

VILLAGE OF DOWNERS GROVE
 Report to the Village

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| SUBJECT: | SUBMITTED BY: |
| 2001 63rd Street - Planned Unit Development Amendment, Special Use and Plat of Subdivision | Stan Popovich, AICP Director of Community Development |

SYNOPSIS

The petitioner is seeking approval of a Planned Unit Development amendment to construct a new drug store, a Special Use for a drive-through facility, and a Plat of Subdivision to create a new out-lot for future commercial development at 2001 63rd Street.

STRATEGIC PLAN ALIGNMENT

The goals for 2017-2019 include *Strong and Diverse Local Economy*.

FISCAL IMPACT

n/a

RECOMMENDATION

Approval on the March 6, 2018 active agenda. The Plan Commission removed staff condition #9 and unanimously recommended approval 7:0. The Plan Commission found that the proposal is an appropriate use in the district, compatible with the Comprehensive Plan, and meets all standards of approval for a Planned Unit Development Amendment (Section 28.12.040), Special Use (Section 28.12.050) and Plat of Subdivision (Section 20.505).

Staff recommends that condition #6, which requires the building design feature be extended across the entire length of the roof-line facing Woodward Avenue, remain. Staff believes the extended design feature creates a more attractive facade facing Woodward Avenue.

BACKGROUND

The applicant is proposing to construct a 10,500 square foot Walgreens pharmacy at 2001 63rd Street, the southwest corner of 63rd Street and Woodward Avenue. The property is zoned B-2/PUD, General Retail Business/Planned Unit Development and encompasses existing Planned Unit Development #1. The petitioner is requesting:

- A PUD Amendment to permit the construction of a Walgreens
- A Special Use for the construction of a drive-through
- A Plat of Subdivision to create the Walgreens out-lot and a second out-lot for future development

The proposed development would include the demolition of a vacant restaurant building to make way for the convenience store and drive-through pharmacy. The petitioner is also proposing to create an additional out-lot for future commercial use. There are no immediate plans to develop the out-lot. In the interim, the existing pavement will be removed, and the entire lot will be returned to greenspace per the landscape requirements, reducing the shopping center's overall impervious surface.

A similar proposal (16-PLC-0062) was approved in August 2017, although the siting, orientation, size, and architectural design of the building has been revised (see table below for comparison).

| Walgreens | Previous Proposal 16-PLC-0062 | Current Proposal 17-PLC- 0041 |
|--------------------------------------|----------------------------------|---|
| Exterior Finish Materials | Gray EIFS with red accents | Nichiha fiber cement board (cedar, white brick, light brown block), EIFS overhang |
| Building Location | Western side of lot | Northeast corner |
| Building Size | 14,500 sq ft | 10,500 sq ft |
| Parking Spaces (required/provided) | 51/66 | 37/43 |
| Building Height | 29.3 ft | 20 ft |
| Shopping Center Improvements | Yes | Yes |
| 63 rd Street Improvements | Yes | Yes |
| Subdivision (2 original lots) | Reconfigured | Addition of 1 out-lot |

Similar to the previous approval, Walgreens final approval and occupancy is contingent on making significant improvements to the entire shopping center including facade improvements and site improvements, including the consolidation of two curb cuts onto 63rd Street into a single three-quarters access curb cut. The petitioner has started to implement some of these improvements, and is in for permit review of the at-grade site work. The occupancy of Walgreens is still contingent on implementing all of the identified site and building façade improvements under 16-PLC-0062.

Compliance with the Zoning Ordinance

The property is zoned B-2/PUD, General Retail Business/Planned Unit Development (#1), established in the 1970s. The proposed Walgreens development is consistent with the requirements of the Zoning Ordinance, excepting the drive-through setback minimum distance and the location of the trash enclosure / loading dock in the street yard along Woodward Avenue. Staff finds the drive-through setback reduction of 4.5 feet is acceptable considering the proposed location further improves on-site circulation and better screens the service. Staff finds that the proposed dumpster/loading dock location is acceptable because it is a corner lot with the western primary entrance practically creating a third main street yard. The proposal screens the enclosure with identical building materials that complement the overall building while also providing substantial landscaping. The proposed Walgreens and site improvements will not negatively impact the amount of remaining parking for the rest of the shopping center. The applicant's proposal is consistent with the Village's Zoning Ordinance.

Compliance with the Comprehensive Plan

The Comprehensive Plan designates this property as Mixed Use, and it is identified as the only catalyst site within the 63rd Street focus area plan. The proposed Walgreens development meets many of the key concepts identified in the Plan:

- Encourage commercial expansion at key intersections where it is necessary to improve commercial vitality

- Beautify and enhance landscaping at major intersections
- Enhance access and visibility
- Reduce the heat island effect through the use of light-colored building materials and shade

For this site, the Plan notes that new out-lots should be developed in conjunction with existing out-lots on this site to provide more visible and convenient shopping uses. The applicant's proposal is consistent with the Comprehensive Plan.

Compliance with the Subdivision Ordinance

The petitioner is proposing to create two new lots from the existing Lot 2. Lot 3 will be used for Walgreens and Lot 4 is the additional out-lot. The revised Lot 2, new Lots 3 and 4 will meet the minimum lot width and lot area requirements outlined in Section 20.301 of the Village's Subdivision Ordinance. The other two existing lots (Lot 1 and Lot 5) will remain the same size. The petitioner is providing a cross-access easement that connects the new 63rd Street access point to the northernmost access points along Belmont Road and Woodward Avenue. The easement is further extended to include the drive aisle in front of the Meadowbrook Shopping Center, and also wraps around Lot 5 to the south. This will ensure perpetual access through a non-exclusive easement for the benefit of all lots in the subdivision.

Engineering\Public Improvements

Post Construction Best Management Practices (PCBMPs) and detention are not required since the proposal results in a decrease in impervious area. The drainage for the site will tie into the existing stormwater system for the shopping center.

The petitioner is proposing to eliminate the dual full-access points onto 63rd Street and replace them with a single three-quarters access point. The eastbound right-turn lane will be extended at the request of DuPage County and will require land dedication. As a result of this, the petitioner may have to relocate or protect an existing Village water main. This will be determined during the permitting for the project. The traffic study found that the development's impact on the existing road network will be minimal, generating less than a two percent increase on 63rd Street traffic, with a significant number of pass-by trips. The study also concluded that the single full movement access drive will be adequate in accommodating the projected traffic and onsite vehicle deliveries.

Public Comment

No members of the public attended the Plan Commission meeting, and no inquiries were received about the project.

ATTACHMENTS

Ordinances

Resolution

Aerial Map

Staff Report with attachments dated February 5, 2018

Draft Minutes of the Plan Commission Hearing dated February 5, 2018

Revised east elevation

ORDINANCE NO. _____**AN ORDINANCE APPROVING AN
AMENDMENT TO PLANNED UNIT DEVELOPMENT #1
TO ALLOW CONSTRUCTION OF A CONVENIENCE STORE WITH DRIVE-THROUGH
AT 2001 63RD STREET**

WHEREAS, the Village Council has previously adopted Ordinance No. 1354, on September 16, 1968, designating the property described therein as Planned Unit Development #1 and subsequent amendments thereto; and,

WHEREAS, the Village Council has previously adopted Ordinance No. 5639, on August 8, 2017, approving an amendment to Planned Unit Development #1 to allow construction of a convenience store with drive-through at 2001 63rd Street; and,

WHEREAS, the Owners have filed a written petition with the Village conforming to the requirements of the Comprehensive Zoning Ordinance and requesting an amendment to Planned Unit Development #1 to allow construction of a convenience store with drive-through on the Meadowbrook & 63rd Street Shopping Center property located at 2001 63rd Street and,

WHEREAS, such request was referred to the Plan Commission of the Village of Downers Grove, and the Plan Commission has given the required public notice, conducted a public hearing for the petition on February 5, 2018, and has made its findings and recommendations, all in accordance with the statutes of the State of Illinois and the ordinances of the Village of Downers Grove; and,

WHEREAS, the Plan Commission has recommended approval of the requested petition, subject to certain conditions; and,

WHEREAS, the Village Council has considered the record before the Plan Commission, as well as the recommendations of Plan Commission.

NOW, THEREFORE, BE IT ORDAINED by the Council of the Village of Downers Grove, DuPage County, Illinois, as follows:

SECTION 1. That the provisions of the preamble are incorporated into and made a part of this ordinance as if fully set forth herein.

SECTION 2. That a Planned Unit Development Amendment is hereby authorized to approve construction of a convenience store with drive-through at 2001 63rd Street.

SECTION 3. The approval set forth in Section 2 of this ordinance is subject to the findings and recommendations of the Downers Grove Plan Commission regarding File 17-PLC-0041 as set forth in the minutes of their February 5, 2018 meeting.

SECTION 4. The approval set forth in Section 2 of this ordinance is subject to the following conditions:

1. The Planned Unit Development, Special Use and Plat of Subdivision shall substantially conform to the staff report dated February 5, 2018; and drawings prepared by Manhard Consulting Ltd, dated 12/20/2017 and resubmitted on 01/18/2018, except as such plans may be modified to conform to the Village codes and ordinances.
2. The site improvement work for the property must be completed per the Site Improvement Exhibit, dated 11/28/16, revised plan dated 01/19/2017, and approved by Village Council on August 8, 2017 prior to the issuance of the Certificate of Occupancy for Walgreens.
3. The Walgreens building shall be equipped with an automatic fire suppression system and an automatic and manual fire alarm system.
4. A fire hydrant shall be installed, including water/fire service line on the vacant out-lot for future use.
5. A separate sign permit will be required prior to installation of any wall or monument sign.
6. The white exterior insulation and finish system (EIFS) shall be extended across the entire length of the roof-line facing Woodward Avenue.
7. The EIFS on the building shall be maintained in accordance with the Village's currently adopted edition of the International Property Maintenance Code.
8. No building permits can be issued until the Final Plat of Subdivision is recorded.

SECTION 5. That the convenience store with drive-through is consistent with and complimentary to the overall planned unit development site plan and with the requirements of the "B-2/PUD, General Retail Business/Planned Unit Development" zoning district.

SECTION 6. That all ordinances or parts of ordinances in conflict with the provisions of this ordinance are hereby repealed.

SECTION 7. That this ordinance shall be in full force and effect from and after its passage and publication in pamphlet form as provided by law.

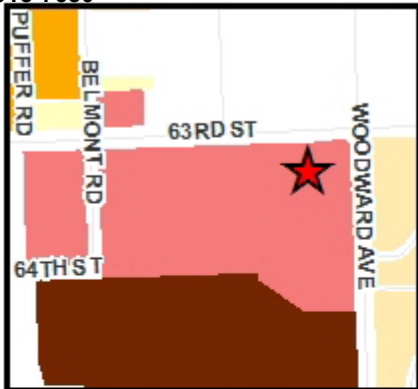
Mayor

Passed:

Published:

Attest: _____

Village Clerk



2001 63rd Street - Location Map



**VILLAGE OF DOWNERS GROVE
REPORT FOR THE PLAN COMMISSION
FEBRUARY 5, 2018 AGENDA**

| SUBJECT: | TYPE: | SUBMITTED BY: |
|---|---|---|
| 17-PLC-0041 2001 63 rd Street | PUD Amendment, Special Use and Plat of Subdivision | Rebecca Leitschuh, AICP Senior Planner |

REQUEST

The petitioner is requesting approval for an amendment to Planned Unit Development #1 to allow the construction of a new Walgreens store, a Special Use to allow a drive-through pharmacy and a Plat of Subdivision at 2001 63rd Street.

NOTICE

The application has been filed in conformance with applicable procedural and public notice requirements.

GENERAL INFORMATION

OWNER & APPLICANT: FL Cedar, LLC
477 Elm Place
Highland Park, IL 60035

PROPERTY INFORMATION

EXISTING ZONING: B-2/PUD, General Retail Business/Planned Unit Development
EXISTING LAND USE: Shopping Center
PROPERTY SIZE: 69,753 sq ft (1.6 acres)
PINS: 08-24-202-008 & -009

SURROUNDING ZONING AND LAND USES

ZONING

FUTURE LAND USE

| | | |
|---------------|--|--|
| NORTH: | R-4, Single Family Unincorporated DuPage County | Single-Family Attached |
| SOUTH: | R-6, Residential Apartment/Condo 6 | Multi-Family Residential |
| EAST: | R-3, Residential Detached House 3 | Single-Family Attached, Neighborhood Commercial |
| WEST: | B-2, General Retail Business | Mixed Use |

ANALYSIS

SUBMITTALS

This report is based on the following documents, which are on file with the Department of Community Development:

1. Project Narrative
2. Plat of Survey
3. Architectural Plans
4. Engineering Plans
5. Landscape Plan
6. Photometric Plan
7. Elevations and Renderings
8. Traffic Impact Study
9. Neighborhood Meeting Summary Report
10. Plat of Subdivision

PROJECT DESCRIPTION

The applicant is proposing to construct a Walgreens pharmacy at 2001 63rd Street. A similar proposal (16-PLC-0062) was approved in August 2017, although the siting, orientation, size, and architectural design of the building has been revised. The subject area involves 1.6 acres of the 18.86 acre shopping center property, located at the southwest corner of 63rd Street and Woodward Avenue. The property is zoned B-2/PUD, General Retail Business/Planned Unit Development and encompasses existing Planned Unit Development #1. The petitioner is requesting:

- A PUD Amendment to permit the construction of a Walgreens
- A Special Use for the construction of a drive-through
- A Plat of Subdivision to create the Walgreens out-lot and a second out-lot for future development

The petitioner is proposing to build a new 10,500-square-foot Walgreens building at the southwest corner of the intersection of Woodward Avenue and 63rd Street. The proposed development would include the demolition of a vacant restaurant building at this location. The project site for this new building is approximately 1.08 acres and will include a convenience store and drive-through pharmacy with 43 parking spaces.

The petitioner is also proposing to create an additional out-lot (0.52 acres) for future commercial use through the final Plat of Subdivision. There are no immediate plans to develop the out-lot. In the interim, the existing pavement will be removed, and the entire lot will be seeded per the landscape requirements, reducing the shopping center's overall impervious surface.

The drive-through facility will be located on the south side of the building with one-way only traffic allowed with appropriate signage to direct traffic. An ADA accessible path is proposed from the corner of the 63rd/Woodward intersection to the entrance of the building. Parking is provided on the western side of the building, with four rows of parking and two full-access drive aisles, and exceeds the requirements of parking per the Zoning Ordinance. There will be two ADA accessible parking spaces adjacent to the building's main entrance as required. The trash compactor, transformer, and tote enclosure are located on the east side of the building, fully enclosed by a wall matching the style of the building.

The petitioner is proposing landscaping in conformance with the Village requirements. Landscaping is provided on all four sides of the property. A total of 34 shade trees are intermixed with shrubs and ornamental grasses around the perimeter, the interior parking lot islands, and the drive aisles. Parking lot and site lighting complies with Village requirements.

The previously approved Walgreens was clad principally in an exterior insulation and finish system (EIFS). The color scheme was principally gray with some red accents. Based on previous discussions, the petitioner has revised the exterior design of the building to minimize the use of EIFS, use fiber cement

architectural panels as the principal material and provide a variety of colors and textures. The facades are broken up by a light brown (Tuscan) modern block face, a white (Chantilly Lace) smaller brick face, and a horizontal wood panel (Cedar), all made of fiber cement board. These materials are further varied with the use of a white EIFS overhang that runs along the roof line of the building on its street facing walls and main western entrance. The main entrance incorporates all of these elements, with the addition of windows and a white horizontal band, breaking the light brown block face. A sign is featured over the entrance.

The 63rd Street facing (north) elevation wraps the corners with the wood panel elements, and breaks up the expanse with windows on both corners. A vertical pier made of the small white brick panel anchors the main corner. A second building sign is proposed on the north elevation.

The Woodward Avenue (east) elevation continues to wrap the corner with wood panels, windows, and the white EIFS overhang. Staff requests a condition be made to extend the EIFS design element across the entire length of the eastern wall so as to bring greater design detail to the east elevation. A vertical pier, identical to the one on the western elevation, intersects the horizontal planes. The dumpster enclosure, while in a street yard, is designed to complement the building, incorporating the same fiber cement panel system.

The rear (south) wall has a canopy over the drive-through window, surrounded by the wood panel design. The horizontal white accent band continues from the other elevations. Roof top mechanical units will be screened from the public right-of-ways. All proposed signage for Walgreens complies with the square-footage requirements of the sign ordinance, including a single tenant monument sign at the northwest corner. The side interior wall sign, although not a location permitted by-right, is supported by staff since it identifies the main entrance.

A reference table is provided below with a quick comparison of the previously approved petition (16-PLC-0062) and the revised submittal.

| Walgreens | 16-PLC-0062 | 17-PLC-0041 |
|--------------------------------------|----------------------------|---|
| Exterior Finish Materials | Gray EIFS with red accents | Nichiha fiber cement board (cedar, white brick, light brown block), EIFS overhang |
| Building Location | Western side of lot | Northeast corner |
| Building Size | 14,500 sq ft | 10,500 sq ft |
| Parking Spaces (required/provided) | 51/66 | 37/43 |
| Building Height | 29.3 ft | 20 ft |
| Shopping Center Improvements | Yes | Yes |
| 63 rd Street Improvements | Yes | Yes |
| Subdivision (2 original lots) | Reconfigured | Addition of 1 out-lot |

In the previous submittal to Plan Commission (16-PLC-0062), Walgreens final approval and occupancy was contingent on making significant improvements to the entire shopping center. The petitioner has started to implement some of these improvements, and is in for permit review of the at-grade site work. The occupancy of Walgreens will still be contingent on implementing all of the identified site and building façade improvements under 16-PLC-0062. These improvements include the following:

- Removal of the existing 63rd Street dual access points and replacement with a single three-quarter access point
- Façade renovations for all shopping center buildings including new EIFS facades with corner treatments, accent bands and new column enclosures.
- Installation of new curbed landscape islands within the front parking lot
- Repaired parking lot and drive aisle along 63rd Street
- Repair of rear access drive and replacement of speed bumps within the rear access drive
- Removal of rear southernmost access point to Belmont Road
- Removal of excess pavement in southwest corner of the shopping center
- Repair of low lying area in the rear of the center which leads to ponding water

COMPLIANCE WITH THE COMPREHENSIVE PLAN

The Comprehensive Plan's Future Land Use Map designates this property as Mixed Use, and it is identified as the only catalyst site within the 63rd Street focus area plan. As a Mixed Use property, the plan recommends "a mix of land uses within a contiguous geographic boundary" serving more than one purpose. The 63rd Street Focus Area Plan notes that the Village should encourage commercial expansion at key intersections where existing commercial uses exist and where it is necessary to improve their vitality. The plan also identifies the enhancement of access and visibility within nearby parcels, and to connect nearby residential areas to shopping and services through pedestrian and bicycle access. In addition, commercial developments should attempt to reduce the urban heat island effect through shading and the use of light-colored building materials; of which both elements are incorporated in this proposal.

As a catalyst site, the plan notes that Meadowbrook Shopping Center should include a mix of uses, and that the site could be redeveloped to include both residential and commercial uses. The plan does not mandate that both residential and commercial uses be a part of a redevelopment. The plan merely identifies the potential for a mix of residential and commercial if the property is no longer viable as a commercial center. The Commercial and Office Area Goal #2 includes the objectives to promote the "... redevelopment of the Meadowbrook Mall and other outdated shopping centers" and to identify and work with "...underperforming and underutilized" sites. The goal encourages the Village to enhance the economic vitality, productivity, appearance and function of commercial corridors including 63rd Street. Additionally, the 63rd Street redevelopment concept graphic identifies commercial out-lots along 63rd Street, supporting the creation of two out-lots.

The proposed redevelopment is consistent with the goals of the Comprehensive Plan.

COMPLIANCE WITH ZONING ORDINANCE

The property is zoned B-2/PUD, General Retail Business/Planned Unit Development, established in the 1970s.

The bulk requirements of the proposed Walgreens development in the B-2/PUD zoning district are summarized in the following table:

Zoning Requirements

| 2001 63rd Street (Lot 3) | Required | Proposed |
|--|-----------------|-----------------|
| North Setback (Street Yard – 63 rd Street) - Building | 25 ft | 25.32 ft |
| East Setback (Street Yard – Woodward Avenue) - Building | 25 ft | 51.5 ft |
| South Setback (Rear Yard) - Building | n/a | 31.5 ft |
| West Setback (Side Yard) - Building | n/a | 133.5 ft |
| West Setback - Parking | n/a | 3.5 ft |
| North Setback - Parking | 25 ft | 27.5 ft |
| South Setback – Parking | n/a | 8 ft |
| Landscaped Open Space | 4,714 sf (10%) | 12,256 sf (26%) |
| Street yard Landscaped Open Space | 2,357 sf (5%) | 10,163 sf (22%) |
| Floor Area Ratio | 0.75 (max) | 0.22 |
| Building Height | 35 ft (max) | 20 ft |
| Loading Setback | 50 ft | 67 ft |
| Parking Spaces | 37 | 43 |
| Drive-through Stacking | 3 | 3 |
| Drive-through Setback | 25 ft | 20.5 ft |

The proposed Walgreens development is consistent with the requirements of the Zoning Ordinance, excepting the drive-through setback minimum distance. However, staff finds the setback reduction of 4.5 feet is acceptable considering the proposed location of the drive-through further improves on-site circulation and better screens the service. The proposed Walgreens and site improvements will not negatively impact the amount of remaining parking for the rest of the shopping center. The applicant's proposal is consistent with the Village's Zoning Ordinance.

COMPLIANCE WITH SUBDIVISION ORDINANCE

The petitioner is proposing to create two new lots out of the existing Lot 2 in the shopping center. Lot 3 will be used for Walgreens and Lot 4 is the additional out-lot. The revised Lot 2, new Lot 3 and Lot 4 will meet the minimum lot width and lot area requirements outlined in Section 20.301 of the Village's Subdivision Ordinance. The other two existing lots (Lot 1 and Lot 5) will remain the same size.

| Meadowbrook Subdivision | Lot Width (req. 100 ft.) | Lot Depth (req. 140 ft.) | Lot Area (req. 10, 500 sq. ft.) |
|--------------------------------|---------------------------------|---------------------------------|--|
| Lot 2 | 450.16 ft | 475.81 ft | 225,238 sq. ft. (5.17 ac) |
| Lot 3 | 163.41 ft | 285 ft | 47,138 sq. ft. (1.08 ac) |
| Lot 4 | 140.37 ft | 163.41 ft | 22,615 sq. ft. (0.52 ac) |

The petitioner is providing a cross-access easement that connects the new 63rd Street access point to the northernmost access points along Belmont Road and Woodward Avenue. The easement is further extended

to include the drive aisle in front of the Meadowbrook Shopping Center, and also wraps around Lot 5 to the south. This will ensure perpetual access through a non-exclusive easement for the benefit of all lots in the subdivision. Additionally, the petitioner is providing the required five-foot wide public utility and drainage easements along the side lot lines and the ten-foot wide public utility and drainage easements along the rear lot lines for Lots 3 and 4.

ENGINEERING/PUBLIC IMPROVEMENTS

There is a net decrease in the impervious area and therefore new stormwater detention is not required. The drainage for the site will tie into the existing stormwater system for the shopping center. The petitioner will be required to meet all Village engineering standards and comply with all applicable codes when formally submitting for a permit.

The petitioner is proposing to eliminate the dual full-access points onto 63rd Street and replace them with a single three-quarters access point. The two current 63rd Street access points are approximately 30 feet from each other. These two access points can create confusion and conflict points for both drivers entering and exiting the site and also for pedestrians walking along the 63rd Street sidewalk. In conjunction with DuPage County, the petitioner has proposed to combine these access points into a single access point. The single access will allow both eastbound and westbound 63rd Street traffic to enter the site, but will limit the exit point to a right-turn (eastbound) only. DuPage County is requiring the petitioner to dedicate land along 63rd Street to extend the turn lane going east. At time of permit, the petitioner will have to verify the location and elevation of an existing Village water main. As a result of the right-turn lane extension, the petitioner may have to relocate or protect the existing water system.

TRAFFIC

A traffic impact study for the proposed development was completed by the petitioner. The study examined the existing 63rd Street and Woodward Avenue traffic conditions and the future conditions based on the proposed development.

The study found that the proposed new store will generate new trips during the weekday evening and Saturday midday peak hours; however, this will not have a detrimental effect on the shopping center or surrounding properties given the multiple access points on the site. The total existing traffic on 63rd Street is over 27,000 vehicles per day, which will be increased by less than 2% with the proposed development. Also noted in the study is the significant number of pass-by trips. Pass-by trips are vehicles that are already using 63rd Street but will now stop at the proposed development and do not represent an increase in traffic.

The study also considers the conversion of the dual full-access points on 63rd Street to a three-quarter access, prohibiting left turns on 63rd Street, as an improvement and reduction in traffic conflict points for the property. Removing these conflict points will reduce the opportunity for crashes. The proposed development's impact on the geometry of 63rd Street should provide a safety benefit.

PUBLIC SAFETY REQUIREMENTS

The Fire Prevention Division reviewed the proposed development and determined that sufficient access to and around the site is provided for emergency vehicles. The site layout permits Fire Department apparatus the opportunity to enter and exit the site from both 63rd Street and Woodward Avenue. The building will be required to include a fire alarm and sprinkler system that meet the Village's code requirements. A fire hydrant will be required to be installed as part of this approval on the vacant lot for future use.

NEIGHBORHOOD COMMENT

Notice was provided to all property owners 250 feet or less from the property in addition to posting public

hearing notice signs and publishing the legal notice in the *Downers Grove Suburban Life*. No public comments have been received by staff.

The petitioner held a neighborhood meeting on November 28, 2017 with four current tenants in attendance. Questions were asked about construction timing, and shopping center signage and improvements. The applicant responded to each of these topics during the meeting and has provided a summary of the meeting that is attached.

FINDINGS OF FACT

The petitioner is requesting a Planned Unit Development Amendment, a Special Use and a Plat of Subdivision to construct a new retail and pharmacy store at 2001 63rd Street. Staff finds that the proposal meets the standards for granting a Planned Unit Development Amendment, a Special Use and Plat of Subdivision as outlined below:

Section 28.12.040.C.6 Review and Approval Criteria

The decision to amend the zoning map to approve a PUD development plan and to establish a PUD overlay district are matters of legislative discretion that are not controlled by any single standard. In making recommendations and decisions regarding approval of planned unit developments, review and decision-making bodies must consider at least the following factors:

a. The zoning map amendment review and approval criteria of Sec. 12.030.I.

As previously noted, the shopping center was approved as a Planned Development in the 1970s. Section 4.030.C of the Zoning Ordinance, adopted in 2014, notes that all previously approved Planned Developments were reclassified as Planned Unit Developments. As such, a rezoning is not required and this standard does not apply.

b. Whether the proposed PUD development plan and map amendment would be consistent with the comprehensive plan and any other adopted plans for the subject area.

The proposed project is consistent with the Comprehensive Plan. The plan identifies this area as *Mixed Use*. This property is an existing shopping center and the proposed use will be compatible with the other uses. The site is well suited to accommodate a drive-through pharmacy. The proposed development is consistent with the policy recommendation that mixed use areas provide a variety of land uses within a pedestrian accessible neighborhood. The proposed improvements will enhance the economic vitality productivity, appearance and function of the shopping center as identified in Commercial and Office Area Goal #2.

The proposed project is consistent with the Comprehensive Plan and the 63rd Street focus area plan. The project is designed in a manner that is compatible with surrounding land uses. The proposed Walgreens and both the building and site improvements to the shopping center will improve the vitality of the center. The proposed removal of two access points onto 63rd Street and the installation of a single three-quarters access point will enhance access to 63rd Street while improving safety. A second new out-lot is proposed, that is consistent with the redevelopment concept sketch while also revitalizing an aged shopping center.

The Focus Ara Plan notes a mix of uses could be provided if the property is no longer viable as a commercial center. The improvements proposed by the property owner show that the owner believes a redevelopment of the commercial space is feasible and will lead to enhanced economic vitality in the center.

This standard has been met.

- c. *Whether PUD development plan complies with the PUD overlay district provisions of Sec. 4.030.***
The proposed project meets several of the PUD overlay district provisions and objectives as found in Section 4.030 of the Zoning Ordinance. The PUD is consistent with and helps advance the goals of the Comprehensive Plan. The development also meets the PUD overlay district provisions by providing a high quality building that is compatible with other developments along 63rd Street while providing attractive, high-quality landscaping for the Walgreens site and numerous upgrades to the property. Improvements have been proposed to improve motorized and non-motorized travel on-site such as a reduction of dual access points along 63rd Street to a single access point which increases safety along the public right-of-way and an accessible route connecting the front entrance of the building to the sidewalk. This standard has been met.
- d. *Whether the proposed development will result in public benefits that are greater than or at least equal to those that would have resulted from development under conventional zoning regulations.***
The proposed development will result in a new convenience store and pharmacy for the neighborhood, and an additional out-lot for future commercial development, in compliance with the Comprehensive Plan. The proposed development meets many objectives of the Comprehensive Plan and furthers the vision of the Village to improve 63rd Street. The new building will enhance the aesthetics of the shopping center and 63rd Street. The public benefits include the replacement of dual access points to 63rd Street with a single three-quarters access point. This will eliminate conflicts between vehicles and vehicles and pedestrians. The building and site improvements will enhance the vitality of the shopping center and this section of 63rd Street. This standard has been met.
- e. *Whether appropriate terms and conditions have been imposed on the approval to protect the interests of surrounding property owners and residents, existing and future residents of the PUD and the general public.***
There are several conditions noted below that will protect the interests of the surrounding neighborhood and the general public. The conditions below are being requested to ensure that the proposed development satisfies all applicable codes and requirements. The project will advance many goals and objective laid out in the current and updated Comprehensive Plan and the conditions listed below will ensure that these goals and objectives are met. Several improvements provided by the petitioner for the existing shopping center (through 16-PLC-0062) will enhance the overall property and will be an improvement for the neighborhood. This standard has been met.

Section 28.12.050.H Approval Criteria

No special use may be recommended for approval or approved unless the respective review or decision-making body determines that the proposed special use is constituent with and in substantial compliance with all Village Council policies and plans and that the applicant has presented evidence to support each of the following conclusions:

- 1. *That the proposed use is expressly authorized as a Special Use in the district in which it is to be located;***
The property is located in the B-2/PUD, General Retail Business/Planned Unit Development zoning district. Under Section 5.010 of the Zoning Ordinance, a drive-through facility is listed as an allowable Special Use in the B-2 zoning district. This standard has been met.
- 2. *That the proposed use at the proposed location is necessary or desirable to provide a service or a facility that is in the interest of public convenience and will contribute to the general welfare of the neighborhood or community.***
The proposed drive through pharmacy is a desirable service to the community and will contribute to the general welfare of the Village. The drive-through pharmacy provides a convenient service to the

community. The development will cater to the local customers as desired in the existing Comprehensive Plan and will meet many goals and objectives outlined in both the current and updated Comprehensive Plan. This standard has been met.

3. *That the proposed use will not, in the particular case, be detrimental to the health, safety or general welfare of persons residing or working in the vicinity or be injurious to property values or improvements in the vicinity.*

The proposed drive-through will not be detrimental to the health, safety or general welfare of persons residing in or working in the vicinity and will not be injurious to property values or improvements in the vicinity. The drive-through is located along the southern wall, away from residential properties and from adjacent public sidewalks, with ample landscaping to screen properties across the street. The location of the building will lead to other improvements including the elimination of dual access points onto 63rd Street which will create a safer driving and walking environment in this area. This standard is met.

Section 20.301 – Plat of Subdivision

The proposed subdivision meets the minimum lot area and width requirements of Sections 20.301 of the Subdivision Ordinance.

RECOMMENDATIONS

The proposed Planned Unit Development Amendment, Special Use for a drive through and the Plat of Subdivision for the new development in Meadowbrook Shopping Center at 2001 63rd Street is consistent with the current and updated Comprehensive Plans, the Zoning Ordinance, the Subdivision Ordinance and surrounding zoning and land use classifications. Based on the findings listed above, staff recommends the Plan Commission recommend the Village Council **approve** the requested Planned Unit Development Amendment, Special Use and Plat of Subdivision as requested in case 17-PLC-0041 subject to the following conditions:

1. The Planned Unit Development, Special Use and Plat of Subdivision shall substantially conform to the staff report; and drawings prepared by Manhard Consulting Ltd, dated 12/20/2017 and resubmitted on 01/18/2018, except as such plans may be modified to conform to the Village codes and ordinances.
2. The site improvement work for the property must be completed per the Site Improvement Exhibit, dated 11/28/16, revised plan dated 01/19/2017, and approved by Village Council in August 2017 prior to the issuance of the Certificate of Occupancy for Walgreens.
3. The Walgreens building shall be equipped with an automatic suppression system and an automatic and manual fire alarm system.
4. A fire hydrant shall be installed, including water/fire service line on the vacant out-lot for future use.
5. A separate sign permit will be required prior to installation of any wall or monument sign.
6. The white exterior insulation and finish system (EIFS) shall be extended across the entire length of the roof-line facing Woodward Avenue.
7. The EIFS on the building shall be maintained in accordance with the Village's currently adopted edition of the International Property Maintenance Code.
8. No building permits can be issued until the Final Plat of Subdivision is recorded.
9. A pedestrian connection shall be provided from Woodward Avenue across the southern property line of Lot 3.

17-PLC-0041; 2001 63rd Street
February 5, 2018

Page 10

10. The petitioner shall provide elevations of new pavement over the water main in response to the right-turn lane extension per DuPage County. Petitioner shall protect and/or relocate existing water system if necessary.

Staff Report Approved By:



Stanley J. Popovich, AICP
Director of Community Development

SP; rl
-att

December 20, 2017
Revised: January 18, 2018

Mr. Stan Popovich
Village of Downers Grove
801 Burlington Avenue
Downers Grove, IL 60515

Re: Project Summary/Narrative
Proposed Walgreens
SW Corner 63 Street and Woodward Avenue

Dear Mr. Popovich:

Please accept this letter as a request by FL Cedar, LLC (Owner) for approval of the application for Special Use for a Drive-Thru, Amendment to Existing PUD, and a Plat of Subdivision.

The original Petition for Plan Commission of this PUD Amendment was submitted to the Village of Downers Grove on November 29, 2016 under Village Project No. 16-PLC-0062. Upon Village review, the PUD Amendment was approved by the Village Council (Ordinance No. 5640 and 5641) on August 8, 2017. Since that time, the end user has chosen to modify the total square footage of the proposed Walgreens facility. As a result of this change, the site plan and parking configuration has been updated accordingly. Furthermore, the proposed lot configuration has been adjusted based on the revised site plan. A convenience store, pharmacy, and drive thru pharmacy will remain part of the new Petition for Plan Commission.

The project site is located at the southwest corner of 63rd Street and Woodward Avenue. The project site is approximately 1.6 acres, and it is currently occupied by an existing building. This existing building was formerly used as a restaurant. The site has frontage along the 63rd Street to the north and Woodward Avenue to the east. The south and west boundaries are abutting the existing shopping center parking lot and drive aisles. The project site has access to 63rd Street and Woodward Avenue via the existing shopping center. The site is currently zoned B-2 PD (General Retail Business Planned Development).

The Owner proposes to demolish the existing unoccupied building and construct a new 10,500 SF Walgreens store including a pharmacy drive-thru, 41 parking spaces, and associated landscaping. The Walgreens will consist of a convenience store, pharmacy, and drive thru pharmacy. The store will employ approximately 35 part-time and full-time employees that will work on various assigned shifts. The hours of operation are proposed to be 8:00 am to 10:00 pm for the store and 8:00 am to 8:00 pm for the pharmacy and the drive-thru. The Owner is proposing to combine the existing dual access points off of 63rd Street into one access point. Coordination with DuPage County Division of Transportation is ongoing.

In addition to the proposed Walgreens, the Owner is also coordinating with the Village on providing numerous upgrades to the existing shopping center including existing façade improvements, asphalt repairs to the east-west drive aisle and north parking lot, asphalt repairs and traffic calming measures to the rear drive aisle in the southern portion of the shopping center, and landscaping improvements to the shopping center parking lot. The referenced overall shopping center improvements are currently under review by the Village of Downers Grove Staff, and an ordinance approving the Amendment to Planned Unit Development (P.U.D.) was approved by the Village Council (Ordinance No. 5641) on August 8, 2017.

The proposed Walgreens is a permitted use by right in the B-2 General Retail Business district. The drive-thru requires a Special Use approval.

For additional detailed information, please also refer to the submitted plans titled Proposed Walgreens, dated 12/20/2017 (Revised 01/18/18) prepared by Manhard Consulting, as well as plans, elevations and renderings dated 12/20/2017 (Revised 01/18/18) prepared by Camburas & Theodore, Ltd.

The requested Special Use Approval, Plat of Subdivision, and Amendment to Existing PUD are in conformance with the Village Municipal Code standards and the following is the evidence to support these request:

Request for Special Use Approval Criteria (Section 28.12.050.H)

No special use may be recommended for approval or approved unless the respective review or decision-making body determines that the proposed special use is consistent with and in substantial compliance with all village council policies and plans and that the applicant has presented evidence to support each of the following conclusions:

1. that the proposed use is expressly authorized as a special use in the district in which it is to be located; **The proposed use is expressly authorized as a special use in the B-2 General Retail Business district.**
2. that the proposed use at the proposed location is necessary or desirable to provide a service or a facility that is in the interest of public convenience and will contribute to the general welfare of the neighborhood or community; **The proposed use at the proposed location is necessary and desirable as it provides a convenience to the community as well as additional safety for customers of the pharmacy. Customers, as a result of the drive-thru facility, are not required to park, exit their vehicle, and walk into the store in order to get a prescription filled.**
3. that the proposed use will not, in the particular case, be detrimental to the health, safety, or general welfare of persons residing or working in the vicinity or be injurious to property values or improvements in the vicinity. **The proposed use will be a benefit to the health, safety, and general welfare of the community as the drive thru provides for additional safety as customers are not required to park, exit their vehicle, and walk into the store in order to get a prescription filled. The proposed use will not be injurious to property values or improvements in the vicinity as there will be a newly constructed building, new parking lot, new landscaping, and a new ADA accessible route from the building to the adjacent roadway right-of-way. These proposed improvements will be in conformance with the Village Municipal Code and standards.**

Request Amendment to Existing PUD (Section 28.12.040.C.6)

The decision to amend the zoning map to approve a PUD development plan and to establish a PUD overlay district are matters of legislative discretion that are not controlled by any single standard. In making recommendations and decisions regarding approval of planned unit developments, review and decision-making bodies must consider at least the following factors:

- a. the zoning map amendment review and approval criteria of Sec. 12.030I in the case of new Planned Unit Development proposals; **This is an existing PUD.**
- b. whether the proposed PUD development plan and map amendment would be consistent with the comprehensive plan and any other adopted plans for the subject area; **The PUD Development Plan is consistent with the comprehensive plan as this site is located within the Corridor Commercial area.**
- c. whether PUD development plan complies with the PUD overlay district provisions of Sec. 4.030; **The PUD Development plan is in conformance with the vision and goals of the comprehensive plan.**

d. whether the proposed development will result in public benefits that are greater than or at least equal to those that would have resulted from development under conventional zoning regulations; and **The public benefits are greater than those that would have resulted from the conventional zoning because of the added convenience to the community as well as additional safety for customers who are not required to park, exit their vehicle, and walk into the store in order to get a prescription filled.**

e. whether appropriate terms and conditions have been imposed on the approval to protect the interests of surrounding property owners and residents, existing and future residents of the PUD and the general public. **The proposed use is unobtrusive and does not create noise issues. The proposed use is buffered effectively by not only the natural terrain, but by both 63rd Street and Woodward Avenue to the north and east respectively.**

Planned Unit Development Overlay District Provisions (Section 4.030.A.2)

Different types of PUDs will achieve different planning goals. In general, however, PUDs should include elements that further some or all of the following objectives:

a. implementation of and consistency with the comprehensive plan and other relevant plans and policies; **The subject site is located in, and consistent with, the Corridor Commercial area.**

b. flexibility and creativity in responding to changing social, economic and market conditions allowing greater public benefits than could be achieved using conventional zoning and development regulations; **Not applicable**

c. efficient and economical provision of public facilities and services; **Not applicable**

d. variety in housing types and sizes to accommodate households of all ages, sizes, incomes and lifestyle choices; **Not applicable**

e. compact, mixed-use development patterns where residential, commercial, civic and open spaces are located in close proximity to one another; **Not applicable**

f. a coordinated transportation system that includes an inter-connected hierarchy of facilities for motorized and non-motorized travel; **The existing subject site includes cross access drive aisles for motorized travel that connect Woodward Avenue and Belmont Road without having to utilize 63rd Street. The proposed development will utilize these existing cross access drives. Additionally, the reduction of dual access points along 63rd Street to a single access point is an increase in the safety of the public. Non-motorized travel will be accommodated by the proposed accessible route from the front of the proposed building to the existing sidewalks at the southwest corner of 63rd Street and Woodward Avenue.**

g. high-quality buildings and improvements that are compatible with surrounding areas, as determined by their arrangement, massing, form, character and landscaping; **In addition to the proposed Walgreens, the Owner is also coordinating with the Village on providing numerous upgrades to the existing shopping center including existing façade improvements, asphalt repairs to the east-west drive aisle and the north parking lot area, asphalt repairs and traffic calming measures to the rear drive aisle in the southern portion of the shopping center, and additional landscaping in the existing shopping center parking lot. The referenced overall shopping center improvements are currently under review by the Village of Downers Grove.**

h. the protection and enhancement of open space amenities and natural resource features; **In addition to the proposed Walgreens landscaping and open space, the Owner is also coordinating with the Village on providing numerous landscaping improvements within the existing shopping center parking lot. The referenced overall shopping center improvements are currently under review by the Village of Downers Grove.**

i. the incorporation of sustainable development features including green infrastructure practices in landscapes and parking area, to maximize the aesthetic and water quality benefits of best practices in stormwater management; and **The proposed Walgreens landscaping and open space meets, and in some cases exceeds, the Village Municipal Code and standards.**

j. attractive, high-quality landscaping, lighting, architecture and signage, including the use of native landscaping that reflects the unique character of the village and the surrounding area. **The proposed Walgreens landscaping, open space, lighting, and signage meets the Village Municipal Code and standards and does reflect the unique character of the village. The numerous upgrades to the existing shopping center façade, parking lot, and landscaping will also provide a significant enhancement to the surrounding area. The referenced overall shopping center improvements are currently under review by the Village of Downers Grove.**

Developer's Statement of Intent Section 4.030.D

The proposed project is an amendment to an existing PUD and consists of the demolition of an existing building (former restaurant) and construction of a new 10,500 SF Walgreens store including a pharmacy drive-thru, 41 parking spaces, new site lighting, and landscaping. The project also includes numerous upgrades to the existing shopping center including existing façade improvements, asphalt repairs to the east-west drive aisle and the north parking lot area, asphalt repairs and traffic calming measures to the rear drive aisle in the southern portion of the shopping center, and additional landscaping in the shopping center parking lot.

There are many benefits to the existing shopping center, the surrounding area, and the community. Access to over the counter and prescription medication is a rudimentary need and a necessity to the community. The proposed drive-thru will be a benefit to the health, safety, and general welfare of the community as the drive thru provides for additional safety as customers are not required to park, exit their vehicle, and walk into the store in order to get a prescription filled. The reduction of dual access points along 63rd Street to a single access point is an increase in the safety of the public. The new ADA accessible route from the building to the adjacent roadway right-of-way will provide a benefit to the pedestrians along Woodward Avenue and 63rd Street. The numerous upgrades to the existing shopping center will enhance the overall appearance of the shopping center while also making the shopping center much better and safer for the community.

We appreciate the opportunity to present this project to you for approval. If you have any questions or require additional clarification, please do not hesitate to contact us at 773-571-4199.

Sincerely,



Perrine Knight
FL Cedar, LLC

DEMOLITION LEGEND

| | |
|--------------|--|
| | PAVEMENT AND BASE TO BE REMOVED |
| | BUILDING TO BE REMOVED |
| | COLD MILL ASPHALT MILL 1.5" SURFACE COURSE |
| | SAWCUT LINE |
| | FENCE, RETAINING WALL, RAILROAD TIES, POLES, CURB AND GUTTER, ETC. TO BE REMOVED |
| | UTILITY STRUCTURE TO BE REMOVED |
| | UTILITY LINE REMOVAL, FILL OR ABANDONMENT (REFER TO SPECIFICATIONS) |
| | TREE TO BE REMOVED |
| (TBR) | TO BE REMOVED |

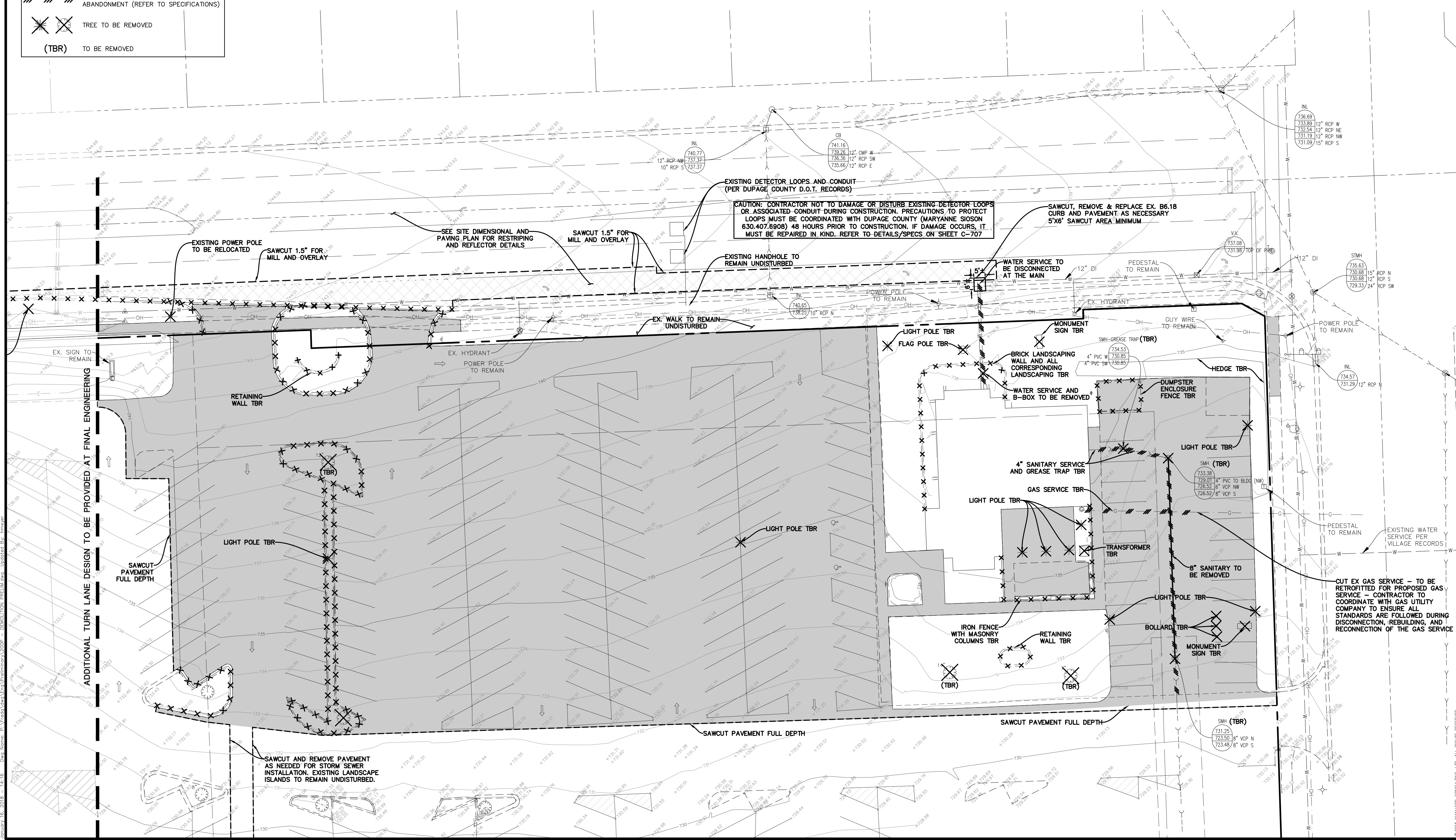
DEMOLITION NOTES:

1. THE CONTRACTOR SHALL COORDINATE WITH RESPECTIVE UTILITY COMPANIES PRIOR TO THE REMOVAL AND/OR RELOCATION OF UTILITIES. THE CONTRACTOR SHALL COORDINATE WITH THE UTILITY COMPANY CONCERNING PORTIONS OF WORK WHICH MAY BE PERFORMED BY THE UTILITY COMPANY'S FORCES AND ANY FEES WHICH ARE TO BE PAID TO THE UTILITY COMPANY FOR THEIR SERVICES. THE CONTRACTOR IS RESPONSIBLE FOR PAYING FOR ALL FEES AND CHARGES.
2. SHOULD REMOVAL AND/OR RELOCATION ACTIVITIES DAMAGE FEATURES INDICATED TO REMAIN, THE CONTRACTOR SHALL PROVIDE NEW MATERIALS/STRUCTURES IN ACCORDANCE WITH THE CONTRACT DOCUMENTS. EXCEPT FOR MATERIALS DESIGNED TO BE RELOCATED ON THIS PLAN, ALL OTHER CONSTRUCTION MATERIALS SHALL BE NEW.
3. PRIOR TO DEMOLITION OCCURRING, ALL EROSION CONTROL DEVICES ARE TO BE INSTALLED.
4. ALL EXISTING UTILITY LINES AND CONDUITS LOCATED UNDER PROPOSED BUILDINGS SHALL BE REMOVED AND PROPERLY BACKFILLED. ALL UTILITY LINES AND CONDUITS LOCATED UNDER DRIVES, ON-SITE ROADS, PARKING LOTS OR SIDEWALKS SHALL BE FILLED WITH A FLOWABLE BACKFILL AND END PLUGGED. ALL EXISTING STRUCTURES SHALL BE REMOVED. ALL EXISTING UTILITY LINES LOCATED UNDER LANDSCAPE AREAS SHALL BE LEFT IN PLACE AND PLUGGED AT ALL STRUCTURES.
5. THE CONTRACTOR IS RESPONSIBLE FOR DEMOLITION, REMOVAL AND LAWFUL DISPOSAL (IN A LOCATION APPROVED BY ALL GOVERNING AUTHORITIES) OF ALL STRUCTURES, PADS, WALLS, FLUMES, FOUNDATIONS, PARKING, DRIVES, DRAINAGE STRUCTURES, UTILITIES, ETC., SUCH THAT THE IMPROVEMENTS SHOWN ON THESE PLANS CAN BE CONSTRUCTED. ALL DEMOLITION WORK SHALL BE IN ACCORDANCE WITH ALL APPLICABLE FEDERAL, STATE AND LOCAL REQUIREMENTS. ALL FACILITIES TO BE REMOVED SHALL BE UNDERCUT TO SUITABLE MATERIAL AND BROUGHT TO GRADE WITH SUITABLE COMPACTED FILL MATERIAL PER THE SPECIFICATIONS.
6. THE CONTRACTOR IS RESPONSIBLE FOR OBTAINING ALL PERMITS REQUIRED FOR DEMOLITION AND DISPOSAL.
7. ELECTRICAL, TELEPHONE, CABLE, WATER, FIBER OPTIC CABLE AND/OR GAS LINES NEEDING TO BE REMOVED SHALL BE COORDINATED BY THE CONTRACTOR WITH THE AFFECTED UTILITY COMPANY.
8. CONTRACTOR MUST PROTECT THE PUBLIC AT ALL TIMES WITH FENCING, BARRICADES, ENCLOSURES, AND OTHER APPROPRIATE BEST MANAGEMENT PRACTICES.
9. CONTINUOUS ACCESS SHALL BE MAINTAINED FOR SURROUNDING PROPERTIES AT ALL TIMES DURING DEMOLITION.
10. VILLAGE WATER DEPARTMENT TO RECLAIM OLD WATER METER AND MTU WHEN WATER SERVICE IS DISCONNECTED

EXISTING CONDITIONS NOTES:

1. EXISTING CONDITIONS AND DEMOLITION PLAN REPRESENT SITE CONDITIONS AS OF 9/8/14. CONTRACTOR SHALL INSPECT SITE PRIOR TO BIDDING WORK TO VERIFY ACTUAL FIELD CONDITIONS AS PORTIONS OF THE DEMOLITION WORK MAY HAVE SINCE BEEN COMPLETED. CONTRACTOR SHALL BE RESPONSIBLE TO COMPLETE ALL DEMOLITION WORK AS PER PLANS TO PREPARE THE SITE FOR CONSTRUCTION OF PROPOSED IMPROVEMENTS.
2. THE UNDERGROUND UTILITY INFORMATION AS SHOWN HEREON IS BASED, IN PART, UPON INFORMATION FURNISHED BY UTILITY COMPANIES AND THE LOCAL MUNICIPALITY. WHILE THIS INFORMATION IS BELIEVED TO BE RELIABLE, ITS ACCURACY AND COMPLETENESS CANNOT BE GUARANTEED NOR CERTIFIED TO.

GRAPHIC SCALE
 0 10 20 40 60
 (IN FEET)
 1 inch = 20 ft.



| DATE | REVISIONS | HCM |
|------|-----------|-----|
| | | |
| | | |
| | | |
| | | |

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PROPOSED WALGREENS - PRELIMINARY
VILLAGE OF DOWNERS GROVE, ILLINOIS
EXISTING CONDITIONS AND DEMOLITION PLAN

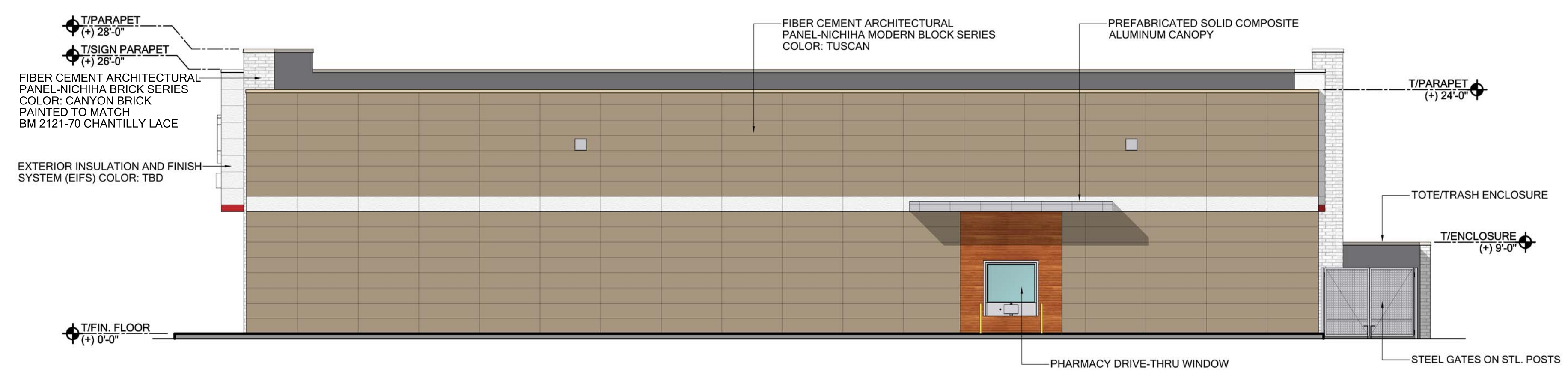
PROJ. MGR.: SMS
 PROJ. ASSOC.: HCM
 DRAWN BY: HCM
 DATE: 12-20-17
 SCALE: 1"=20'
 SHEET: **C-200**
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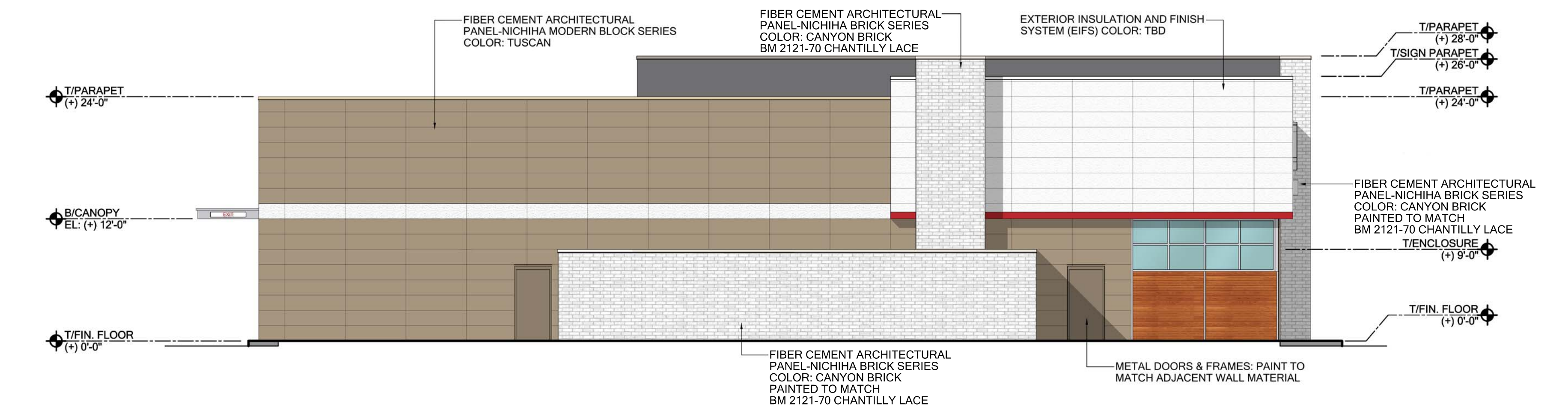
PRELIMINARY PLAN - NOT FOR CONSTRUCTION



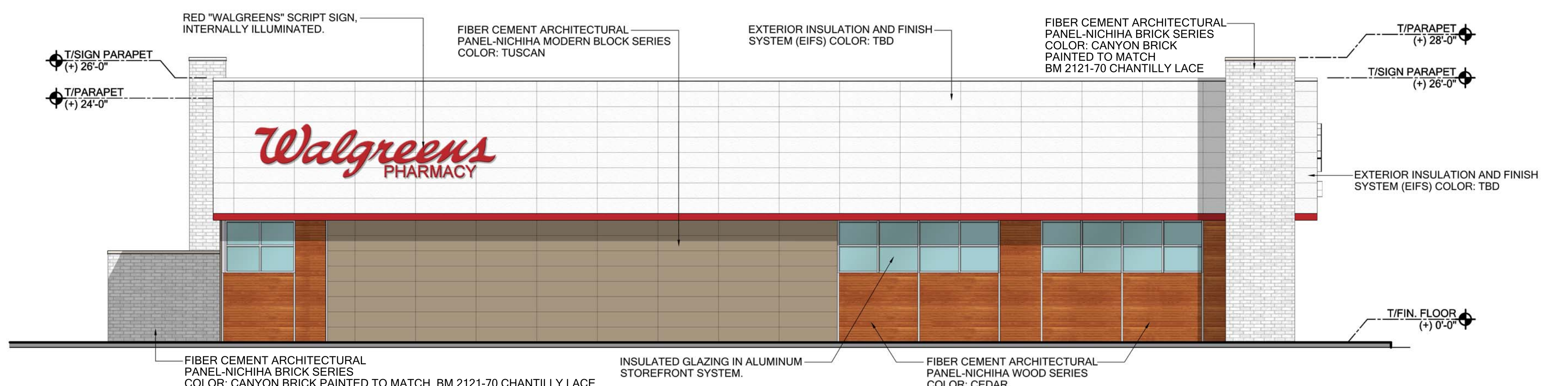
WEST ELEVATION



SOUTH ELEVATION



EAST ELEVATION



NORTH ELEVATION

SIGN SUMMARY

| WEST ELEVATION | |
|---|--------------------|
| 23'-8 1/2" SCRIPT LED LETTER SET W/ SECONDARY 14" PHARMACY LED LETTER SET | 124.89 S.F. |
| 3'-6" x 2'-11" 1/8" LED SUSPENDED INTERIOR TOWER SIGN | 10.39 S.F. |
| "DRIVE-THRU" SIGN (6' x 3'-6") | 1.75 S.F. |
| NON-ILLUM. "CLEARANCE" SIGN (5' x 4'-0") | 1.64 S.F. |
| TOTAL | 138.67 S.F. |

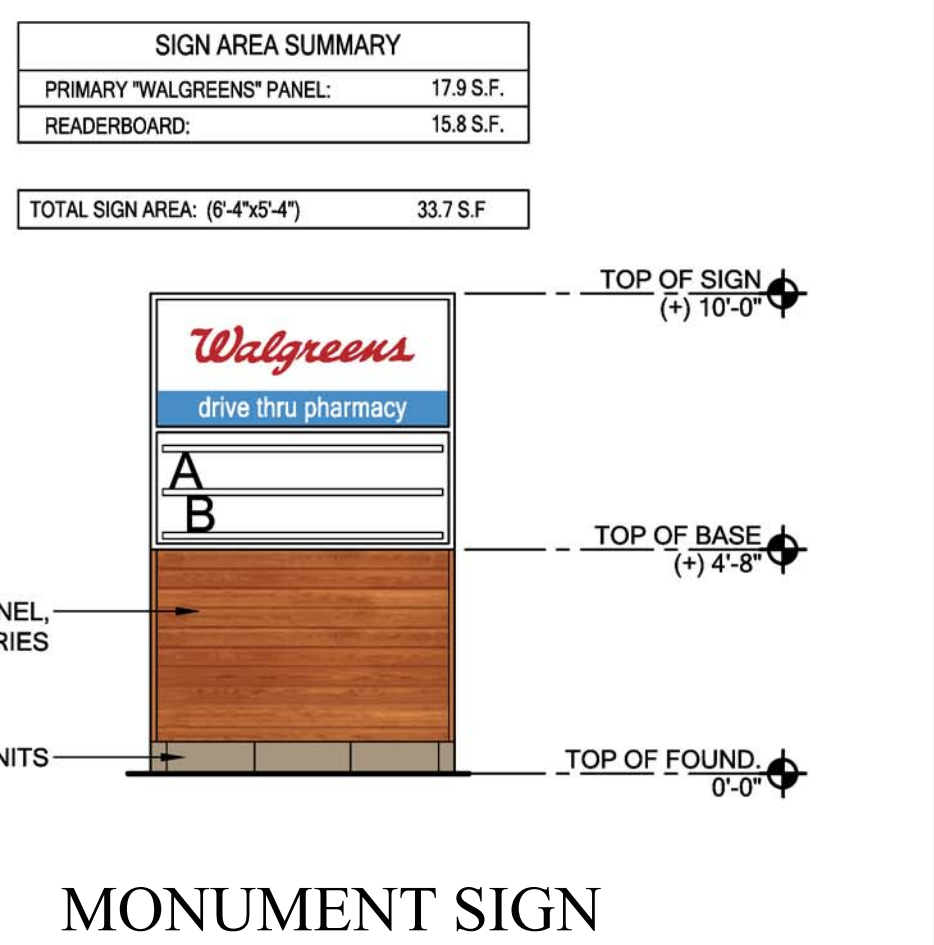
| NORTH ELEVATION | |
|---|--------------------|
| 23'-8 1/2" SCRIPT LED LETTER SET W/ SECONDARY 14" PHARMACY LED LETTER SET | 124.89 S.F. |
| TOTAL | 124.89 S.F. |

| EAST ELEVATION | |
|--------------------------|------------------|
| "EXIT" SIGN (6' x 3'-6") | 1.75 S.F. |
| TOTAL | 1.75 S.F. |

| MONUMENT SIGN | |
|---------------------------------|------------------|
| TOTAL SIGN AREA (6'-4" x 5'-4") | 33.7 S.F. |
| TOTAL | 67.4 S.F. |

WALL SIGNS:
THE TOTAL AREA OF WALL SIGNS AFFIXED TO A BUILDING WALL SHALL NOT EXCEED THE ALLOWABLE AREA. THE ALLOWABLE AREA IS BASED UPON THE FOLLOWING:
(1.5 SQUARE FEET PER LINEAR FOOT OF TENANT FRONTAGE) = MAX WALL SIGNAGE ALLOWED

PARAPETS ARE SET AT A HEIGHT TO SCREEN ROOF TOP UNITS PER ZONING ORDINANCE (VODG 28.040.C)



| REVISIONS | DATE | BY | APP |
|---------------------------|----------|----|-----|
| PLAN COMMISSION SUBMITTAL | 12-20-17 | AB | |
| PLAN COMMISSION SUBMITTAL | 1-18-18 | AB | |

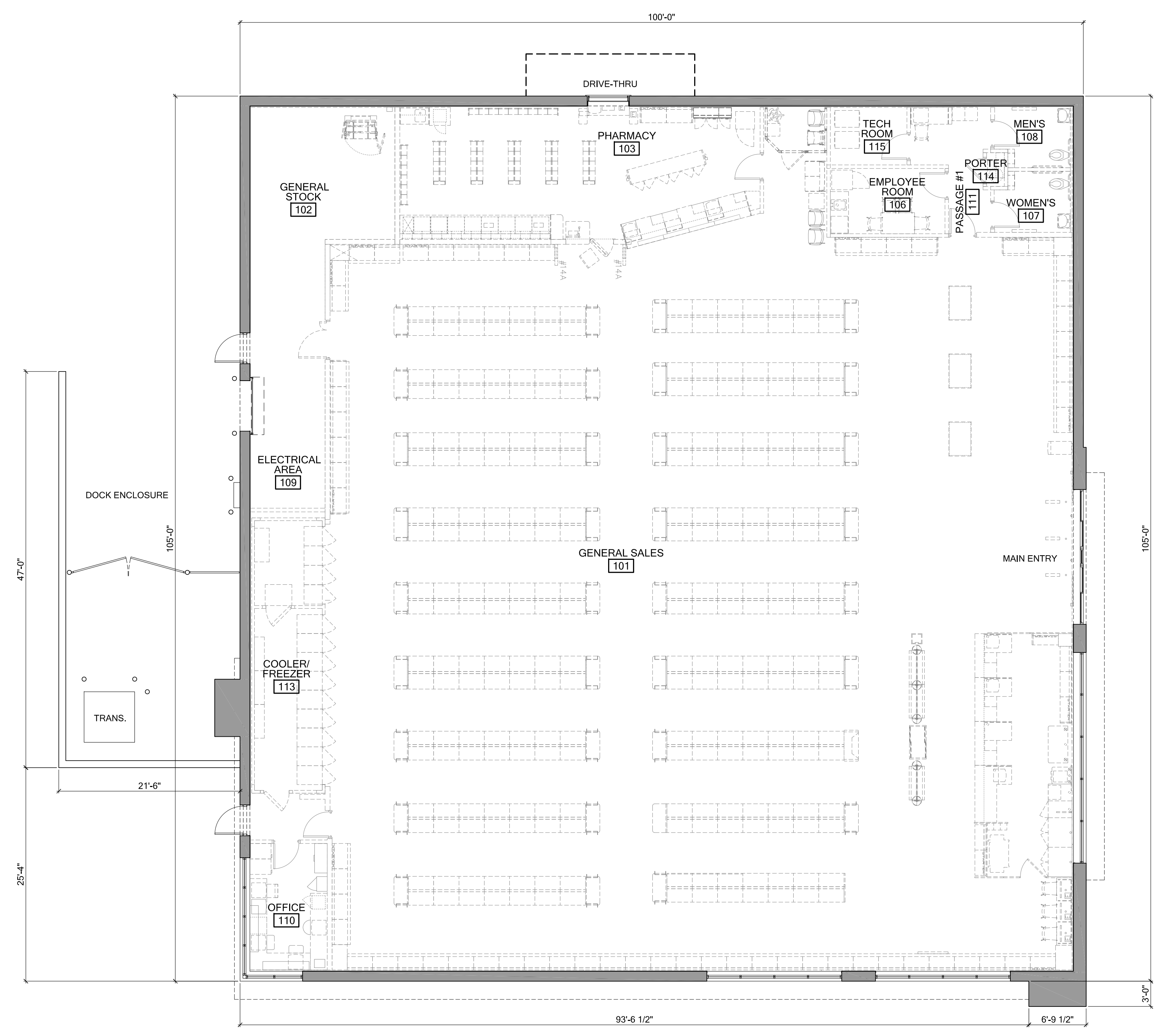
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est. 1949

63RD STREET AND WOODWARD AVENUE
VILLAGE OF DOWNERS GROVE, ILLINOIS
EXTERIOR ELEVATIONS

PROJ. MGR.:
PROJ. ASSOC.:
DRAWN BY: AB
DATE: 12-20-17
SCALE: AS NOTED

SHEET
A-210



1 GENERAL FLOOR PLAN
SCALE: 1/8" = 1'-0"



| DATE | REVISIONS |
|----------|---------------------------------|
| 12-20-17 | AB PLAN COMMISSION SUBMITTAL |
| 1-18-18 | AB PLAN COMMISSION SUBMITTAL |

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63RD STREET AND WOODWARD AVENUE
VILLAGE OF DOWNERS GROVE, ILLINOIS
 FLOOR PLAN

| | |
|---------------|----------|
| PROJ. MGR.: | |
| PROJ. ASSOC.: | |
| DRAWN BY: | AB |
| DATE: | 12-20-17 |
| SCALE: | AS NOTED |

SHEET
A-111

- SITE DIMENSIONAL AND PAVING NOTES:**
- ALL DIMENSIONS ARE FACE OF CURB TO FACE OF CURB OR BUILDING FOUNDATION UNLESS NOTED OTHERWISE.
 - ALL PROPOSED CURB AND GUTTER SHALL BE B6.12 UNLESS OTHERWISE NOTED.
 - ALL CURB RADII SHALL BE 3' MEASURED TO FACE OF CURB UNLESS NOTED OTHERWISE.
 - TIE ALL PROPOSED CURB AND GUTTER TO EXISTING CURB AND GUTTER WITH 2-#6 BARS x 18" LONG DOWELED INTO EXISTING CURB.
 - BUILDING DIMENSIONS AND ADJACENT PARKING HAVE BEEN PREPARED BASED UPON ARCHITECTURAL INFORMATION CURRENT AT THE DATE OF THIS DRAWING. SUBSEQUENT ARCHITECTURAL CHANGES MAY EXIST. THEREFORE CONTRACTOR SHALL REFER TO ARCHITECTURAL PLANS FOR PRECISE BUILDING DIMENSIONS AND NOTIFY THE ARCHITECT AND ENGINEER OF ANY DISCREPANCIES PRIOR TO CONSTRUCTION. BUILDING DIMENSIONS SHOWN SHOULD NOT BE USED FOR CONSTRUCTION LAYOUT OF BUILDING.

- IMPROVEMENTS ADJACENT TO BUILDING, IF SHOWN, SUCH AS TRUCK DOCK, RETAINING WALLS, SIDEWALKS, CURBING, FENCES, CANOPIES, RAMPS, HANDICAP ACCESS, PLANTERS, DUMPSTERS, AND TRANSFORMERS ETC. HAVE BEEN SHOWN FOR APPROXIMATE LOCATION ONLY. REFER TO ARCHITECTURAL PLANS FOR EXACT LOCATIONS, SPECIFICATIONS AND DETAILS.
- LOCATION OF PRIVATE SIDEWALKS SHALL BE COORDINATED WITH PROPOSED DOORWAY. CONTRACTOR TO VERIFY ACTUAL BUILDING PLAN LOCATIONS WITH ARCHITECT/DEVELOPER PRIOR TO CONSTRUCTING THE SIDEWALKS.
- ALL ROADWAY AND PARKING LOT SIGNAGE, STRIPING, SYMBOLS, ETC. SHALL BE IN ACCORDANCE WITH LATEST JURISDICTIONAL GOVERNMENTAL ENTITY DETAILS.
- SOME EXISTING ITEMS TO BE REMOVED HAVE BEEN DELETED FROM THIS PLAN FOR CLARITY. SEE DEMOLITION PLAN FOR ITEMS DELETED.
- DEPRESS CURB & GUTTER AT ALL SIDEWALK AND PATH LOCATIONS FOR HANDICAP ACCESS PER FEDERAL AND STATE STANDARDS.
- THE CONTRACTOR SHALL CONTACT J.U.L.I.E. (1-800-892-0123) PRIOR TO ANY WORK TO LOCATE UTILITIES AND SHALL CONTACT THE OWNER SHOULD UTILITIES APPEAR TO BE IN CONFLICT WITH THE PROPOSED IMPROVEMENT.
- ALL TRAFFIC SIGNS SHALL BE INSTALLED AT 7' HIGH, MEASURED FROM THE GROUND ELEVATION TO THE BOTTOM OF THE SIGN.
- THE STOP BAR AT 63RD STREET DRIVEWAY SHALL BE THERMOPLASTIC
- TRAFFIC SIGNS INSTALLED WITHIN THE COUNTY R.O.W. SHALL NOT BE INSTALLED WITH A CONCRETE BASE
- THERE SHALL BE NO DECORATIVE STONE, LANDSCAPING OR TREES PLANTED WITHIN THE COUNTY R.O.W.
- ALL TRAFFIC SIGNS FACING THE COUNTY HIGHWAY SHALL BE INSTALLED BY THE DUPAGE COUNTY.
- THERMOPLASTIC PAVEMENT MARKING IS REQUIRED WITHIN LIMITS OF THE ACCESS DRIVEWAY AND IN THE 63RD ST. COUNTY RIGHT OF WAY.
- ALL EXISTING SIDEWALK DAMAGED IN THE 63RD ST. COUNTY RIGHT OF WAY SHALL BE REPLACED IN LIKE KIND.

| OVERALL PARKING DATA | | | |
|----------------------|------------|-----------------|------------|
| | EXISTING | | PROPOSED |
| SHOPPING CENTER | 774 SPACES | SHOPPING CENTER | 651 SPACES |
| ROUNDHEAD'S PIZZA | 21 SPACES | WALGREENS | 43 SPACES |
| TOTAL | 795 SPACES | TOTAL | 694 SPACES |

| SITE DATA | |
|------------------------|------------|
| TOTAL PROPERTY AREA | 1.60 ACRES |
| LOT 4 (OUTLOT) AREA | 0.52 ACRES |
| LOT 3 (WALGREENS) AREA | 1.08 ACRES |
| PARKING PROVIDED | 43 SPACES |
| HANDICAP PROVIDED | 2 SPACES |

- SIGN LEGEND**
- R1-1 STOP SIGN
 - R7-8 HANDICAP PARKING SIGN
 - FIRE LANE - NO PARKING SIGN
 - R5-1 DO NOT ENTER
 - R6-25 ONE WAY
 - R3-5R RIGHT TURN ONLY

- PAVEMENT LEGEND**
- STANDARD DUTY PAVEMENT**
 1 1/2" BITUMINOUS SURFACE COURSE, HOT-MIX ASPHALT, MIX D, N50
 2 1/4" BITUMINOUS BINDER COURSE, HOT-MIX ASPHALT, IL-19, N50
 8" AGGREGATE BASE COURSE, TYPE B
- HEAVY DUTY PAVEMENT**
 2" BITUMINOUS SURFACE COURSE, HOT-MIX ASPHALT, MIX D, N50
 3" BITUMINOUS BINDER COURSE, HOT-MIX ASPHALT, IL-19, N50
 10" AGGREGATE BASE COURSE, TYPE B
- WALGREENS CONCRETE PAVEMENT**
 6 1/2" PORTLAND CEMENT CONCRETE PAVEMENT W/ 6 X 6 W1.4 WWF
 6" COMPACTED AGGREGATE BASE, TYPE B
- DUPAGE COUNTY D.O.T. DRIVEWAY APRON CONCRETE**
 1 1/2" BITUMINOUS CONCRETE SURFACE COURSE
 1 1/2" BITUMINOUS CONCRETE BINDER COURSE
 6" BITUMINOUS CONCRETE BASE MATERIAL (B.A.M.)
 8" CA-6 (GRADE 8) COMPACTED GRAVEL SUB-BASE
- THICKENED SIDEWALK AND MEDIAN CONCRETE PAVEMENT**
 8" PORTLAND CEMENT CONCRETE PAVEMENT W/ 6 X 6 W1.4 WWF
 8" CA-6 (GRADE 8) COMPACTED GRAVEL SUB-BASE
- CONCRETE SIDEWALK**
 5" PORTLAND CEMENT CONCRETE PAVEMENT W/ 6 X 6 W1.4 WWF
 4" COMPACTED AGGREGATE BASE, TYPE B
- B6.12 CONCRETE CURB & GUTTER**
- REVERSE PITCH B6.12 CONCRETE CURB & GUTTER**

BENCHMARK:

SOURCE BENCHMARK 1:
 DUPAGE COUNTY 2006 GEODETIC SURVEY MONUMENT 0214, PID DK3151, DESCRIBED AS BRASS DISK LOCATED IN THE CONCRETE BASE OF A LIGHT POLE LOCATED 21' SOUTH OF THE CENTERLINE OF 71ST STREET AND 70' WEST OF THE CENTERLINE OF BINDER ROAD.
 ELEVATION=774.53 NAVD 88

SOURCE BENCHMARK 2:
 DUPAGE COUNTY 2006 GEODETIC SURVEY MONUMENT, PID MF1251, DESCRIBED AS TOP OF A STEEL ROD LOCATED IN PVC SLEEVE WITH BERTSEN LID, LOCATED 145' SOUTH OF THE CENTERLINE OF 63RD STREET AND 42' WEST OF CENTERLINE OF DUNHAM ROAD.
 ELEVATION=745.59 NAVD 88

SITE BENCHMARK 1:
 TAG BOLT OF FIRST FIRE HYDRANT WEST OF WOODWARD AVENUE ON THE SOUTH SIDE OF 63RD STREET.
 ELEVATION=738.96 NAVD 88

SITE BENCHMARK 2:
 TAG BOLT OF SECOND HYDRANT WEST OF WOODWARD AVENUE ON THE SOUTH SIDE OF 63RD STREET.
 ELEVATION=744.51 NAVD 88

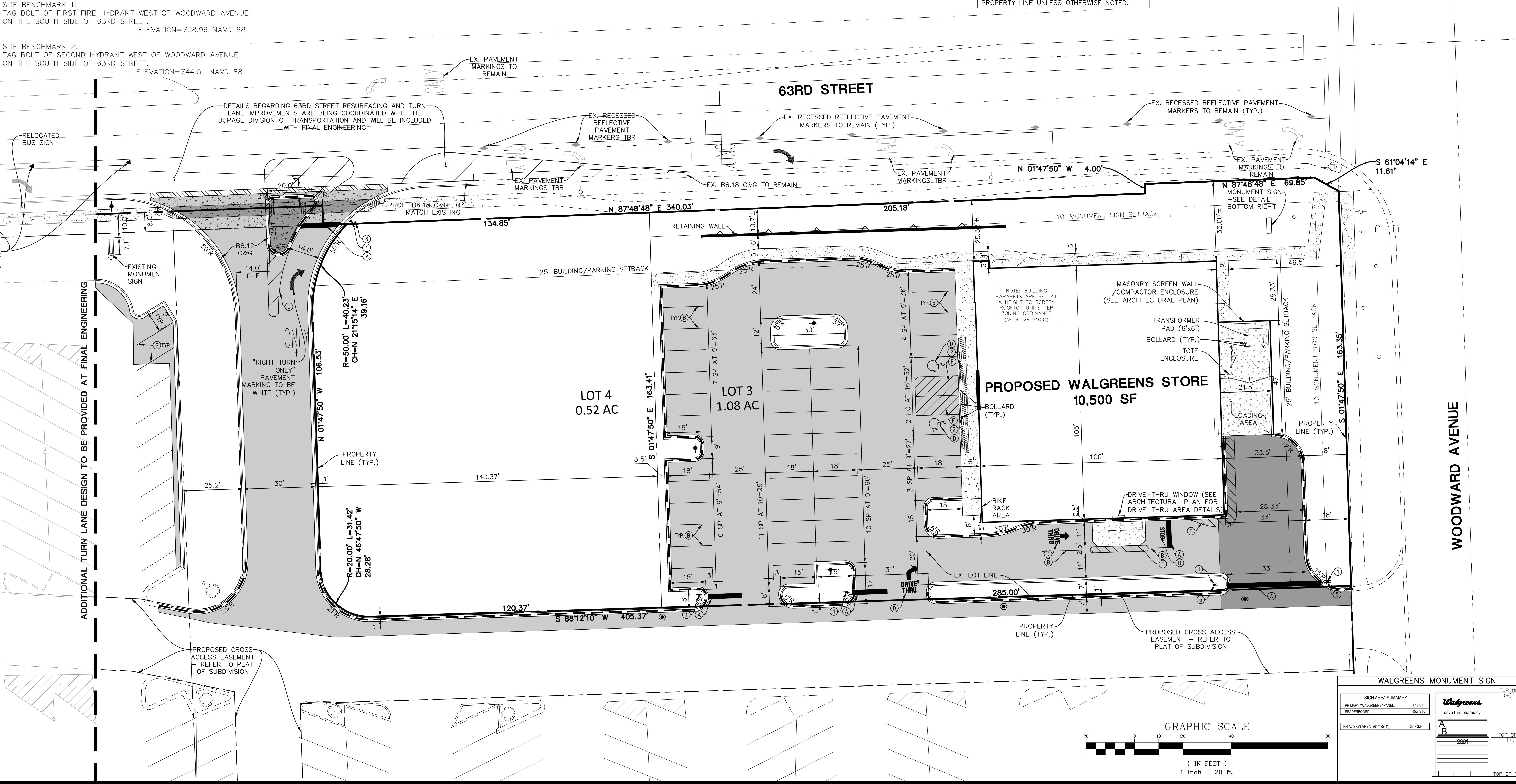
Project Name: 63RD STREET AND WOODWARD AVENUE
Address: 2001 63rd Street
Pin: 08-24-202-009
Zoning District: B-2
Existing Use: Commercial Restaurant (Roundhead's Pub)
Proposed Use: Commercial Retail (Walgreens)
Petition Type: Planned Unit Development / Special Use Permit
Deviations: Proposed Drive - Thru (Authorized Special Use in Zone B-2)

| Requirement | Factor | Required | Proposed/Existing | Meets Req.? | Difference |
|--------------|---------|---------------------------------|---------------------------------------|-------------|------------|
| Lot Frontage | Minimum | - | 125.04' (Lot 4) - 454.01' (Lot 3) | - | - |
| Lot Area | Minimum | - | 22,615 SF (Lot 4) - 47,138 SF (Lot 3) | - | - |
| Lot Width | Minimum | - | 140.37' (Lot 4) - 285.00' (Lot 3) | - | - |
| Street Yard | Minimum | 25' | 25.32 | Yes | +0.32' |
| Rear Yard | Minimum | - | - | - | - |
| Side Yard | Minimum | - | - | Yes | - |
| Open Space | Maximum | 35' | 29' 4" | Yes | -5' 8" |
| Height | Minimum | 10% | 24% (Lot 3) | Yes | +14% |
| FAR | Maximum | 0.75 | 0.22 (Lot 3) | Yes | -0.53 |
| Parking | Minimum | 3.5 Spaces per 1,000 sq ft (37) | 4.1 Spaces per 1,000 sq ft (43) | Yes | +6 spaces |
| Donations* | Minimum | - | - | - | - |

- PAVEMENT MARKING LEGEND**
- 24" WHITE STOP BAR
 - 4" YELLOW LINE
 - 6" SOLID WHITE
 - LETTERS AND SYMBOLS PAVEMENT MARKINGS
 - 6" WHITE - 2' DASH 6' SKIP
 - 4" YELLOW DIAGONAL AT 45° SPACED 2' O.C. W/ 4" YELLOW BORDER
 - 12" YELLOW DIAGONAL AT 45° (3 SPACED EVENLY) W/ DOUBLE 4" YELLOW (11" O.C.) BORDER

CONTRACTOR SHALL REFER TO ARCHITECTURAL PLANS FOR EXACT LOCATION OF SIDEWALKS, SIDEWALK SCORING, BENCHES, BIKE RACKS, FLAG POLES, ETC., DIMENSIONS OF VESTIBULE, RAMPS AND TRUCK DOCKS, PRECISE BUILDING DIMENSIONS AND EXACT UTILITY ENTRANCE LOCATIONS

CONTROL POINT FOR LAYOUT OF ALL PROPOSED IMPROVEMENTS IS THE SOUTHEAST PROPERTY CORNER BUILDING AND PARKING TO BE PARALLEL AND PERPENDICULAR TO EAST PROPERTY LINE UNLESS OTHERWISE NOTED.



WALGREENS MONUMENT SIGN

| SIGN AREA SUMMARY | |
|----------------------------|-----------|
| PRIMARY WALGREENS PANEL | 17.9 S.F. |
| READERBOARD | 15.8 S.F. |
| TOTAL SIGN AREA (6-475-47) | 33.7 S.F. |

GRAPHIC SCALE
 (IN FEET)
 1 inch = 20 ft.

DATE: 12-20-17
SCALE: 1"=20'
SHEET: C-300
FREDG

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PROPOSED WALGREENS - PRELIMINARY
VILLAGE OF DOWNERS GROVE, ILLINOIS
SITE DIMENSIONAL AND PAVING PLAN

PRELIMINARY PLAN - NOT FOR CONSTRUCTION

| NO. | DATE | REVISIONS | BY | CHK |
|-----|------|-----------|----|-----|
| | | | | |

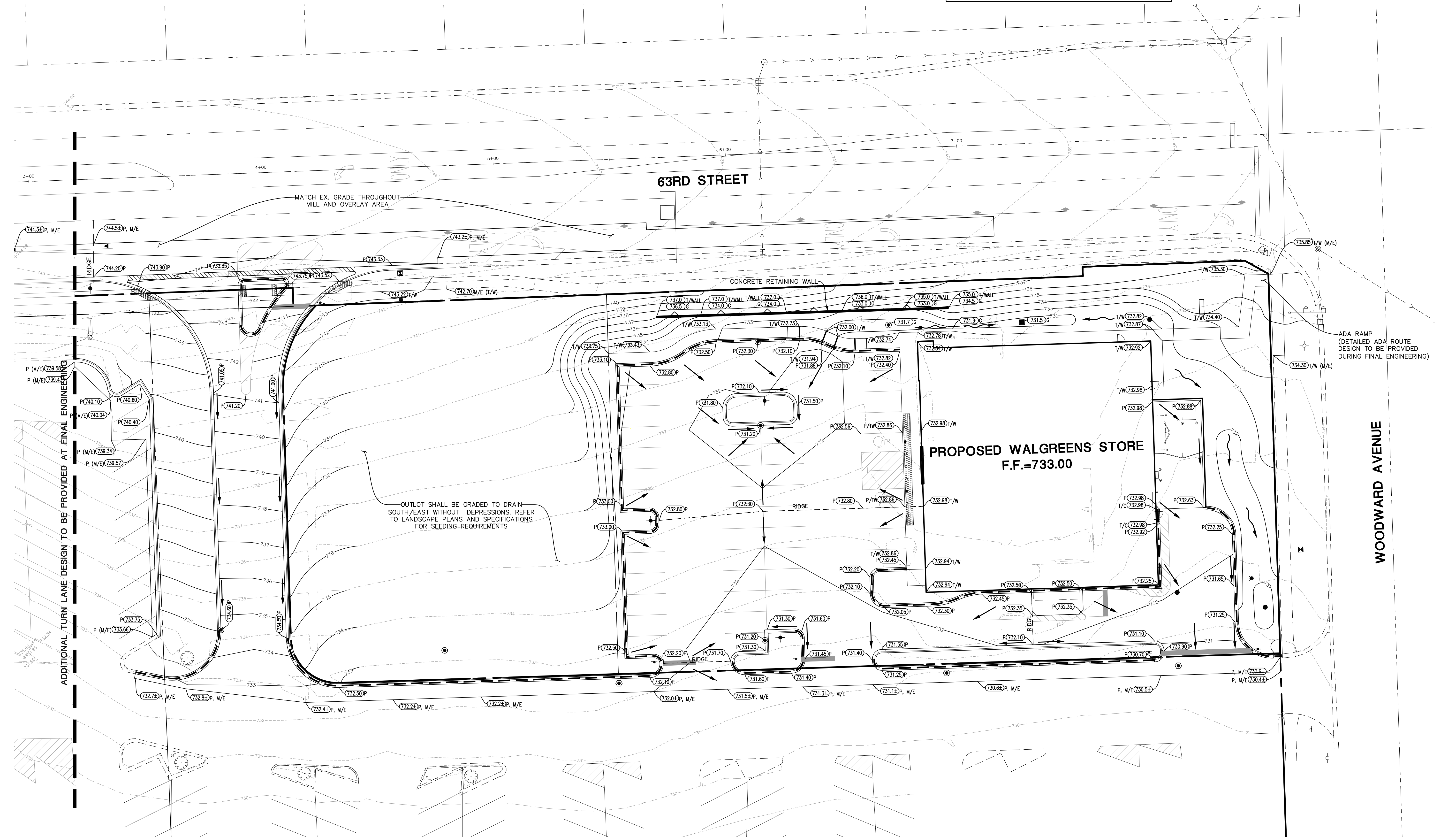
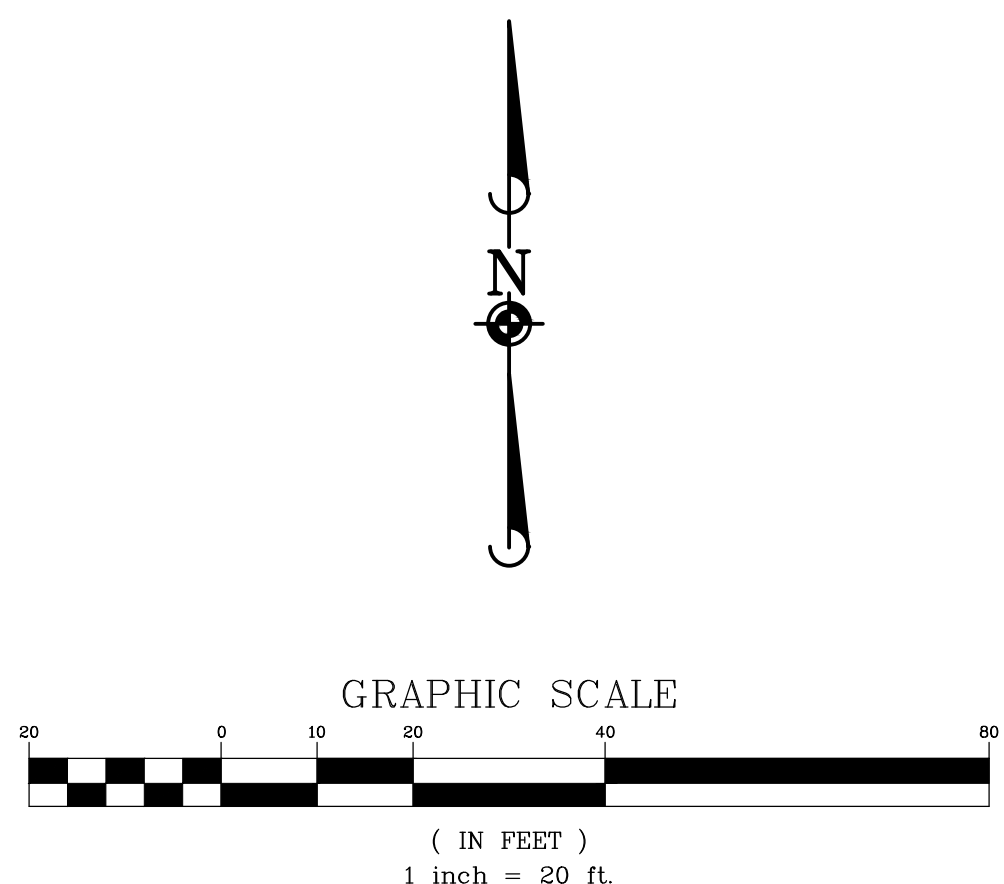
PROJ. MGR.: SMS
 PROJ. ASSOC.: HCM
 DRAWN BY: HCM
 DATE: 12-20-17
 SCALE: 1"=20'
 SHEET: C-300
 FREDG

- GRADING NOTES:**
1. RETAINING WALL DESIGN TO BE PROVIDED BY OTHERS.
 2. PAVEMENT SLOPES THROUGH HANDICAP ACCESSIBLE PARKING AREAS SHALL BE 2.00% MAXIMUM IN ANY DIRECTION.
 3. ALL HANDICAP RAMPS SHALL BE CONSTRUCTED WITH A MAXIMUM CROSS SLOPE OF 2.00% OR LESS.
 4. MEET EXISTING GRADE AT PROPERTY LIMITS UNLESS NOTED OTHERWISE.
 5. CONTRACTOR SHALL REFER TO THE SOIL EROSION AND SEDIMENT CONTROL PLAN AND DETAILS FOR CONSTRUCTION SCHEDULING AND EROSION CONTROL MEASURES TO BE INSTALLED PRIOR TO BEGINNING GRADING OPERATIONS.
 6. THE CONTRACTOR SHALL CONTACT J.U.L.I.E. (1-800-892-0123) PRIOR TO ANY WORK TO LOCATE UTILITIES AND SHALL CONTACT THE OWNER SHOULD UTILITIES APPEAR TO BE IN CONFLICT WITH THE PROPOSED IMPROVEMENT.
 7. THE CONTRACTOR IS SPECIFICALLY CAUTIONED THAT THE LOCATION AND/OR ELEVATION OF EXISTING UTILITIES AS SHOWN ON THESE PLANS IS BASED ON RECORDS OF THE VARIOUS UTILITY COMPANIES, AND WHERE POSSIBLE, MEASUREMENTS TAKEN IN THE FIELD. THE INFORMATION IS NOT TO BE RELIED ON AS BEING EXACT OR COMPLETE. THE CONTRACTOR MUST CALL THE APPROPRIATE UTILITY COMPANIES AT LEAST 72 HOURS BEFORE ANY EXCAVATION TO REQUEST EXACT FIELD LOCATION OF UTILITIES. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO RELOCATE ALL EXISTING UTILITIES WHICH CONFLICT WITH THE PROPOSED IMPROVEMENTS SHOWN ON THE PLANS.
 8. IF ANY EXISTING STRUCTURES TO REMAIN ARE DAMAGED DURING CONSTRUCTION IT SHALL BE THE CONTRACTORS RESPONSIBILITY TO REPAIR AND/OR REPLACE THE EXISTING STRUCTURE AS NECESSARY TO RETURN IT TO EXISTING CONDITION OR BETTER.
 9. ALL UNSURFACED AREAS DISTURBED BY GRADING OPERATION SHALL RECEIVE 6 INCHES OF TOPSOIL. CONTRACTOR SHALL APPLY STABILIZATION FABRIC TO ALL SLOPES 3H:1V OR STEEPER. CONTRACTOR SHALL STABILIZE DISTURBED AREAS IN ACCORDANCE WITH GOVERNING SPECIFICATIONS UNTIL A HEALTHY STAND OF VEGETATION IS OBTAINED.
 10. EXISTING TOPOGRAPHY SHOWN REPRESENTS SITE CONDITIONS AS PREPARED BY MANHARD CONSULTING LTD ON MARCH 15, 2015. CONTRACTOR SHALL FIELD CHECK EXISTING ELEVATIONS AND CONDITIONS PRIOR TO CONSTRUCTION AND NOTIFY ARCHITECT AND ENGINEER OF ANY DISCREPANCIES PRIOR TO STARTING CONSTRUCTION. IF THE CONTRACTOR DOES NOT ACCEPT EXISTING TOPOGRAPHY AS SHOWN ON THE PLANS, WITHOUT EXCEPTION, THEN THE CONTRACTOR SHALL SUPPLY, AT THEIR EXPENSE, A TOPOGRAPHIC SURVEY BY A REGISTERED LAND SURVEYOR TO THE OWNER FOR REVIEW.
 11. TRANSITIONS FROM DEPRESSED CURB TO FULL HEIGHT CURB SHALL BE TAPERED AT 2H:1V UNLESS OTHERWISE NOTED.

- VILLAGE OF DOWNERS GROVE:**
- GENERAL NOTES:**
- A FINAL GRADING SURVEY IS REQUIRED AT THE COMPLETION OF THE PROJECT, INCLUDING AN ELECTRONIC COPY. IT SHALL INCLUDE, BUT IS NOT LIMITED TO, THE FOLLOWING ITEMS:
- SUMP PUMP DISCHARGE LOCATION, DISCHARGE PATH, AND THE LOCATION, SIZE, AND MATERIAL OF ANY ASSOCIATED PIPING. (SUMP PUMP DISCHARGE SHALL BE AT LEAST 20 FEET FROM THE DOWNSTREAM PROPERTY LINE)
 - DOWNSPOUT LOCATION, DISCHARGE PATH, AND THE LOCATION, SIZE, AND MATERIAL OF ANY ASSOCIATED PIPING. (DOWNSPOUT DISCHARGE SHALL BE AT LEAST 20 FEET FROM THE DOWNSTREAM PROPERTY LINE)
 - TOP OF FOUNDATION ELEVATIONS OF ALL NEW STRUCTURES
 - SPOT GRADES ADJACENT TO THE FOUNDATIONS OF ALL NEW STRUCTURES
 - ALL NEW IMPERVIOUS AREAS INCLUDING THOSE MADE OF CONCRETE, ASPHALT, AND BRICK
 - STOOPS OUTSIDE OF DOORWAYS
 - WINDOW WELL LOCATIONS, ELEVATIONS, AND ADJACENT GRADE
 - UPDATED CALCULATIONS OF THE AS-BUILT IMPERVIOUS AREAS, TABULATED TO SHOW THE NET INCREASE IN IMPERVIOUS AREA. (ANY INCREASE IN IMPERVIOUS AREA FROM THE PROPOSED WILL RESULT IN AN ADDITIONAL FEE)

GRADING PLAN LEGEND

| | |
|--|---|
| | PROPOSED 1 FOOT CONTOURS |
| | PROPOSED SPOT ELEVATION |
| | PROPOSED FINISHED FLOOR ELEVATION |
| | PROPOSED GRADE AT FOUNDATION |
| | PROPOSED PAVEMENT ELEVATION |
| | PROPOSED TOP OF CURB |
| | PROPOSED TOP OF WALK |
| | PROPOSED TOP OF WALL |
| | MEET EXISTING |
| | PROPOSED GROUND GRADE OR GROUND AT BASE OF RETAINING WALL |
| | PROPOSED DITCH OR SWALE |
| | PROPOSED DIRECTION OF FLOW |
| | OVERFLOW RELIEF SWALE |
| | PROPOSED RIDGE LINE |
| | PROPOSED DEPTH OF PONDING |
| | RETAINING WALL |
| | PROPOSED SWALE LOW POINT |
| | PROPOSED SWALE SUMMIT |



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PROPOSED WALGREENS - PRELIMINARY
VILLAGE OF DOWNERS GROVE, ILLINOIS
GRADING PLAN

PROJ. MGR.: SMS
 PROJ. ASSOC.: HCM
 DRAWN BY: SWL
 DATE: 12-20-17
 SCALE: 1" = 20'

SHEET
C-400
 FREDG

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PRELIMINARY PLAN - NOT FOR CONSTRUCTION

J:\000016_2018_14-19_Dwg Name: P:\FredG\Draws\Pre\manhard\400P - GRADING PRELIM.dwg, Updated By: hmcwv

| Plant List | | | | | |
|---------------------------|----------|------------------------|---|---------|------------------|
| Key | Quantity | Common Name | Botanical Name | Size | Comments |
| Canopy Trees | | | | | |
| ACFR | 9 | AUTUMN BLAZE MAPLE | Acer freemani | 2.5" BB | |
| GIBI | 9 | GINKGO (MALE) | Ginkgo biloba 'Autumn Gold' | 2.5" BB | |
| GLTI | 7 | SKYLINE HONEYLOCUST | Gleditsia tricanthos inermis | 2.5" BB | |
| QRU | 9 | RED OAK | Quercus rubra | 2.5" BB | |
| Deciduous Shrubs | | | | | |
| COSA | 5 | ARCTIC SUN DOGWOOD | Cornus sanguinea 'Cato' | 36" HT | |
| HYLL | 13 | LITTLE LIME HYDRANGEA | Hydrangea 'Little Lime' | 36" HT | |
| ROSK | 15 | KNOCKOUT SHRUB ROSE | Rosa 'Knockout' | 36" HT | |
| SYMB | 15 | BLOOMERANG LILAC | Syringa 'Bloomerang Purple Lilac' | 36" HT | |
| Evergreen Shrubs | | | | | |
| JUCK | 29 | KALLAY COMPACT JUNIPER | Juniperus chinensis 'Kallay' | 36" HT | |
| Ornamental Grasses | | | | | |
| CALK | 26 | FEATHER REED GRASS | Calamagrostis acutiflora 'Karl Forerster' | #1 | |
| MISP | 26 | PURPLE MAIDEN GRASS | Miscanthus sinsensis 'Purpurascens' | #1 | |
| PEAH | 18 | DWARF FOUNTAIN GRASS | Pennisetum alopecuroides 'Hamel' | #1 | |
| Groundcovers | | | | | |
| GERR | 24 | ROZANNE GERANIUM | Geranium 'Rozanne' | #1 | Planted 30" O.C. |

Note: 4" of shredded bark mulch in all beds unless otherwise noted

Landscaped Open Space Table

Requirement: 10% of lot to be landscaped open space. At least 50% of the landscaped open space must be located in the street yard.

| | Area | % |
|----------------------------------|-----------|------|
| Lot 3 Impervious Surface | 34,882 sf | 74% |
| Lot 3 Landscaped Open Space | 12,256 sf | 26% |
| Lot 3 Area | 47,138 sf | 100% |
| Lot 3 Street Yard Open Space | 10,163 sf | 83% |
| Lot 3 Non-Street Yard Open Space | 2,093 sf | 17% |
| Lot 3 Landscaped Open Space | 17,471 sf | 100% |

LEGEND

- .plp Parking Lot Perimeter
- .pli Parking Lot Islands
- .si Signage Landscaping
- .scr Screening Landscaping

TURF GRASS
 *SOD PER WALGREENS SPECS

Village of Downers Grove Required Landscaping

PARKING LOT PERIMETER (.plp)

Requirement: When a parking lot is located across the street from a residential zoning district, parking lot perimeter landscaping must be provided along 100% of the street frontage. When across from a nonresidential zoning district, landscaping must be provided along at least 75% of the parking lot perimeter. Shade trees to be provided at a rate of at least one tree per 30 lf of street frontage. Parkway trees may be counted.

63rd Street: 235 LF @100% landscaping and 8 trees (incl. bldg)

East & West Sides: 139 LF (Each side) @ 50% landscaping and 5 trees each side

On Plan - 18 canopy trees total (plus 4 additional at entrance)
63rd Street = 100% screening
E & W Sides = 50% screening

PARKING LOT ISLANDS (.pli)

At least one shade tree must be provided for each 150 sf of landscape island. Islands must be located at the end of each parking row and between every 20 spaces.

At least 50% of every landscape island and landscape divider median must be planted with live plant material, such as perennials, ground cover, shrubs, or turf grass to a maximum height of 30 inches at maturity.

Required - 11 shade trees in islands, 50% coverage with live plant material (1650 sf of islands)
On Plan - 11 shade trees in islands, more than 50% coverage with live plant material

SIGNAGE LANDSCAPING

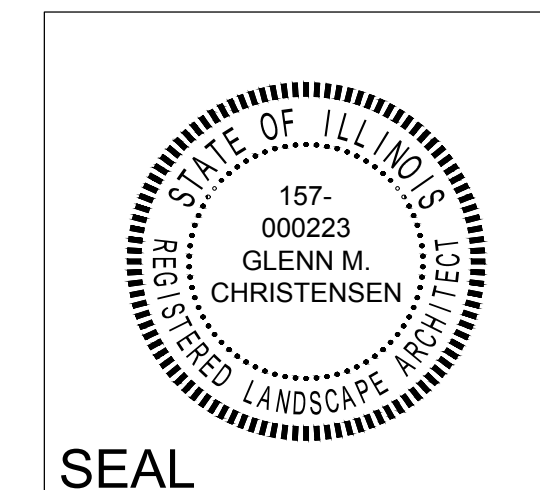
The base of all monument signs must be landscaped. Signs must be surrounded by a landscaped area of at least 3 ft.in width.

On Plan - Meets ordinance

SCREENING LANDSCAPING

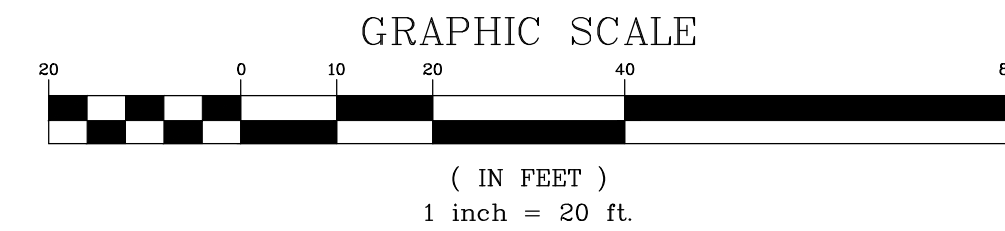
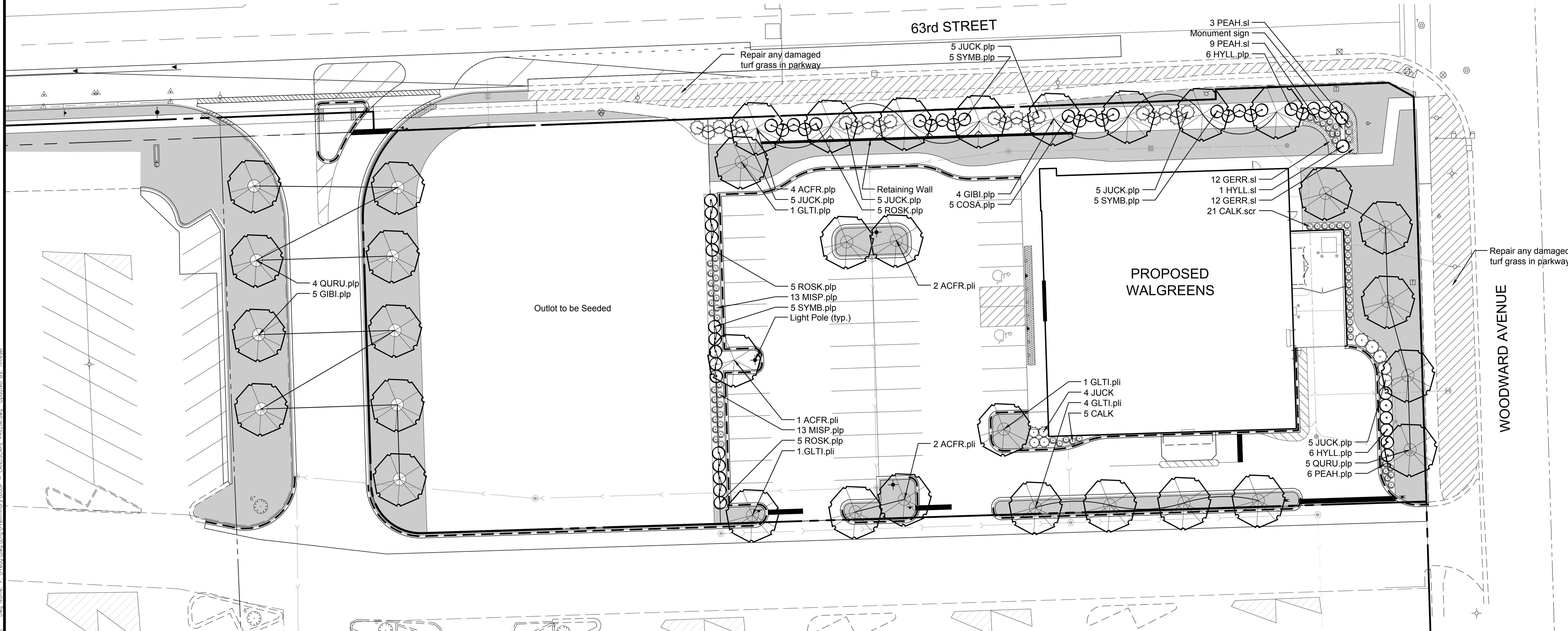
Screening of refuse areas must be screened with a wall or other screening material providing at least 80% direct visual screening at least 6' in height.

On Plan - Screening wall and landscaping; meets ordinance



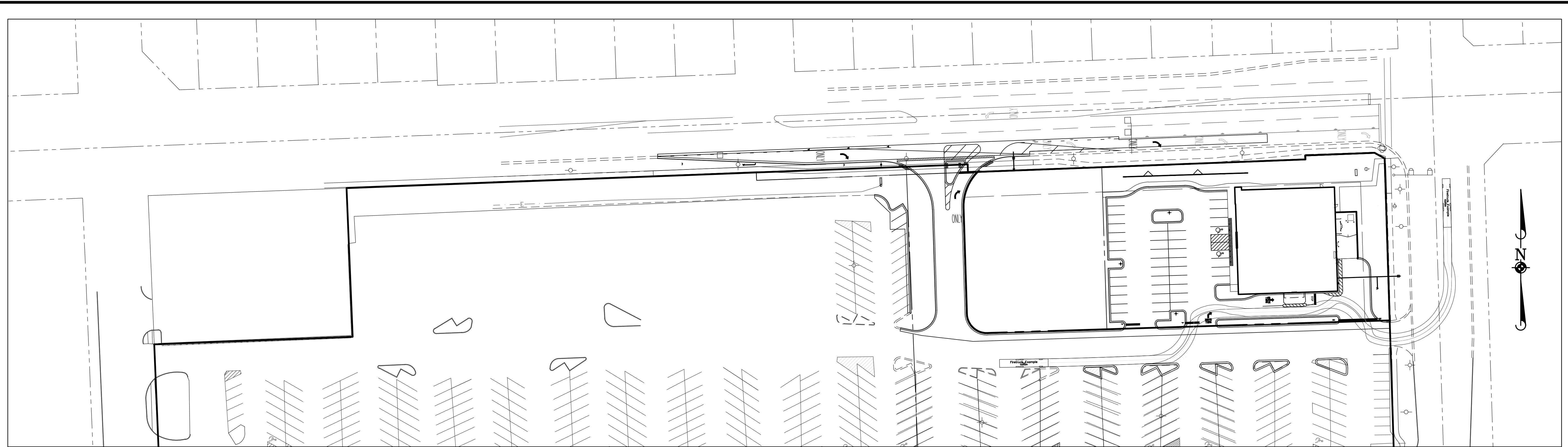
SEAL
Glenn M. Christensen, ASLA, RLA
Illinois Registered Landscape Architect # 157-000223

Glenn M. Christensen

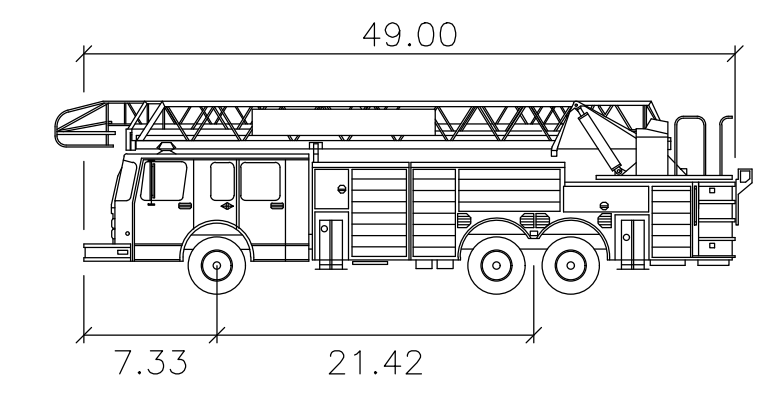


January 16, 2018 - 14:26 Day Name: P:\Fredg\dwg\Prop\Pre\manh\000P - LANDSCAPE PRELIM.dwg Updated By: hmcv

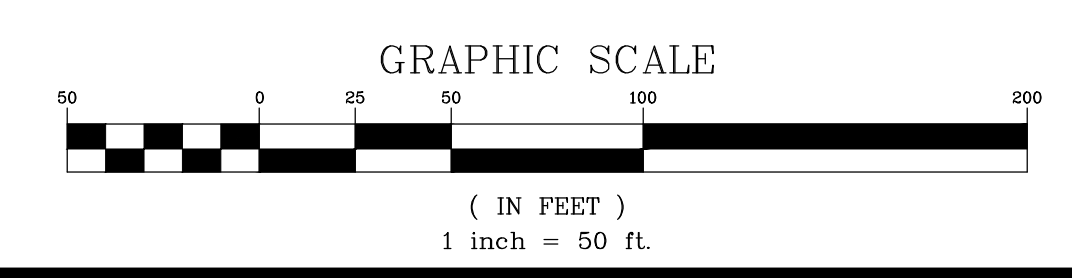
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| DRAWN BY | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | | REVISIONS | | DATE | | 157-000223 | | 157-000223 | | 01-18-18 | |
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AERIAL FIRE TRUCK



- Firetruck Example
- Width : 8.00
 - Track : 8.00
 - Lock to Lock Time : 6.0
 - Steering Angle : 32.6



December 20, 2017 - 15:49 - Dwg Name: P:\Fredg\Draws\Final Drawings\Exhibits\Fredg\Autoturn Exhibit REVISED SITEPLAN.dwg - Updated By: hmc

| DATE | REVISIONS | DRAWN BY |
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| 01-18-18 | REVISED PER VILLAGE REVIEW | HCM |

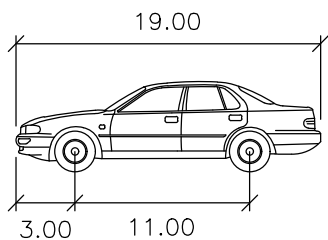
Manhard CONSULTING LTD.
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WALGREENS
VILLAGE OF DOWNERS GROVE
AUTOTURN EXHIBIT

PROJ. MGR.: TP
 PROJ. ASSOC.: SW
 DRAWN BY: HCM
 DATE: 12-20-17
 SCALE: 1" = 50'

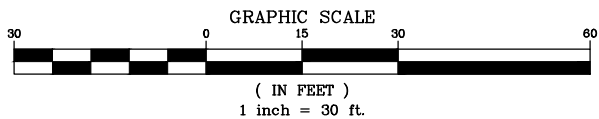
SHEET
2 OF **2**
 FREDG

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P feet
 Width : 7.00
 Track : 6.00
 Lock to Lock Time : 6.0
 Steering Angle : 31.6

**PROPOSED
 WALGREENS**



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 Civil Engineers • Surveyors • Water Resource Engineers • Water & Wastewater Engineers
 Construction Managers • Environmental Scientists • Landscape Architects • Planners

| | |
|------------------------------------|------------------|
| WALGREENS | |
| VILLAGE OF DOWNERS GROVE | |
| DRIVE THRU STACKING EXHIBIT | |
| PROJ. MGR.: <u>SMS</u> | SHEET |
| DRAWN BY: <u>HCM</u> | EXHIBIT 1 |
| DATE: <u>01/29/17</u> | FREDG |
| SCALE: <u>1"=30'</u> | |

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MEMORANDUM TO: Mitchell P Kahn
Frontline Real Estate Partners, LLC

FROM: Javier Millan
Senior Consultant

Luay Aboona, PE
Principal

DATE: December 15, 2017

SUBJECT: Trip Generation Comparison
Revised Walgreens Development Plan
Downers Grove, Illinois

This memorandum provides a comparison of the estimated traffic to be generated by the (1) approved plan and (2) current proposed plan for the Walgreens Drive-Through Pharmacy store to be located in the southwest corner of the intersection of 63rd Street with Woodward Avenue within the Meadowbrook shopping center in Downers Grove, Illinois. The approved development plan included an approximate 14,500 square-foot Walgreens pharmacy with drive-through facility. As currently proposed, the development plan calls for a smaller Walgreens of approximately 10,500 square feet to be located on the east end of the parcel. The west end of the parcel will be developed by others at a later time. Access to the Walgreens pharmacy will continue to be provided via the existing access drives serving the Meadowbrook shopping center.

The number of peak hour vehicle trips estimated to be generated by the approved development plan and the current proposed development plan were based on trip data for land use code 881 (Pharmacy/Drugstore w/Drive-Through) published by the Institute of Transportation Engineers (ITE) in its *Trip Generation Manual*, 9th Edition. **Table 1** shows the traffic estimated to be generated by the approved development plan and the current proposed development plan. With the reduction in size, it can be seen that the current proposed development plan is estimated to generate less traffic than the approved development plan. As such, the findings and conclusions of the original traffic impact evaluation dated February 2, 2017 remain.

Table 2
EXISTING AND ESTIMATED TRAFFIC VOLUMES – PROPOSED WALGREENS

| Land-Use Code | Type/Size | Weekday Morning Peak Hour | | | Weekday Evening Peak Hour | | | Saturday Midday Peak Hour | | | Daily Two-Way Traffic |
|--|---|---------------------------|------------|------------|---------------------------|------------|------------|---------------------------|------------|------------|-----------------------|
| | | In | Out | Total | In | Out | Total | In | Out | Total | Total |
| Approved Development Plan | | | | | | | | | | | |
| 881 | Pharmacy/Drugstore w/ Drive-Through (14,500 s.f.) | 26 | 24 | 50 | 72 | 72 | 144 | 59 | 61 | 120 | 1,406 |
| | <i>Pass-By Trip Reduction (50%):</i> | <u>-13</u> | <u>-12</u> | <u>-25</u> | <u>-36</u> | <u>-36</u> | <u>-72</u> | <u>-30</u> | <u>-30</u> | <u>-60</u> | <u>-702</u> |
| Total New Trips Generated: | | 13 | 12 | 25 | 36 | 36 | 72 | 29 | 31 | 60 | 704 |
| Current Proposed Development Plan | | | | | | | | | | | |
| 881 | Pharmacy/Drugstore w/ Drive-Through (10,500 s.f.) | 19 | 17 | 36 | 52 | 52 | 104 | 42 | 44 | 86 | 1,018 |
| | <i>Pass-By Trip Reduction (50%):</i> | <u>-10</u> | <u>-8</u> | <u>-18</u> | <u>-26</u> | <u>-26</u> | <u>-52</u> | <u>-21</u> | <u>-22</u> | <u>-43</u> | <u>-509</u> |
| Total New Trips Generated: | | 9 | 9 | 18 | 26 | 26 | 52 | 21 | 22 | 43 | 509 |
| Difference | | -4 | -3 | -7 | -10 | -10 | -20 | -8 | -9 | -17 | -195 |



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MEMORANDUM TO: Mitchell P. Kahn
Frontline Real Estate Partners, LLC

FROM: Javier Milan
Senior Consultant

Luay R. Aboona, PE
Principal

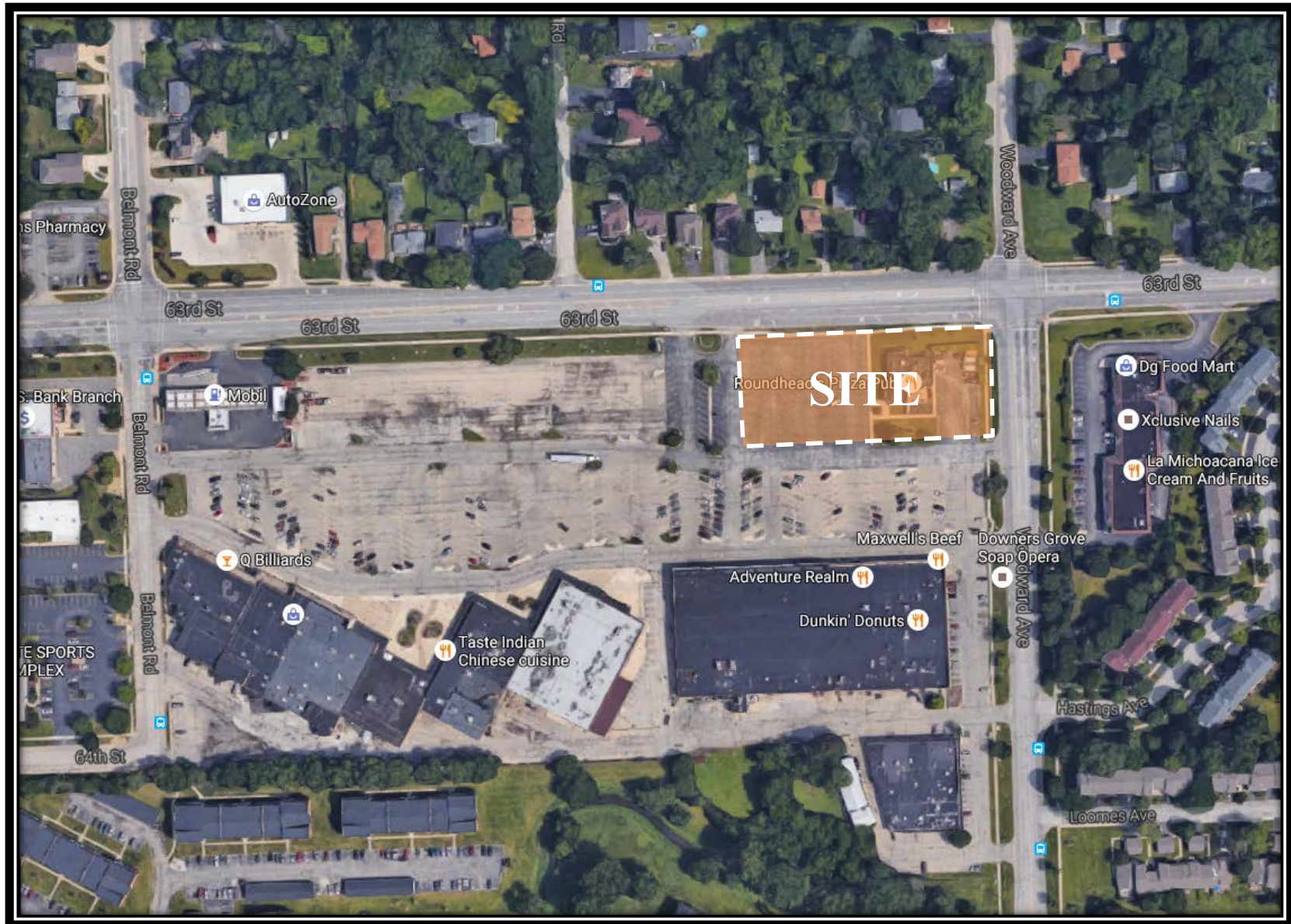
DATE: February 2, 2017

SUBJECT: Traffic Impact Evaluation
Proposed Walgreens Drive-Through Pharmacy Store
Downers Grove, Illinois

This memorandum summarizes the results of a traffic impact evaluation conducted by Kenig, Lindgren, O'Hara, Aboona, Inc. (KLOA, Inc.) for the proposed Walgreens Drive-Through Pharmacy Store in Downers Grove, Illinois. The plans call for relocating the existing Walgreens store in the northwest corner of the intersection of 63rd Street with Belmont Avenue to the southwest corner of the intersection of 63rd Street with Woodward Avenue. The proposed location, which is currently occupied by Roundhead's Pizza Pub, is located within the Meadowbrook shopping center. **Figure 1** shows an aerial view of the site area.

The purpose of this evaluation is to address concerns raised regarding existing traffic operations at the intersection of 63rd Street with Woodward Avenue, which include the following:

- Queueing and delays experienced by traffic on Woodward Avenue
- The impact of the additional traffic that the proposed Walgreens development will generate
- The projected increase in traffic on Woodward Avenue north of 63rd Street



Aerial View of Site Location

Figure 1

Existing Roadways

The existing roadways and traffic control characteristics of the adjacent roadways are described below.

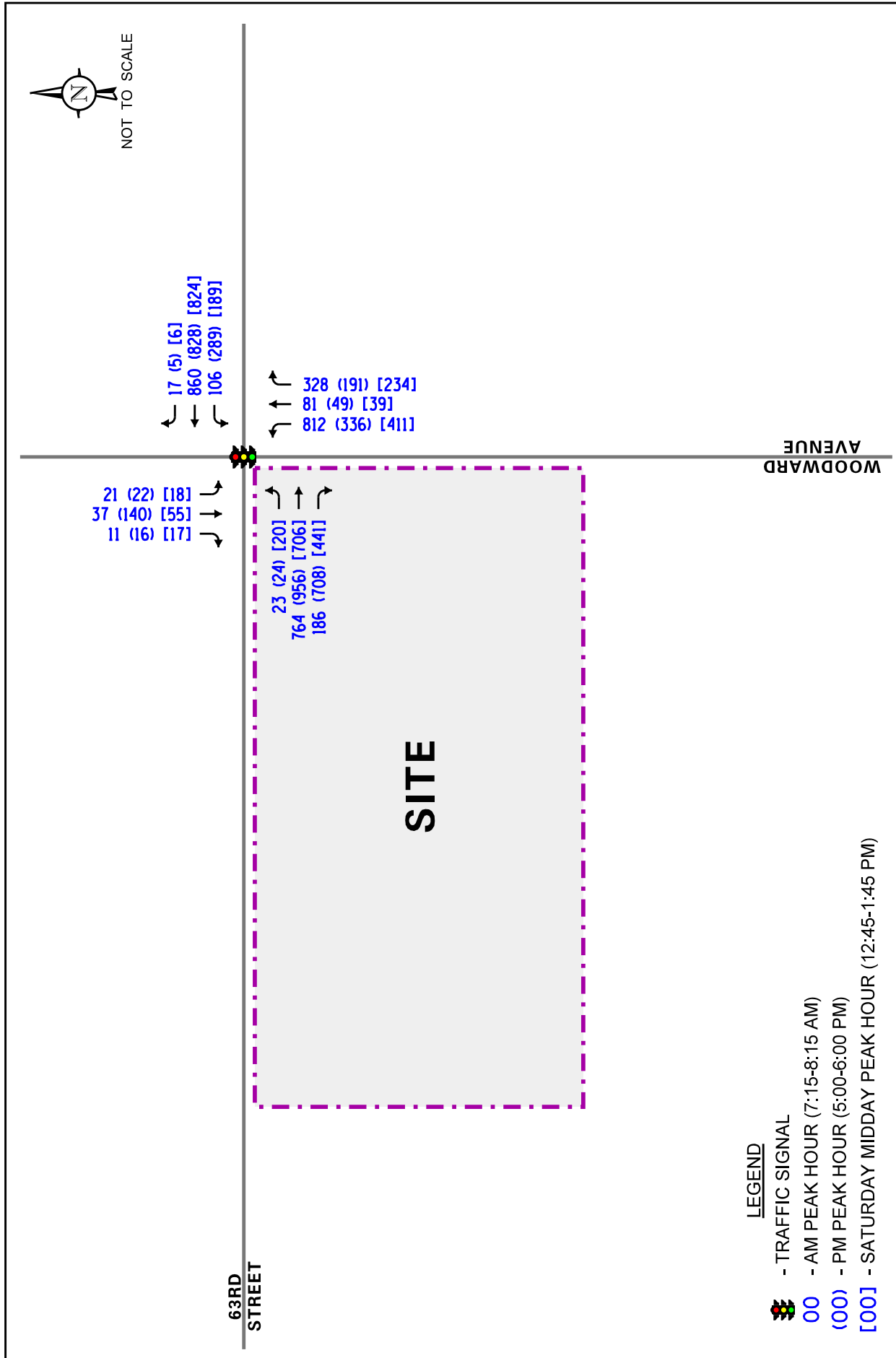
63rd Street (DuPage County Route 38) is an east-west roadway with a five-lane cross section that in the vicinity of the site provides two lanes in each direction divided by a striped median. At its signalized intersection with Woodward Avenue, 63rd Street provides an exclusive left-turn lane, two exclusive through lanes, and an exclusive right-turn lane on the eastbound approach and an exclusive left-turn lane, an exclusive through lane, and a shared through/right-turn lane on the westbound approach. Standard style crosswalks are provided on all legs of the intersection along with pedestrian signals. 63rd Street is under the jurisdiction of the DuPage County Division of Transportation (DuDOT) and has a posted speed limit of 40 miles per hour (mph).

Woodward Avenue is a north-south roadway that provides one lane in each direction north of 63rd Street and two lanes in each direction south of 63rd Street. At its signalized intersection with 63rd Street, Woodward Avenue provides a shared left-turn/through lane and a shared through/right-turn lane on the southbound approach and an exclusive left-turn lane, a shared left-turn/through lane, and an exclusive right-turn lane on the northbound approach. Woodward Avenue is under the jurisdiction of Lisle Township and Downers Grove Township north of 63rd street and the Village of Downers Grove south of 63rd Street. Woodward Avenue has a posted speed limit of 25 mph north of 63rd Street increasing to 30 mph south of 63rd Street. Through traffic and trucks over eight tons are prohibited on Woodward Avenue via signage north of 63rd Street.

Existing Traffic Volumes

In order to determine current traffic conditions at the intersection of 63rd Street with Woodward Avenue, KLOA, Inc. conducted peak period traffic counts on Saturday, January 21, 2017 and on Tuesday, January 24, 2017 during the weekday morning (7:00 A.M. to 9:00 A.M.) and weekday evening (2:30 P.M. to 6:00 P.M.) peak periods and on Saturday January 21, 2017 during the midday (12:00 P.M. to 2:00 P.M.) peak period. The results of the traffic counts showed that the weekday morning peak hour of traffic occurs from 7:15 A.M. to 8:15 A.M., the weekday evening peak hour of traffic occurs from 4:00 P.M. to 5:00 P.M., and the Saturday midday peak hour of traffic occurs from 12:45 P.M. to 1:45 P.M. **Figure 2** illustrates the existing peak hour traffic volumes. Summaries of the traffic counts can be found in the Appendix.

In addition, the results of the traffic counts were compared with counts previously conducted by DuDOT in 2014 and were found to be generally consistent. It should also be noted that the traffic counts were conducted while the nearby Indian Trail Elementary School was in session and school-related traffic was included in the traffic counts. A review of the traffic counts showed that approximately 30 percent more traffic traveled through the intersection of 63rd Street with Woodward Avenue during the evening peak hour (5:00 P.M. to 6:00 P.M.) than during the afternoon peak hour (2:30 P.M. to 3:30 P.M.) when the school ends. As such, the higher evening traffic volumes were utilized in the evaluation.



KLOA
Job No: 16-297

Figure: 2

TITLE:
Existing Traffic Volumes

PROJECT:
Proposed
Walgreens Development
Downers Grove, Illinois

Traffic Operations of the 63rd Street/Woodward Avenue Intersection

The intersection of 63rd Street with Woodward Avenue is under traffic signal control with split phasing for the northbound and southbound approaches. This intersection is part of a coordinated system on 63rd Street that extends from Main Street to the east to Leonard Avenue to the west. The intersection of 63rd Street with Woodward Avenue is fully actuated on all approaches and provides protected/permissive left-turn phases on the eastbound and westbound approaches and right-turn overlap phases on the eastbound and northbound approaches. A sign facing southbound on Woodward Avenue north of 63rd Street prohibits non-local traffic on Woodward Avenue. However, no such signs are provided on Woodward Avenue at its intersection with Maple Avenue or on 61st Street or 59th Street at their respective intersections with Belmont Avenue. As such, cut-through traffic is only prohibited from traveling northbound on Woodward Avenue, not southbound.

Capacity analyses were conducted at the intersection under existing conditions utilizing the existing signal timings and phasing. The results of the capacity analyses expressed in terms of Level of Service (LOS) and average delays are summarized in **Table 1**. As can be seen, the intersection overall operates at an acceptable LOS C during all three peak hours. However, the northbound and southbound approaches operate at LOS D/E which is primarily due to the limited green time allocated to these approaches, the split phase, and the high volume of traffic on northbound Woodward Avenue.

The results of the capacity analyses were also confirmed by the following observations that were made of existing conditions:

- Morning Peak Hour
 - Traffic queues on northbound Woodward Avenue were consistently observed to extend between Hastings Avenue and Loomes Avenue for approximately 30 to 60 seconds; however, the queues cleared the intersection most of the time with each green phase.
 - Traffic queues on southbound Woodward Avenue were observed to consist of a maximum of four to eight vehicles each cycle and cleared the intersection with each green phase.
- Evening Peak Hour
 - Traffic queues on southbound Woodward Avenue were observed to consistently extend past the shared left-turn/through lane storage length and taper with combined lane queues of approximately 10 to 16 vehicles per cycle.
 - A significant portion of traffic traveling on Woodward Avenue north of 63rd Street during the peak hours was observed to be cut-through traffic traveling to/from Belmont Avenue to the west and Maple Avenue from the north.

- Based on the previous observation, the sign prohibiting non-local traffic on northbound Woodward Avenue north of 63rd Street is not being adhered to.
- Additional signage prohibiting cut-through traffic needs to be posted at 59th Street, 61st Street, and Maple Avenue.

The following is a summary of the reasons for the long delays and queues experienced at this intersection:

- The north-south split phasing nature of the intersection is the primary cause for the intersection's poor level of service, extensive queueing, and significant delay.
 - The split phasing at this intersection is required because of the striping on the south leg to accommodate the high volume of northbound left-turn movements and the limitation of sufficient right-of-way to geometrically improve the intersection to accommodate the existing traffic volumes without split phasing.
 - The northbound and southbound phases are allocated a proportionate amount of green time based on their respective traffic volumes which results in a minimal amount of green time for the southbound phase (approximately 8, 15, and 13 seconds during the weekday morning, weekday evening, and Saturday midday peak hours, respectively), thus resulting in the delays on the approach.
- The cause for the high volume of northbound left-turn movements on Woodward Avenue and the high volume of eastbound right-turn movements on 63rd Street is the result of how the area roadways are configured.
 - As previously mentioned, Woodward Avenue north of 63rd Street is restricted to local traffic only and does not allow vehicles over eight tons.
 - Vehicles traveling on Woodward Avenue that desire to continue to travel northbound must turn left onto westbound 63rd Street and then turn right onto northbound Belmont Street approximately one-quarter mile to the west of Woodward Avenue.
 - These two north-south streets act as non-continuous arterial roadways. Instead of being continuous or directly connected, traffic must travel on 63rd Street to continue to travel either north or south via Woodward or Belmont Avenue.

Table 1
 CAPACITY ANALYSIS RESULTS – 63RD STREET WITH WOODWARD AVENUE – SIGNALIZED

| | Peak Hour | Eastbound | | | Westbound | | | Northbound | | | Southbound | | | Overall |
|----------------------------|----------------------------------|-----------|-----------|-----------|-----------|-----------|-----------|------------|-----------|-----------|------------|---|-----------|----------|
| | | L | T | R | L | T | R | L | T | R | L | T | R | |
| Existing Conditions | Weekday Morning Peak Hour | C 22.8 | C 23.0 | A 6.3 | C 21.6 | C 22.7 | C 23.0 | E 60.1 | E 59.5 | D 37.3 | E 62.1 | | E 61.8 | C – 34.4 |
| | | B – 19.8 | | | C – 22.7 | | | D – 53.8 | | | E – 61.9 | | | |
| | Weekday Evening Peak Hour | B 13.8 | B 12.9 | B 17.0 | B 14.7 | A 7.4 | A 7.5 | E 60.7 | E 64.6 | D 44.5 | E 69.3 | | E 67.0 | C – 22.4 |
| | | B – 14.6 | | | A – 9.3 | | | E – 56.7 | | | E – 68.2 | | | |
| | Saturday Midday Peak Hour | B 14.2 | B 13.4 | B 10.7 | B 13.1 | B 11.1 | B 11.2 | D 47.7 | D 47.3 | D 37.8 | D 51.7 | | D 51.6 | C – 20.6 |
| | | B – 12.4 | | | B – 11.5 | | | D – 44.2 | | | D – 51.7 | | | |

Development Traffic Generation

The traffic to be generated by the proposed development was estimated using trip data published by the Institute of Transportation Engineers (ITE) in its *Trip Generation Manual*, 9th Edition. The trip rates were applied for the weekday morning and evening peak hours and on a daily basis for a Pharmacy/Drugstore with Drive-Through Window (Land-Use Code 881). In addition, the traffic currently generated by the existing Walgreens was observed and the resulting trip generation was compared with the ITE estimates. **Table 2** shows the trip generation comparison, which indicates that the estimated trips are very similar. It is important to note that surveys conducted by ITE have shown that up to 50 percent of trips made to pharmacy/drugstores with drive-through are diverted from the existing traffic on the roadway system. Such diverted trips are referred to as pass-by traffic. As such, a 50 percent pass-by reduction was applied to the trip generation estimates of the proposed development.

It is also important to note that the proposed Walgreens is a relocation of the existing Walgreens store located approximately one block west of the site. As such, the majority of its traffic is already traversing the intersection of 63rd Street with Woodward Avenue and as a result is not expected to add a significant amount of new traffic to the intersection, with its current operations expected to remain largely unchanged. However, in order to provide a conservative analysis, the traffic that will be generated by the proposed Walgreens was assumed to all be new to the area roadways.

In order to project Year 2018 conditions, existing traffic volumes on 63rd Street and Woodward Avenue were increased by one percent based on projections provided by the Chicago Metropolitan Agency for Planning (CMAP). In addition, traffic to be generated by the proposed development was assigned to the roadways as determined from the traffic counts. The assignment of traffic was determined as follows:

- 40 percent traveling to and from the west on 63rd Street
- 30 percent traveling to and from the east on 63rd Street
- 30 percent traveling to and from the south on Woodward Avenue
- Five percent traveling to and from the north of Woodward Avenue

The Year 2018 projected conditions for the intersection of 63rd Street with Woodward Avenue were analyzed. **Table 3** summarizes the intersection's LOS and delay for Year 2018 projected conditions during the peak hours.

Table 2
EXISTING AND ESTIMATED TRAFFIC VOLUMES – PROPOSED WALGREENS

| Land-Use Code | Type/Size | Weekday Morning Peak Hour | | | Weekday Evening Peak Hour | | | Saturday Midday Peak Hour | | | Daily Two-Way Traffic |
|---------------|---|---------------------------|------------|------------|---------------------------|------------|------------|---------------------------|------------|------------|-----------------------|
| | | In | Out | Total | In | Out | Total | In | Out | Total | Total |
| | Existing Walgreens Traffic | 12 | 7 | 19 | 65 | 77 | 142 | 58 | 59 | 117 | N/A |
| 881 | Pharmacy/Drugstore w/ Drive-Through (14,500 s.f.) | 26 | 24 | 50 | 72 | 72 | 144 | 59 | 61 | 120 | 1,406 |
| | <i>Pass-By Trip Reduction (50%):</i> | <u>-13</u> | <u>-12</u> | <u>-25</u> | <u>-36</u> | <u>-36</u> | <u>-72</u> | <u>-30</u> | <u>-30</u> | <u>-60</u> | <u>702</u> |
| | Total New Trips Generated: | 13 | 12 | 25 | 36 | 36 | 72 | 29 | 31 | 60 | 704 |

Table 3
CAPACITY ANALYSIS RESULTS – 63RD STREET WITH WOODWARD AVENUE – SIGNALIZED

| | Peak Hour | Eastbound | | | Westbound | | | Northbound | | | Southbound | | | Overall |
|---|---------------------------|-----------|-----------|-----------|-----------|-----------|-----------|------------|-----------|-----------|------------|---|-----------|----------|
| | | L | T | R | L | T | R | L | T | R | L | T | R | |
| Projected Conditions | Weekday Morning Peak Hour | C 23.1 | C 23.4 | A 6.3 | C 21.9 | C 23.1 | C 23.4 | E 60.7 | E 60.4 | D 37.2 | E 62.1 | | E 61.8 | C – 34.8 |
| | | C – 20.2 | | | C – 23.1 | | | D – 54.3 | | | E – 62.0 | | | |
| | Weekday Evening Peak Hour | B 14.1 | B 13.4 | B 17.4 | B 15.4 | A 7.7 | A 7.8 | E 60.4 | E 65.0 | D 44.1 | E 69.2 | | E 67.0 | C – 22.7 |
| | | B – 15.1 | | | A – 9.7 | | | E – 56.6 | | | E – 68.2 | | | |
| | Saturday Midday Peak Hour | B 14.4 | B 13.6 | B 10.8 | B 13.3 | B 11.3 | B 11.4 | D 47.8 | D 47.5 | D 37.6 | D 51.8 | | D 51.6 | C – 20.8 |
| | | B – 12.6 | | | B – 11.7 | | | D – 44.2 | | | D – 51.7 | | | |
| LOS – Level of Service Delay is measured in seconds. | | | | | | | | | | | | | | |

As can be seen, the intersection is expected to continue to operate at an overall LOS C during the weekday morning, weekday evening, and Saturday midday peak hours with an increase in overall delay during each of the peak hours of one second or less. Similarly, the increase in delay of the southbound approach as a result of the increase in traffic will be minimal (less than one second). As such, the proposed development will have a minimal impact on the operation of the intersection.

Potential Intersection and Roadway Improvements

While the proposed relocation of Walgreens will have a negligible impact on the intersection, the following improvements to the intersection and/or roadways could be considered:

- In order to reduce cut through traffic and reduce queues on southbound Woodward Avenue, signs prohibiting non-local traffic should be placed on Woodward Avenue just south of Maple Avenue and on 61st Street and 59th Street just east of Belmont Avenue.
- In order to reduce delays on Woodward Avenue, additional green time could be allocated to the northbound and/or southbound movements.
 - Preliminary analysis showed that providing an additional five seconds of green time for the southbound approach during the evening peak hour would reduce delay for all southbound movements by approximately five to six seconds.
 - However, this will increase the overall intersection delay by approximately seven seconds during the evening peak hour.
 - It is important to note that reducing delay for the southbound approach may encourage additional cut-through traffic on Woodward Avenue.
 - Any change to signal timing will require DuDOT review and may not be approved due to its impact on 63rd Street traffic and the interconnect system.

Conclusion

Based on the preceding evaluation, the following conclusions are made:

- The proposed Walgreens is a relocation of the existing store located at the northwest corner of the intersection of 63rd Street with Belmont Avenue.
- The signalized intersection of 63rd Street with Woodward Avenue currently operates as a split phase intersection causing queues and delays on both the northbound and southbound approaches.
- Despite these delays, queues were generally observed to clear the intersection with each green phase.
- Cut-through traffic is utilizing Woodward Avenue to travel to/from Belmont Avenue to the west and Maple Avenue from the north.
- The proposed Walgreens will not add a significant amount of new traffic to 63rd Street or Woodward Avenue and, as such, will have a minimal impact on the operations of the intersection of 63rd Street with Woodward Avenue.
- A minimal amount of traffic generated by the proposed development will travel to/from the north on Woodward Avenue.
- Signs prohibiting cut-through traffic should be placed on Woodward Avenue just south of Maple Avenue and on 61st Street and 59th Street just east of Belmont Avenue to reduce the traffic volumes of Woodward Avenue north of 63rd Street.
- Traffic delays on the southbound approach of Woodward Avenue may be reduced by allocating additional green time which may cause increases in overall intersection delays. Such modifications would be subject to DuDOT review.

Appendix

Traffic Count Summary Sheets
Level of Service Criteria
Capacity Analysis Summary Sheets

Traffic Count Summary Sheets



Kenig Lindgren O'Hara Aboona, Inc.
 9575 W. Higgins Rd., Suite 400
 Rosemont, Illinois, United States 60018
 (847)518-9990

Count Name: 63rd Street with Woodward Avenue
 Site Code:
 Start Date: 01/21/2017
 Page No: 1

Turning Movement Data

| Start Time | 63rd Street Eastbound | | | | | | 63rd Street Westbound | | | | | | Woodward Avenue Northbound | | | | | | Woodward Avenue Southbound | | | | | | Int. Total |
|---------------|-----------------------|------|------|-------|------|------------|-----------------------|------|------|-------|------|------------|----------------------------|------|------|-------|------|------------|----------------------------|------|------|-------|------|------------|------------|
| | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | |
| 12:00 PM | 0 | 1 | 161 | 120 | 0 | 282 | 0 | 50 | 175 | 2 | 0 | 227 | 0 | 89 | 9 | 65 | 0 | 163 | 0 | 1 | 22 | 3 | 1 | 26 | 698 |
| 12:15 PM | 0 | 6 | 169 | 111 | 0 | 286 | 0 | 56 | 192 | 2 | 0 | 250 | 0 | 95 | 13 | 46 | 0 | 154 | 0 | 1 | 20 | 5 | 0 | 26 | 716 |
| 12:30 PM | 0 | 5 | 197 | 111 | 0 | 313 | 0 | 61 | 170 | 4 | 0 | 235 | 0 | 100 | 11 | 60 | 2 | 171 | 0 | 2 | 18 | 4 | 0 | 24 | 743 |
| 12:45 PM | 0 | 3 | 159 | 94 | 0 | 256 | 0 | 55 | 205 | 1 | 0 | 261 | 0 | 116 | 12 | 60 | 0 | 188 | 0 | 5 | 20 | 2 | 0 | 27 | 732 |
| Hourly Total | 0 | 15 | 686 | 436 | 0 | 1137 | 0 | 222 | 742 | 9 | 0 | 973 | 0 | 400 | 45 | 231 | 2 | 676 | 0 | 9 | 80 | 14 | 1 | 103 | 2889 |
| 1:00 PM | 0 | 8 | 196 | 117 | 0 | 321 | 0 | 42 | 195 | 1 | 0 | 238 | 0 | 85 | 9 | 56 | 0 | 150 | 0 | 5 | 6 | 9 | 0 | 20 | 729 |
| 1:15 PM | 0 | 2 | 178 | 104 | 0 | 284 | 0 | 46 | 202 | 2 | 0 | 250 | 0 | 102 | 10 | 51 | 1 | 163 | 0 | 4 | 16 | 1 | 0 | 21 | 718 |
| 1:30 PM | 0 | 7 | 175 | 126 | 0 | 308 | 0 | 46 | 222 | 2 | 0 | 270 | 0 | 108 | 8 | 67 | 1 | 183 | 0 | 4 | 13 | 5 | 0 | 22 | 783 |
| 1:45 PM | 0 | 1 | 191 | 107 | 1 | 299 | 0 | 47 | 150 | 2 | 0 | 199 | 0 | 98 | 7 | 65 | 0 | 170 | 0 | 3 | 6 | 4 | 0 | 13 | 681 |
| Hourly Total | 0 | 18 | 740 | 454 | 1 | 1212 | 0 | 181 | 769 | 7 | 0 | 957 | 0 | 393 | 34 | 239 | 2 | 666 | 0 | 16 | 41 | 19 | 0 | 76 | 2911 |
| *** BREAK *** | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - |
| 7:00 AM | 0 | 1 | 179 | 37 | 0 | 217 | 0 | 16 | 212 | 2 | 0 | 230 | 0 | 194 | 9 | 50 | 0 | 253 | 0 | 0 | 7 | 1 | 0 | 8 | 708 |
| 7:15 AM | 0 | 3 | 191 | 40 | 0 | 234 | 0 | 15 | 193 | 0 | 0 | 208 | 0 | 218 | 17 | 72 | 0 | 307 | 0 | 3 | 5 | 2 | 0 | 10 | 759 |
| 7:30 AM | 0 | 7 | 194 | 55 | 0 | 256 | 0 | 26 | 232 | 2 | 0 | 260 | 0 | 213 | 26 | 85 | 0 | 324 | 0 | 3 | 14 | 4 | 0 | 21 | 861 |
| 7:45 AM | 0 | 9 | 204 | 43 | 0 | 256 | 0 | 43 | 235 | 9 | 0 | 287 | 0 | 212 | 29 | 97 | 0 | 338 | 0 | 9 | 2 | 2 | 0 | 13 | 894 |
| Hourly Total | 0 | 20 | 768 | 175 | 0 | 963 | 0 | 100 | 872 | 13 | 0 | 985 | 0 | 837 | 81 | 304 | 0 | 1222 | 0 | 15 | 28 | 9 | 0 | 52 | 3222 |
| 8:00 AM | 0 | 4 | 175 | 48 | 0 | 227 | 0 | 22 | 200 | 6 | 0 | 228 | 0 | 169 | 9 | 74 | 0 | 252 | 0 | 6 | 16 | 3 | 1 | 25 | 732 |
| 8:15 AM | 0 | 9 | 149 | 51 | 0 | 209 | 0 | 40 | 212 | 1 | 0 | 253 | 0 | 141 | 14 | 53 | 0 | 208 | 0 | 5 | 10 | 4 | 1 | 19 | 689 |
| 8:30 AM | 0 | 2 | 167 | 41 | 0 | 210 | 0 | 44 | 209 | 3 | 1 | 256 | 0 | 111 | 14 | 46 | 1 | 171 | 0 | 2 | 10 | 4 | 0 | 16 | 653 |
| 8:45 AM | 0 | 3 | 156 | 54 | 0 | 213 | 0 | 24 | 180 | 0 | 0 | 204 | 0 | 112 | 4 | 35 | 0 | 151 | 0 | 1 | 11 | 2 | 0 | 14 | 582 |
| Hourly Total | 0 | 18 | 647 | 194 | 0 | 859 | 0 | 130 | 801 | 10 | 1 | 941 | 0 | 533 | 41 | 208 | 1 | 782 | 0 | 14 | 47 | 13 | 2 | 74 | 2656 |
| *** BREAK *** | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - |
| 2:30 PM | 0 | 5 | 149 | 104 | 0 | 258 | 0 | 51 | 167 | 2 | 0 | 220 | 0 | 71 | 9 | 50 | 0 | 130 | 0 | 1 | 11 | 2 | 0 | 14 | 622 |
| 2:45 PM | 0 | 4 | 164 | 103 | 0 | 271 | 0 | 52 | 138 | 2 | 0 | 192 | 0 | 79 | 21 | 64 | 0 | 164 | 0 | 2 | 11 | 4 | 0 | 17 | 644 |
| Hourly Total | 0 | 9 | 313 | 207 | 0 | 529 | 0 | 103 | 305 | 4 | 0 | 412 | 0 | 150 | 30 | 114 | 0 | 294 | 0 | 3 | 22 | 6 | 0 | 31 | 1266 |
| 3:00 PM | 0 | 5 | 145 | 112 | 0 | 262 | 0 | 65 | 139 | 7 | 0 | 211 | 0 | 68 | 11 | 40 | 0 | 119 | 0 | 4 | 20 | 4 | 2 | 28 | 620 |
| 3:15 PM | 0 | 3 | 198 | 109 | 0 | 310 | 0 | 47 | 146 | 5 | 0 | 198 | 0 | 77 | 10 | 43 | 0 | 130 | 0 | 5 | 19 | 0 | 0 | 24 | 662 |
| 3:30 PM | 0 | 4 | 185 | 129 | 0 | 318 | 0 | 67 | 194 | 4 | 1 | 265 | 0 | 87 | 8 | 68 | 0 | 163 | 0 | 2 | 20 | 1 | 1 | 23 | 769 |
| 3:45 PM | 0 | 3 | 213 | 144 | 0 | 360 | 0 | 69 | 226 | 1 | 1 | 296 | 0 | 91 | 7 | 49 | 0 | 147 | 0 | 2 | 30 | 4 | 0 | 36 | 839 |
| Hourly Total | 0 | 15 | 741 | 494 | 0 | 1250 | 0 | 248 | 705 | 17 | 2 | 970 | 0 | 323 | 36 | 200 | 0 | 559 | 0 | 13 | 89 | 9 | 3 | 111 | 2890 |
| 4:00 PM | 0 | 4 | 223 | 157 | 1 | 384 | 0 | 49 | 239 | 0 | 0 | 288 | 0 | 92 | 5 | 45 | 0 | 142 | 0 | 3 | 23 | 3 | 2 | 29 | 843 |
| 4:15 PM | 0 | 3 | 234 | 157 | 0 | 394 | 0 | 69 | 157 | 1 | 0 | 227 | 0 | 102 | 6 | 43 | 0 | 151 | 0 | 0 | 26 | 3 | 0 | 29 | 801 |
| 4:30 PM | 0 | 2 | 242 | 166 | 0 | 410 | 0 | 75 | 218 | 1 | 0 | 294 | 0 | 78 | 11 | 47 | 1 | 136 | 0 | 4 | 36 | 3 | 0 | 43 | 883 |
| 4:45 PM | 0 | 2 | 220 | 146 | 0 | 368 | 0 | 63 | 211 | 1 | 0 | 275 | 0 | 90 | 10 | 43 | 0 | 143 | 0 | 0 | 16 | 6 | 0 | 22 | 808 |
| Hourly Total | 0 | 11 | 919 | 626 | 1 | 1556 | 0 | 256 | 825 | 3 | 0 | 1084 | 0 | 362 | 32 | 178 | 1 | 572 | 0 | 7 | 101 | 15 | 2 | 123 | 3335 |
| 5:00 PM | 0 | 8 | 228 | 158 | 0 | 394 | 0 | 66 | 226 | 2 | 0 | 294 | 0 | 69 | 12 | 49 | 0 | 130 | 0 | 9 | 29 | 3 | 0 | 41 | 859 |
| 5:15 PM | 0 | 6 | 238 | 178 | 0 | 422 | 0 | 95 | 197 | 1 | 0 | 293 | 0 | 96 | 13 | 50 | 0 | 159 | 0 | 5 | 42 | 0 | 0 | 47 | 921 |

| | | | | | | | | | | | | | | | | | | | | | | | | | |
|----------------------|-----|------|------|------|-------|------|-----|------|------|-------|---|------|-----|------|------|------|-------|------|-----|------|------|------|-------|------|-------|
| 5:30 PM | 0 | 2 | 251 | 202 | 0 | 455 | 0 | 66 | 202 | 0 | 0 | 268 | 0 | 75 | 11 | 41 | 0 | 127 | 0 | 5 | 39 | 5 | 0 | 49 | 899 |
| 5:45 PM | 0 | 8 | 239 | 168 | 0 | 415 | 0 | 62 | 203 | 2 | 0 | 267 | 0 | 96 | 13 | 51 | 0 | 160 | 0 | 3 | 30 | 8 | 0 | 41 | 883 |
| Hourly Total | 0 | 24 | 956 | 706 | 0 | 1686 | 0 | 289 | 828 | 5 | 0 | 1122 | 0 | 336 | 49 | 191 | 0 | 576 | 0 | 22 | 140 | 16 | 0 | 178 | 3562 |
| Grand Total | 0 | 130 | 5770 | 3292 | 2 | 9192 | 0 | 1529 | 5847 | 68 | 3 | 7444 | 0 | 3334 | 348 | 1665 | 6 | 5347 | 0 | 99 | 548 | 101 | 8 | 748 | 22731 |
| Approach % | 0.0 | 1.4 | 62.8 | 35.8 | - | - | 0.0 | 20.5 | 78.5 | 0.9 | - | - | 0.0 | 62.4 | 6.5 | 31.1 | - | - | 0.0 | 13.2 | 73.3 | 13.5 | - | - | - |
| Total % | 0.0 | 0.6 | 25.4 | 14.5 | - | 40.4 | 0.0 | 6.7 | 25.7 | 0.3 | - | 32.7 | 0.0 | 14.7 | 1.5 | 7.3 | - | 23.5 | 0.0 | 0.4 | 2.4 | 0.4 | - | 3.3 | - |
| Lights | 0 | 129 | 5601 | 3255 | - | 8985 | 0 | 1494 | 5682 | 64 | - | 7240 | 0 | 3292 | 339 | 1625 | - | 5256 | 0 | 93 | 537 | 97 | - | 727 | 22208 |
| % Lights | - | 99.2 | 97.1 | 98.9 | - | 97.7 | - | 97.7 | 97.2 | 94.1 | - | 97.3 | - | 98.7 | 97.4 | 97.6 | - | 98.3 | - | 93.9 | 98.0 | 96.0 | - | 97.2 | 97.7 |
| Buses | 0 | 0 | 112 | 27 | - | 139 | 0 | 18 | 83 | 2 | - | 103 | 0 | 22 | 6 | 35 | - | 63 | 0 | 4 | 10 | 2 | - | 16 | 321 |
| % Buses | - | 0.0 | 1.9 | 0.8 | - | 1.5 | - | 1.2 | 1.4 | 2.9 | - | 1.4 | - | 0.7 | 1.7 | 2.1 | - | 1.2 | - | 4.0 | 1.8 | 2.0 | - | 2.1 | 1.4 |
| Single-Unit Trucks | 0 | 1 | 50 | 9 | - | 60 | 0 | 16 | 66 | 2 | - | 84 | 0 | 15 | 1 | 5 | - | 21 | 0 | 2 | 1 | 1 | - | 4 | 169 |
| % Single-Unit Trucks | - | 0.8 | 0.9 | 0.3 | - | 0.7 | - | 1.0 | 1.1 | 2.9 | - | 1.1 | - | 0.4 | 0.3 | 0.3 | - | 0.4 | - | 2.0 | 0.2 | 1.0 | - | 0.5 | 0.7 |
| Articulated Trucks | 0 | 0 | 7 | 1 | - | 8 | 0 | 1 | 16 | 0 | - | 17 | 0 | 5 | 0 | 0 | - | 5 | 0 | 0 | 0 | 1 | - | 1 | 31 |
| % Articulated Trucks | - | 0.0 | 0.1 | 0.0 | - | 0.1 | - | 0.1 | 0.3 | 0.0 | - | 0.2 | - | 0.1 | 0.0 | 0.0 | - | 0.1 | - | 0.0 | 0.0 | 1.0 | - | 0.1 | 0.1 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 2 | 0 | - | 2 | 0 | 0 | 0 | 0 | - | 0 | 2 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.6 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Pedestrians | - | - | - | - | 2 | - | - | - | - | 3 | - | - | - | - | - | - | 6 | - | - | - | - | - | 8 | - | - |
| % Pedestrians | - | - | - | - | 100.0 | - | - | - | - | 100.0 | - | - | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | - | - |



Kenig Lindgren O'Hara Aboona, Inc.
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Rosemont, Illinois, United States 60018
(847)518-9990

Count Name: 63rd Street with Woodward Avenue
Site Code:
Start Date: 01/21/2017
Page No: 4

Turning Movement Peak Hour Data (12:45 PM)

| Start Time | 63rd Street Eastbound | | | | | | 63rd Street Westbound | | | | | | Woodward Avenue Northbound | | | | | | Woodward Avenue Southbound | | | | | | Int. Total |
|----------------------|-----------------------|-----------|------------|------------|----------|-------------|-----------------------|------------|------------|----------|----------|-------------|----------------------------|------------|-----------|------------|----------|------------|----------------------------|-----------|-----------|-----------|----------|------------|-------------|
| | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | |
| 12:45 PM | 0 | 3 | 159 | 94 | 0 | 256 | 0 | 55 | 205 | 1 | 0 | 261 | 0 | 116 | 12 | 60 | 0 | 188 | 0 | 5 | 20 | 2 | 0 | 27 | 732 |
| 1:00 PM | 0 | 8 | 196 | 117 | 0 | 321 | 0 | 42 | 195 | 1 | 0 | 238 | 0 | 85 | 9 | 56 | 0 | 150 | 0 | 5 | 6 | 9 | 0 | 20 | 729 |
| 1:15 PM | 0 | 2 | 178 | 104 | 0 | 284 | 0 | 46 | 202 | 2 | 0 | 250 | 0 | 102 | 10 | 51 | 1 | 163 | 0 | 4 | 16 | 1 | 0 | 21 | 718 |
| 1:30 PM | 0 | 7 | 175 | 126 | 0 | 308 | 0 | 46 | 222 | 2 | 0 | 270 | 0 | 108 | 8 | 67 | 1 | 183 | 0 | 4 | 13 | 5 | 0 | 22 | 783 |
| Total | 0 | 20 | 708 | 441 | 0 | 1169 | 0 | 189 | 824 | 6 | 0 | 1019 | 0 | 411 | 39 | 234 | 2 | 684 | 0 | 18 | 55 | 17 | 0 | 90 | 2962 |
| Approach % | 0.0 | 1.7 | 60.6 | 37.7 | - | - | 0.0 | 18.5 | 80.9 | 0.6 | - | - | 0.0 | 60.1 | 5.7 | 34.2 | - | - | 0.0 | 20.0 | 61.1 | 18.9 | - | - | - |
| Total % | 0.0 | 0.7 | 23.9 | 14.9 | - | 39.5 | 0.0 | 6.4 | 27.8 | 0.2 | - | 34.4 | 0.0 | 13.9 | 1.3 | 7.9 | - | 23.1 | 0.0 | 0.6 | 1.9 | 0.6 | - | 3.0 | - |
| PHF | 0.000 | 0.625 | 0.903 | 0.875 | - | 0.910 | 0.000 | 0.859 | 0.928 | 0.750 | - | 0.944 | 0.000 | 0.886 | 0.813 | 0.873 | - | 0.910 | 0.000 | 0.900 | 0.688 | 0.472 | - | 0.833 | 0.946 |
| Lights | 0 | 19 | 701 | 439 | - | 1159 | 0 | 188 | 810 | 6 | - | 1004 | 0 | 411 | 38 | 234 | - | 683 | 0 | 17 | 55 | 17 | - | 89 | 2935 |
| % Lights | - | 95.0 | 99.0 | 99.5 | - | 99.1 | - | 99.5 | 98.3 | 100.0 | - | 98.5 | - | 100.0 | 97.4 | 100.0 | - | 99.9 | - | 94.4 | 100.0 | 100.0 | - | 98.9 | 99.1 |
| Buses | 0 | 0 | 4 | 0 | - | 4 | 0 | 1 | 8 | 0 | - | 9 | 0 | 0 | 0 | 0 | - | 0 | 0 | 1 | 0 | 0 | - | 1 | 14 |
| % Buses | - | 0.0 | 0.6 | 0.0 | - | 0.3 | - | 0.5 | 1.0 | 0.0 | - | 0.9 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 5.6 | 0.0 | 0.0 | - | 1.1 | 0.5 |
| Single-Unit Trucks | 0 | 1 | 3 | 2 | - | 6 | 0 | 0 | 5 | 0 | - | 5 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 11 |
| % Single-Unit Trucks | - | 5.0 | 0.4 | 0.5 | - | 0.5 | - | 0.0 | 0.6 | 0.0 | - | 0.5 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.4 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 1 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.1 | 0.0 | - | 0.1 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 1 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 2.6 | 0.0 | - | 0.1 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Pedestrians | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 2 | - | - | - | - | - | 0 | - | - |
| % Pedestrians | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | 100.0 | - | - | - | - | - | - | - | - |



Kenig Lindgren O'Hara Aboona, Inc.
9575 W. Higgins Rd., Suite 400

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(847)518-9990

Count Name: 63rd Street with Woodward Avenue
Site Code:
Start Date: 01/21/2017
Page No: 6

Turning Movement Peak Hour Data (7:15 AM)

| Start Time | 63rd Street Eastbound | | | | | | 63rd Street Westbound | | | | | | Woodward Avenue Northbound | | | | | | Woodward Avenue Southbound | | | | | | Int. Total |
|----------------------|-----------------------|-------|-------|-------|------|------------|-----------------------|-------|-------|-------|------|------------|----------------------------|-------|-------|-------|------|------------|----------------------------|-------|-------|-------|-------|------------|------------|
| | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | |
| 7:15 AM | 0 | 3 | 191 | 40 | 0 | 234 | 0 | 15 | 193 | 0 | 0 | 208 | 0 | 218 | 17 | 72 | 0 | 307 | 0 | 3 | 5 | 2 | 0 | 10 | 759 |
| 7:30 AM | 0 | 7 | 194 | 55 | 0 | 256 | 0 | 26 | 232 | 2 | 0 | 260 | 0 | 213 | 26 | 85 | 0 | 324 | 0 | 3 | 14 | 4 | 0 | 21 | 861 |
| 7:45 AM | 0 | 9 | 204 | 43 | 0 | 256 | 0 | 43 | 235 | 9 | 0 | 287 | 0 | 212 | 29 | 97 | 0 | 338 | 0 | 9 | 2 | 2 | 0 | 13 | 894 |
| 8:00 AM | 0 | 4 | 175 | 48 | 0 | 227 | 0 | 22 | 200 | 6 | 0 | 228 | 0 | 169 | 9 | 74 | 0 | 252 | 0 | 6 | 16 | 3 | 1 | 25 | 732 |
| Total | 0 | 23 | 764 | 186 | 0 | 973 | 0 | 106 | 860 | 17 | 0 | 983 | 0 | 812 | 81 | 328 | 0 | 1221 | 0 | 21 | 37 | 11 | 1 | 69 | 3246 |
| Approach % | 0.0 | 2.4 | 78.5 | 19.1 | - | - | 0.0 | 10.8 | 87.5 | 1.7 | - | - | 0.0 | 66.5 | 6.6 | 26.9 | - | - | 0.0 | 30.4 | 53.6 | 15.9 | - | - | - |
| Total % | 0.0 | 0.7 | 23.5 | 5.7 | - | 30.0 | 0.0 | 3.3 | 26.5 | 0.5 | - | 30.3 | 0.0 | 25.0 | 2.5 | 10.1 | - | 37.6 | 0.0 | 0.6 | 1.1 | 0.3 | - | 2.1 | - |
| PHF | 0.000 | 0.639 | 0.936 | 0.845 | - | 0.950 | 0.000 | 0.616 | 0.915 | 0.472 | - | 0.856 | 0.000 | 0.931 | 0.698 | 0.845 | - | 0.903 | 0.000 | 0.583 | 0.578 | 0.688 | - | 0.690 | 0.908 |
| Lights | 0 | 23 | 734 | 176 | - | 933 | 0 | 104 | 826 | 16 | - | 946 | 0 | 798 | 77 | 315 | - | 1190 | 0 | 20 | 37 | 11 | - | 68 | 3137 |
| % Lights | - | 100.0 | 96.1 | 94.6 | - | 95.9 | - | 98.1 | 96.0 | 94.1 | - | 96.2 | - | 98.3 | 95.1 | 96.0 | - | 97.5 | - | 95.2 | 100.0 | 100.0 | - | 98.6 | 96.6 |
| Buses | 0 | 0 | 21 | 8 | - | 29 | 0 | 2 | 12 | 0 | - | 14 | 0 | 10 | 4 | 12 | - | 26 | 0 | 1 | 0 | 0 | - | 1 | 70 |
| % Buses | - | 0.0 | 2.7 | 4.3 | - | 3.0 | - | 1.9 | 1.4 | 0.0 | - | 1.4 | - | 1.2 | 4.9 | 3.7 | - | 2.1 | - | 4.8 | 0.0 | 0.0 | - | 1.4 | 2.2 |
| Single-Unit Trucks | 0 | 0 | 7 | 2 | - | 9 | 0 | 0 | 15 | 1 | - | 16 | 0 | 4 | 0 | 1 | - | 5 | 0 | 0 | 0 | 0 | - | 0 | 30 |
| % Single-Unit Trucks | - | 0.0 | 0.9 | 1.1 | - | 0.9 | - | 0.0 | 1.7 | 5.9 | - | 1.6 | - | 0.5 | 0.0 | 0.3 | - | 0.4 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.9 |
| Articulated Trucks | 0 | 0 | 2 | 0 | - | 2 | 0 | 0 | 7 | 0 | - | 7 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 9 |
| % Articulated Trucks | - | 0.0 | 0.3 | 0.0 | - | 0.2 | - | 0.0 | 0.8 | 0.0 | - | 0.7 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.3 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Pedestrians | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 1 | - | - |
| % Pedestrians | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | 100.0 | - | - |



Kenig Lindgren O'Hara Aboona, Inc.
9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018
(847)518-9990

Count Name: 63rd Street with Woodward Avenue
Site Code:
Start Date: 01/21/2017
Page No: 8

Turning Movement Peak Hour Data (5:00 PM)

| Start Time | 63rd Street Eastbound | | | | | | 63rd Street Westbound | | | | | | Woodward Avenue Northbound | | | | | | Woodward Avenue Southbound | | | | | | Int. Total |
|----------------------|-----------------------|-------|-------|-------|------|------------|-----------------------|-------|-------|-------|------|------------|----------------------------|-------|-------|-------|------|------------|----------------------------|-------|-------|-------|------|------------|------------|
| | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | |
| 5:00 PM | 0 | 8 | 228 | 158 | 0 | 394 | 0 | 66 | 226 | 2 | 0 | 294 | 0 | 69 | 12 | 49 | 0 | 130 | 0 | 9 | 29 | 3 | 0 | 41 | 859 |
| 5:15 PM | 0 | 6 | 238 | 178 | 0 | 422 | 0 | 95 | 197 | 1 | 0 | 293 | 0 | 96 | 13 | 50 | 0 | 159 | 0 | 5 | 42 | 0 | 0 | 47 | 921 |
| 5:30 PM | 0 | 2 | 251 | 202 | 0 | 455 | 0 | 66 | 202 | 0 | 0 | 268 | 0 | 75 | 11 | 41 | 0 | 127 | 0 | 5 | 39 | 5 | 0 | 49 | 899 |
| 5:45 PM | 0 | 8 | 239 | 168 | 0 | 415 | 0 | 62 | 203 | 2 | 0 | 267 | 0 | 96 | 13 | 51 | 0 | 160 | 0 | 3 | 30 | 8 | 0 | 41 | 883 |
| Total | 0 | 24 | 956 | 706 | 0 | 1686 | 0 | 289 | 828 | 5 | 0 | 1122 | 0 | 336 | 49 | 191 | 0 | 576 | 0 | 22 | 140 | 16 | 0 | 178 | 3562 |
| Approach % | 0.0 | 1.4 | 56.7 | 41.9 | - | - | 0.0 | 25.8 | 73.8 | 0.4 | - | - | 0.0 | 58.3 | 8.5 | 33.2 | - | - | 0.0 | 12.4 | 78.7 | 9.0 | - | - | - |
| Total % | 0.0 | 0.7 | 26.8 | 19.8 | - | 47.3 | 0.0 | 8.1 | 23.2 | 0.1 | - | 31.5 | 0.0 | 9.4 | 1.4 | 5.4 | - | 16.2 | 0.0 | 0.6 | 3.9 | 0.4 | - | 5.0 | - |
| PHF | 0.000 | 0.750 | 0.952 | 0.874 | - | 0.926 | 0.000 | 0.761 | 0.916 | 0.625 | - | 0.954 | 0.000 | 0.875 | 0.942 | 0.936 | - | 0.900 | 0.000 | 0.611 | 0.833 | 0.500 | - | 0.908 | 0.967 |
| Lights | 0 | 24 | 948 | 706 | - | 1678 | 0 | 287 | 822 | 5 | - | 1114 | 0 | 334 | 48 | 188 | - | 570 | 0 | 21 | 140 | 16 | - | 177 | 3539 |
| % Lights | - | 100.0 | 99.2 | 100.0 | - | 99.5 | - | 99.3 | 99.3 | 100.0 | - | 99.3 | - | 99.4 | 98.0 | 98.4 | - | 99.0 | - | 95.5 | 100.0 | 100.0 | - | 99.4 | 99.4 |
| Buses | 0 | 0 | 3 | 0 | - | 3 | 0 | 1 | 1 | 0 | - | 2 | 0 | 2 | 0 | 3 | - | 5 | 0 | 0 | 0 | 0 | - | 0 | 10 |
| % Buses | - | 0.0 | 0.3 | 0.0 | - | 0.2 | - | 0.3 | 0.1 | 0.0 | - | 0.2 | - | 0.6 | 0.0 | 1.6 | - | 0.9 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.3 |
| Single-Unit Trucks | 0 | 0 | 3 | 0 | - | 3 | 0 | 1 | 4 | 0 | - | 5 | 0 | 0 | 1 | 0 | - | 1 | 0 | 1 | 0 | 0 | - | 1 | 10 |
| % Single-Unit Trucks | - | 0.0 | 0.3 | 0.0 | - | 0.2 | - | 0.3 | 0.5 | 0.0 | - | 0.4 | - | 0.0 | 2.0 | 0.0 | - | 0.2 | - | 4.5 | 0.0 | 0.0 | - | 0.6 | 0.3 |
| Articulated Trucks | 0 | 0 | 2 | 0 | - | 2 | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 3 |
| % Articulated Trucks | - | 0.0 | 0.2 | 0.0 | - | 0.1 | - | 0.0 | 0.1 | 0.0 | - | 0.1 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.1 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Pedestrians | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - |
| % Pedestrians | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - |

Level of Service Criteria

- LEVEL OF SERVICE CRITERIA

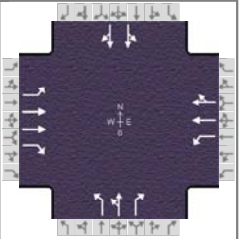
| Signalized Intersections | | |
|-----------------------------------|--|---|
| Level of Service | Interpretation | Average Control Delay (seconds per vehicle) |
| A | Favorable progression. Most vehicles arrive during the green indication and travel through the intersection without stopping. | ≤10 |
| B | Good progression, with more vehicles stopping than for Level of Service A. | >10 - 20 |
| C | Individual cycle failures (i.e., one or more queued vehicles are not able to depart as a result of insufficient capacity during the cycle) may begin to appear. Number of vehicles stopping is significant, although many vehicles still pass through the intersection without stopping. | >20 - 35 |
| D | The volume-to-capacity ratio is high and either progression is ineffective or the cycle length is too long. Many vehicles stop and individual cycle failures are noticeable. | >35 - 55 |
| E | Progression is unfavorable. The volume-to-capacity ratio is high and the cycle length is long. Individual cycle failures are frequent. | >55 - 80 |
| F | The volume-to-capacity ratio is very high, progression is very poor and the cycle length is long. Most cycles fail to clear the queue. | >80.0 |
| Unsignalized Intersections | | |
| Level of Service | Average Total Delay (SEC/VEH) | |
| A | 0 - 10 | |
| B | > 10 - 15 | |
| C | > 15 - 25 | |
| D | > 25 - 35 | |
| E | > 35 - 50 | |
| F | > 50 | |

Source: *Highway Capacity Manual, 2010.*

Capacity Analysis Sheets

HCS 2010 Signalized Intersection Input Data

| General Information | | | | Intersection Information | | | |
|---------------------|---------------------------|--|---------------|----------------------------|------|-----------------|----------|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | | Analysis Date | 1/24/2017 | | Area Type | Other |
| Jurisdiction | DuPage County | | Time Period | AM | | PHF | 0.91 |
| Urban Street | 63rd Street | | Analysis Year | 2017 | | Analysis Period | 1 > 7:00 |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward AMEX.xus | | | |
| Project Description | Existing AM Peak Hour | | | | | | |



| Demand Information | EB | | | WB | | | NB | | | SB | | |
|--------------------|----|-----|-----|-----|-----|----|-----|----|-----|----|----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 23 | 764 | 186 | 106 | 860 | 17 | 812 | 81 | 328 | 21 | 37 | 11 |

| Signal Information | | | | Signal Timing (s) | | | | | | | | | |
|--------------------|-------|-----------------|-------|-------------------|-----|-----|------|------|-----|-----|--|--|--|
| Cycle, s | 130.0 | Reference Phase | 6 | | | | | | | | | | |
| Offset, s | 0 | Reference Point | Begin | Green | 3.1 | 3.9 | 53.6 | 40.0 | 7.5 | 0.0 | | | |
| Uncoordinated | No | Simult. Gap E/W | On | Yellow | 3.0 | 0.0 | 4.5 | 4.5 | 4.5 | 0.0 | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | Red | 1.0 | 0.0 | 1.5 | 1.5 | 1.5 | 0.0 | | | |

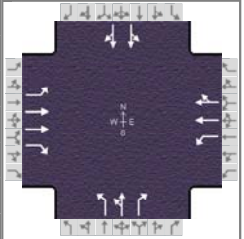
| Traffic Information | EB | | | WB | | | NB | | | SB | | |
|--|------|------|------|------|------|------|------|------|------|------|------|------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 23 | 764 | 186 | 106 | 860 | 17 | 812 | 81 | 328 | 21 | 37 | 11 |
| Initial Queue (Q _b), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Base Saturation Flow Rate (s ₀), veh/h | 1900 | 2000 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Parking (N _m), man/h | | None | | | None | | | None | | | None | |
| Heavy Vehicles (P _{HV}), % | 0 | 4 | 5 | 2 | 4 | | 2 | 5 | 4 | | 0 | |
| Ped / Bike / RTOR, /h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Buses (N _b), buses/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Arrival Type (AT) | 3 | 4 | 3 | 3 | 4 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| Upstream Filtering (I) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Lane Width (W), ft | 12.0 | 12.0 | 12.0 | 12.0 | 12.0 | | 12.0 | 12.0 | 12.0 | | 12.0 | |
| Turn Bay Length, ft | 125 | 0 | 350 | 340 | 0 | | 305 | 0 | 180 | | 0 | |
| Grade (P _g), % | | 0 | | | 0 | | | 0 | | | 0 | |
| Speed Limit, mi/h | 40 | 40 | 40 | 40 | 40 | 40 | 30 | 30 | 30 | 25 | 25 | 25 |

| Phase Information | EBL | EBT | WBL | WBT | NBL | NBT | SBL | SBT |
|---|------|------|------|------|------|------|-----|------|
| Maximum Green (G _{max}) or Phase Split, s | 13.0 | 52.0 | 16.0 | 55.0 | 48.0 | 48.0 | | 14.0 |
| Yellow Change Interval (Y), s | 3.0 | 4.5 | 3.0 | 4.5 | 4.5 | 4.5 | | 4.5 |
| Red Clearance Interval (R _c), s | 1.0 | 1.5 | 1.0 | 1.5 | 1.5 | 1.5 | | 1.5 |
| Minimum Green (G _{min}), s | 3 | 15 | 3 | 15 | 3 | 8 | 3 | 8 |
| Start-Up Lost Time (lt), s | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Extension of Effective Green (e), s | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Passage (PT), s | 3.0 | 7.0 | 3.0 | 7.0 | 3.0 | 4.0 | 3.0 | 4.0 |
| Recall Mode | Off | Min | Off | Min | Off | Off | Off | Off |
| Dual Entry | Yes | Yes | Yes | Yes | No | Yes | No | Yes |
| Walk (Walk), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Pedestrian Clearance Time (PC), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Multimodal Information | EB | | | WB | | | NB | | | SB | | |
|---|-----|------|-----|-----|------|-----|-----|------|-----|-----|------|-----|
| 85th % Speed / Rest in Walk / Corner Radius | 0 | No | 25 | 0 | No | 25 | 0 | No | 25 | 0 | No | 25 |
| Walkway / Crosswalk Width / Length, ft | 9.0 | 12 | 0 | 9.0 | 12 | 0 | 9.0 | 12 | 0 | 9.0 | 12 | 0 |
| Street Width / Island / Curb | 0 | 0 | No | 0 | 0 | No | 0 | 0 | No | 0 | 0 | No |
| Width Outside / Bike Lane / Shoulder, ft | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 |
| Pedestrian Signal / Occupied Parking | No | 0.50 | | No | 0.50 | | No | 0.50 | | No | 0.50 | |

HCS 2010 Signalized Intersection Results Summary

| General Information | | | | Intersection Information | | | |
|---------------------|---------------------------|---------------|----------------------------|--------------------------|----------|--|--|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | Analysis Date | 1/24/2017 | Area Type | Other | | |
| Jurisdiction | DuPage County | Time Period | AM | PHF | 0.91 | | |
| Urban Street | 63rd Street | Analysis Year | 2017 | Analysis Period | 1 > 7:00 | | |
| Intersection | 63rd Street with Woodw... | File Name | 63rd and Woodward AMEX.xus | | | | |
| Project Description | Existing AM Peak Hour | | | | | | |



| Demand Information | EB | | | WB | | | NB | | | SB | | |
|---------------------|----|-----|-----|-----|-----|----|-----|----|-----|----|----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 23 | 764 | 186 | 106 | 860 | 17 | 812 | 81 | 328 | 21 | 37 | 11 |

| Signal Information | | | | Signal Timing (s) | | | | | | | | | | | |
|--------------------|-------|-----------------|-------|-------------------|-----|------|------|-----|-----|--|--|--|--|--|--|
| Cycle, s | 130.0 | Reference Phase | 6 | | | | | | | | | | | | |
| Offset, s | 0 | Reference Point | Begin | | | | | | | | | | | | |
| Uncoordinated | No | Simult. Gap E/W | On | | | | | | | | | | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | | | | | | | | | | | | |
| | | Green | | 3.1 | 3.9 | 53.6 | 40.0 | 7.5 | 0.0 | | | | | | |
| | | Yellow | | 3.0 | 0.0 | 4.5 | 4.5 | 4.5 | 0.0 | | | | | | |
| | | Red | | 1.0 | 0.0 | 1.5 | 1.5 | 1.5 | 0.0 | | | | | | |

| Timer Results | EBL | EBT | WBL | WBT | NBL | NBT | SBL | SBT |
|--|------|------|------|------|-----|------|-----|------|
| Assigned Phase | 5 | 2 | 1 | 6 | | 8 | | 4 |
| Case Number | 1.1 | 3.0 | 1.1 | 4.0 | | 9.0 | | 12.0 |
| Phase Duration, s | 7.1 | 59.6 | 10.9 | 63.5 | | 46.0 | | 13.5 |
| Change Period, (Y+R _c), s | 4.0 | 6.0 | 4.0 | 6.0 | | 6.0 | | 6.0 |
| Max Allow Headway (MAH), s | 4.0 | 0.0 | 4.0 | 0.0 | | 5.1 | | 5.2 |
| Queue Clearance Time (g _s), s | 3.0 | | 6.8 | | | 36.4 | | 4.7 |
| Green Extension Time (g _e), s | 0.0 | 0.0 | 0.2 | 0.0 | | 3.6 | | 0.1 |
| Phase Call Probability | 1.00 | | 1.00 | | | 1.00 | | 0.94 |
| Max Out Probability | 0.00 | | 0.03 | | | 0.97 | | 0.90 |

| Movement Group Results | EB | | | WB | | | NB | | | SB | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|---|-------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Assigned Movement | 5 | 2 | 12 | 1 | 6 | 16 | 3 | 8 | 18 | 7 | 4 | 14 |
| Adjusted Flow Rate (v), veh/h | 25 | 840 | 204 | 116 | 484 | 480 | 491 | 491 | 360 | 40 | | 36 |
| Adjusted Saturation Flow Rate (s), veh/h/ln | 1810 | 1831 | 1533 | 1774 | 1827 | 1814 | 1774 | 1780 | 1548 | 1846 | | 1792 |
| Queue Service Time (g _s), s | 1.0 | 19.3 | 5.6 | 4.8 | 21.8 | 22.0 | 34.4 | 34.2 | 25.2 | 2.7 | | 2.5 |
| Cycle Queue Clearance Time (g _c), s | 1.0 | 19.3 | 5.6 | 4.8 | 21.8 | 22.0 | 34.4 | 34.2 | 25.2 | 2.7 | | 2.5 |
| Green Ratio (g/C) | 0.44 | 0.41 | 0.72 | 0.48 | 0.44 | 0.44 | 0.31 | 0.31 | 0.36 | 0.06 | | 0.06 |
| Capacity (c), veh/h | 250 | 1509 | 1104 | 322 | 807 | 802 | 546 | 548 | 559 | 106 | | 103 |
| Volume-to-Capacity Ratio (X) | 0.101 | 0.556 | 0.185 | 0.362 | 0.599 | 0.599 | 0.899 | 0.895 | 0.645 | 0.374 | | 0.349 |
| Back of Queue (Q), ft/ln (95 th percentile) | 20.2 | 307.6 | 80 | 90.7 | 342.4 | 334.2 | 621.3 | 632.7 | 388.6 | 64 | | 55.6 |
| Back of Queue (Q), veh/ln (95 th percentile) | 0.8 | 11.9 | 3.1 | 3.6 | 13.3 | 13.4 | 24.5 | 24.3 | 15.1 | 2.5 | | 2.2 |
| Queue Storage Ratio (RQ) (95 th percentile) | 0.16 | 0.00 | 0.23 | 0.27 | 0.00 | 0.00 | 2.04 | 0.00 | 2.16 | 0.00 | | 0.00 |
| Uniform Delay (d ₁), s/veh | 22.6 | 21.6 | 5.9 | 20.9 | 19.4 | 19.7 | 43.1 | 43.0 | 34.6 | 59.0 | | 58.9 |
| Incremental Delay (d ₂), s/veh | 0.2 | 1.5 | 0.4 | 0.7 | 3.3 | 3.3 | 17.1 | 16.5 | 2.7 | 3.1 | | 2.9 |
| Initial Queue Delay (d ₃), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 |
| Control Delay (d), s/veh | 22.8 | 23.0 | 6.3 | 21.6 | 22.7 | 23.0 | 60.1 | 59.5 | 37.3 | 62.1 | | 61.8 |
| Level of Service (LOS) | C | C | A | C | C | C | E | E | D | E | | E |
| Approach Delay, s/veh / LOS | 19.8 | | B | 22.7 | | C | 53.8 | | D | 61.9 | | E |
| Intersection Delay, s/veh / LOS | 34.4 | | | | | | C | | | | | |

| Multimodal Results | EB | | WB | | NB | | SB | |
|----------------------------|-----|---|-----|---|-----|---|-----|---|
| Pedestrian LOS Score / LOS | 2.8 | C | 2.3 | B | 2.9 | C | 3.0 | C |
| Bicycle LOS Score / LOS | 1.4 | A | 1.4 | A | 2.7 | B | 0.6 | A |

HCS 2010 Signalized Intersection Intermediate Values

| General Information | | | | Intersection Information | | | |
|---------------------|---------------------------|---------------|---------------|----------------------------|-----------------|----------|--|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | Analysis Date | 1/24/2017 | Area Type | Other | | |
| Jurisdiction | DuPage County | | Time Period | AM | PHF | 0.91 | |
| Urban Street | 63rd Street | | Analysis Year | 2017 | Analysis Period | 1 > 7:00 | |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward AMEX.xus | | | |
| Project Description | Existing AM Peak Hour | | | | | | |

| Demand Information | EB | | | WB | | | NB | | | SB | | |
|---------------------|----|-----|-----|-----|-----|----|-----|----|-----|----|----|----|
| Approach Movement | L | T | R | L | T | R | L | T | R | L | T | R |
| Demand (v), veh/h | 23 | 764 | 186 | 106 | 860 | 17 | 812 | 81 | 328 | 21 | 37 | 11 |

| Signal Information | | | | | | | | | | | | | | | | | | | | | | | | |
|--------------------|-------|-----------------|-------|-------|-----|-----|------|------|-----|-----|--------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| Cycle, s | 130.0 | Reference Phase | 6 | Green | 3.1 | 3.9 | 53.6 | 40.0 | 7.5 | 0.0 | Yellow | 3.0 | 0.0 | 4.5 | 4.5 | 4.5 | 0.0 | Red | 1.0 | 0.0 | 1.5 | 1.5 | 1.5 | 0.0 |
| Offset, s | 0 | Reference Point | Begin | | | | | | | | | | | | | | | | | | | | | |
| Uncoordinated | No | Simult. Gap E/W | On | | | | | | | | | | | | | | | | | | | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | | | | | | | | | | | | | | | | | | | | | |

| Saturation Flow / Delay | EB | | | WB | | | NB | | | SB | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Lane Width Adjustment Factor (f_w) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Heavy Vehicle Adjustment Factor (f_{HV}) | 1.000 | 0.962 | 0.952 | 0.980 | 0.962 | 1.000 | 0.980 | 0.952 | 0.962 | 0.952 | 1.000 | 1.000 |
| Approach Grade Adjustment Factor (f_g) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Parking Activity Adjustment Factor (f_p) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Bus Blockage Adjustment Factor (f_{bb}) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Area Type Adjustment Factor (f_a) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Lane Utilization Adjustment Factor (f_{LU}) | 1.000 | 0.952 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Left-Turn Adjustment Factor (f_{LT}) | 0.952 | 0.000 | | 0.952 | 0.000 | | 0.952 | 0.000 | | 0.000 | 0.972 | |
| Right-Turn Adjustment Factor (f_{RT}) | | 0.000 | 0.847 | | 0.993 | 0.993 | | 0.000 | 0.847 | | 0.937 | 0.943 |
| Left-Turn Pedestrian Adjustment Factor (f_{LPB}) | 1.000 | | | 1.000 | | | 1.000 | | | 1.000 | | |
| Right-Turn Ped-Bike Adjustment Factor (f_{RPB}) | | | 1.000 | | | 1.000 | | | 1.000 | | | 1.000 |
| Movement Saturation Flow Rate (s), veh/h | 1810 | 3662 | 1533 | 1774 | 3571 | 71 | 1774 | 1780 | 1548 | 1071 | 1966 | 601 |
| Proportion of Vehicles Arriving on Green (P) | 0.02 | 0.55 | 0.41 | 0.05 | 0.59 | 0.44 | 0.31 | 0.31 | 0.31 | 0.06 | 0.06 | 0.06 |
| Incremental Delay Factor (k) | 0.11 | 0.50 | 0.50 | 0.11 | 0.50 | 0.50 | 0.40 | 0.40 | 0.24 | 0.15 | | 0.15 |

| Signal Timing / Movement Groups | EBL | EBT/R | WBL | WBT/R | NBL | NBT/R | SBL | SBT/R |
|---|------|-------|------|-------|-----|-------|-----|-------|
| Lost Time (t_L) | 4.0 | 6.0 | 4.0 | 6.0 | | 6.0 | | 6.0 |
| Green Ratio (g/C) | 0.44 | 0.41 | 0.48 | 0.44 | | 0.31 | | 0.06 |
| Permitted Saturation Flow Rate (s_p), veh/h/ln | 592 | 0 | 652 | 0 | | 1774 | | 0 |
| Shared Saturation Flow Rate (s_{sh}), veh/h/ln | | | | | | | | |
| Permitted Effective Green Time (g_p), s | 53.6 | 0.0 | 55.5 | 0.0 | | 0.0 | | 0.0 |
| Permitted Service Time (g_u), s | 33.4 | 0.0 | 34.2 | 0.0 | | 0.0 | | 0.0 |
| Permitted Queue Service Time (g_{ps}), s | 0.9 | | 4.6 | | | | | |
| Time to First Blockage (g_t), s | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | | 0.0 |
| Queue Service Time Before Blockage (g_{ts}), s | | | | | | | | |
| Protected Right Saturation Flow (s_R), veh/h/ln | | 1533 | | | | 1548 | | |
| Protected Right Effective Green Time (g_R), s | | 40.0 | | | | 6.9 | | |

| Multimodal | EB | | | WB | | | NB | | | SB | | |
|----------------------------------|--------|-------|--------|-------|-------|--------|-------|-------|--|----|--|--|
| Pedestrian F_w / F_v | 2.107 | 0.00 | 1.557 | 0.00 | 2.107 | 0.00 | 2.224 | 0.00 | | | | |
| Pedestrian F_s / F_{delay} | 0.000 | 0.125 | 0.000 | 0.121 | 0.000 | 0.172 | 0.000 | 0.163 | | | | |
| Pedestrian M_{corner} / M_{cw} | | | | | | | | | | | | |
| Bicycle c_b / d_b | 824.44 | 22.46 | 883.88 | 20.24 | 72.19 | 115.11 | 57.73 | | | | | |
| Bicycle F_w / F_v | -3.64 | 0.88 | -3.64 | 0.89 | -3.64 | 2.21 | -3.64 | 0.06 | | | | |

--- Messages ---

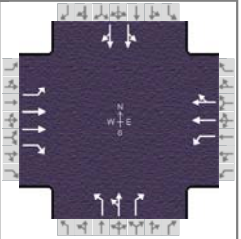
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

WARNING: The shared-plus-exclusive turn lane solution is an approximation of the HCM method, because more than three lane groups cannot be accommodated. Input data for Percent Turns in Shared Lane are used to specify proportion of turning vehicles in the shared lane.

--- Comments ---

HCS 2010 Signalized Intersection Input Data

| General Information | | | | | | Intersection Information | | | | |
|---------------------|---------------------------|--|---------------|----------------------------|--|--------------------------|----------|--|--|--|
| Agency | KLOA, Inc. | | | | | Duration, h | 0.25 | | | |
| Analyst | NJB | | Analysis Date | 1/24/2017 | | Area Type | Other | | | |
| Jurisdiction | DuPage County | | Time Period | PM | | PHF | 0.97 | | | |
| Urban Street | 63rd Street | | Analysis Year | 2017 | | Analysis Period | 1 > 7:00 | | | |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward PMEX.xus | | | | | | |
| Project Description | Existing PM Peak Hour | | | | | | | | | |



| Demand Information | EB | | | WB | | | NB | | | SB | | |
|--------------------|----|-----|-----|-----|-----|---|-----|----|-----|----|-----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 24 | 956 | 706 | 289 | 828 | 5 | 336 | 49 | 191 | 22 | 140 | 16 |

| Signal Information | | | | Signal Timing (s) | | | | | | | | | Signal Phases | | | | |
|--------------------|-------|-----------------|-------|-------------------|-----|-----|------|------|-----|-----|--|--|---------------|--|--|--|--|
| Cycle, s | 130.0 | Reference Phase | 2 | | | | | | | | | | | | | | |
| Offset, s | 0 | Reference Point | Begin | Green | 3.0 | 5.5 | 67.7 | 18.6 | 9.2 | 0.0 | | | | | | | |
| Uncoordinated | No | Simult. Gap E/W | On | Yellow | 3.0 | 3.0 | 4.5 | 4.5 | 4.5 | 0.0 | | | | | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | Red | 1.0 | 1.0 | 1.5 | 1.5 | 1.5 | 0.0 | | | | | | | |

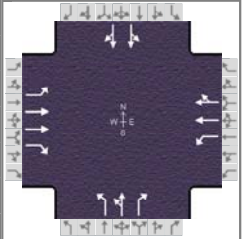
| Traffic Information | EB | | | WB | | | NB | | | SB | | |
|--|------|------|------|------|------|------|------|------|------|------|------|------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 24 | 956 | 706 | 289 | 828 | 5 | 336 | 49 | 191 | 22 | 140 | 16 |
| Initial Queue (Q _b), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Base Saturation Flow Rate (s ₀), veh/h | 1900 | 2000 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Parking (N _m), man/h | | None | | | None | | | None | | | None | |
| Heavy Vehicles (P _{HV}), % | 0 | 1 | 0 | 1 | 1 | | 1 | 2 | 2 | | 0 | |
| Ped / Bike / RTOR, /h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Buses (N _b), buses/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Arrival Type (AT) | 3 | 4 | 3 | 3 | 4 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| Upstream Filtering (I) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Lane Width (W), ft | 12.0 | 12.0 | 12.0 | 12.0 | 12.0 | | 12.0 | 12.0 | 12.0 | | 12.0 | |
| Turn Bay Length, ft | 125 | 0 | 350 | 340 | 0 | | 305 | 0 | 180 | | 0 | |
| Grade (P _g), % | | 0 | | | 0 | | | 0 | | | 0 | |
| Speed Limit, mi/h | 40 | 40 | 40 | 40 | 40 | 40 | 30 | 30 | 30 | 25 | 25 | 25 |

| Phase Information | EBL | EBT | WBL | WBT | NBL | NBT | SBL | SBT |
|---|------|------|------|------|------|------|-----|------|
| Maximum Green (G _{max}) or Phase Split, s | 13.0 | 48.0 | 30.0 | 65.0 | 31.0 | 31.0 | | 21.0 |
| Yellow Change Interval (Y), s | 3.0 | 4.5 | 3.0 | 4.5 | 4.5 | 4.5 | | 4.5 |
| Red Clearance Interval (R _c), s | 1.0 | 1.5 | 1.0 | 1.5 | 1.5 | 1.5 | | 1.5 |
| Minimum Green (G _{min}), s | 3 | 15 | 3 | 15 | 3 | 8 | 3 | 8 |
| Start-Up Lost Time (lt), s | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Extension of Effective Green (e), s | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Passage (PT), s | 3.0 | 7.0 | 3.0 | 7.0 | 3.0 | 4.0 | 3.0 | 4.0 |
| Recall Mode | Off | Min | Off | Min | Off | Off | Off | Off |
| Dual Entry | Yes | Yes | Yes | Yes | No | Yes | No | Yes |
| Walk (Walk), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Pedestrian Clearance Time (PC), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Multimodal Information | EB | | | WB | | | NB | | | SB | | |
|---|-----|------|-----|-----|------|-----|-----|------|-----|-----|------|-----|
| 85th % Speed / Rest in Walk / Corner Radius | 0 | No | 25 | 0 | No | 25 | 0 | No | 25 | 0 | No | 25 |
| Walkway / Crosswalk Width / Length, ft | 9.0 | 12 | 0 | 9.0 | 12 | 0 | 9.0 | 12 | 0 | 9.0 | 12 | 0 |
| Street Width / Island / Curb | 0 | 0 | No | 0 | 0 | No | 0 | 0 | No | 0 | 0 | No |
| Width Outside / Bike Lane / Shoulder, ft | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 |
| Pedestrian Signal / Occupied Parking | No | 0.50 | | No | 0.50 | | No | 0.50 | | No | 0.50 | |

HCS 2010 Signalized Intersection Results Summary

| General Information | | | | Intersection Information | | | |
|---------------------|---------------------------|--|---------------|----------------------------|------|-----------------|----------|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | | Analysis Date | 1/24/2017 | | Area Type | Other |
| Jurisdiction | DuPage County | | Time Period | PM | | PHF | 0.97 |
| Urban Street | 63rd Street | | Analysis Year | 2017 | | Analysis Period | 1 > 7:00 |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward PMEX.xus | | | |
| Project Description | Existing PM Peak Hour | | | | | | |



| Demand Information | EB | | | WB | | | NB | | | SB | | |
|---------------------|----|-----|-----|-----|-----|---|-----|----|-----|----|-----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 24 | 956 | 706 | 289 | 828 | 5 | 336 | 49 | 191 | 22 | 140 | 16 |

| Signal Information | | | | Signal Timing (s) | | | | | | | | | |
|--------------------|-------|-----------------|-------|-------------------|-----|-----|------|------|-----|-----|--|--|--|
| Cycle, s | 130.0 | Reference Phase | 2 | | | | | | | | | | |
| Offset, s | 0 | Reference Point | Begin | Green | 3.0 | 5.5 | 67.7 | 18.6 | 9.2 | 0.0 | | | |
| Uncoordinated | No | Simult. Gap E/W | On | Yellow | 3.0 | 3.0 | 4.5 | 4.5 | 4.5 | 0.0 | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | Red | 1.0 | 1.0 | 1.5 | 1.5 | 1.5 | 0.0 | | | |

| Timer Results | EBL | EBT | WBL | WBT | NBL | NBT | SBL | SBT |
|--|------|------|------|------|-----|------|-----|------|
| Assigned Phase | 5 | 2 | 1 | 6 | | 8 | | 4 |
| Case Number | 1.1 | 3.0 | 1.1 | 4.0 | | 9.0 | | 12.0 |
| Phase Duration, s | 7.0 | 73.7 | 16.5 | 83.2 | | 24.6 | | 15.2 |
| Change Period, (Y+R _c), s | 4.0 | 6.0 | 4.0 | 6.0 | | 6.0 | | 6.0 |
| Max Allow Headway (MAH), s | 4.0 | 0.0 | 4.0 | 0.0 | | 5.2 | | 5.1 |
| Queue Clearance Time (g _s), s | 2.8 | | 11.5 | | | 16.3 | | 8.5 |
| Green Extension Time (g _e), s | 0.0 | 0.0 | 1.0 | 0.0 | | 2.2 | | 0.7 |
| Phase Call Probability | 1.00 | | 1.00 | | | 1.00 | | 1.00 |
| Max Out Probability | 0.00 | | 0.00 | | | 0.45 | | 0.01 |

| Movement Group Results | EB | | | WB | | | NB | | | SB | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|---|-------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Assigned Movement | 5 | 2 | 12 | 1 | 6 | 16 | 3 | 8 | 18 | 7 | 4 | 14 |
| Adjusted Flow Rate (v), veh/h | 25 | 986 | 728 | 298 | 430 | 429 | 191 | 206 | 197 | 96 | | 87 |
| Adjusted Saturation Flow Rate (s), veh/h/ln | 1810 | 1885 | 1610 | 1792 | 1881 | 1877 | 1792 | 1809 | 1579 | 1878 | | 1837 |
| Queue Service Time (g _s), s | 0.8 | 16.0 | 36.1 | 9.5 | 8.9 | 9.0 | 13.3 | 14.3 | 14.1 | 6.5 | | 6.0 |
| Cycle Queue Clearance Time (g _c), s | 0.8 | 16.0 | 36.1 | 9.5 | 8.9 | 9.0 | 13.3 | 14.3 | 14.1 | 6.5 | | 6.0 |
| Green Ratio (g/C) | 0.54 | 0.52 | 0.66 | 0.63 | 0.59 | 0.59 | 0.14 | 0.14 | 0.24 | 0.07 | | 0.07 |
| Capacity (c), veh/h | 430 | 1962 | 1068 | 456 | 1117 | 1115 | 256 | 259 | 378 | 133 | | 130 |
| Volume-to-Capacity Ratio (X) | 0.058 | 0.502 | 0.681 | 0.653 | 0.385 | 0.385 | 0.744 | 0.798 | 0.521 | 0.724 | | 0.669 |
| Back of Queue (Q), ft/ln (95 th percentile) | 15.1 | 236.2 | 464.8 | 169.8 | 142.4 | 142.5 | 269.4 | 298.4 | 244.1 | 164.4 | | 140.3 |
| Back of Queue (Q), veh/ln (95 th percentile) | 0.6 | 9.4 | 18.6 | 6.7 | 5.7 | 5.7 | 10.7 | 11.7 | 9.6 | 6.3 | | 5.6 |
| Queue Storage Ratio (RQ) (95 th percentile) | 0.12 | 0.00 | 1.33 | 0.50 | 0.00 | 0.00 | 0.88 | 0.00 | 1.36 | 0.00 | | 0.00 |
| Uniform Delay (d ₁), s/veh | 13.7 | 12.0 | 13.4 | 13.1 | 6.4 | 6.5 | 53.4 | 53.9 | 43.0 | 59.1 | | 58.9 |
| Incremental Delay (d ₂), s/veh | 0.1 | 0.9 | 3.5 | 1.6 | 1.0 | 1.0 | 7.3 | 10.7 | 1.6 | 10.1 | | 8.1 |
| Initial Queue Delay (d ₃), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 |
| Control Delay (d), s/veh | 13.8 | 12.9 | 17.0 | 14.7 | 7.4 | 7.5 | 60.7 | 64.6 | 44.5 | 69.3 | | 67.0 |
| Level of Service (LOS) | B | B | B | B | A | A | E | E | D | E | | E |
| Approach Delay, s/veh / LOS | 14.6 | | B | 9.3 | | A | 56.7 | | E | 68.2 | | E |
| Intersection Delay, s/veh / LOS | 22.4 | | | | | | C | | | | | |

| Multimodal Results | EB | | WB | | NB | | SB | |
|----------------------------|-----|---|-----|---|-----|---|-----|---|
| Pedestrian LOS Score / LOS | 2.8 | C | 2.3 | B | 2.9 | C | 3.0 | C |
| Bicycle LOS Score / LOS | 1.9 | A | 1.4 | A | 1.5 | A | 0.6 | A |

HCS 2010 Signalized Intersection Intermediate Values

| General Information | | | | Intersection Information | | | |
|---------------------|---------------------------|---------------|---------------|----------------------------|-----------------|----------|--|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | Analysis Date | 1/24/2017 | Area Type | Other | | |
| Jurisdiction | DuPage County | | Time Period | PM | PHF | 0.97 | |
| Urban Street | 63rd Street | | Analysis Year | 2017 | Analysis Period | 1 > 7:00 | |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward PMEX.xus | | | |
| Project Description | Existing PM Peak Hour | | | | | | |

| Demand Information | EB | | | WB | | | NB | | | SB | | |
|---------------------|----|-----|-----|-----|-----|---|-----|----|-----|----|-----|----|
| Approach Movement | L | T | R | L | T | R | L | T | R | L | T | R |
| Demand (v), veh/h | 24 | 956 | 706 | 289 | 828 | 5 | 336 | 49 | 191 | 22 | 140 | 16 |

| Signal Information | | | | | | | | | | | | | | | | | | | | | | | | |
|--------------------|-------|-----------------|-------|---------------|-----|-----------------|------|------------|-------|-----------------|--------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| Cycle, s | 130.0 | Reference Phase | 2 | Green | 3.0 | 5.5 | 67.7 | 18.6 | 9.2 | 0.0 | Yellow | 3.0 | 3.0 | 4.5 | 4.5 | 4.5 | 0.0 | Red | 1.0 | 1.0 | 1.5 | 1.5 | 1.5 | 0.0 |
| Offset, s | 0 | Reference Point | Begin | Uncoordinated | No | Simult. Gap E/W | On | Force Mode | Fixed | Simult. Gap N/S | On | | | | | | | | | | | | | |

| Saturation Flow / Delay | EB | | | WB | | | NB | | | SB | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Lane Width Adjustment Factor (f_w) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Heavy Vehicle Adjustment Factor (f_{HV}) | 1.000 | 0.990 | 1.000 | 0.990 | 0.990 | 1.000 | 0.990 | 0.980 | 0.980 | 0.952 | 1.000 | 1.000 |
| Approach Grade Adjustment Factor (f_g) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Parking Activity Adjustment Factor (f_p) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Bus Blockage Adjustment Factor (f_{bb}) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Area Type Adjustment Factor (f_a) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Lane Utilization Adjustment Factor (f_{LU}) | 1.000 | 0.952 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Left-Turn Adjustment Factor (f_{LT}) | 0.952 | 0.000 | | 0.952 | 0.000 | | 0.952 | 0.000 | | 0.000 | 0.988 | |
| Right-Turn Adjustment Factor (f_{RT}) | | 0.000 | 0.847 | | 0.998 | 0.998 | | 0.000 | 0.847 | | 0.966 | 0.967 |
| Left-Turn Pedestrian Adjustment Factor (f_{LPB}) | 1.000 | | | 1.000 | | | 1.000 | | | 1.000 | | |
| Right-Turn Ped-Bike Adjustment Factor (f_{RPB}) | | | 1.000 | | | 1.000 | | | 1.000 | | | 1.000 |
| Movement Saturation Flow Rate (s), veh/h | 1810 | 3770 | 1610 | 1792 | 3736 | 23 | 1792 | 1809 | 1579 | 442 | 2926 | 348 |
| Proportion of Vehicles Arriving on Green (P) | 0.02 | 0.69 | 0.52 | 0.10 | 0.79 | 0.59 | 0.14 | 0.14 | 0.14 | 0.07 | 0.07 | 0.07 |
| Incremental Delay Factor (k) | 0.11 | 0.50 | 0.50 | 0.11 | 0.50 | 0.50 | 0.19 | 0.22 | 0.15 | 0.15 | | 0.15 |

| Signal Timing / Movement Groups | EBL | EBT/R | WBL | WBT/R | NBL | NBT/R | SBL | SBT/R |
|---|------|-------|------|-------|-----|-------|-----|-------|
| Lost Time (t_L) | 4.0 | 6.0 | 4.0 | 6.0 | | 6.0 | | 6.0 |
| Green Ratio (g/C) | 0.54 | 0.52 | 0.63 | 0.59 | | 0.14 | | 0.07 |
| Permitted Saturation Flow Rate (s_p), veh/h/ln | 654 | 0 | 574 | 0 | | 1792 | | 0 |
| Shared Saturation Flow Rate (s_{sh}), veh/h/ln | | | | | | | | |
| Permitted Effective Green Time (g_p), s | 67.7 | 0.0 | 69.7 | 0.0 | | 0.0 | | 0.0 |
| Permitted Service Time (g_u), s | 66.2 | 0.0 | 51.7 | 0.0 | | 0.0 | | 0.0 |
| Permitted Queue Service Time (g_{ps}), s | 0.1 | | 19.4 | | | | | |
| Time to First Blockage (g_t), s | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | | 0.0 |
| Queue Service Time Before Blockage (g_{ts}), s | | | | | | | | |
| Protected Right Saturation Flow (s_R), veh/h/ln | | | 1610 | | | 1579 | | |
| Protected Right Effective Green Time (g_R), s | | | 18.6 | | | 12.5 | | |

| Multimodal | EB | | | WB | | | NB | | | SB | | |
|----------------------------------|---------|-------|---------|-------|-------|-------|--------|-------|--|----|--|--|
| Pedestrian F_w / F_v | 2.107 | 0.00 | 1.557 | 0.00 | 2.107 | 0.00 | 2.224 | 0.00 | | | | |
| Pedestrian F_s / F_{delay} | 0.000 | 0.108 | 0.000 | 0.095 | 0.000 | 0.172 | 0.000 | 0.161 | | | | |
| Pedestrian M_{corner} / M_{cw} | | | | | | | | | | | | |
| Bicycle c_b / d_b | 1041.02 | 14.94 | 1187.69 | 10.72 | | 72.19 | 141.77 | 56.11 | | | | |
| Bicycle F_w / F_v | -3.64 | 1.43 | -3.64 | 0.95 | -3.64 | 0.98 | -3.64 | 0.15 | | | | |

--- Messages ---

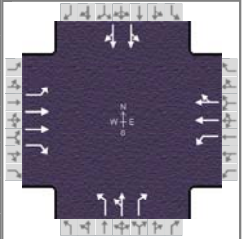
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

WARNING: The shared-plus-exclusive turn lane solution is an approximation of the HCM method, because more than three lane groups cannot be accommodated. Input data for Percent Turns in Shared Lane are used to specify proportion of turning vehicles in the shared lane.

--- Comments ---

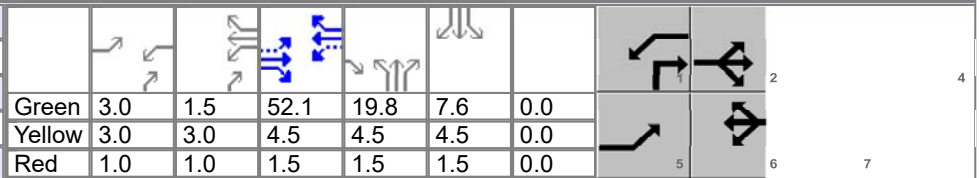
HCS 2010 Signalized Intersection Input Data

| General Information | | | | Intersection Information | | | |
|---------------------|-------------------------------|--|---------------|-----------------------------|------|-----------------|----------|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | | Analysis Date | 1/24/2017 | | Area Type | Other |
| Jurisdiction | DuPage County | | Time Period | SAT Midday | | PHF | 0.95 |
| Urban Street | 63rd Street | | Analysis Year | 2017 | | Analysis Period | 1 > 7:00 |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward SATEX.xus | | | |
| Project Description | Existing SAT Midday Peak Hour | | | | | | |



| Demand Information | EB | | | WB | | | NB | | | SB | | |
|--------------------|----|-----|-----|-----|-----|---|-----|----|-----|----|----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 20 | 708 | 441 | 189 | 824 | 6 | 411 | 39 | 234 | 18 | 55 | 17 |

| Signal Information | | | |
|--------------------|-------|-----------------|-------|
| Cycle, s | 110.0 | Reference Phase | 2 |
| Offset, s | 0 | Reference Point | Begin |
| Uncoordinated | No | Simult. Gap E/W | On |
| Force Mode | Fixed | Simult. Gap N/S | On |



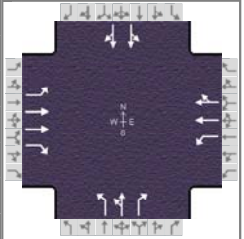
| Traffic Information | EB | | | WB | | | NB | | | SB | | |
|--|------|------|------|------|------|------|------|------|------|------|------|------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 20 | 708 | 441 | 189 | 824 | 6 | 411 | 39 | 234 | 18 | 55 | 17 |
| Initial Queue (Q _b), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Base Saturation Flow Rate (s ₀), veh/h | 1900 | 2000 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Parking (N _m), man/h | | None | | | None | | | None | | | None | |
| Heavy Vehicles (P _{HV}), % | 5 | 1 | 1 | 1 | 2 | | 0 | 3 | 0 | | 0 | |
| Ped / Bike / RTOR, /h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Buses (N _b), buses/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Arrival Type (AT) | 3 | 4 | 3 | 3 | 4 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| Upstream Filtering (I) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Lane Width (W), ft | 12.0 | 12.0 | 12.0 | 12.0 | 12.0 | | 12.0 | 12.0 | 12.0 | | 12.0 | |
| Turn Bay Length, ft | 125 | 0 | 350 | 340 | 0 | | 305 | 0 | 180 | | 0 | |
| Grade (P _g), % | | 0 | | | 0 | | | 0 | | | 0 | |
| Speed Limit, mi/h | 40 | 40 | 40 | 40 | 40 | 40 | 30 | 30 | 30 | 25 | 25 | 25 |

| Phase Information | EBL | EBT | WBL | WBT | NBL | NBT | SBL | SBT |
|---|------|------|------|------|------|------|-----|------|
| Maximum Green (G _{max}) or Phase Split, s | 13.0 | 39.0 | 19.0 | 45.0 | 33.0 | 33.0 | | 19.0 |
| Yellow Change Interval (Y), s | 3.0 | 4.5 | 3.0 | 4.5 | 4.5 | 4.5 | | 4.5 |
| Red Clearance Interval (R _c), s | 1.0 | 1.5 | 1.0 | 1.5 | 1.5 | 1.5 | | 1.5 |
| Minimum Green (G _{min}), s | 3 | 15 | 3 | 15 | 3 | 8 | 3 | 8 |
| Start-Up Lost Time (lt), s | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Extension of Effective Green (e), s | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Passage (PT), s | 3.0 | 7.0 | 3.0 | 7.0 | 3.0 | 4.0 | 3.0 | 4.0 |
| Recall Mode | Off | Min | Off | Min | Off | Off | Off | Off |
| Dual Entry | Yes | Yes | Yes | Yes | No | Yes | No | Yes |
| Walk (Walk), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Pedestrian Clearance Time (PC), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Multimodal Information | EB | | | WB | | | NB | | | SB | | |
|---|-----|------|-----|-----|------|-----|-----|------|-----|-----|------|-----|
| 85th % Speed / Rest in Walk / Corner Radius | 0 | No | 25 | 0 | No | 25 | 0 | No | 25 | 0 | No | 25 |
| Walkway / Crosswalk Width / Length, ft | 9.0 | 12 | 0 | 9.0 | 12 | 0 | 9.0 | 12 | 0 | 9.0 | 12 | 0 |
| Street Width / Island / Curb | 0 | 0 | No | 0 | 0 | No | 0 | 0 | No | 0 | 0 | No |
| Width Outside / Bike Lane / Shoulder, ft | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 |
| Pedestrian Signal / Occupied Parking | No | 0.50 | | No | 0.50 | | No | 0.50 | | No | 0.50 | |

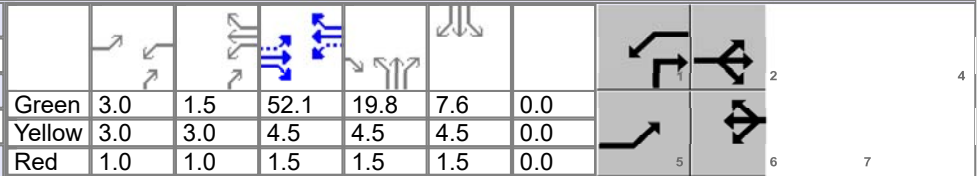
HCS 2010 Signalized Intersection Results Summary

| General Information | | | | Intersection Information | | | |
|---------------------|-------------------------------|--|---------------|-----------------------------|------|-----------------|----------|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | | Analysis Date | 1/24/2017 | | Area Type | Other |
| Jurisdiction | DuPage County | | Time Period | SAT Midday | | PHF | 0.95 |
| Urban Street | 63rd Street | | Analysis Year | 2017 | | Analysis Period | 1 > 7:00 |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward SATEX.xus | | | |
| Project Description | Existing SAT Midday Peak Hour | | | | | | |



| Demand Information | EB | | | WB | | | NB | | | SB | | |
|---------------------|----|-----|-----|-----|-----|---|-----|----|-----|----|----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 20 | 708 | 441 | 189 | 824 | 6 | 411 | 39 | 234 | 18 | 55 | 17 |

| Signal Information | | | |
|--------------------|-------|-----------------|-------|
| Cycle, s | 110.0 | Reference Phase | 2 |
| Offset, s | 0 | Reference Point | Begin |
| Uncoordinated | No | Simult. Gap E/W | On |
| Force Mode | Fixed | Simult. Gap N/S | On |



| Timer Results | EBL | EBT | WBL | WBT | NBL | NBT | SBL | SBT |
|--|------|------|------|------|-----|------|-----|------|
| Assigned Phase | 5 | 2 | 1 | 6 | | 8 | | 4 |
| Case Number | 1.1 | 3.0 | 1.1 | 4.0 | | 9.0 | | 12.0 |
| Phase Duration, s | 7.0 | 58.1 | 12.5 | 63.6 | | 25.8 | | 13.6 |
| Change Period, (Y+R _c), s | 4.0 | 6.0 | 4.0 | 6.0 | | 6.0 | | 6.0 |
| Max Allow Headway (MAH), s | 4.0 | 0.0 | 4.0 | 0.0 | | 5.2 | | 5.2 |
| Queue Clearance Time (g _s), s | 2.7 | | 7.9 | | | 16.8 | | 4.8 |
| Green Extension Time (g _e), s | 0.0 | 0.0 | 0.6 | 0.0 | | 3.1 | | 0.3 |
| Phase Call Probability | 1.00 | | 1.00 | | | 1.00 | | 0.94 |
| Max Out Probability | 0.00 | | 0.00 | | | 0.39 | | 0.00 |

| Movement Group Results | EB | | | WB | | | NB | | | SB | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|---|-------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Assigned Movement | 5 | 2 | 12 | 1 | 6 | 16 | 3 | 8 | 18 | 7 | 4 | 14 |
| Adjusted Flow Rate (v), veh/h | 21 | 745 | 464 | 199 | 437 | 436 | 238 | 236 | 246 | 50 | | 45 |
| Adjusted Saturation Flow Rate (s), veh/h/ln | 1723 | 1885 | 1594 | 1792 | 1863 | 1858 | 1810 | 1816 | 1610 | 1865 | | 1773 |
| Queue Service Time (g _s), s | 0.7 | 10.9 | 15.6 | 5.9 | 11.3 | 11.4 | 13.7 | 13.5 | 14.8 | 2.8 | | 2.7 |
| Cycle Queue Clearance Time (g _c), s | 0.7 | 10.9 | 15.6 | 5.9 | 11.3 | 11.4 | 13.7 | 13.5 | 14.8 | 2.8 | | 2.7 |
| Green Ratio (g/C) | 0.50 | 0.47 | 0.65 | 0.57 | 0.52 | 0.52 | 0.18 | 0.18 | 0.26 | 0.07 | | 0.07 |
| Capacity (c), veh/h | 359 | 1787 | 1043 | 474 | 976 | 973 | 326 | 327 | 414 | 128 | | 122 |
| Volume-to-Capacity Ratio (X) | 0.059 | 0.417 | 0.445 | 0.420 | 0.448 | 0.448 | 0.730 | 0.721 | 0.595 | 0.389 | | 0.369 |
| Back of Queue (Q), ft/ln (95 th percentile) | 12.1 | 182.5 | 223.2 | 103.1 | 184.8 | 183.2 | 268.1 | 271.3 | 249.7 | 66.9 | | 57.5 |
| Back of Queue (Q), veh/ln (95 th percentile) | 0.5 | 7.2 | 8.9 | 4.1 | 7.3 | 7.3 | 10.7 | 10.6 | 10.0 | 2.6 | | 2.3 |
| Queue Storage Ratio (RQ) (95 th percentile) | 0.10 | 0.00 | 0.64 | 0.30 | 0.00 | 0.00 | 0.88 | 0.00 | 1.39 | 0.00 | | 0.00 |
| Uniform Delay (d ₁), s/veh | 14.1 | 12.6 | 9.3 | 12.5 | 9.6 | 9.7 | 42.6 | 42.5 | 35.8 | 49.0 | | 48.9 |
| Incremental Delay (d ₂), s/veh | 0.1 | 0.7 | 1.4 | 0.6 | 1.5 | 1.5 | 5.1 | 4.8 | 1.9 | 2.7 | | 2.6 |
| Initial Queue Delay (d ₃), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 |
| Control Delay (d), s/veh | 14.2 | 13.4 | 10.7 | 13.1 | 11.1 | 11.2 | 47.7 | 47.3 | 37.8 | 51.7 | | 51.6 |
| Level of Service (LOS) | B | B | B | B | B | B | D | D | D | D | | D |
| Approach Delay, s/veh / LOS | 12.4 | | B | 11.5 | | B | 44.2 | | D | 51.7 | | D |
| Intersection Delay, s/veh / LOS | 20.6 | | | | | | C | | | | | |

| Multimodal Results | EB | | | WB | | | NB | | | SB | | |
|----------------------------|-----|--|---|-----|--|---|-----|--|---|-----|--|---|
| Pedestrian LOS Score / LOS | 2.8 | | C | 2.3 | | B | 2.9 | | C | 3.0 | | C |
| Bicycle LOS Score / LOS | 1.5 | | A | 1.4 | | A | 1.7 | | A | 0.6 | | A |

HCS 2010 Signalized Intersection Intermediate Values

| General Information | | | | Intersection Information | | | |
|---------------------|-------------------------------|---------------|-----------------------------|--------------------------|----------|--|--|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | Analysis Date | 1/24/2017 | Area Type | Other | | |
| Jurisdiction | DuPage County | Time Period | SAT Midday | PHF | 0.95 | | |
| Urban Street | 63rd Street | Analysis Year | 2017 | Analysis Period | 1 > 7:00 | | |
| Intersection | 63rd Street with Woodw... | File Name | 63rd and Woodward SATEX.xus | | | | |
| Project Description | Existing SAT Midday Peak Hour | | | | | | |

| Demand Information | EB | | | WB | | | NB | | | SB | | |
|---------------------|----|-----|-----|-----|-----|---|-----|----|-----|----|----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 20 | 708 | 441 | 189 | 824 | 6 | 411 | 39 | 234 | 18 | 55 | 17 |

| Signal Information | | | | | | | | | | | | |
|--------------------|-----------------|-----------------|----|--|--|--|--|--|--|--|--|--|
| Cycle, s | Reference Phase | Reference Point | | | | | | | | | | |
| 110.0 | 2 | Begin | | | | | | | | | | |
| Offset, s | 0 | On | | | | | | | | | | |
| Uncoordinated | No | Simult. Gap E/W | On | | | | | | | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | | | | | | | | | |

| Saturation Flow / Delay | EB | | | WB | | | NB | | | SB | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Lane Width Adjustment Factor (f_w) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Heavy Vehicle Adjustment Factor (f_{HV}) | 0.952 | 0.990 | 0.990 | 0.990 | 0.980 | 1.000 | 1.000 | 0.971 | 1.000 | 0.943 | 1.000 | 1.000 |
| Approach Grade Adjustment Factor (f_g) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Parking Activity Adjustment Factor (f_p) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Bus Blockage Adjustment Factor (f_{bb}) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Area Type Adjustment Factor (f_a) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Lane Utilization Adjustment Factor (f_{LU}) | 1.000 | 0.952 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Left-Turn Adjustment Factor (f_{LT}) | 0.952 | 0.000 | | 0.952 | 0.000 | | 0.952 | 0.000 | | 0.000 | 0.981 | |
| Right-Turn Adjustment Factor (f_{RT}) | | 0.000 | 0.847 | | 0.997 | 0.997 | | 0.000 | 0.847 | | 0.929 | 0.933 |
| Left-Turn Pedestrian Adjustment Factor (f_{LPB}) | 1.000 | | | 1.000 | | | 1.000 | | | 1.000 | | |
| Right-Turn Ped-Bike Adjustment Factor (f_{RPB}) | | | 1.000 | | | 1.000 | | | 1.000 | | | 1.000 |
| Movement Saturation Flow Rate (s), veh/h | 1723 | 3770 | 1594 | 1792 | 3694 | 27 | 1810 | 1816 | 1610 | 709 | 2222 | 706 |
| Proportion of Vehicles Arriving on Green (P) | 0.03 | 0.63 | 0.47 | 0.08 | 0.70 | 0.52 | 0.18 | 0.18 | 0.18 | 0.07 | 0.07 | 0.07 |
| Incremental Delay Factor (k) | 0.11 | 0.50 | 0.50 | 0.11 | 0.50 | 0.50 | 0.18 | 0.17 | 0.15 | 0.15 | | 0.15 |

| Signal Timing / Movement Groups | EBL | EBT/R | WBL | WBT/R | NBL | NBT/R | SBL | SBT/R |
|---|------|-------|------|-------|-----|-------|-----|-------|
| Lost Time (t_L) | 4.0 | 6.0 | 4.0 | 6.0 | | 6.0 | | 6.0 |
| Green Ratio (g/C) | 0.50 | 0.47 | 0.57 | 0.52 | | 0.18 | | 0.07 |
| Permitted Saturation Flow Rate (s_p), veh/h/ln | 614 | 0 | 719 | 0 | | 1810 | | 0 |
| Shared Saturation Flow Rate (s_{sh}), veh/h/ln | | | | | | | | |
| Permitted Effective Green Time (g_p), s | 52.1 | 0.0 | 54.1 | 0.0 | | 0.0 | | 0.0 |
| Permitted Service Time (g_u), s | 44.2 | 0.0 | 41.3 | 0.0 | | 0.0 | | 0.0 |
| Permitted Queue Service Time (g_{ps}), s | 0.3 | | 4.9 | | | | | |
| Time to First Blockage (g_t), s | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | | 0.0 |
| Queue Service Time Before Blockage (g_{ts}), s | | | | | | | | |
| Protected Right Saturation Flow (s_R), veh/h/ln | | | 1594 | | | 1610 | | |
| Protected Right Effective Green Time (g_R), s | | | 19.8 | | | 8.5 | | |

| Multimodal | EB | | | WB | | | NB | | | SB | | |
|----------------------------------|--------|-------|---------|-------|-------|--------|-------|-------|--|----|--|--|
| Pedestrian F_w / F_v | 2.107 | 0.00 | 1.557 | 0.00 | 2.107 | 0.00 | 2.224 | 0.00 | | | | |
| Pedestrian F_s / F_{delay} | 0.000 | 0.109 | 0.000 | 0.101 | 0.000 | 0.166 | 0.000 | 0.155 | | | | |
| Pedestrian M_{corner} / M_{cw} | | | | | | | | | | | | |
| Bicycle c_b / d_b | 947.97 | 15.22 | 1047.82 | 12.47 | 62.22 | 137.41 | 47.70 | | | | | |
| Bicycle F_w / F_v | -3.64 | 1.02 | -3.64 | 0.88 | -3.64 | 1.19 | -3.64 | 0.08 | | | | |

--- Messages ---

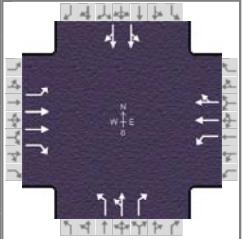
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

WARNING: The shared-plus-exclusive turn lane solution is an approximation of the HCM method, because more than three lane groups cannot be accommodated. Input data for Percent Turns in Shared Lane are used to specify proportion of turning vehicles in the shared lane.

--- Comments ---

HCS 2010 Signalized Intersection Input Data

| General Information | | | | | | Intersection Information | | | |
|---------------------|---------------------------|--|---------------|----------------------------|--|--------------------------|----------|--|--|
| Agency | KLOA, Inc. | | | | | Duration, h | 0.25 | | |
| Analyst | NJB | | Analysis Date | 1/24/2017 | | Area Type | Other | | |
| Jurisdiction | DuPage County | | Time Period | AM | | PHF | 0.91 | | |
| Urban Street | 63rd Street | | Analysis Year | 2018 | | Analysis Period | 1 > 7:00 | | |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward AMFU.xus | | | | | |
| Project Description | Future AM Peak Hour | | | | | | | | |



| Demand Information | EB | | | WB | | | NB | | | SB | | |
|--------------------|----|-----|-----|-----|-----|----|-----|----|-----|----|----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 23 | 775 | 188 | 107 | 873 | 17 | 820 | 83 | 331 | 21 | 38 | 11 |

| Signal Information | | | | Signal Timing (s) | | | | | | | | | |
|--------------------|-------|-----------------|-------|-------------------|-----|-----|------|------|-----|-----|--|--|--|
| Cycle, s | 130.0 | Reference Phase | 6 | | | | | | | | | | |
| Offset, s | 0 | Reference Point | Begin | | | | | | | | | | |
| Uncoordinated | No | Simult. Gap E/W | On | Green | 3.1 | 3.9 | 53.3 | 40.2 | 7.5 | 0.0 | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | Yellow | 3.0 | 0.0 | 4.5 | 4.5 | 4.5 | 0.0 | | | |
| | | | | Red | 1.0 | 0.0 | 1.5 | 1.5 | 1.5 | 0.0 | | | |

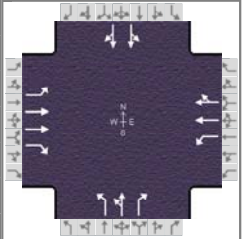
| Traffic Information | EB | | | WB | | | NB | | | SB | | |
|--|------|------|------|------|------|------|------|------|------|------|------|------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 23 | 775 | 188 | 107 | 873 | 17 | 820 | 83 | 331 | 21 | 38 | 11 |
| Initial Queue (Q _b), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Base Saturation Flow Rate (s ₀), veh/h | 1900 | 2000 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Parking (N _m), man/h | | None | | | None | | | None | | | None | |
| Heavy Vehicles (P _{HV}), % | 0 | 4 | 5 | 2 | 4 | | 2 | 5 | 4 | | 0 | |
| Ped / Bike / RTOR, /h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Buses (N _b), buses/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Arrival Type (AT) | 3 | 4 | 3 | 3 | 4 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| Upstream Filtering (I) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Lane Width (W), ft | 12.0 | 12.0 | 12.0 | 12.0 | 12.0 | | 12.0 | 12.0 | 12.0 | | 12.0 | |
| Turn Bay Length, ft | 125 | 0 | 350 | 340 | 0 | | 305 | 0 | 180 | | 0 | |
| Grade (P _g), % | | 0 | | | 0 | | | 0 | | | 0 | |
| Speed Limit, mi/h | 40 | 40 | 40 | 40 | 40 | 40 | 30 | 30 | 30 | 25 | 25 | 25 |

| Phase Information | EBL | EBT | WBL | WBT | NBL | NBT | SBL | SBT |
|---|------|------|------|------|------|------|-----|------|
| Maximum Green (G _{max}) or Phase Split, s | 13.0 | 52.0 | 16.0 | 55.0 | 48.0 | 48.0 | | 14.0 |
| Yellow Change Interval (Y), s | 3.0 | 4.5 | 3.0 | 4.5 | 4.5 | 4.5 | | 4.5 |
| Red Clearance Interval (R _c), s | 1.0 | 1.5 | 1.0 | 1.5 | 1.5 | 1.5 | | 1.5 |
| Minimum Green (G _{min}), s | 3 | 15 | 3 | 15 | 3 | 8 | 3 | 8 |
| Start-Up Lost Time (lt), s | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Extension of Effective Green (e), s | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Passage (PT), s | 3.0 | 7.0 | 3.0 | 7.0 | 3.0 | 4.0 | 3.0 | 4.0 |
| Recall Mode | Off | Min | Off | Min | Off | Off | Off | Off |
| Dual Entry | Yes | Yes | Yes | Yes | No | Yes | No | Yes |
| Walk (Walk), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Pedestrian Clearance Time (PC), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Multimodal Information | EB | | | WB | | | NB | | | SB | | |
|---|-----|------|-----|-----|------|-----|-----|------|-----|-----|------|-----|
| 85th % Speed / Rest in Walk / Corner Radius | 0 | No | 25 | 0 | No | 25 | 0 | No | 25 | 0 | No | 25 |
| Walkway / Crosswalk Width / Length, ft | 9.0 | 12 | 0 | 9.0 | 12 | 0 | 9.0 | 12 | 0 | 9.0 | 12 | 0 |
| Street Width / Island / Curb | 0 | 0 | No | 0 | 0 | No | 0 | 0 | No | 0 | 0 | No |
| Width Outside / Bike Lane / Shoulder, ft | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 |
| Pedestrian Signal / Occupied Parking | No | 0.50 | | No | 0.50 | | No | 0.50 | | No | 0.50 | |

HCS 2010 Signalized Intersection Results Summary

| General Information | | | | Intersection Information | | | |
|---------------------|---------------------------|---------------|----------------------------|--------------------------|----------|--|--|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | Analysis Date | 1/24/2017 | Area Type | Other | | |
| Jurisdiction | DuPage County | Time Period | AM | PHF | 0.91 | | |
| Urban Street | 63rd Street | Analysis Year | 2018 | Analysis Period | 1 > 7:00 | | |
| Intersection | 63rd Street with Woodw... | File Name | 63rd and Woodward AMFU.xus | | | | |
| Project Description | Future AM Peak Hour | | | | | | |



| Demand Information | EB | | | WB | | | NB | | | SB | | |
|---------------------|----|-----|-----|-----|-----|----|-----|----|-----|----|----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 23 | 775 | 188 | 107 | 873 | 17 | 820 | 83 | 331 | 21 | 38 | 11 |

| Signal Information | | | | Signal Timing (s) | | | | | | | | | | | |
|--------------------|-------|-----------------|-------|-------------------|-----|------|------|-----|-----|--|--|--|--|--|--|
| Cycle, s | 130.0 | Reference Phase | 6 | | | | | | | | | | | | |
| Offset, s | 0 | Reference Point | Begin | | | | | | | | | | | | |
| Uncoordinated | No | Simult. Gap E/W | On | | | | | | | | | | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | | | | | | | | | | | | |
| | | Green | | 3.1 | 3.9 | 53.3 | 40.2 | 7.5 | 0.0 | | | | | | |
| | | Yellow | | 3.0 | 0.0 | 4.5 | 4.5 | 4.5 | 0.0 | | | | | | |
| | | Red | | 1.0 | 0.0 | 1.5 | 1.5 | 1.5 | 0.0 | | | | | | |

| Timer Results | EBL | EBT | WBL | WBT | NBL | NBT | SBL | SBT |
|--|------|------|------|------|-----|------|-----|------|
| Assigned Phase | 5 | 2 | 1 | 6 | | 8 | | 4 |
| Case Number | 1.1 | 3.0 | 1.1 | 4.0 | | 9.0 | | 12.0 |
| Phase Duration, s | 7.1 | 59.3 | 11.0 | 63.2 | | 46.2 | | 13.5 |
| Change Period, (Y+R _c), s | 4.0 | 6.0 | 4.0 | 6.0 | | 6.0 | | 6.0 |
| Max Allow Headway (MAH), s | 4.0 | 0.0 | 4.0 | 0.0 | | 5.1 | | 5.2 |
| Queue Clearance Time (g _s), s | 3.0 | | 6.8 | | | 36.8 | | 4.7 |
| Green Extension Time (g _e), s | 0.0 | 0.0 | 0.2 | 0.0 | | 3.4 | | 0.1 |
| Phase Call Probability | 1.00 | | 1.00 | | | 1.00 | | 0.94 |
| Max Out Probability | 0.00 | | 0.04 | | | 1.00 | | 1.00 |

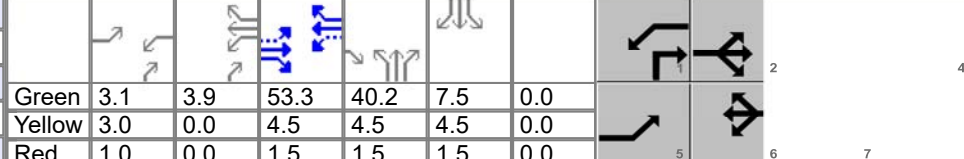
| Movement Group Results | EB | | | WB | | | NB | | | SB | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|---|-------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Assigned Movement | 5 | 2 | 12 | 1 | 6 | 16 | 3 | 8 | 18 | 7 | 4 | 14 |
| Adjusted Flow Rate (v), veh/h | 25 | 852 | 207 | 118 | 491 | 487 | 496 | 497 | 364 | 40 | | 37 |
| Adjusted Saturation Flow Rate (s), veh/h/ln | 1810 | 1831 | 1533 | 1774 | 1827 | 1814 | 1774 | 1781 | 1548 | 1847 | | 1793 |
| Queue Service Time (g _s), s | 1.0 | 19.9 | 5.7 | 4.8 | 22.5 | 22.7 | 34.8 | 34.7 | 25.4 | 2.7 | | 2.5 |
| Cycle Queue Clearance Time (g _c), s | 1.0 | 19.9 | 5.7 | 4.8 | 22.5 | 22.7 | 34.8 | 34.7 | 25.4 | 2.7 | | 2.5 |
| Green Ratio (g/C) | 0.43 | 0.41 | 0.72 | 0.48 | 0.44 | 0.44 | 0.31 | 0.31 | 0.36 | 0.06 | | 0.06 |
| Capacity (c), veh/h | 244 | 1501 | 1103 | 316 | 804 | 799 | 549 | 551 | 562 | 107 | | 104 |
| Volume-to-Capacity Ratio (X) | 0.103 | 0.567 | 0.187 | 0.372 | 0.610 | 0.610 | 0.903 | 0.902 | 0.647 | 0.379 | | 0.353 |
| Back of Queue (Q), ft/ln (95 th percentile) | 20.2 | 314.9 | 81.1 | 91.9 | 351 | 343 | 629.4 | 644.5 | 391.5 | 64.9 | | 56.4 |
| Back of Queue (Q), veh/ln (95 th percentile) | 0.8 | 12.2 | 3.1 | 3.6 | 13.6 | 13.7 | 24.8 | 24.8 | 15.2 | 2.5 | | 2.3 |
| Queue Storage Ratio (RQ) (95 th percentile) | 0.16 | 0.00 | 0.23 | 0.27 | 0.00 | 0.00 | 2.06 | 0.00 | 2.17 | 0.00 | | 0.00 |
| Uniform Delay (d ₁), s/veh | 22.9 | 21.9 | 5.9 | 21.2 | 19.7 | 20.0 | 43.0 | 43.0 | 34.5 | 59.0 | | 58.9 |
| Incremental Delay (d ₂), s/veh | 0.2 | 1.6 | 0.4 | 0.7 | 3.4 | 3.5 | 17.7 | 17.4 | 2.8 | 3.1 | | 2.9 |
| Initial Queue Delay (d ₃), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 |
| Control Delay (d), s/veh | 23.1 | 23.4 | 6.3 | 21.9 | 23.1 | 23.4 | 60.7 | 60.4 | 37.2 | 62.1 | | 61.8 |
| Level of Service (LOS) | C | C | A | C | C | C | E | E | D | E | | E |
| Approach Delay, s/veh / LOS | 20.2 | | C | 23.1 | | C | 54.3 | | D | 62.0 | | E |
| Intersection Delay, s/veh / LOS | 34.8 | | | | | | C | | | | | |

| Multimodal Results | EB | | WB | | NB | | SB | |
|----------------------------|-----|---|-----|---|-----|---|-----|---|
| Pedestrian LOS Score / LOS | 2.8 | C | 2.3 | B | 2.9 | C | 3.0 | C |
| Bicycle LOS Score / LOS | 1.4 | A | 1.4 | A | 2.7 | B | 0.6 | A |

HCS 2010 Signalized Intersection Intermediate Values

| General Information | | | | Intersection Information | | | |
|---------------------|---------------------------|---------------|---------------|----------------------------|-----------------|----------|--|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | Analysis Date | 1/24/2017 | Area Type | Other | | |
| Jurisdiction | DuPage County | | Time Period | AM | PHF | 0.91 | |
| Urban Street | 63rd Street | | Analysis Year | 2018 | Analysis Period | 1 > 7:00 | |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward AMFU.xus | | | |
| Project Description | Future AM Peak Hour | | | | | | |

| Demand Information | EB | | | WB | | | NB | | | SB | | |
|---------------------|----|-----|-----|-----|-----|----|-----|----|-----|----|----|----|
| Approach Movement | L | T | R | L | T | R | L | T | R | L | T | R |
| Demand (v), veh/h | 23 | 775 | 188 | 107 | 873 | 17 | 820 | 83 | 331 | 21 | 38 | 11 |

| Signal Information | | | |  | | | | | | | | | | |
|--------------------|-------|-----------------|-------|--|-----|-----|------|------|-----|-----|--|--|--|--|
| Cycle, s | 130.0 | Reference Phase | 6 | Green | 3.1 | 3.9 | 53.3 | 40.2 | 7.5 | 0.0 | | | | |
| Offset, s | 0 | Reference Point | Begin | Yellow | 3.0 | 0.0 | 4.5 | 4.5 | 4.5 | 0.0 | | | | |
| Uncoordinated | No | Simult. Gap E/W | On | Red | 1.0 | 0.0 | 1.5 | 1.5 | 1.5 | 0.0 | | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | | | | | | | | | | | |

| Saturation Flow / Delay | EB | | | WB | | | NB | | | SB | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Lane Width Adjustment Factor (f_w) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Heavy Vehicle Adjustment Factor (f_{HV}) | 1.000 | 0.962 | 0.952 | 0.980 | 0.962 | 1.000 | 0.980 | 0.952 | 0.962 | 0.952 | 1.000 | 1.000 |
| Approach Grade Adjustment Factor (f_g) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Parking Activity Adjustment Factor (f_p) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Bus Blockage Adjustment Factor (f_{bb}) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Area Type Adjustment Factor (f_a) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Lane Utilization Adjustment Factor (f_{LU}) | 1.000 | 0.952 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Left-Turn Adjustment Factor (f_{LT}) | 0.952 | 0.000 | | 0.952 | 0.000 | | 0.952 | 0.000 | | 0.000 | 0.972 | |
| Right-Turn Adjustment Factor (f_{RT}) | | 0.000 | 0.847 | | 0.993 | 0.993 | | 0.000 | 0.847 | | 0.938 | 0.944 |
| Left-Turn Pedestrian Adjustment Factor (f_{LPB}) | 1.000 | | | 1.000 | | | 1.000 | | | 1.000 | | |
| Right-Turn Ped-Bike Adjustment Factor (f_{RPB}) | | | 1.000 | | | 1.000 | | | 1.000 | | | 1.000 |
| Movement Saturation Flow Rate (s), veh/h | 1810 | 3662 | 1533 | 1774 | 3572 | 70 | 1774 | 1781 | 1548 | 1056 | 1991 | 593 |
| Proportion of Vehicles Arriving on Green (P) | 0.02 | 0.55 | 0.41 | 0.05 | 0.59 | 0.44 | 0.31 | 0.31 | 0.31 | 0.06 | 0.06 | 0.06 |
| Incremental Delay Factor (k) | 0.11 | 0.50 | 0.50 | 0.11 | 0.50 | 0.50 | 0.41 | 0.41 | 0.24 | 0.15 | | 0.15 |

| Signal Timing / Movement Groups | EBL | EBT/R | WBL | WBT/R | NBL | NBT/R | SBL | SBT/R |
|---|------|-------|------|-------|-----|-------|-----|-------|
| Lost Time (t_L) | 4.0 | 6.0 | 4.0 | 6.0 | | 6.0 | | 6.0 |
| Green Ratio (g/C) | 0.43 | 0.41 | 0.48 | 0.44 | | 0.31 | | 0.06 |
| Permitted Saturation Flow Rate (s_p), veh/h/ln | 584 | 0 | 645 | 0 | | 1774 | | 0 |
| Shared Saturation Flow Rate (s_{sh}), veh/h/ln | | | | | | | | |
| Permitted Effective Green Time (g_p), s | 53.3 | 0.0 | 55.2 | 0.0 | | 0.0 | | 0.0 |
| Permitted Service Time (g_u), s | 32.6 | 0.0 | 33.4 | 0.0 | | 0.0 | | 0.0 |
| Permitted Queue Service Time (g_{ps}), s | 0.9 | | 4.9 | | | | | |
| Time to First Blockage (g_t), s | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | | 0.0 |
| Queue Service Time Before Blockage (g_{ts}), s | | | | | | | | |
| Protected Right Saturation Flow (s_R), veh/h/ln | | | 1533 | | | 1548 | | |
| Protected Right Effective Green Time (g_R), s | | | 40.2 | | | 7.0 | | |

| Multimodal | EB | | | WB | | | NB | | | SB | | |
|----------------------------------|--------|-------|--------|-------|-------|--------|-------|-------|--|----|--|--|
| Pedestrian F_w / F_v | 2.107 | 0.00 | 1.557 | 0.00 | 2.107 | 0.00 | 2.224 | 0.00 | | | | |
| Pedestrian F_s / F_{delay} | 0.000 | 0.125 | 0.000 | 0.121 | 0.000 | 0.172 | 0.000 | 0.163 | | | | |
| Pedestrian M_{corner} / M_{cw} | | | | | | | | | | | | |
| Bicycle c_b / d_b | 820.05 | 22.62 | 880.32 | 20.37 | 72.19 | 115.42 | 57.71 | | | | | |
| Bicycle F_w / F_v | -3.64 | 0.89 | -3.64 | 0.90 | -3.64 | 2.24 | -3.64 | 0.06 | | | | |

--- Messages ---

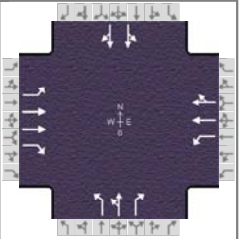
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

WARNING: The shared-plus-exclusive turn lane solution is an approximation of the HCM method, because more than three lane groups cannot be accommodated. Input data for Percent Turns in Shared Lane are used to specify proportion of turning vehicles in the shared lane.

--- Comments ---

HCS 2010 Signalized Intersection Input Data

| General Information | | | | | | Intersection Information | | | | |
|---------------------|---------------------------|--|---------------|----------------------------|--|--------------------------|----------|--|--|--|
| Agency | KLOA, Inc. | | | | | Duration, h | 0.25 | | | |
| Analyst | NJB | | Analysis Date | 1/24/2017 | | Area Type | Other | | | |
| Jurisdiction | DuPage County | | Time Period | PM | | PHF | 0.97 | | | |
| Urban Street | 63rd Street | | Analysis Year | 2018 | | Analysis Period | 1 > 7:00 | | | |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward PMFU.xus | | | | | | |
| Project Description | Future PM Peak Hour | | | | | | | | | |



| Demand Information | EB | | | WB | | | NB | | | SB | | |
|--------------------|----|-----|-----|-----|-----|---|-----|----|-----|----|-----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 24 | 977 | 713 | 292 | 847 | 5 | 339 | 52 | 193 | 22 | 143 | 16 |

| Signal Information | | | | Signal Timing (s) | | | | | | | | | Signal Phases | | | |
|--------------------|-------|-----------------|-------|-------------------|-----|-----|------|------|-----|-----|--|--|---------------|--|--|--|
| Cycle, s | 130.0 | Reference Phase | 2 | | | | | | | | | | | | | |
| Offset, s | 0 | Reference Point | Begin | Green | 3.0 | 5.7 | 67.1 | 18.9 | 9.3 | 0.0 | | | | | | |
| Uncoordinated | No | Simult. Gap E/W | On | Yellow | 3.0 | 3.0 | 4.5 | 4.5 | 4.5 | 0.0 | | | | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | Red | 1.0 | 1.0 | 1.5 | 1.5 | 1.5 | 0.0 | | | | | | |

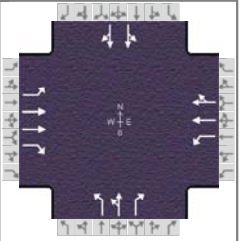
| Traffic Information | EB | | | WB | | | NB | | | SB | | |
|--|------|------|------|------|------|------|------|------|------|------|------|------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 24 | 977 | 713 | 292 | 847 | 5 | 339 | 52 | 193 | 22 | 143 | 16 |
| Initial Queue (Q _b), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Base Saturation Flow Rate (s ₀), veh/h | 1900 | 2000 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Parking (N _m), man/h | | None | | | None | | | None | | | None | |
| Heavy Vehicles (P _{HV}), % | 0 | 1 | 0 | 1 | 1 | | 1 | 2 | 2 | | 0 | |
| Ped / Bike / RTOR, /h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Buses (N _b), buses/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Arrival Type (AT) | 3 | 4 | 3 | 3 | 4 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| Upstream Filtering (I) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Lane Width (W), ft | 12.0 | 12.0 | 12.0 | 12.0 | 12.0 | | 12.0 | 12.0 | 12.0 | | 12.0 | |
| Turn Bay Length, ft | 125 | 0 | 350 | 340 | 0 | | 305 | 0 | 180 | | 0 | |
| Grade (P _g), % | | 0 | | | 0 | | | 0 | | | 0 | |
| Speed Limit, mi/h | 40 | 40 | 40 | 40 | 40 | 40 | 30 | 30 | 30 | 25 | 25 | 25 |

| Phase Information | EBL | EBT | WBL | WBT | NBL | NBT | SBL | SBT |
|---|------|------|------|------|------|------|-----|------|
| Maximum Green (G _{max}) or Phase Split, s | 13.0 | 48.0 | 30.0 | 65.0 | 31.0 | 31.0 | | 21.0 |
| Yellow Change Interval (Y), s | 3.0 | 4.5 | 3.0 | 4.5 | 4.5 | 4.5 | | 4.5 |
| Red Clearance Interval (R _c), s | 1.0 | 1.5 | 1.0 | 1.5 | 1.5 | 1.5 | | 1.5 |
| Minimum Green (G _{min}), s | 3 | 15 | 3 | 15 | 3 | 8 | 3 | 8 |
| Start-Up Lost Time (lt), s | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Extension of Effective Green (e), s | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Passage (PT), s | 3.0 | 7.0 | 3.0 | 7.0 | 3.0 | 4.0 | 3.0 | 4.0 |
| Recall Mode | Off | Min | Off | Min | Off | Off | Off | Off |
| Dual Entry | Yes | Yes | Yes | Yes | No | Yes | No | Yes |
| Walk (Walk), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Pedestrian Clearance Time (PC), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Multimodal Information | EB | | | WB | | | NB | | | SB | | |
|---|-----|------|-----|-----|------|-----|-----|------|-----|-----|------|-----|
| 85th % Speed / Rest in Walk / Corner Radius | 0 | No | 25 | 0 | No | 25 | 0 | No | 25 | 0 | No | 25 |
| Walkway / Crosswalk Width / Length, ft | 9.0 | 12 | 0 | 9.0 | 12 | 0 | 9.0 | 12 | 0 | 9.0 | 12 | 0 |
| Street Width / Island / Curb | 0 | 0 | No | 0 | 0 | No | 0 | 0 | No | 0 | 0 | No |
| Width Outside / Bike Lane / Shoulder, ft | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 |
| Pedestrian Signal / Occupied Parking | No | 0.50 | | No | 0.50 | | No | 0.50 | | No | 0.50 | |

HCS 2010 Signalized Intersection Results Summary

| General Information | | | | Intersection Information | | | |
|---------------------|---------------------------|--|---------------|----------------------------|------|-----------------|----------|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | | Analysis Date | 1/24/2017 | | Area Type | Other |
| Jurisdiction | DuPage County | | Time Period | PM | | PHF | 0.97 |
| Urban Street | 63rd Street | | Analysis Year | 2018 | | Analysis Period | 1 > 7:00 |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward PMFU.xus | | | |
| Project Description | Future PM Peak Hour | | | | | | |



| Demand Information | EB | | | WB | | | NB | | | SB | | |
|---------------------|----|-----|-----|-----|-----|---|-----|----|-----|----|-----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 24 | 977 | 713 | 292 | 847 | 5 | 339 | 52 | 193 | 22 | 143 | 16 |

| Signal Information | | | | Signal Timing (s) | | | | | | | | |
|--------------------|-------|-----------------|-------|-------------------|-----|-----|--|--|--|--|--|--|
| Cycle, s | 130.0 | Reference Phase | 2 | | | | | | | | | |
| Offset, s | 0 | Reference Point | Begin | | | | | | | | | |
| Uncoordinated | No | Simult. Gap E/W | On | | | | | | | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | | | | | | | | | |
| Green | 3.0 | 5.7 | 67.1 | 18.9 | 9.3 | 0.0 | | | | | | |
| Yellow | 3.0 | 3.0 | 4.5 | 4.5 | 4.5 | 0.0 | | | | | | |
| Red | 1.0 | 1.0 | 1.5 | 1.5 | 1.5 | 0.0 | | | | | | |

| Timer Results | EBL | EBT | WBL | WBT | NBL | NBT | SBL | SBT |
|--|------|------|------|------|-----|------|-----|------|
| Assigned Phase | 5 | 2 | 1 | 6 | | 8 | | 4 |
| Case Number | 1.1 | 3.0 | 1.1 | 4.0 | | 9.0 | | 12.0 |
| Phase Duration, s | 7.0 | 73.1 | 16.7 | 82.8 | | 24.9 | | 15.3 |
| Change Period, (Y+R _c), s | 4.0 | 6.0 | 4.0 | 6.0 | | 6.0 | | 6.0 |
| Max Allow Headway (MAH), s | 4.0 | 0.0 | 4.0 | 0.0 | | 5.2 | | 5.1 |
| Queue Clearance Time (g _s), s | 2.8 | | 11.7 | | | 16.7 | | 8.6 |
| Green Extension Time (g _e), s | 0.0 | 0.0 | 1.0 | 0.0 | | 2.2 | | 0.7 |
| Phase Call Probability | 1.00 | | 1.00 | | | 1.00 | | 1.00 |
| Max Out Probability | 0.00 | | 0.00 | | | 0.49 | | 0.02 |

| Movement Group Results | EB | | | WB | | | NB | | | SB | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|---|-------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Assigned Movement | 5 | 2 | 12 | 1 | 6 | 16 | 3 | 8 | 18 | 7 | 4 | 14 |
| Adjusted Flow Rate (v), veh/h | 25 | 1007 | 735 | 301 | 440 | 439 | 192 | 211 | 199 | 98 | | 89 |
| Adjusted Saturation Flow Rate (s), veh/h/ln | 1810 | 1885 | 1610 | 1792 | 1881 | 1877 | 1792 | 1810 | 1579 | 1878 | | 1838 |
| Queue Service Time (g _s), s | 0.8 | 16.8 | 37.0 | 9.7 | 9.4 | 9.5 | 13.4 | 14.7 | 14.2 | 6.6 | | 6.1 |
| Cycle Queue Clearance Time (g _c), s | 0.8 | 16.8 | 37.0 | 9.7 | 9.4 | 9.5 | 13.4 | 14.7 | 14.2 | 6.6 | | 6.1 |
| Green Ratio (g/C) | 0.54 | 0.52 | 0.66 | 0.63 | 0.59 | 0.59 | 0.15 | 0.15 | 0.24 | 0.07 | | 0.07 |
| Capacity (c), veh/h | 420 | 1945 | 1064 | 448 | 1111 | 1109 | 260 | 263 | 384 | 135 | | 132 |
| Volume-to-Capacity Ratio (X) | 0.059 | 0.518 | 0.691 | 0.672 | 0.396 | 0.396 | 0.739 | 0.803 | 0.518 | 0.728 | | 0.671 |
| Back of Queue (Q), ft/ln (95 th percentile) | 15.3 | 246.2 | 476.8 | 174.1 | 149.3 | 149.6 | 270.7 | 304.5 | 245 | 167.2 | | 142.6 |
| Back of Queue (Q), veh/ln (95 th percentile) | 0.6 | 9.8 | 19.1 | 6.9 | 5.9 | 6.0 | 10.7 | 12.0 | 9.6 | 6.4 | | 5.7 |
| Queue Storage Ratio (RQ) (95 th percentile) | 0.12 | 0.00 | 1.36 | 0.51 | 0.00 | 0.00 | 0.89 | 0.00 | 1.36 | 0.00 | | 0.00 |
| Uniform Delay (d ₁), s/veh | 14.0 | 12.5 | 13.7 | 13.7 | 6.6 | 6.7 | 53.2 | 53.8 | 42.6 | 59.1 | | 58.8 |
| Incremental Delay (d ₂), s/veh | 0.1 | 1.0 | 3.7 | 1.8 | 1.1 | 1.1 | 7.2 | 11.2 | 1.5 | 10.1 | | 8.1 |
| Initial Queue Delay (d ₃), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 |
| Control Delay (d), s/veh | 14.1 | 13.4 | 17.4 | 15.4 | 7.7 | 7.8 | 60.4 | 65.0 | 44.1 | 69.2 | | 67.0 |
| Level of Service (LOS) | B | B | B | B | A | A | E | E | D | E | | E |
| Approach Delay, s/veh / LOS | 15.1 | | B | 9.7 | | A | 56.6 | | E | 68.2 | | E |
| Intersection Delay, s/veh / LOS | 22.7 | | | | | | C | | | | | |

| Multimodal Results | EB | | WB | | NB | | SB | |
|----------------------------|-----|---|-----|---|-----|---|-----|---|
| Pedestrian LOS Score / LOS | 2.8 | C | 2.3 | B | 2.9 | C | 3.0 | C |
| Bicycle LOS Score / LOS | 1.9 | A | 1.5 | A | 1.5 | A | 0.6 | A |

HCS 2010 Signalized Intersection Intermediate Values

| General Information | | | | | | Intersection Information | | | | | |
|---------------------|---------------------------|--|---------------|----------------------------|--|--------------------------|----------|--|--|--|--|
| Agency | KLOA, Inc. | | | | | Duration, h | 0.25 | | | | |
| Analyst | NJB | | Analysis Date | 1/24/2017 | | Area Type | Other | | | | |
| Jurisdiction | DuPage County | | Time Period | PM | | PHF | 0.97 | | | | |
| Urban Street | 63rd Street | | Analysis Year | 2018 | | Analysis Period | 1 > 7:00 | | | | |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward PMFU.xus | | | | | | | |
| Project Description | Future PM Peak Hour | | | | | | | | | | |

| Demand Information | EB | | | WB | | | NB | | | SB | | |
|---------------------|----|-----|-----|-----|-----|---|-----|----|-----|----|-----|----|
| Approach Movement | L | T | R | L | T | R | L | T | R | L | T | R |
| Demand (v), veh/h | 24 | 977 | 713 | 292 | 847 | 5 | 339 | 52 | 193 | 22 | 143 | 16 |

| Signal Information | | | | | | | | | | | | |
|--------------------|-------|-----------------|-------|--|--|--|--|--|--|--|--|--|
| Cycle, s | 130.0 | Reference Phase | 2 | | | | | | | | | |
| Offset, s | 0 | Reference Point | Begin | | | | | | | | | |
| Uncoordinated | No | Simult. Gap E/W | On | | | | | | | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | | | | | | | | | |

| Saturation Flow / Delay | EB | | | WB | | | NB | | | SB | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Lane Width Adjustment Factor (f_w) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Heavy Vehicle Adjustment Factor (f_{HV}) | 1.000 | 0.990 | 1.000 | 0.990 | 0.990 | 1.000 | 0.990 | 0.980 | 0.980 | 0.952 | 1.000 | 1.000 |
| Approach Grade Adjustment Factor (f_g) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Parking Activity Adjustment Factor (f_p) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Bus Blockage Adjustment Factor (f_{bb}) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Area Type Adjustment Factor (f_a) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Lane Utilization Adjustment Factor (f_{LU}) | 1.000 | 0.952 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Left-Turn Adjustment Factor (f_{LT}) | 0.952 | 0.000 | | 0.952 | 0.000 | | 0.952 | 0.000 | | 0.000 | 0.989 | |
| Right-Turn Adjustment Factor (f_{RT}) | | 0.000 | 0.847 | | 0.998 | 0.998 | | 0.000 | 0.847 | | 0.967 | 0.968 |
| Left-Turn Pedestrian Adjustment Factor (f_{LPB}) | 1.000 | | | 1.000 | | | 1.000 | | | 1.000 | | |
| Right-Turn Ped-Bike Adjustment Factor (f_{RPB}) | | | 1.000 | | | 1.000 | | | 1.000 | | | 1.000 |
| Movement Saturation Flow Rate (s), veh/h | 1810 | 3770 | 1610 | 1792 | 3736 | 22 | 1792 | 1810 | 1579 | 434 | 2940 | 342 |
| Proportion of Vehicles Arriving on Green (P) | 0.02 | 0.69 | 0.52 | 0.10 | 0.79 | 0.59 | 0.15 | 0.15 | 0.15 | 0.07 | 0.07 | 0.07 |
| Incremental Delay Factor (k) | 0.11 | 0.50 | 0.50 | 0.11 | 0.50 | 0.50 | 0.19 | 0.23 | 0.15 | 0.15 | | 0.15 |

| Signal Timing / Movement Groups | EBL | EBT/R | WBL | WBT/R | NBL | NBT/R | SBL | SBT/R |
|---|------|-------|------|-------|-----|-------|-----|-------|
| Lost Time (t_L) | 4.0 | 6.0 | 4.0 | 6.0 | | 6.0 | | 6.0 |
| Green Ratio (g/C) | 0.54 | 0.52 | 0.63 | 0.59 | | 0.15 | | 0.07 |
| Permitted Saturation Flow Rate (s_p), veh/h/ln | 642 | 0 | 563 | 0 | | 1792 | | 0 |
| Shared Saturation Flow Rate (s_{sh}), veh/h/ln | | | | | | | | |
| Permitted Effective Green Time (g_p), s | 67.1 | 0.0 | 69.1 | 0.0 | | 0.0 | | 0.0 |
| Permitted Service Time (g_u), s | 65.3 | 0.0 | 50.2 | 0.0 | | 0.0 | | 0.0 |
| Permitted Queue Service Time (g_{ps}), s | 0.1 | | 21.7 | | | | | |
| Time to First Blockage (g_t), s | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | | 0.0 |
| Queue Service Time Before Blockage (g_{ts}), s | | | | | | | | |
| Protected Right Saturation Flow (s_R), veh/h/ln | | | 1610 | | | 1579 | | |
| Protected Right Effective Green Time (g_R), s | | | 18.9 | | | 12.7 | | |

| Multimodal | EB | | | WB | | | NB | | | SB | | |
|----------------------------------|---------|-------|---------|-------|-------|--------|-------|-------|--|----|--|--|
| Pedestrian F_w / F_v | 2.107 | 0.00 | 1.557 | 0.00 | 2.107 | 0.00 | 2.224 | 0.00 | | | | |
| Pedestrian F_s / F_{delay} | 0.000 | 0.109 | 0.000 | 0.096 | 0.000 | 0.172 | 0.000 | 0.161 | | | | |
| Pedestrian M_{corner} / M_{cw} | | | | | | | | | | | | |
| Bicycle c_b / d_b | 1031.61 | 15.24 | 1181.49 | 10.89 | 72.19 | 143.49 | 56.01 | | | | | |
| Bicycle F_w / F_v | -3.64 | 1.46 | -3.64 | 0.97 | -3.64 | 0.99 | -3.64 | 0.15 | | | | |

--- Messages ---

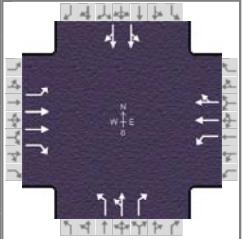
WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

WARNING: The shared-plus-exclusive turn lane solution is an approximation of the HCM method, because more than three lane groups cannot be accommodated. Input data for Percent Turns in Shared Lane are used to specify proportion of turning vehicles in the shared lane.

--- Comments ---

HCS 2010 Signalized Intersection Input Data

| General Information | | | | Intersection Information | | | |
|---------------------|-----------------------------|--|---------------|-----------------------------|------|-----------------|----------|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | | Analysis Date | 1/24/2017 | | Area Type | Other |
| Jurisdiction | DuPage County | | Time Period | SAT Midday | | PHF | 0.95 |
| Urban Street | 63rd Street | | Analysis Year | 2018 | | Analysis Period | 1 > 7:00 |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward SATFU.xus | | | |
| Project Description | Future SAT Midday Peak Hour | | | | | | |



| Demand Information | EB | | | WB | | | NB | | | SB | | |
|--------------------|----|-----|-----|-----|-----|---|-----|----|-----|----|----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 20 | 724 | 445 | 191 | 841 | 6 | 415 | 41 | 236 | 18 | 57 | 17 |

| Signal Information | | | | Signal Timing (s) | | | | | | | | | | | | | |
|--------------------|-------|-----------------|-------|-------------------|-----|-----|------|------|-----|-----|--------|-----|-----|-----|-----|-----|-----|
| Cycle, s | 110.0 | Reference Phase | 2 | | | | | | | | | | | | | | |
| Offset, s | 0 | Reference Point | Begin | Green | 3.0 | 1.6 | 51.9 | 19.9 | 7.6 | 0.0 | Yellow | 3.0 | 3.0 | 4.5 | 4.5 | 4.5 | 0.0 |
| Uncoordinated | No | Simult. Gap E/W | On | Red | 1.0 | 1.0 | 1.5 | 1.5 | 1.5 | 0.0 | | | | | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | | | | | | | | | | | | | | |

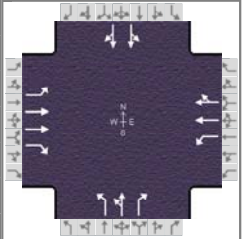
| Traffic Information | EB | | | WB | | | NB | | | SB | | |
|--|------|------|------|------|------|------|------|------|------|------|------|------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 20 | 724 | 445 | 191 | 841 | 6 | 415 | 41 | 236 | 18 | 57 | 17 |
| Initial Queue (Q _b), veh/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Base Saturation Flow Rate (s ₀), veh/h | 1900 | 2000 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Parking (N _m), man/h | | None | | | None | | | None | | | None | |
| Heavy Vehicles (P _{HV}), % | 5 | 1 | 1 | 1 | 2 | | 0 | 3 | 0 | | 0 | |
| Ped / Bike / RTOR, /h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Buses (N _b), buses/h | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Arrival Type (AT) | 3 | 4 | 3 | 3 | 4 | 3 | 3 | 3 | 3 | 3 | 3 | 3 |
| Upstream Filtering (I) | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Lane Width (W), ft | 12.0 | 12.0 | 12.0 | 12.0 | 12.0 | | 12.0 | 12.0 | 12.0 | | 12.0 | |
| Turn Bay Length, ft | 125 | 0 | 350 | 340 | 0 | | 305 | 0 | 180 | | 0 | |
| Grade (P _g), % | | 0 | | | 0 | | | 0 | | | 0 | |
| Speed Limit, mi/h | 40 | 40 | 40 | 40 | 40 | 40 | 30 | 30 | 30 | 25 | 25 | 25 |

| Phase Information | EBL | EBT | WBL | WBT | NBL | NBT | SBL | SBT |
|---|---|------|------|------|------|------|------|-----|
| | Maximum Green (G _{max}) or Phase Split, s | 13.0 | 39.0 | 19.0 | 45.0 | 33.0 | 33.0 | |
| Yellow Change Interval (Y), s | 3.0 | 4.5 | 3.0 | 4.5 | 4.5 | 4.5 | | 4.5 |
| Red Clearance Interval (R _c), s | 1.0 | 1.5 | 1.0 | 1.5 | 1.5 | 1.5 | | 1.5 |
| Minimum Green (G _{min}), s | 3 | 15 | 3 | 15 | 3 | 8 | 3 | 8 |
| Start-Up Lost Time (lt), s | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Extension of Effective Green (e), s | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 | 2.0 |
| Passage (PT), s | 3.0 | 7.0 | 3.0 | 7.0 | 3.0 | 4.0 | 3.0 | 4.0 |
| Recall Mode | Off | Min | Off | Min | Off | Off | Off | Off |
| Dual Entry | Yes | Yes | Yes | Yes | No | Yes | No | Yes |
| Walk (Walk), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |
| Pedestrian Clearance Time (PC), s | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 |

| Multimodal Information | EB | | | WB | | | NB | | | SB | | |
|--|---|------|-----|-----|------|-----|-----|------|-----|-----|------|-----|
| | 85th % Speed / Rest in Walk / Corner Radius | 0 | No | 25 | 0 | No | 25 | 0 | No | 25 | 0 | No |
| Walkway / Crosswalk Width / Length, ft | 9.0 | 12 | 0 | 9.0 | 12 | 0 | 9.0 | 12 | 0 | 9.0 | 12 | 0 |
| Street Width / Island / Curb | 0 | 0 | No | 0 | 0 | No | 0 | 0 | No | 0 | 0 | No |
| Width Outside / Bike Lane / Shoulder, ft | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 | 12 | 5.0 | 2.0 |
| Pedestrian Signal / Occupied Parking | No | 0.50 | | No | 0.50 | | No | 0.50 | | No | 0.50 | |

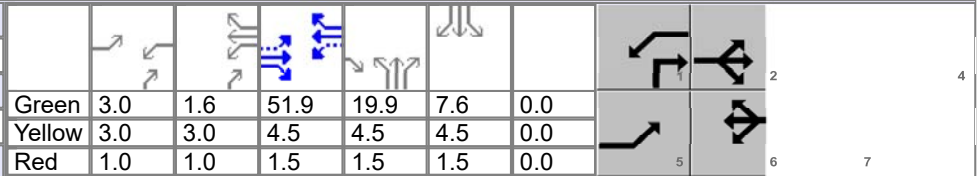
HCS 2010 Signalized Intersection Results Summary

| General Information | | | | Intersection Information | | | |
|---------------------|-----------------------------|---------------|-----------------------------|--------------------------|----------|--|--|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | Analysis Date | 1/24/2017 | Area Type | Other | | |
| Jurisdiction | DuPage County | Time Period | SAT Midday | PHF | 0.95 | | |
| Urban Street | 63rd Street | Analysis Year | 2018 | Analysis Period | 1 > 7:00 | | |
| Intersection | 63rd Street with Woodw... | File Name | 63rd and Woodward SATFU.xus | | | | |
| Project Description | Future SAT Midday Peak Hour | | | | | | |



| Demand Information | EB | | | WB | | | NB | | | SB | | |
|---------------------|----|-----|-----|-----|-----|---|-----|----|-----|----|----|----|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Approach Movement | | | | | | | | | | | | |
| Demand (v), veh/h | 20 | 724 | 445 | 191 | 841 | 6 | 415 | 41 | 236 | 18 | 57 | 17 |

| Signal Information | | | |
|--------------------|-------|-----------------|-------|
| Cycle, s | 110.0 | Reference Phase | 2 |
| Offset, s | 0 | Reference Point | Begin |
| Uncoordinated | No | Simult. Gap E/W | On |
| Force Mode | Fixed | Simult. Gap N/S | On |



| Timer Results | EBL | EBT | WBL | WBT | NBL | NBT | SBL | SBT |
|--|------|------|------|------|-----|------|-----|------|
| Assigned Phase | 5 | 2 | 1 | 6 | | 8 | | 4 |
| Case Number | 1.1 | 3.0 | 1.1 | 4.0 | | 9.0 | | 12.0 |
| Phase Duration, s | 7.0 | 57.9 | 12.6 | 63.5 | | 25.9 | | 13.6 |
| Change Period, (Y+R _c), s | 4.0 | 6.0 | 4.0 | 6.0 | | 6.0 | | 6.0 |
| Max Allow Headway (MAH), s | 4.0 | 0.0 | 4.0 | 0.0 | | 5.2 | | 5.2 |
| Queue Clearance Time (g _s), s | 2.7 | | 8.0 | | | 16.9 | | 4.9 |
| Green Extension Time (g _e), s | 0.0 | 0.0 | 0.6 | 0.0 | | 3.1 | | 0.3 |
| Phase Call Probability | 1.00 | | 1.00 | | | 1.00 | | 0.95 |
| Max Out Probability | 0.00 | | 0.00 | | | 0.40 | | 0.00 |

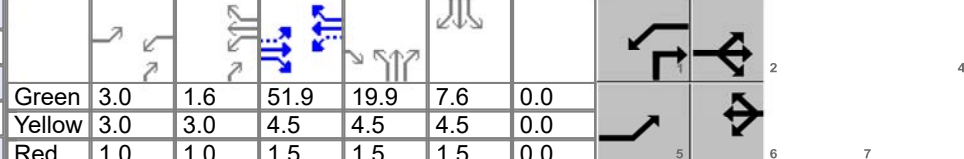
| Movement Group Results | EB | | | WB | | | NB | | | SB | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|---|-------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Assigned Movement | 5 | 2 | 12 | 1 | 6 | 16 | 3 | 8 | 18 | 7 | 4 | 14 |
| Adjusted Flow Rate (v), veh/h | 21 | 762 | 468 | 201 | 446 | 445 | 240 | 240 | 248 | 51 | | 46 |
| Adjusted Saturation Flow Rate (s), veh/h/ln | 1723 | 1885 | 1594 | 1792 | 1863 | 1858 | 1810 | 1816 | 1610 | 1865 | | 1775 |
| Queue Service Time (g _s), s | 0.7 | 11.3 | 15.9 | 6.0 | 11.7 | 11.8 | 13.8 | 13.7 | 14.9 | 2.9 | | 2.7 |
| Cycle Queue Clearance Time (g _c), s | 0.7 | 11.3 | 15.9 | 6.0 | 11.7 | 11.8 | 13.8 | 13.7 | 14.9 | 2.9 | | 2.7 |
| Green Ratio (g/C) | 0.50 | 0.47 | 0.65 | 0.57 | 0.52 | 0.52 | 0.18 | 0.18 | 0.26 | 0.07 | | 0.07 |
| Capacity (c), veh/h | 352 | 1778 | 1041 | 467 | 973 | 971 | 328 | 329 | 418 | 129 | | 122 |
| Volume-to-Capacity Ratio (X) | 0.060 | 0.429 | 0.450 | 0.431 | 0.459 | 0.459 | 0.732 | 0.728 | 0.595 | 0.396 | | 0.375 |
| Back of Queue (Q), ft/ln (95 th percentile) | 12.2 | 189.6 | 225.9 | 104.8 | 191 | 189.3 | 270.7 | 276 | 251 | 68.4 | | 58.8 |
| Back of Queue (Q), veh/ln (95 th percentile) | 0.5 | 7.5 | 9.0 | 4.2 | 7.5 | 7.6 | 10.8 | 10.8 | 10.0 | 2.6 | | 2.4 |
| Queue Storage Ratio (RQ) (95 th percentile) | 0.10 | 0.00 | 0.65 | 0.31 | 0.00 | 0.00 | 0.89 | 0.00 | 1.39 | 0.00 | | 0.00 |
| Uniform Delay (d ₁), s/veh | 14.3 | 12.9 | 9.4 | 12.7 | 9.7 | 9.8 | 42.5 | 42.5 | 35.7 | 49.0 | | 48.9 |
| Incremental Delay (d ₂), s/veh | 0.1 | 0.8 | 1.4 | 0.6 | 1.6 | 1.6 | 5.2 | 5.1 | 1.9 | 2.8 | | 2.7 |
| Initial Queue Delay (d ₃), s/veh | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 |
| Control Delay (d), s/veh | 14.4 | 13.6 | 10.8 | 13.3 | 11.3 | 11.4 | 47.8 | 47.5 | 37.6 | 51.8 | | 51.6 |
| Level of Service (LOS) | B | B | B | B | B | B | D | D | D | D | | D |
| Approach Delay, s/veh / LOS | 12.6 | | B | 11.7 | | B | 44.2 | | D | 51.7 | | D |
| Intersection Delay, s/veh / LOS | 20.8 | | | | | | C | | | | | |

| Multimodal Results | EB | | WB | | NB | | SB | |
|----------------------------|-----|---|-----|---|-----|---|-----|---|
| Pedestrian LOS Score / LOS | 2.8 | C | 2.3 | B | 2.9 | C | 3.0 | C |
| Bicycle LOS Score / LOS | 1.5 | A | 1.4 | A | 1.7 | A | 0.6 | A |

HCS 2010 Signalized Intersection Intermediate Values

| General Information | | | | Intersection Information | | | |
|---------------------|-----------------------------|---------------|---------------|-----------------------------|-----------------|----------|--|
| Agency | KLOA, Inc. | | | Duration, h | 0.25 | | |
| Analyst | NJB | Analysis Date | 1/24/2017 | Area Type | Other | | |
| Jurisdiction | DuPage County | | Time Period | SAT Midday | PHF | 0.95 | |
| Urban Street | 63rd Street | | Analysis Year | 2018 | Analysis Period | 1 > 7:00 | |
| Intersection | 63rd Street with Woodw... | | File Name | 63rd and Woodward SATFU.xus | | | |
| Project Description | Future SAT Midday Peak Hour | | | | | | |

| Demand Information | EB | | | WB | | | NB | | | SB | | |
|---------------------|----|-----|-----|-----|-----|---|-----|----|-----|----|----|----|
| Approach Movement | L | T | R | L | T | R | L | T | R | L | T | R |
| Demand (v), veh/h | 20 | 724 | 445 | 191 | 841 | 6 | 415 | 41 | 236 | 18 | 57 | 17 |

| Signal Information | | | |  | | | | | | | | | | | | | | | | | | | | |
|--------------------|-------|-----------------|-------|--|-----|-----|------|------|-----|-----|--------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| Cycle, s | 110.0 | Reference Phase | 2 | Green | 3.0 | 1.6 | 51.9 | 19.9 | 7.6 | 0.0 | Yellow | 3.0 | 3.0 | 4.5 | 4.5 | 4.5 | 0.0 | Red | 1.0 | 1.0 | 1.5 | 1.5 | 1.5 | 0.0 |
| Offset, s | 0 | Reference Point | Begin | | | | | | | | | | | | | | | | | | | | | |
| Uncoordinated | No | Simult. Gap E/W | On | | | | | | | | | | | | | | | | | | | | | |
| Force Mode | Fixed | Simult. Gap N/S | On | | | | | | | | | | | | | | | | | | | | | |

| Saturation Flow / Delay | EB | | | WB | | | NB | | | SB | | |
|--|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|-------|
| | L | T | R | L | T | R | L | T | R | L | T | R |
| Lane Width Adjustment Factor (f_w) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Heavy Vehicle Adjustment Factor (f_{HV}) | 0.952 | 0.990 | 0.990 | 0.990 | 0.980 | 1.000 | 1.000 | 0.971 | 1.000 | 0.943 | 1.000 | 1.000 |
| Approach Grade Adjustment Factor (f_g) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Parking Activity Adjustment Factor (f_p) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Bus Blockage Adjustment Factor (f_{bb}) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Area Type Adjustment Factor (f_a) | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Lane Utilization Adjustment Factor (f_{LU}) | 1.000 | 0.952 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 | 1.000 |
| Left-Turn Adjustment Factor (f_{LT}) | 0.952 | 0.000 | | 0.952 | 0.000 | | 0.952 | 0.000 | | 0.000 | 0.982 | |
| Right-Turn Adjustment Factor (f_{RT}) | | 0.000 | 0.847 | | 0.997 | 0.997 | | 0.000 | 0.847 | | 0.931 | 0.934 |
| Left-Turn Pedestrian Adjustment Factor (f_{LPB}) | 1.000 | | | 1.000 | | | 1.000 | | | 1.000 | | |
| Right-Turn Ped-Bike Adjustment Factor (f_{RPB}) | | | 1.000 | | | 1.000 | | | 1.000 | | | 1.000 |
| Movement Saturation Flow Rate (s), veh/h | 1723 | 3770 | 1594 | 1792 | 3694 | 26 | 1810 | 1816 | 1610 | 694 | 2255 | 692 |
| Proportion of Vehicles Arriving on Green (P) | 0.03 | 0.63 | 0.47 | 0.08 | 0.70 | 0.52 | 0.18 | 0.18 | 0.18 | 0.07 | 0.07 | 0.07 |
| Incremental Delay Factor (k) | 0.11 | 0.50 | 0.50 | 0.11 | 0.50 | 0.50 | 0.18 | 0.18 | 0.15 | 0.15 | | 0.15 |

| Signal Timing / Movement Groups | EBL | EBT/R | WBL | WBT/R | NBL | NBT/R | SBL | SBT/R |
|---|------|-------|------|-------|-----|-------|-----|-------|
| Lost Time (t_L) | 4.0 | 6.0 | 4.0 | 6.0 | | 6.0 | | 6.0 |
| Green Ratio (g/C) | 0.50 | 0.47 | 0.57 | 0.52 | | 0.18 | | 0.07 |
| Permitted Saturation Flow Rate (s_p), veh/h/ln | 604 | 0 | 708 | 0 | | 1810 | | 0 |
| Shared Saturation Flow Rate (s_{sh}), veh/h/ln | | | | | | | | |
| Permitted Effective Green Time (g_p), s | 51.9 | 0.0 | 53.9 | 0.0 | | 0.0 | | 0.0 |
| Permitted Service Time (g_u), s | 43.6 | 0.0 | 40.6 | 0.0 | | 0.0 | | 0.0 |
| Permitted Queue Service Time (g_{ps}), s | 0.3 | | 5.3 | | | | | |
| Time to First Blockage (g_t), s | 0.0 | 0.0 | 0.0 | 0.0 | | 0.0 | | 0.0 |
| Queue Service Time Before Blockage (g_{ts}), s | | | | | | | | |
| Protected Right Saturation Flow (s_R), veh/h/ln | | | 1594 | | | 1610 | | |
| Protected Right Effective Green Time (g_R), s | | | 19.9 | | | 8.6 | | |

| Multimodal | EB | | | WB | | | NB | | | SB | | |
|----------------------------------|--------|-------|---------|-------|-------|--------|-------|-------|--|----|--|--|
| Pedestrian F_w / F_v | 2.107 | 0.00 | 1.557 | 0.00 | 2.107 | 0.00 | 2.224 | 0.00 | | | | |
| Pedestrian F_s / F_{delay} | 0.000 | 0.110 | 0.000 | 0.101 | 0.000 | 0.166 | 0.000 | 0.155 | | | | |
| Pedestrian M_{corner} / M_{cw} | | | | | | | | | | | | |
| Bicycle c_b / d_b | 943.42 | 15.35 | 1045.00 | 12.54 | 62.22 | 137.91 | 47.68 | | | | | |
| Bicycle F_w / F_v | -3.64 | 1.03 | -3.64 | 0.90 | -3.64 | 1.20 | -3.64 | 0.08 | | | | |

--- Messages ---

WARNING: Since queue spillover from turn lanes and spillback into upstream intersections is not accounted for in the HCM procedures, use of a simulation tool may be advised in situations where the Queue Storage Ratio exceeds 1.0.

WARNING: The shared-plus-exclusive turn lane solution is an approximation of the HCM method, because more than three lane groups cannot be accommodated. Input data for Percent Turns in Shared Lane are used to specify proportion of turning vehicles in the shared lane.

--- Comments ---

Summary of Neighborhood Meeting
Meadowbrook Shopping Center, Downers Grove
Walgreens Development
November 28, 2017 7:00pm, Horizon Church, Meadowbrook Center

Attendance:

Owner: Perri Knight (project manager)

Residents/Tenants: No residents in attendance, 4 current tenants attended

Presentation materials included a site plan for Walgreens, a site improvement plan for overall center, renderings of Walgreens and façade, Walgreens provided statistics

Meeting began a few minutes after 7pm.

Questions related to timing of construction for façade improvements.

Answer: We are in for permit on the façade and hope to submit revised Walgreens plans for permit on 12/20. We hope to get approval from the village in February and start both WAG and facade in March.

Explained that due to the timing delay on getting the WAG approvals, WAG has renegotiated their lease/building and we are required to start the approval process over.

Questions related to roof replacements.

Answer: Unfortunately, do to the other aesthetic and ancillary improvements we are required to make, a full roof replacement is not in the budget.

Question re: Construction timing/staging

Answer: Perri committed to having another meeting with the tenants once we have approval on both projects to discuss staging for construction equipment as well as timelines for façade—e.g. starting at one end first and working our way down versus in the middle etc.

Question re: signage

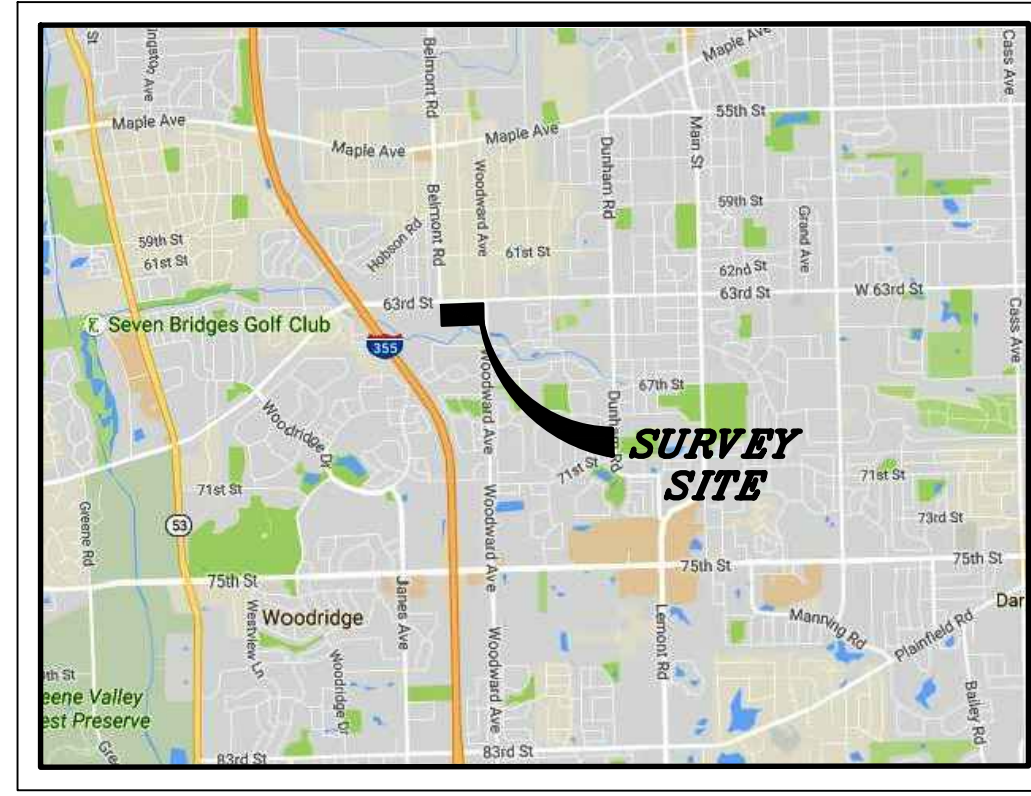
Answer: Perri met with sign company who is working on proposal for entire center to unify signage with channel letters in accordance with village signage ordinances. Costs yet to be determined as is decision whether we move forward with new signage or keep the old.

FINAL PLAT OF MEADOWBROOK ADDITION TO DOWNERS GROVE

BEING A SUBDIVISION OF PART OF THE NORTHEAST QUARTER OF SECTION 24, TOWNSHIP 38 NORTH, RANGE 10 EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOIS

PIN'S

08-24-202-005
08-24-202-008
08-24-202-009
08-24-203-004

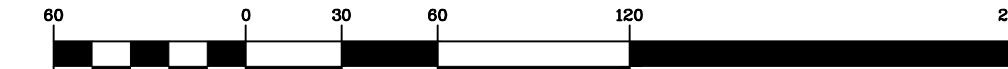


LOCATION MAP

NOT TO SCALE



GRAPHIC SCALE



(IN FEET)
1 inch = 60 ft.

BASIS OF BEARINGS

COORDINATES AND BEARINGS ARE BASED UPON THE ILLINOIS STATE PLANE COORDINATE SYSTEM, EAST ZONE (NAD 83), ADJUSTED TO GROUND VALUES, AS ESTABLISHED BY A REAL-TIME KINEMATIC (RTK) GLOBAL NAVIGATION SATELLITE SYSTEM (GNSS) UTILIZING THE TRIMBLE VRS NOW NETWORK.

SURVEY PREPARED FOR

FRONTLINE REAL ESTATE PARTNERS, LLC
707 SKOKIE BOULEVARD
NORTHBROOK, ILLINOIS 60062

SUBMITTED BY/RETURN TO:

THE VILLAGE OF DOWNERS GROVE
801 BURLINGTON AVENUE
DOWNERS GROVE, ILLINOIS 60515
PHONE: 630-434-5500

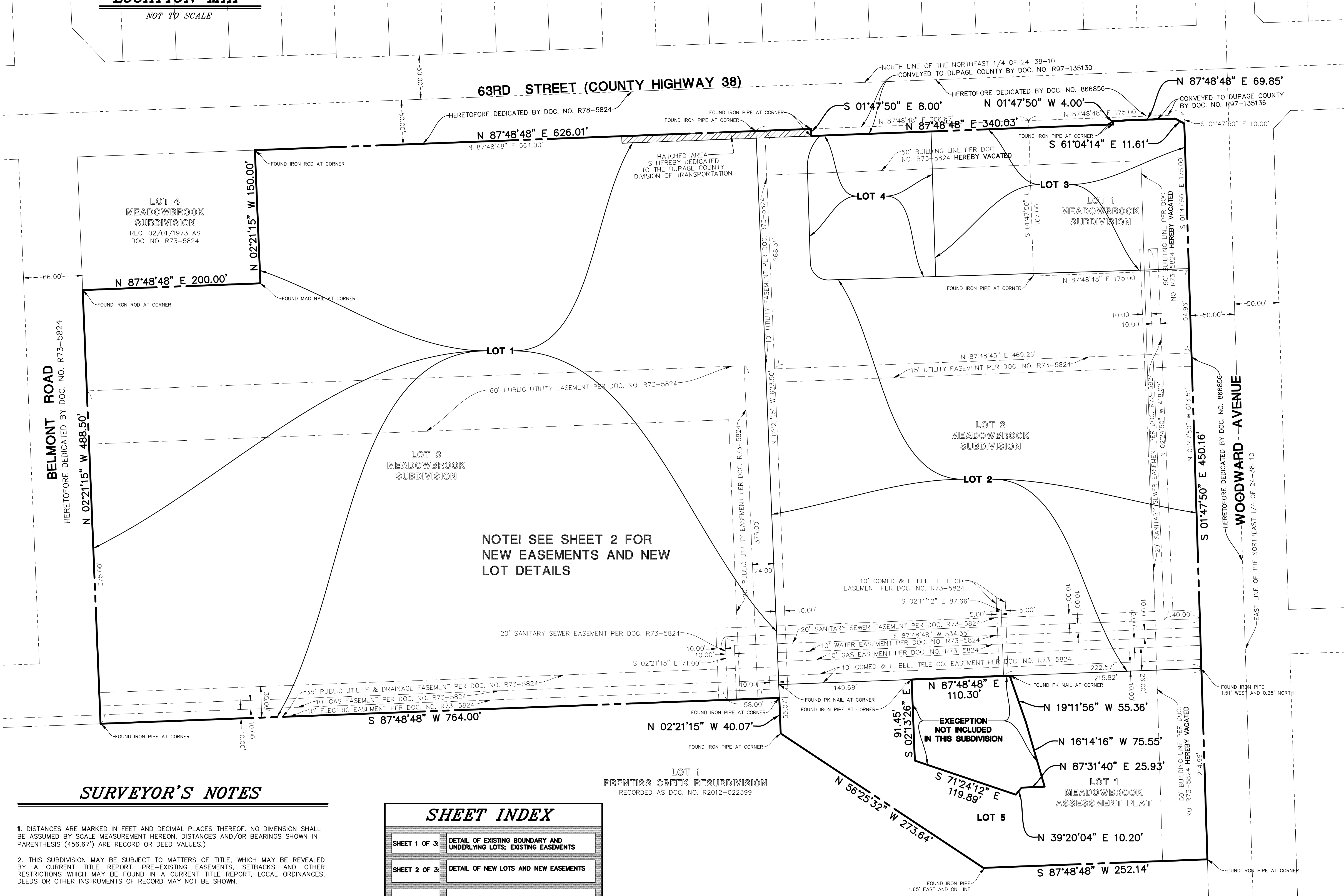
Table with columns for DATE and REVISIONS.

Manhard CONSULTING LTD.
700 Springer Drive, Lombard, IL 60148
Cell Engineers • Surveyors • Visual Resources Engineers • Water & Wastewater Engineers
Construction Managers • Environmental Scientists • Landscape Architects • Planners

MEADOWBROOK FIRST RESUBDIVISION
DOWNERS GROVE, ILLINOIS
FINAL PLAT OF RESUBDIVISION

PROJ. MOR: TP
PROJ. ASSOC.: WW
DRAWN BY: WW-CM
DATE: 01/18/18
SCALE: 1"=60'
SHEET 1 OF 3
FREDG

FINAL PLAT ISSUED FOR REVIEW 01/18/18



NOTE! SEE SHEET 2 FOR NEW EASEMENTS AND NEW LOT DETAILS

SURVEYOR'S NOTES

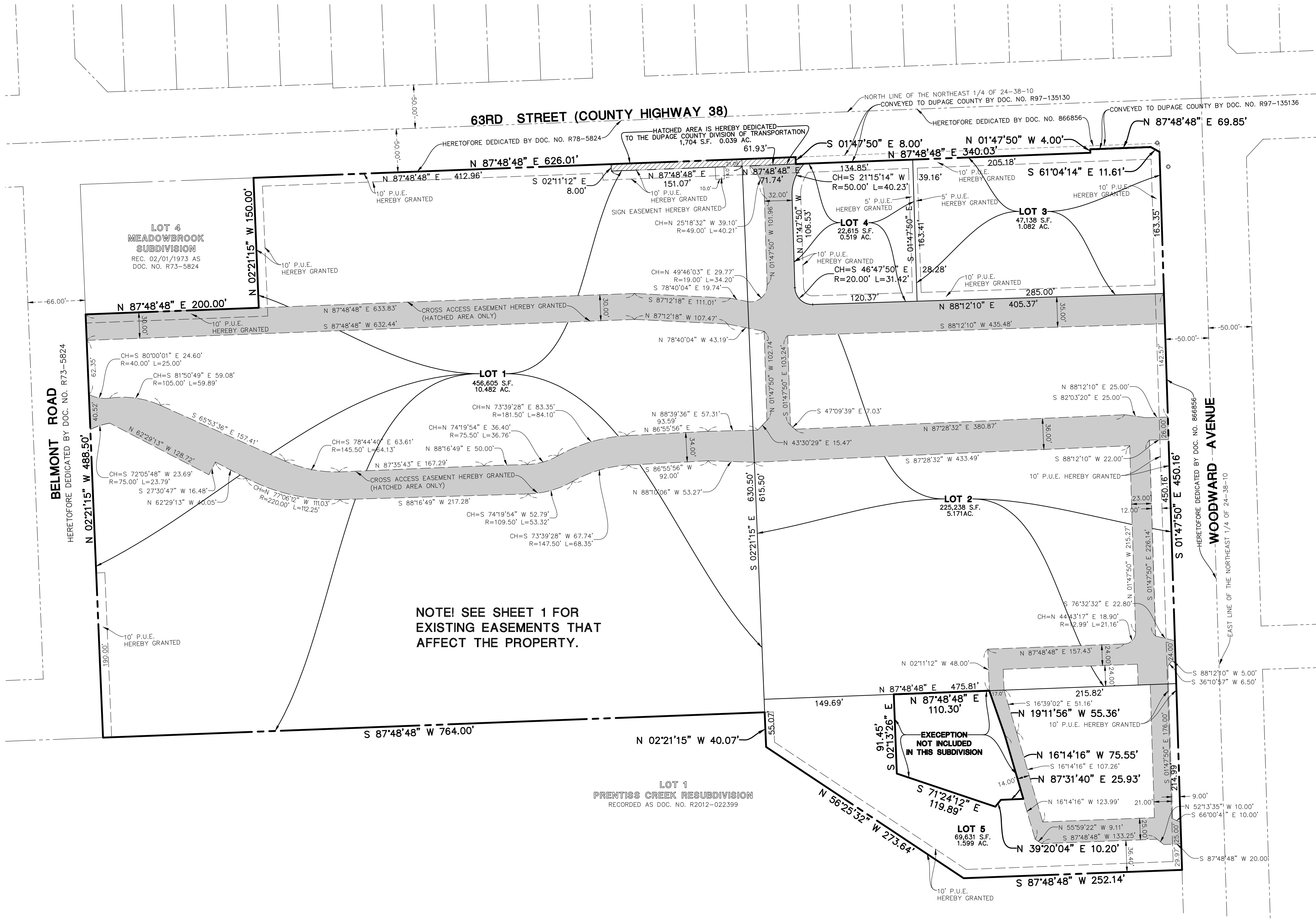
- 1. DISTANCES ARE MARKED IN FEET AND DECIMAL PLACES THEREOF. NO DIMENSION SHALL BE ASSUMED BY SCALE MEASUREMENT HEREON. DISTANCES AND/OR BEARINGS SHOWN IN PARENTHESIS (456.67) ARE RECORD OR DEED VALUES.
- 2. THIS SUBDIVISION MAY BE SUBJECT TO MATTERS OF TITLE, WHICH MAY BE REVEALED BY A CURRENT TITLE REPORT. PRE-EXISTING EASEMENTS, SETBACKS AND OTHER RESTRICTIONS WHICH MAY BE FOUND IN A CURRENT TITLE REPORT, LOCAL ORDINANCES, DEEDS OR OTHER INSTRUMENTS OF RECORD MAY NOT BE SHOWN.
- 3. THIS PROFESSIONAL SERVICE CONFORMS TO THE CURRENT ILLINOIS MINIMUM STANDARDS FOR A SUBDIVISION SURVEY. MANHARD CONSULTING, LTD. IS A PROFESSIONAL DESIGN FIRM, REGISTRATION NUMBER 184003350, EXPIRES APRIL 30, 2017.
- 4. 5/8" DIA. IRON RODS WILL BE SET AT ALL PROPERTY CORNERS UNLESS OTHERWISE NOTED.

SHEET INDEX

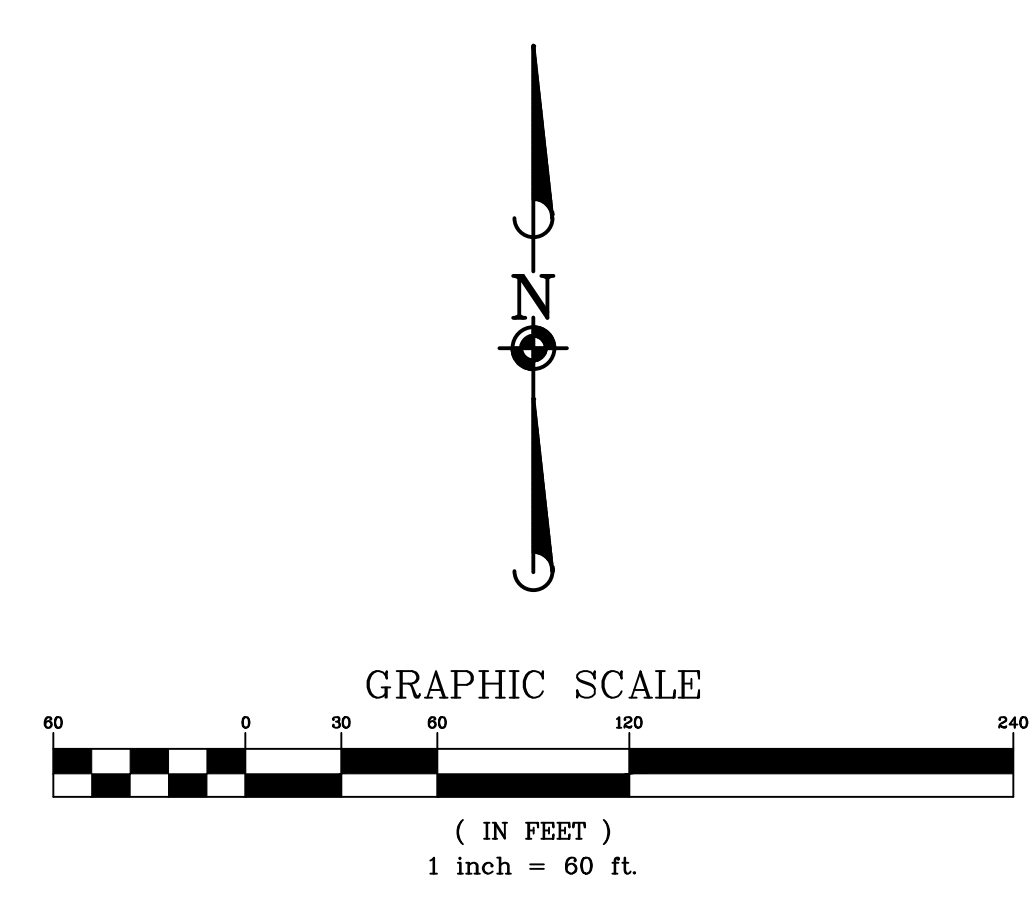
| | |
|---------------|---|
| SHEET 1 OF 3: | DETAIL OF EXISTING BOUNDARY AND UNDERLYING LOTS, EXISTING EASEMENTS |
| SHEET 2 OF 3: | DETAIL OF NEW LOTS AND NEW EASEMENTS |
| SHEET 3 OF 3: | CERTIFICATES; EASEMENT PROVISIONS |

FINAL PLAT OF MEADOWBROOK ADDITION TO DOWNERS GROVE

BEING A SUBDIVISION OF PART OF THE NORTHEAST QUARTER OF SECTION 24, TOWNSHIP 38
NORTH, RANGE 10 EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOIS



NOTE! SEE SHEET 1 FOR
EXISTING EASEMENTS THAT
AFFECT THE PROPERTY.



| DATE | REVISIONS |
|------|-----------|
| | |
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Manhard
CONSULTING LTD.
700 Springer Drive, Lombard, IL 60148
Cell Engineers - Surveyors - Water Resources - Vistas & Wetlands
Construction Managers - Environmental Scientists - Landscape Architects - Planners

MEADOWBROOK FIRST RESUBDIVISION
DOWNERS GROVE, ILLINOIS
FINAL PLAT OF RESUBDIVISION

| | |
|--------------|----------|
| PROJ. MGR. | SAS |
| PROJ. ASSOC. | WW |
| DRAWN BY | WW |
| DATE | 01/18/18 |
| SCALE | 1"=60' |

SHEET
2 OF **3**
FREDG

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FINAL PLAT ISSUED 01/18/18

January 18, 2018 - 09:09 - Day Name: P:\Fresh\Man\Survey\Drawings\Plat\Plat\2018\02-05.dwg - Plotted By: gwright

DRAFT PC MINUTES – 2/5/18

17-PLC-0041: A petition seeking approval of a Planned Unit Development Amendment to construct a new convenience goods store, a Special Use for a drive-through facility, and a Plat of Subdivision. The property is zoned B-2/PUD, General Retail Business/Planned Unit Development. The property is located at the southwest corner of 63rd Street and Woodward Avenue, commonly known as 2001 63rd Street, Downers Grove, IL (PINs 08-24-202-008, -009) FL Cedar, LLC, Petitioner and Owner.

Ms. Leitschuh said that a similar petition was before the Plan Commission in August of 2017 as Case #16-PLC-0062, and was referred to the Village Council with a positive recommendation, and subsequently approved by the Village Council. This PUD amendment would substitute some of the revised elements while maintaining the other previously established conditions including improvements to the Meadowbrook Shopping Center that are currently under review or have already been approved. This petition is focusing on the Walgreens area of the development and a new adjacent outlet. Previously approved conditions will be connected to this request as part of the PUD, unless something is rescinded.

Ms. Leitschuh displayed a plan showing the location of the proposed Walgreen's. The site has an existing vacant restaurant that will be demolished and a 10,500 square foot building will be constructed at the location. She reviewed the surrounding zoning. When the Petitioner was last before the Commission, the Comprehensive Plan was still under review; however, with the approval of the Comprehensive Plan this shopping center is now shown as mixed use for future consideration.

Ms. Leitschuh said the plan is substantially different in design from the original presentation. She provided comparison photos of the previous and present plans. This proposal creates two lots which includes a new outlet. Lot 3 will consist of 1.08 acres, Lot 4 will have 0.52 acres with a combined acreage of 1.6 acres. The shopping plaza is 18.86 acres. Lot 4 will be reserved for future commercial development. In the interim the pavement will be eliminated and that lot will be seeded to reduce the shopping center's overall impervious surface.

Regarding the Walgreen's building, it is proposed to be 10,500 square feet on Lot 3. The previous proposed building was 14,500 square feet on the western side of the lot with 66 parking spaces and a drive-thru on the western side. The loading, storage, trash area was located on the southern portion of the property. The current site plan is for a building reduced by 4,000 square feet, provides 43 parking spaces and a drive-thru located along the southern portion of the building. The loading area and trash enclosure are along the eastern wall. DuPage County said they would like a portion of the right-of-way dedicated to them because of the existence of a watermain at that location. The only nonconformities in the proposal are: the location of the storage and trash area, the setback of the drive-thru, and the pedestrian connection to Woodward Avenue. Staff noted that the location of the loading/trash area has a substantial amount of screening.

DRAFT PC MINUTES – 2/5/18

Ms. Leitschuh displayed elevation drawings for the current site plan. The facades are broken up by a light brown modern block face, a white smaller brick face, and a horizontal wood panel, all made of fiber cement board. She reviewed other design elements for the building. She pointed out that a condition of approval was included in the staff report relative to extending the EIFS overhang along the eastern wall because of its location adjacent to a major intersection. Walgreens is allowed to have signage as proposed, as it complies with the square-footage requirements of the sign ordinance, including a single tenant monument sign at the northwestern corner.

Regarding landscaping, 26% of the property will be open space, and 22% of the property is street yard open space. Technically only 5% is required. A total of 34 shade trees will be provided, 13 within the street yard, 12 in the interior islands or dividers, and 9 within the entrance aisle. They surpass the requirements. There will be substantial screening around the main corner, the dumpster enclosure and the loading area.

Ms. Leitschuh reviewed the traffic turning radius exhibits to explain that these were reviewed by the Fire Department to assure that all access requirements have been met. The drive-thru will be one-directional with a bypass lane. All requirements for the Subdivision Ordinance are met. They are reconfiguring four lots into five lots. She displayed how the lots are impacted. There is a newly created Lot 2, Lot 3 which is the Walgreens lot, and Lot 4 which is the outlot. She explained there is a reciprocal agreement between all these properties that they shall have continuous shared access between them. There is also the standard public utility and drainage easement that will be required on Lots 3 and Lot 4.

Ms. Leitschuh reviewed the Zoning Requirements as shown on Page 5 of Staff's report dated February 5, 2018. The street yard dumpster loading area provides adequate screening using physical walls and landscape screening. The setback between the drive-thru and interior lot line does not negatively impact any one aspect of the development. Regarding a pedestrian connection, Staff recommends that a condition be included to provide a pedestrian connection from Woodward Avenue across the southern property line of lot 3.

Ms. Leitschuh stated that under the Comprehensive Plan this area is identified as a mixed use, which is new for this area. It is a mix of land uses within a continuous geographic boundary, with the 63rd Street Focus Plan encouraging commercial expansion at key intersections and improving the vitality of aging shopping centers. This is a catalyst site for reinvestment with the uses potentially being expanded to include a mix of commercial and residential, although it does not have to be. It merely provides the opportunity for that type of mix. The criteria for a Planned Unit Development are met. She noted that Staff has recommended ten conditions for consideration in evaluating approval of the petition. The special use is specifically for the drive-thru and Staff finds that the drive-thru is an appropriate use and is placed appropriately on the site.

DRAFT PC MINUTES – 2/5/18

Ch. Rickard asked where on the site plan the pedestrian connection is located. Ms. Leitschuh showed that location.

Mr. Boyle (??) asked about the direction of the drive-thru. Ms. Leitschuh showed the travel path of the drive-thru. He asked whether there is screening at the exit onto Woodward, and Ms. Leitschuh said that they are planning dense evergreens at that exit point. Mr. Boyle asked about the shared access to the south of the buildings and whether there is a shared-access easement for the back of the property. Ms. Leitschuh said it was not part of the current proposal.

Ch. Rickard then called upon the Petitioner to make its presentation.

Perrine Knight, representing the owner said there were other members of their staff present to respond to specific questions. She reviewed their previous appearance before the Commission and Village Council. They have worked closely with Staff on the present plan before the Commission. She brought samples of the materials to be used in the construction of the building. Previous concerns about the EIFS product were addressed with a change of materials that is being widely used today in commercial construction. Many revisions were made based on Staff's recommendations for the location of the building. Ms. Knight said that the improvements to the shopping center are ready to proceed as soon as the construction of the Walgreens begins. The drive-thru location addresses concerns about lights disturbing residential areas.

Ch. Rickard asked whether the Petitioner is in agreement with Staff's condition regarding extending the EIFS, and its condition regarding the pedestrian connection. Ms. Knight said that Walgreens is in agreement with those conditions if they are required to obtain approval. She did note that the grading of the lot is very challenging and they would prefer to keep the trees if they can.

Mr. Boyle asked what the typical size is for a Walgreens and Ms. Knight replied that 10,500 square feet is their new standard store. Mr. Boyle said he thought the finishes were good and asked if this is a drastic change in materials for Walgreens.

John Bradshaw, architect for Walgreens, said this is not standard for Walgreens. He said that the entry is a new design as well because it is the most convenient spot for the handicap stalls. He said this may be the first location to introduce Walgreens' new design.

Steve Shanholtzer of Manhard Consulting responded to a question by Mr. Boyle concerning the proximity of the drive-thru, saying they added signage and a stop bar at the drive-thru and intersection for safety.

Mr. Kulovany asked about the height of the screening at the exit point. Mr. Shanholtzer said there are parkway trees required by the Ordinance. Screening is further north, so the only barrier is the curb between the access and Woodward.

DRAFT PC MINUTES – 2/5/18

Ms. Gassen said that overall she thought the changes showed great improvements. She appreciates all the concerns that were addressed by the Petitioner. Regarding the EIFS on the east façade, she would have no problem eliminating that as a condition for approval.

Ch. Rickard said he also doesn't believe it is necessary as a condition.

Mr. Kulovany said he appreciates the petitioner changing the materials which are much more durable and built to last.

Ms. Gassen then asked about Condition #9 and the grading issue that was alluded to earlier. Mr. Shanholtzer said that 63rd street is relatively high and then the site slopes down. They wanted to get the building as high as possible for better visibility by the public. The sidewalk along Woodward which also slopes downward could never be extended to the west and still be ADA compliant. ADA requires 5% as the maximum grade. The connection would exceed that grade. The original petition showed a connection along the northeast corner for connectivity for both sidewalks. If there was future development on Outlot 4 the walk could continue west. Ms. Gassen asked if there would be two on the north side, and whether there would be a connection point to the store. Mr. Shanholtzer then used the site plan to show how the connection would occur. He described the location of private and public sidewalks, and noted the amount of grade transition. Ms. Gassen said she wasn't sure whether they should keep that condition or not, because in driving that location she could see the grade changes.

Mr. Kulovany said he had conflicting thoughts on this regarding mixing pedestrians with the vehicular traffic. He noted also that Prentiss Creek's apartment complex is just south of there, and there might be residents of that complex who would prefer to walk to Walgreens and would need that access. Mr. Shanholtzer said he has no data re foot-traffic. They want to be sure that pedestrians cross at the safest point possible. They would encourage everyone to come to the front of the store and then cross over. Ms. Knight said they have worked with Staff on this but noted that it is difficult for them to be ADA compliant with the challenges of the site. They are working with what the location presents.

Ch. Rickard did not ask for public input, as there were no members of the public present.

Ch. Rickard said that with all the parking in the shopping center, everyone is walking through drive aisles to get to buildings. He thinks people will head through the parking lot. Mr. Kulovany agrees that people would cut across the parking area.

Mr. Quirk said he doesn't expect to see anyone walking to Walgreens. He doesn't think it makes a difference where the connection is located, and he thinks it will be underutilized. They are looking at a new configuration of the drive-thru and assurance that the plan meets the standards.

DRAFT PC MINUTES – 2/5/18

Mr. Maurer raised a question regarding semi-trailers making deliveries and he said he'd like to see how a semi-trailer can get in there without blocking the drive-thru. Ms. Knight replied that they did review that. Deliveries to Walgreens are once a week and last about an hour and a half. It is a limited window of inconvenience, but there should still be no access problem.

Ch. Rickard asked for closing comments from the Petitioner, and Ms. Knight thanked the Commission and appreciated their comments about the changes that were made.

Ch. Richard closed the public hearing.

Ch. Rickard noted that Staff feels all the standards have been met as documented in their report dated February 5, 2018. He asked if any Commissioners had a differing opinion and none did. Ch. Rickard then asked whether Conditions 6 and 9 are still thought to be necessary after previous discussion. No one expressed opposition to removing those conditions. Mr. Quirk raised a question about adding sidewalk for wheelchair people. Mr. Kulovany said he thought the shopping center would be a dangerous place to try and introduce pedestrians. He believes people will cut the corner. He is more concerned about mixing traffic and pedestrians.

Regarding improvements to the shopping center, Ms. Leitschuh said that this petition only rescinds things relevant to the specific site plan. Everything previously approved a year ago must be completed. Any changes made were related to the façade, but the Village Council made no real modifications to the plan at that time. She said the Petitioner would be held accountable to what was previously approved, including the overall improvements to the shopping center.

Ms. Gassen moved with regard to File 17-PLC-0041 that the Plan Commission forward a positive recommendation to the Village Council to approve this request for a PUD, Special Use and Plat of Subdivision subject to the conditions listed on Page 9 and 10 of Staff's February 5, 2018 report, with the exception of condition 6 related to the extension of the EIFS along the Woodward side, and condition 9 concerning the pedestrian connection from Woodward Avenue across the southern property line of lot 3. Mr. Quirk seconded the Motion.

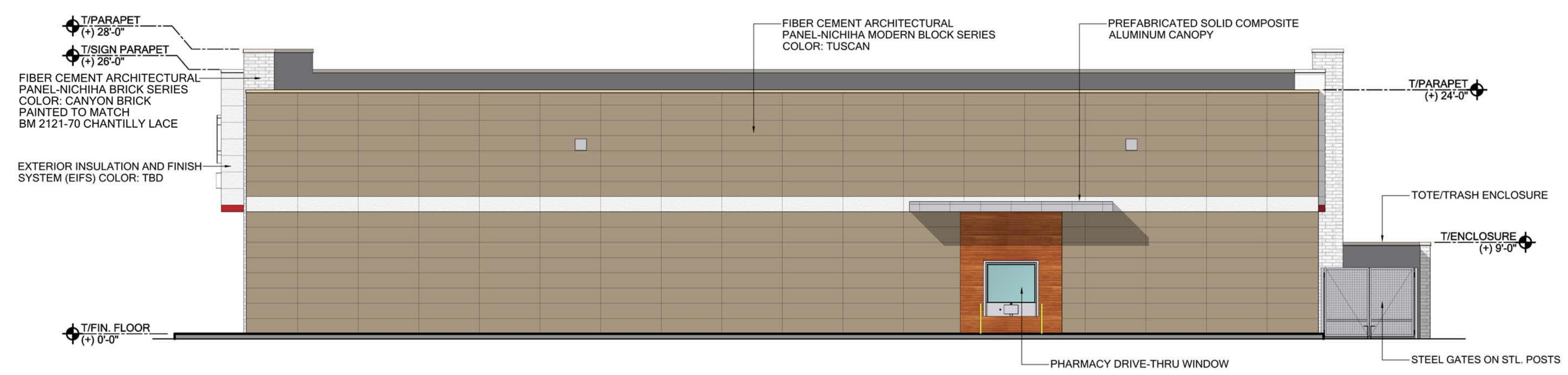
**AYES: Ms. Gassen, Mr. Quirk, Mr. Boyle, Mr. Kulovany, Mr. Maurer,
Ms. Rollins, Ch. Rickard**

NAYS: None

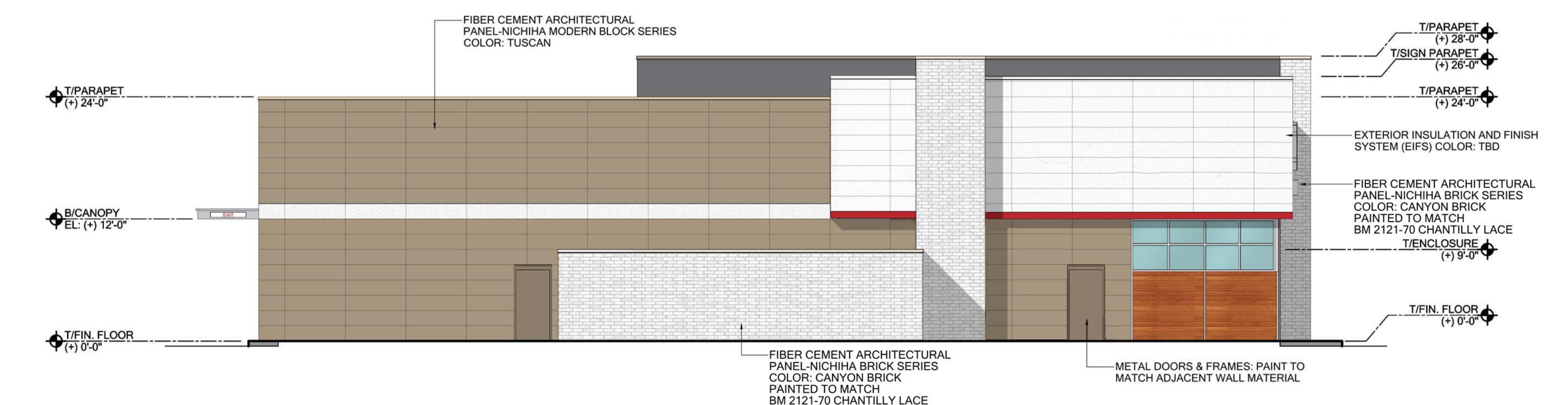
The Motion passed unanimously.



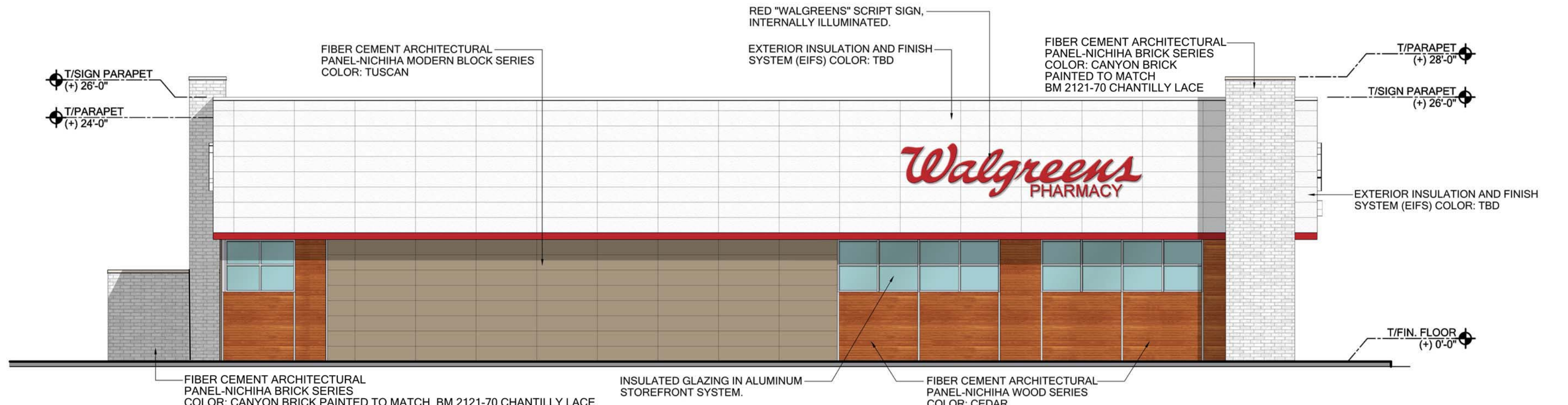
WEST ELEVATION



SOUTH ELEVATION



EAST ELEVATION

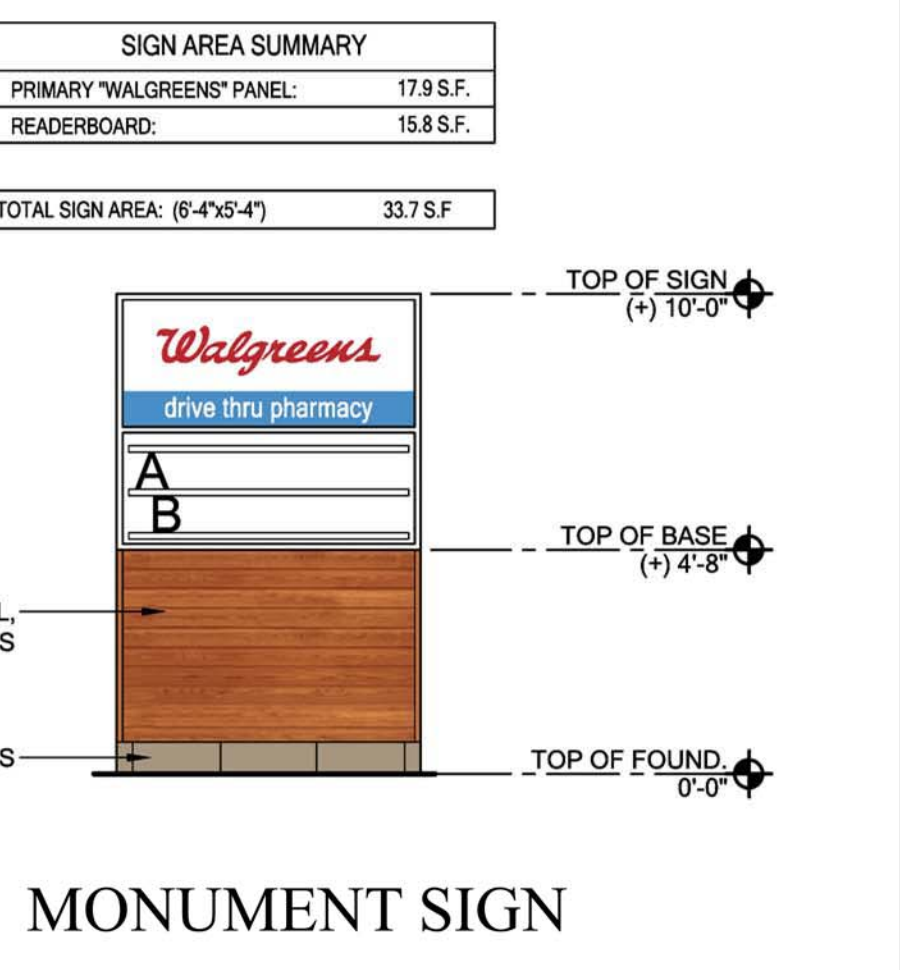


NORTH ELEVATION

SIGN SUMMARY

| WEST ELEVATION | |
|---|--------------------|
| 23'-8 1/2" SCRIPT LED LETTER SET W/ SECONDARY 14" PHARMACY LED LETTER SET | 124.89 S.F. |
| 3'-6" x 2'-11 1/8" LED SUSPENDED INTERIOR TOWER SIGN | 10.39 S.F. |
| "DRIVE-THRU" SIGN (6' x 3'-6") | 1.75 S.F. |
| NON-ILLUM. "CLEARANCE" SIGN (5' x 4'-0") | 1.64 S.F. |
| TOTAL | 138.67 S.F. |
| NORTH ELEVATION | |
| 23'-8 1/2" SCRIPT LED LETTER SET W/ SECONDARY 14" PHARMACY LED LETTER SET | 124.89 S.F. |
| TOTAL | 124.89 S.F. |
| EAST ELEVATION | |
| 23'-8 1/2" SCRIPT LED LETTER SET W/ SECONDARY 14" PHARMACY LED LETTER SET | 124.89 S.F. |
| "EXIT" SIGN (6' x 3'-6") | 1.75 S.F. |
| TOTAL | 126.64 S.F. |
| MONUMENT SIGN | |
| TOTAL SIGN AREA (6'-4" x 5'-4") | 33.7 S.F. |
| TOTAL | 67.4 S.F. |

WALL SIGNS:
THE TOTAL AREA OF WALL SIGNS AFFIXED TO A BUILDING WALL SHALL NOT EXCEED THE ALLOWABLE AREA. THE ALLOWABLE AREA IS BASED UPON THE FOLLOWING:
(1.5 SQUARE FEET PER LINEAR FOOT OF TENANT FRONTAGE) = MAX WALL SIGNAGE ALLOWED



MONUMENT SIGN

| REVISION | DATE | BY | APP'D |
|---------------------------|----------|----|-------|
| PLAN COMMISSION SUBMITTAL | 12-20-17 | AB | |

Manhard CONSULTING LTD.
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est. 1949

63RD STREET AND WOODWARD AVENUE
VILLAGE OF DOWNERS GROVE, ILLINOIS
EXTERIOR ELEVATIONS

PROJ. MGR.:
PROJ. ASSOC.:
DRAWN BY: AB
DATE: 12-20-17
SCALE: AS NOTED
SHEET
A-210