	_	0	0	1	249	0	18
Domana (1), 1	011/11			Ü,	1000				0.2				2.0		10
Signal Informa	tion									\top					1
Cycle, s	140.0	Reference Phase	2	1	E .	\parallel	<u></u> ⊵.4	2				_	$\boldsymbol{\leftrightarrow}$		4
Offset, s	0	Reference Point	Begin	Green	1 3	97.2	23.0	0.0	0.0	0.0		1	2	3	
Uncoordinated	No	Simult. Gap E/W	On	Yellow		4.5	4.5	0.0		0.0		7	}		κŤ
Force Mode	Fixed	Simult. Gap N/S	On	Red	0.0	1.5	1.5	0.0		0.0		5	6	7	Y
					-										
Timer Results				EBI	-	EBT	WB	L	WBT	NB	-	NBT	SBI	-	SBT
Assigned Phase	e			5	_	2		_	6			8			4
Case Number				1.0		4.0		_	6.3			8.0			6.0
Phase Duration				7.8	_	111.0		_	103.2			29.0			29.0
Change Period	, (Y+R	c), S		3.5		6.0			6.0			6.0			6.0
Max Allow Head		· · · · · · · · · · · · · · · · · · ·		4.0		0.0			0.0			5.1			5.1
Queue Clearan	ce Time	e (g s), s		4.2								2.1			25.0
Green Extension		(g e), s		0.1		0.0			0.0			1.4			0.0
Phase Call Pro	bability			1.00)							1.00			1.00
Max Out Proba	bility			0.10)							0.00			1.00
M	D							١٨/٦			ND			OD	
Movement Gro	-	suits			EB	Τ.		WE			NB -			SB	
Approach Move				L	T	R	L	T	R	L	T	R	L	T	R
Assigned Move		·		5	2	12	1	6	16	3	8	18	7	4	14
Adjusted Flow F		,		101	1878	+	0	724			0		259	19	
		ow Rate (s), veh/h/	in	1810	1870	0	246	1850			0		1416	1610	
Queue Service				2.2	0.0	0.0	0.0	8.4			0.0		22.9	1.4	
Cycle Queue C		e Time (g c), s		2.2	0.0	0.0	0.0	8.4			0.0		23.0	1.4	
Green Ratio (g				0.74	0.75		0.69	0.69					0.16	0.16	
Capacity (c), v		tia (V)		321	2806	0.000	51	1289			0.000		283	265	
Volume-to-Capa)	0.315	_	+	0.000	0.56			0.000		0.915	0.071 25.8	
	· ,	/In(95 th percentile eh/In(95 th percent		34.8 1.4	0.9	0.0	0.0	107. 4.2			0.0		17.6	1.0	
		RQ) (95 th percen		0.35	0.00	0.00	0.00	0.00	_		0.00		1.49	0.00	
Uniform Delay	•	· · ·	,	7.6	0.0	-	0.0	1.9					59.8	49.5	
Incremental De	, ,			0.6	1.3	0.0	0.0	1.8	\rightarrow		0.0		32.7	0.2	
Initial Queue De	- '	,		0.0	0.0	0.0	0.0	0.0			0.0		0.0	0.0	
Control Delay (8.2	1.3		0.0	3.7					92.4	49.6	
Level of Service				A	A			A	A				F	D	
Approach Delay				1.6		Α	6.1		Α	48.9	9	D	89.5		F
Intersection De							0.0						В		
Multime and all D								100			ND			0.5	
Multimodal Re		// 00		0.0	EB	D	0.0	WE		0.0	NB		0.0	SB	
Pedestrian LOS				2.0		В	2.2	_	В	2.9		C	2.9	_	C
Bicycle LOS So	ore / LC	JS		2.1		В	1.6		В	0.5		Α	0.9		Α

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		HCS7	Siç	gnal	ized	Inter	sectio	n In	ter	media	ite Va	lues					
Canaval Informa	tion									Intore	ootion.	Inform	antin:				l b.L.
General Informa	- 1	I/I OA Ina								-	section	-		1	- 1	.↓ Ļ	1 4 A
Agency		KLOA, Inc.				D 1	D 40	004	,	Durat		_	.25		_1		<u>.</u>
Analyst		BSM		\rightarrow			Dec 12,			Area	туре	-	ther			w∳E	4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4
Jurisdiction		IDOT		-	me Pe		AM Pea	к но	ur	PHF		_	.96			8 8	· -
Urban Street		Ogden Avenue	_		nalysis		2023				sis Peri		> 7:00				<u>~</u>
Intersection		Ogden Avenue with 0			le Nar	ne	Ogden /	Avenu	ie wi	th Cross	Street	- AMP	'R.xus	3	_	•	
Project Description	on	AM Projected Peak F	lou	r	_									_		ጎላ ተቀ"	777
Demand Informa	ation			Т		EB			١	NB			NB			SB	
Approach Movem	nent				L	Т	R	L		Т	R	L	Т	R	L	Т	R
Demand (v), vel	h/h				97	1803	0	0	ç	964 3	372	0	0	1	249	0	18
Signal Informati	11	D (D)	_		L	2	. , 5	W.							A		人
	140.0	Reference Phase	2		F	₹		- 54	2					1	↔ ₂	3	4
Offset, s	0		3eg	— ს	reen	4.3	97.2	23.0	0	.0 C	0.0	0.0			Ā		
Uncoordinated	No	Simult. Gap E/W	Or		ellow		4.5	4.5	_			0.0	_/	7	Z		₩.
Force Mode F	Fixed	Simult. Gap N/S	Or	ı <u>R</u>	ed	0.0	1.5	1.5	0	.0 0	0.0	0.0		5	6	7	8
Saturation Flow	/ Dolo		T	1	Т	R		1 -	Т	R		Т		R	1	Т	R
Lane Width Adjus		•	1	.000	1.000		1.000		000	1.000	1.000	1.00	0 1	.000	1.000	1.000	_
			-	.000	0.984	_			977	1.000	1.000	1.00	_	.000	0.984	1.000	
	avy Vehicles and Grade Factor (f _{HVg}) king Activity Adjustment Factor (f _P)						1.000	\rightarrow	000	1.000	1.000	1.00	_	.000	1.000	1.000	
Bus Blockage Ad		,	-	.000	1.000	_			000	1.000	1.000	1.00	_	.000	1.000	1.000	_
Area Type Adjust	-	· , ,	-	.000	1.000	_			000	1.000	1.000	1.00	_	.000	1.000	1.000	1.000
Lane Utilization A		· · ·	_	.000	1.000				000	1.000	1.000	1.00	-	.000	1.000	1.000	\rightarrow
Left-Turn Adjustm		, ,	-).952	0.000		0.130	_	000		1.000	0.84	_		0.745	0.000	
Right-Turn Adjust			Ť		1.000	_		_	905	0.905	1.000	0.00	_	.847	0.7 10	0.847	\rightarrow
		ljustment Factor (f _{Lpb})	1	.000		1100	1.000	_		0.000	1.000	0.00			1.000	0.0	0.011
		justment Factor (f _{Rpb})	-			1.000				1.000			1	.000			1.000
Work Zone Adjus		. , ,	-	.000	1.000			1.0	000	1.000	1.000	1.00	_	.000	1.000	1.000	-
		low Rate (s), veh/h	-	1810	3741	_	246		60	975	0	0	_	610	1416	0	1610
		Arriving on Green (P)	-	0.03	1.00	0.00		_	93	0.69	0.00	0.00	_	0.16	0.16	0.00	0.16
Incremental Dela			-	0.11	0.50	_	1		50	0.50					0.44	0.15	
			ì														
Signal Timing / I	Moven	nent Groups	I	EBI	-	EBT/R	WI	ВL	W	/BT/R	NB	L	NBT	Γ/R	SBL	.	SBT/R
Lost Time (t _L)			L	3.5		6.0				6.0			6.0	0			6.0
Green Ratio (g/C	;)		L	0.74	l _	0.75			(0.69			0.1	6			0.16
		ow Rate (<i>s₀</i>), veh/h/ln	L	394		0				246			141	16			1416
		Rate (ssh), veh/h/ln											0				
Permitted Effective		(3.).	1	99.2	_	0.0			_	0.0			0.0	_			23.0
Permitted Service		,- ,	1	76.0		0.0	-			0.0			0.0	0			22.9
Permitted Queue		(3.).	1	8.0	_		_		-	0.0		_				_	22.9
Time to First Bloo		,	1	0.0		0.0	-			0.0			23.	.0		\perp	0.0
		efore Blockage (gfs), s	_		\perp		-					_				\perp	
		ion Flow (s _R), veh/h/lr	1				-										
	=πectiv	e Green Time (g _R), s	╄														
Multimodal	-		+	4.00	EB	0.00	1 -		VB	2.00	0.10	NE		\ <u>\</u>	0.40	SB	0.00
Pedestrian F _w / F			+	1.38	_	0.00	1.5		_	0.00	2.10	-	0.0	_	2.10	_	0.00
Pedestrian F _s / F _d			+	0.00	U	0.059	0.0	UU	U	.075	0.00	10	0.1	00	0.000	J	0.156
Pedestrian Mcorne	er I IVI cw		+	1500	00	4.20	4000	0.04		2 5 2	200	5.7	40.4	90	200.5	7	40.00
Bicycle <i>c_b</i> / <i>d_b</i> Bicycle <i>F_w</i> / <i>F_v</i>			+	1500. -3.6	_	4.38 1.63	1388		_	6.53 1.15	-3.6	_	48.8	_	328.5 -3.6 ²		48.89 0.46
Dicycle Fw / Fv				-3.04	†	1.03	-3.0	U 4		1.10	_∥ -ა.ნ	4	0.0	,0	-3.04	<u> </u>	0.40

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--- Messages ---

WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

--- Comments ---

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HCS7	Signa	alized	Inter	sectio	on In	put Da	ata				r uş	ge or or
General Information						Intersec	tion Inf	ormatic	nn	<u> </u>	14141	₽ ľ
Agency KLOA, Inc.					_	Duration,		0.25	711		47	
Analyst BSM	Analys	sis Date	Dec 1	2 2017		Area Typ		Other				<u>*</u> _ &
Jurisdiction IDOT	Time F		+	eak Hou	-	PHF	<u> </u>	0.95		^	w∔e	<u>*</u> }-
Urban Street Ogden Avenue		is Year		ak i iou		Analysis	Poriod	1> 7:0	20	_ _ ₹ _ ₹		√
	—		_	Ανοριι								
Intersection Ogden Avenue with Cro Project Description PM Projected Peak Hour	File Na	ame	Ogder	1 Avenu	e with	Cross St	reet - P	MPR.XI	JS	- ×	* 1 4 Y	7 4
Demand Information		ГР		1	۱۸/۱	.		ND			SB	
		EB		٠.	WI		٠.	NB	Т Б	+ -	1	
Approach Movement	L	T	R	L	T		L	T	R	L 404	T	R
Demand (v), veh/h	45	1245	1	1	167	77 249	2	0	2	494	0	116
Signal Information	1	Т		1 11:								
Cycle, s 140.0 Reference Phase 2		<u> -</u> 2		- α ₄₂	_					Z	l l	小
Offset, s 0 Reference Point Begin			-3				\perp		1	2	3	4
Uncoordinated No Simult. Gap E/W On	Green		89.2	32.0	0.0		0.0		_	5		-4-
Force Mode Fixed Simult. Gap N/S On	Yellow Red	0.0	4.5 1.5	4.5 1.5	0.0		0.0		5	6	7	Y
Porce Mode Pixed Simult. Gap N/3 On	Neu	10.0	1.5	1.5	10.0	0.0	10.0	-	3		-	0
Traffic Information		EB			WB			NB			SB	
Approach Movement		T	R	L	T	R	L	T	R		T	R
Demand (v), veh/h	45	1245	1	1	1677		2	0	2	494	0	116
Initial Queue (Q _b), veh/h	0	0	0	0	0	0	0	0	0	0	0	0
Base Saturation Flow Rate (s _o), veh/h	1900	1900	1900	1900	1900		1900	1900	1900	1900	1900	1900
Parking (N _m), man/h	1900	None	1900	1900	None		1900	None	1900	1900	None	1900
Heavy Vehicles (<i>Phv</i>), %	2	1		0	1	5		0		1	0	_
Ped / Bike / RTOR, /h	0	0	0	0	0	0	0	0	0	0	0	
·	0	0	0	0	0	0	_	0	0	0	0	0
Buses (Nb), buses/h	3	4	3	3	4	3	3	3	3	3	_	3
Arrival Type (AT)			_			_		_	-	_	3	-
Upstream Filtering (/)	1.00	1.00	1.00	1.00	1.00		1.00	1.00	1.00	1.00	1.00	1.00
Lane Width (W), ft	12.0			12.0	12.0			12.0			_	-
Turn Bay Length, ft	100	0		0	0			0		300	0	-
Grade (<i>Pg</i>), %	40	0	40	25	0	0.5	45	0	45	20	0	20
Speed Limit, mi/h	40	40	40	35	35	35	15	15	15	30	30	30
Phase Information	EBL	.	EBT	WBL	-	WBT	NBL	-	NBT	SBL	-	SBT
Maximum Green (<i>G_{max}</i>) or Phase Split, s	11.0) 1	02.0			91.0			38.0			38.0
Yellow Change Interval (Y), s	3.5		4.5			4.5			4.5			4.5
Red Clearance Interval (Rc), s	0.0		1.5			1.5			1.5			1.5
Minimum Green (Gmin), s	3		15	6		15	6		8	3		8
Start-Up Lost Time (It), s	2.0		2.0	2.0		2.0	2.0		2.0	2.0		2.0
Extension of Effective Green (e), s	2.0		2.0	2.0		2.0	2.0		2.0	2.0		2.0
Passage (<i>PT</i>), s	3.0		7.0	2.0		7.0	2.0		4.0	2.0		4.0
Recall Mode	Off		Min	Off		Min	Off		Off	Off		Off
Dual Entry	Yes		Yes	No		Yes	No		Yes	No		Yes
Walk (<i>Walk</i>), s	0.0		0.0	0.0		0.0	0.0		0.0	0.0		0.0
Pedestrian Clearance Time (PC), s	0.0		0.0	0.0		0.0	0.0		0.0	0.0	\bot	0.0
Multimodal Information		EB			WB			NB			SB	
85th % Speed / Rest in Walk / Corner Radius	0	No	25	0	No	25	0	No	25	0	No	25
Walkway / Crosswalk Width / Length, ft	9.0	12	0	9.0	12	0	9.0	12	0	9.0	12	0
Street Width / Island / Curb	0	0	No	0	0	No	0	0	No	0	0	No
Width Outside / Bike Lane / Shoulder, ft	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0	12	5.0	2.0
Pedestrian Signal / Occupied Parking	No		0.50	No		0.50	No		0.50	No		0.50

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		HCS	7 Sig	nalize	d Int	ersec	tion F	Resi	ılts Sı	mma	ary					
									1 -							
General Inforn	nation	,							Interse		V		n		1 1	Da L
Agency		KLOA, Inc.							Duratio		0.2					
Analyst		BSM				e Dec 1	2, 2017		Area Ty	ре		her		≯≯		ے.
Jurisdiction		IDOT		Time F			eak Hou	ır	PHF		0.9	95		♦ ₹	w	←
Urban Street		Ogden Avenue		Analys	sis Yea				Analysi			7:0		*		,
Intersection		Ogden Avenue with	n Cro	File N	ame	Ogde	n Avenu	e wit	h Cross	Street -	PMPF	₹.xu	IS		*	
Project Descrip	tion	PM Projected Peak	k Hour											*	4 1 4 Y	7 1
Demand Inforr	nation				EB			V	/B		1	ΝB			SB	
Approach Move	ement			L	Т	R	L	Τ-	T R		_	Т	R	L	T	R
Demand (v), v	eh/h			45	1245	5 1	1	16	77 24	9 2	2	0	2	494	0	116
Signal Informa	tion			1			. :			-						
Signal Informa	ır	Defense Dhana				17								,		人
Cycle, s	140.0		2		R		` SA	2					1	☆ 2	3	4
Offset, s	0	Reference Point	Begin	Green		89.2	32.0	0.						<u> </u>		
Uncoordinated	No	Simult. Gap E/W	On	Yellow		4.5	4.5	0.				_/	^ _	Y		Ψ
Force Mode	Fixed	Simult. Gap N/S	On	Red	0.0	1.5	1.5	0.	0.0	0	.0		5	6	7	8
Timer Results				EBI		EBT	WB	L	WBT	N	IBL		NBT	SBI	_	SBT
Assigned Phas	e			5		2			6				8			4
Case Number				1.0		4.0			6.3				8.0			6.0
Phase Duration	1, S			6.8		102.0		\neg	95.2			- (38.0		\neg	38.0
Change Period		c). s		3.5	_	6.0			6.0				6.0			6.0
Max Allow Hea	•	,		4.0	_	0.0		\neg	0.0				5.1		\neg	5.1
Queue Clearan				3.2	_			\neg					10.9			34.0
Green Extension		,		0.0		0.0		\neg	0.0				3.9			0.0
Phase Call Pro		(90),0		1.00	-	0.0		$\overline{}$	0.0	1			1.00			1.00
Max Out Proba				0.72	_			_					0.04		_	1.00
				-												
Movement Gro	oup Res	sults			EB			WI	3		N	В			SB	
Approach Move	ement			L	Т	R	L	Т	R	L	Т		R	L	Т	R
Assigned Move	ment			5	2	12	1	6	16	3	8		18	7	4	14
Adjusted Flow I	Rate (v	'), veh/h		47	656	656	1	101	4 1014		4			520	122	
Adjusted Satura	ation Flo	ow Rate (s), veh/h/	'In	1781	1885	1885	426	188	5 1801		114	14		1426	1610	
Queue Service	Time (g s), s		1.2	7.8	7.8	0.1	39.	9 54.2		0.0	0		23.1	8.9	
Cycle Queue C	learanc	e Time (<i>g c</i>), s		1.2	7.8	7.8	1.2	39.	9 54.2		8.9	9		32.0	8.9	
Green Ratio (g	/C)			0.67	0.69	0.69	0.64	0.6	4 0.64		0.2	23		0.23	0.23	
Capacity (c), v	/eh/h			145	1293	1292	320	120	2 1148		30	0		287	368	
Volume-to-Cap	acity Ra	atio (X)		0.326	0.507	0.507	0.003	0.84	14 0.883	3	0.0	14		1.811	0.332	
Back of Queue	(Q), ft	/In (95 th percentile)	36.7	102.8	102.3	0.6	334	4 596.8	3	5.4	4		1624.6	164.6	
Back of Queue	(Q), v	eh/ln (95 th percent	tile)	1.4	4.1	4.1	0.0	13.	3 23.9		0.:	2		64.5	6.6	
Queue Storage	Ratio (RQ) (95 th percen	itile)	0.37	0.00	0.00	0.00	0.0	0.00		0.0	0		5.42	0.00	
Uniform Delay	(d 1), s	/veh		22.2	2.2	2.2	9.6	6.8	3 10.8		41	9		60.5	45.1	
Incremental De	lay (d 2	2), s/veh		1.3	1.4	1.4	0.0	7.3	9.9		0.0	0		378.6	0.7	
Initial Queue De	elay (<i>d</i>	з), s/veh		0.0	0.0	0.0	0.0	0.0	0.0		0.0	0		0.0	0.0	
Control Delay (d), s/v	eh		23.5	3.6	3.7	9.6	14.	1 20.7		42	.0		439.1	45.8	
Level of Service	e (LOS)			С	Α	А	Α	В	С		D			F	D	
Approach Delay	y, s/veh	/LOS		4.3		Α	17.4	1	В	4.	2.0		D	364.	3	F
Intersection De	lay, s/ve	eh / LOS				68	3.3							E		
Multimodal Re	oulto				ГР			WI))		N.I	D			SB	
Pedestrian LOS		/1.08		2.1	EB	В	2.2	-	<u>В</u>	-	N 2.9	ם	С	2.9	-	С
Bicycle LOS So						В	_	_		_	_				_	
Dicycle LOS Sc	OIE / LC	J.3		1.6		D	2.2		В).5		Α	1.5		В

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		HCS7	Si	ignal	ized	Inters	sectio	n In	teri	media	ite Va	lues					
General Inform	nation									Intore	section	Inform	ation				I b L
	iation	KI OA Ina								Durat		- 1	25		- 1	41	
Agency		KLOA, Inc. BSM		Ι.Δ	ما درام ما	Data	D 10	2047									K.
Analyst				_			Dec 12,			Area	туре	_	ther			w∱E	**************************************
Jurisdiction		IDOT			ime Pe		PM Pea	к нос	ır	PHF	:- D:		95			8	
Urban Street		Ogden Avenue	0		nalysis		2023				sis Peri		> 7:00				<u></u>
Intersection	4	Ogden Avenue with			ile Nan	ne	Ogden <i>A</i>	wenu	ie wi	th Cros	s Street	- PMP	R.xus		- 4	*	V 4. 7
Project Descrip	tion	PM Projected Peak	НО	ur												ነ ተተ	
Demand Inform	nation			т		EB			٧	ΝB			NB			SE	
Approach Move	ement				L	Т	R	L		T	R	L	Т	R	L	Т	R
Demand (v), v	eh/h				45	1245	1	1	1	677 2	249	2	0	2	494	0	116
Signal Informa	r					2	, Ņ	ولل							_		人
Cycle, s	140.0	Reference Phase		2	F	₹	≝ 💆	<u>"</u> ?↑	2					1	♦ 2	3	4
Offset, s	0	Reference Point		gin	reen	3.3	89.2	32.0	0	.0 0	0.0	0.0			Ā		
Uncoordinated	No	Simult. Gap E/W			ellow		4.5	4.5				0.0			7		$ \Psi $
Force Mode	Fixed	Simult. Gap N/S	C	n R	ed	0.0	1.5	1.5	0	.0 0	0.0	0.0		5	6	7	8
Saturation Flo	w / Dola	av	7	<u> </u>	Т	R		T	- 1	R	1	Т	R	_	1	Т	R
Lane Width Adj		•	7	1.000	1.000		1.000		\rightarrow	1.000	1.000	1.00	_	_	1.000	1.000	_
		ade Factor (f _{HVg})	1	0.984	0.992	_		-	\rightarrow	1.000	1.000	1.00	_	-	0.992	1.000	
Parking Activity		· · · · ·	-	1.000	1.000			-	-	1.000	1.000	1.00	_	-	1.000	1.000	
Bus Blockage A		,	1	1.000	1.000	_			\rightarrow	1.000	1.000	1.00	_	_	1.000	1.000	_
Area Type Adju	-	· , ,	7	1.000	1.000	_		_	_	1.000	1.000	1.00	_	-	1.000	1.000	
		ment Factor (f _{LU})	-	1.000	1.000		_		\rightarrow	1.000	1.000	1.00		\rightarrow	1.000	1.000	
Left-Turn Adjus		. ,	+	0.952	0.000		0.224	_	\rightarrow	1.000	0.647	0.60	_		0.751	0.000	
Right-Turn Adju			1	0.332	1.000	+	_	0.9	\rightarrow	0.956	0.047	0.00	_	12	0.731	0.847	\rightarrow
		djustment Factor (f _{Lp}	b)	1.000	1.000	1.000	1.000	_		0.000	1.000	0.00	0.00	-	1.000	0.017	0.017
		djustment Factor (<i>f</i> _{Rp}	-			1.000				1.000			1.00	00			1.000
Work Zone Adju			7	1.000	1.000			1.0	00	1.000	1.000	1.00		_	1.000	1.000	
		Flow Rate (s), veh/h	T	1781	3767	3	426	32	21	466	572	0	57:	2	1426	0	1610
Proportion of Ve	ehicles i	Arriving on Green (P)	0.02	0.91	0.69	0.64	0.8	35	0.64	0.23	0.00	0.2	3	0.23	0.00	0.23
Incremental De				0.11	0.50	0.50	0.50	0.5	50	0.50		0.15	5		0.50	0.15	
						-0											
Signal Timing	/ Move	ment Groups	4	EB		EBT/R	WE	3L		/BT/R	NB	L	NBT/F	₹	SBL		SBT/R
Lost Time (t _L)			4	3.5		6.0	_			6.0		_	6.0	4		_	6.0
Green Ratio (g/			4	0.6	_	0.69	-			0.64			0.23	4			0.23
		low Rate (s _ρ), veh/h/	n	210)	0	-		4	426	_	_	1289	4		_	1426
		v Rate (ssh), veh/h/ln	4				+			20.0			0	-		_	
Permitted Effec		(3.).	4	91.2		0.0				39.2			32.0			_	32.0
Permitted Servi				35.0		0.0				38.2			23.1			-	23.1
Permitted Queu		(3.).	-	16.4	_	0.0				0.1			0.0				23.1
Time to First Bl		(<i>g_f</i>), s efore Blockage (<i>g_{fs}</i>),		0.0		0.0				0.0			0.1				0.0
		tion Flow (s_R), veh/h.	_										0.1			+	
		ve Green Time (g_R),	-		-											-	
Multimodal		is crosm mile (gR),			EB			١٨	VB			NB				SB	
Pedestrian F _w /	Fv		-	1.38		0.00	1.5			0.00	2.10		0.00		2.107		0.00
Pedestrian F _s /			1	0.00	_	0.078	0.00			.089	0.00	-	0.150	-	0.000	_	0.00
Pedestrian Mcor	· ·	/	7	0.00		2.310	3.00		- 0	.550	0.00		0.100	-	0.000		3.100
Bicycle <i>c_b</i> / <i>d_b</i>				1371.	43	6.91	1274	.74	Ç	9.20	457.	15	41.66		457.1	5	41.66
Bicycle F_w / F_v			7	-3.6	_	1.12	-3.6			1.67	-3.6		0.01		-3.64	_	1.06

--- Messages ---

WARNING: If demand exceeds capacity, a multiple-period analysis should be conducted.

WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

--- Comments ---

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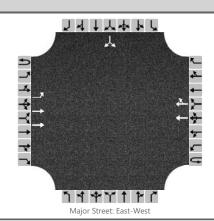
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	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	BSM	Intersection	Ogden with Max Madsen
Agency/Co.	KLOA, Inc.	Jurisdiction	IDOT
Date Performed	12/12/2017	East/West Street	Ogden Avenue
Analysis Year	2023	North/South Street	Max Madsen Access
Time Analyzed	AM Peak Hour	Peak Hour Factor	0.94
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	17-273 - Downers Grove		

Lanes



Vehicle	Volumes	and A	Adjustments
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Approach		Eastk	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	T	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		0	0	0
Configuration		L	T				Т	TR							LR	
Volume, V (veh/h)		0	2052				1335	3						0		1
Percent Heavy Vehicles (%)		0												0		0
Proportion Time Blocked																
Percent Grade (%)															0	
Right Turn Channelized	No No No					١	10									
Median Type/Storage				Left	Only								1			

Critical and Follow-up Headways

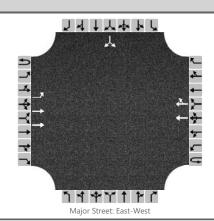
Base Critical Headway (sec)	4.1						7.5	6.9
Critical Headway (sec)	4.10						6.80	6.90
Base Follow-Up Headway (sec)	2.2						3.5	3.3
Follow-Up Headway (sec)	2.20						3.50	3.30

Flow Rate, v (veh/h)	0								1	
Capacity, c (veh/h)	485								380	
v/c Ratio	0.00								0.00	
95% Queue Length, Q ₉₅ (veh)	0.0								0.0	
Control Delay (s/veh)	12.4								14.5	
Level of Service, LOS	В								В	
Approach Delay (s/veh)	0	.0						14	.5	
Approach LOS								E	3	

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	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	BSM	Intersection	Ogden with Max Madsen
Agency/Co.	KLOA, Inc.	Jurisdiction	IDOT
Date Performed	12/12/2017	East/West Street	Ogden Avenue
Analysis Year	2023	North/South Street	Max Madsen Access
Time Analyzed	PM Peak Hour	Peak Hour Factor	0.94
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	17-273 - Downers Grove		

Lanes



Vehicle \	/olumes	and A	Adjustments
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Approach		Eastb	ound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		0	0	0
Configuration		L	Т				Т	TR							LR	
Volume, V (veh/h)		2	1737				1919	1						1		7
Percent Heavy Vehicles (%)		0												0		0
Proportion Time Blocked																
Percent Grade (%)														(0	
Right Turn Channelized		Ν	10			Ν	10			N	lo			Ν	lo	
Median Type/Storage				Left	Only								1			

Critical and Follow-up Headways

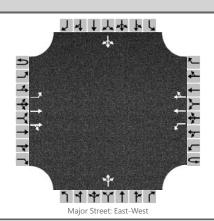
Base Critical Headway (sec)	4.1						7.5	6.9
Critical Headway (sec)	4.10						6.80	6.90
Base Follow-Up Headway (sec)	2.2						3.5	3.3
Follow-Up Headway (sec)	2.20						3.50	3.30

Flow Rate, v (veh/h)	2									8	
Capacity, c (veh/h)	280									181	
v/c Ratio	0.01									0.04	
95% Queue Length, Q ₉₅ (veh)	0.0									0.1	
Control Delay (s/veh)	17.9									25.9	
Level of Service, LOS	С									D	
Approach Delay (s/veh)	0.0								25	5.9	
Approach LOS									[)	

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	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	BSM	Intersection	Ogden with Gerber/Fairway
Agency/Co.	KLOA, Inc.	Jurisdiction	IDOT
Date Performed	12/12/2017	East/West Street	Ogden Avenue
Analysis Year	2023	North/South Street	Gerber/Fairway Access
Time Analyzed	AM Peak Hour	Peak Hour Factor	0.95
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	17-273 - Downers Grove		

Lanes



Vehicle Volumes and Adjustments

Approach		Eastb	ound			Westl	oound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration		L	T	TR		L	Т	TR			LTR				LTR	
Volume, V (veh/h)		4	2041	2		0	1330	7		10	0	33		4	0	0
Percent Heavy Vehicles (%)		0				0				0	0	0		0	0	0
Proportion Time Blocked																
Percent Grade (%)										()			(0	
Right Turn Channelized		N	lo			N	lo			N	lo			N	lo	
Median Type/Storage				Left	Only								1			

Critical and Follow-up Headways

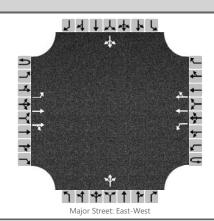
base Critical Headway (sec)								
Critical Headway (sec)								
Base Follow-Up Headway (sec)								
Follow-Up Headway (sec)								

Flow Rate, v (veh/h)	4			0				46			4	
Capacity, c (veh/h)	491			254				108			83	
v/c Ratio	0.01			0.00				0.42			0.05	
95% Queue Length, Q ₉₅ (veh)	0.0			0.0				1.8			0.1	
Control Delay (s/veh)	12.4			19.2				60.7			50.4	
Level of Service, LOS	В			С				F			F	
Approach Delay (s/veh)	0	.0		0	.0		60).7		50).4	
Approach LOS							F	=			=	

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	HCS7 Two-Way Stop	o-Control Report	
General Information		Site Information	
Analyst	BSM	Intersection	Ogden with Gerber/Fairway
Agency/Co.	KLOA, Inc.	Jurisdiction	IDOT
Date Performed	12/12/2017	East/West Street	Ogden Avenue
Analysis Year	2023	North/South Street	Gerber/Fairway Access
Time Analyzed	PM Peak Hour	Peak Hour Factor	0.94
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	17-273 - Downers Grove		

Lanes



ľ	V	eł	ni	icl	le	V	o	lum	ıes	and	<i> </i>	١d	jus	st	m	en	ıts

Approach		Eastb	ound			West	bound			North	bound			South	bound	
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	1	2	0	0	1	2	0		0	1	0		0	1	0
Configuration		L	Т	TR		L	Т	TR			LTR				LTR	
Volume, V (veh/h)		3	1721	14		13	1903	2		4	0	14		2	0	9
Percent Heavy Vehicles (%)		0				0				0	0	0		0	0	0
Proportion Time Blocked																
Percent Grade (%)										()			(0	
Right Turn Channelized		lo			١	10			N	lo			Ν	lo		
Median Type/Storage				Left	Only								1			

Critical and Follow-up Headways

Base Critical Headway (sec)								
Critical Headway (sec)								
Base Follow-Up Headway (sec)								
Follow-Up Headway (sec)								

Delay, Queue Length, and Level of Service

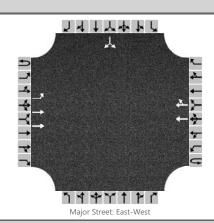
Flow Rate, v (veh/h)		3			14				19			12		
Capacity, c (veh/h)	2	84			334				150			137		
v/c Ratio	0	.01			0.04				0.13			0.09		
95% Queue Length, Q ₉₅ (veh)	C	0.0			0.1				0.4			0.3		
Control Delay (s/veh)	1	7.8			16.3				32.4			33.8		
Level of Service, LOS		С			С				D			D		
Approach Delay (s/veh)	0.0			0.1			32	2.4		33.8				

Approach LOS

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HCS7 Two-Way Stop-Control Report												
General Information		Site Information										
Analyst	BSM	Intersection	Ogden with Proposed Acces									
Agency/Co.	KLOA, Inc.	Jurisdiction	IDOT									
Date Performed	12/12/2017	East/West Street	Ogden Avenue									
Analysis Year	2023	North/South Street	Proposed Access Drive									
Time Analyzed	AM Peak Hour	Peak Hour Factor	0.95									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	17-273 - Downers Grove											

Lanes



Vehicle	Volumes	and Ad	justments
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Approach		Eastb	ound			Westl	oound			North	bound		Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	Т	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		0	0	0	
Configuration		L	Т				Т	TR							LR		
Volume, V (veh/h)		6	2046				1336	4						1		2	
Percent Heavy Vehicles (%)		0												0		0	
Proportion Time Blocked																	
Percent Grade (%)													0				
Right Turn Channelized		Ν	10			Ν	lo			N	lo			Ν	lo		
Median Type/Storage			Left Only						1								

Critical and Follow-up Headways

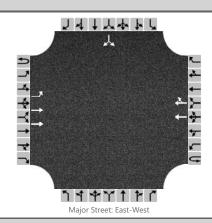
base Critical Headway (sec)								
Critical Headway (sec)								
Base Follow-Up Headway (sec)								
Follow-Up Headway (sec)								

Flow Rate, v (veh/h)	6									3	
Capacity, c (veh/h)	490									217	
v/c Ratio	0.01									0.01	
95% Queue Length, Q ₉₅ (veh)	0.0									0.0	
Control Delay (s/veh)	12.4									21.8	
Level of Service, LOS	В									С	
Approach Delay (s/veh)	0	.0							21	.8	
Approach LOS									(

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HCS7 Two-Way Stop-Control Report												
General Information		Site Information										
Analyst	BSM	Intersection	Ogden with Proposed Acces									
Agency/Co.	KLOA, Inc.	Jurisdiction	IDOT									
Date Performed	12/12/2017	East/West Street	Ogden Avenue									
Analysis Year	2023	North/South Street	Proposed Access Drive									
Time Analyzed	PM Peak Hour	Peak Hour Factor	0.95									
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25									
Project Description	17-273 - Downers Grove											

Lanes



Vehicle	Volumes	and	Adjustments
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Approach		Eastb	ound			Westi	oound			North	bound		Southbound				
Movement	U	L	Т	R	U	L	Т	R	U	L	Т	R	U	L	T	R	
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12	
Number of Lanes	0	1	2	0	0	0	2	0		0	0	0		0	0	0	
Configuration		L	Т				Т	TR							LR		
Volume, V (veh/h)		4	1734				1913	3						4		7	
Percent Heavy Vehicles (%)		0												0		0	
Proportion Time Blocked																	
Percent Grade (%)													0				
Right Turn Channelized		١	lo			Ν	lo			N	lo			N	lo		
Median Type/Storage				Left	ft Only								1				

Critical and Follow-up Headways

base Critical Headway (sec)								
Critical Headway (sec)								
Base Follow-Up Headway (sec)								
Follow-Up Headway (sec)								

Delay, Queue Length, and Level of Service

4													11	
286													127	
0.01													0.09	
0.0													0.3	
17.8													36.0	
С													Е	
0	.0											36	5.0	
	286 0.01 0.0 17.8 C	286 0.01 0.0 17.8	286 0.01 0.0 17.8 C	286 0.01 0.0 17.8 C	286 0.01 0.0 17.8 C	286 0.01 0.0 17.8 C	286 0.01 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	286 0.01 0.0 0.0 17.8 C	286 0.01 0.00 0.00 0.00 0.00 0.00 0.00 0.0	286 127 0.01 0.09 0.0 0.3 17.8 36.0 C E				

Approach LOS

DRAFT MINUTES

VILLAGE OF DOWNERS GROVE PLAN COMMISSION MEETING

17-PLC-0039: A petition seeking approval of a Special Use to construct an automobile dealership. The property is currently zoned B-3, General Services and Highway Business. The property is located on the north side of Ogden Avenue, approximately 385 feet east of Cross Street, commonly known as 2410 Ogden Avenue, Downers Grove, IL (PINs 08-01-303-014, -015, -016, -017). Anas Alkhatib agent of Agri-Pes, LLC, Petitioner; Agri-Pes, LLC, Owner.

Mr. Scott Williams stated that the subject property is zoned B-3 and the petitioner proposes construction of an automobile dealership. He described the surrounding properties, and noted that the site has two access points to Ogden Avenue. The easternmost curb cut along Ogden Avenue will be eliminated. The site has been vacant for about two years and has little existing landscaping. He noted on the site plan the parking areas available on the site. He also pointed out the location of the trash enclosure. The petitioner meets or exceeds all zoning requirements for the location. He noted that there is sufficient space for both vehicle carriers and Fire Department vehicles.

Mr. Williams noted that the property line is almost on the street, and Public Works has requested the granting of a sidewalk easement. The petitioner's landscaping plan meets or exceeds Code requirements as well. The proposed 8500 square foot building is two-story and consists of the actual showroom, as well as rear service bays. He described the elevation as primarily steel, masonry and concrete with a brown colored metal clad paneling, and glazed overhead doors. He said the applicant has submitted a photometric plan with an average foot-candle rating of .1 at centerline of Ogden Avenue. With regard to the Comprehensive Plan Future Land Use Map, the location is shown as Corridor-Commercial.

Mr. Williams said that Staff believes the Special Use criteria have been met as it is an authorized special use, is a redevelopment of a vacant site, it meets the Comprehensive Plan and there have been conditions specific to limiting any potential adverse impact on adjacent properties including test drives in residential areas. Staff recommends approval subject to the seven conditions listed on page 5 of Staff's report dated January 8, 2018.

Ms. Johnson asked about plans for signage. Mr. Williams replied that signage shown will be facing Ogden Avenue.

Mr. Maurer clarified that their only request is for a Special Use for an auto dealership on Ogden Avenue in Downers Grove.

Ch. Gassen called upon the Petitioner to make its presentation.

Paul Chabez, Jr., of Phorma Designs, Inc. of Aurora, said they are taking the existing property that has an abandoned restaurant and replacing that building with a new building, new pavement, and new curb. The dealership will operate between the hours Plan Commission Meeting

Jan. 8, 2018

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DRAFT MINUTES

of 10 AM to 8 PM, Monday through Friday, and 11 AM to 6 PM on Saturday and Sunday. The delivery of vehicles will be scheduled on a weekly basis, with all loading and unloading taking place on the dealership lot.

Ch. Gassen said she thought that automobile dealerships were not allowed to be opened on Sundays. Ms. Leitschuh said she was not sure if there is a law limiting activity on Sunday.

Ms. Johnson noted a different address for Agri-Pes as 857 Willow Lane and asked that the address be corrected. She said she was also surprised to see customer parking in the street yard on their proposed site. Mr. Chabez said they planned to have customer parking in front with automobile display in the back.

Mr. Boyle asked about them reusing the existing building, and Mr. Chabez said the location and condition was not usable. Mr. Boyle asked about the storm drainage.

Mr. Anas Alkhatib replied that the correct address is 857 Willow Lane. As to the existing drainage, the plan is to sheet flow to the southwest corner of the property where there is a culvert that connects to the storm sewer. They will add catch basins to the site, and will keep the same volume and same optimal release.

In response to what kind of automobiles will be sold, Mr. Alkhatib said it is to be highend used cars with service and detailing available. They will have the site staffed every day.

Ch. Gassen called on the public for comments. There were no comments. She then closed the public portion of the hearing.

Ch. Gassen asked about the sidewalk easement, and Mr. Williams said the Village requested that so they can have access for repair and maintenance.

Mr. Maurer said this proposal looks better than what has been there, and he sees no reason to oppose this. They are asking for a Special Use that's allowed in that Zoning District. He sees it as a benefit.

Mr. Maurer moved with regard to File 17-PLC-0039 that the Plan Commission forward a positive recommendation to the Village Council to approve this request subject to Staff's seven conditions listed on page 5 of Staff's report dated January 8, 2018. Mr. Boyle seconded the Motion.

AYES: Mr. Maurer, Mr. Boyle, Ms. Johnson, Mr. Kulovany, Ms. Rollins,

Ch. Gassen

NAYS: None

The Motion passed unanimously.

Mr. Williams said this item would also be forwarded to the Village Council for their review at their February 6, 2018 meeting.

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