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VILLAGE OF DOWNERS GROVE Report for the Village 5/10/2022

| SUBJECT: | SUBMITTED BY: |
|--|--|
| Facilities - FRSP - Plat of Vacation, Plat of Subdivision, Planned Unit Development, Map Amendment and Special Use | Stan Popovich, AICP Director of Community Development |

SYNOPSIS

The Village of Downers Grove (Village), as the petitioner, is proposing to construct a combined Village Hall and Police Station facility (Downers Grove Civic Center (DGCC)) and a new telecommunications tower and demolish the existing Village Hall, Police Station and telecommunications tower. To facilitate the proposed project, the following approvals are required:

- Plat of Vacation Burlington Avenue
- Final Plat of Subdivision
- Planned Unit Development
- Zoning Map Amendment for DT (Downtown Transition) to DT/PUD (Downtown Transition/Planned Unit Development)
- Special Use Telecommunications Tower

STRATEGIC PLAN ALIGNMENT

The goals for 2021-2023 include Steward of Financial, Environmental and Neighborhood Sustainability, Exceptional Municipal Services, Top Quality Infrastructure, and Continual Innovation.

FISCAL IMPACT

N/A

UPDATE & RECOMMENDATION

Based on the Village Council discussion at the May 3, 2022 meeting, staff has prepared the following additional information about the telecommunications tower:

- An 8-foot tall PVC fence is planned to be installed around open areas of the equipment pads and the tower (see rendering below).
- A preliminary enhanced screening option for the southern property line between the Civic Center property and the residential properties to the south has been prepared by the landscape architect (see rendering below).
- The following table includes the proposed tower height and adjusted height if wireless communication services provider equipment and/or future capacity for new equipment were eliminated.

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| 9-1-1 Dispatch, Village Communications, Wireless Communications Service Providers and Future Capacity (proposed) | 199 feet |
|---|-------------------------------------|
| 9-1-1 Dispatch, Village Communications, Wireless Communications Service Providers (existing uses only/no future capacity) | Approx. 183 feet (same as existing) |
| 9-1-1 Dispatch and Village Communications (removal of wireless companies) | Approx. 183 feet (same as existing) |

- The tower will be designed and constructed as a Class III structure, which provides the greatest strength and durability to withstand most tornados, high winds and extreme weather conditions. Monopole towers generally withstand extreme conditions better than lattice towers.
- The proposed location continues to be the only viable location that 1) maintains fall zone separation from existing and proposed residential buildings, 2) adheres to the construction phasing plan and 3) satisfies all other site selection criteria (see FAQ attachment).
- The width of the existing lattice tower at ground level is 24 feet. The diameter of the proposed monopole tower at ground level is 7 feet.
- There are no restrictions that prevent property owners from constructing improvements on their property within the fall zone area of the proposed tower.
- Municipalities are preempted by federal law from considering environmental effects of radio frequency-related emissions in determining telecommunication tower locations. Requirements are established and enforced by the Federal Communications Commission (FCC). The Village will comply with all FCC rules and regulations.
- Camouflaging the tower is not recommended because doing so would:
 - o Substantially increase construction and maintenance costs
 - Create additional liability for the Village due to greater potential for the screening materials to fall from the tower and cause damage
 - o Potentially limit and impact the effectiveness of the type and amount of telecommunications equipment that can be placed on the tower

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BACKGROUND

Proposed Development Plan

An overview of the project can be found on the project summary page at www.downers/facilities. In summary, the Village is proposing the following project components for the development plan:

- Construct a new 79,000 square foot, two-story (with a basement) Downers Grove Civic Center (DGCC) that will house the offices of Village Hall, Police Station and School District 58 (District 58). The new DGCC will be located immediately west of the existing Police Station.
- Construct a new 199-foot tall telecommunications tower. The new telecommunications tower will be 270 feet east of the existing 183-foot telecommunications tower.
- Demolish the existing Village Hall, Police Station and telecommunications tower.
- Construct a 252 space vehicle parking lot for staff and visitors.
- Re-orient the Burlington Avenue and Washington Street intersection by eliminating the eastern leg of Burlington Avenue.
- Provide a pad ready development site for a private multi-family residential development.
- Provide the following environmentally sustainable features around the DGCC:
 - Solar panels
 - o Permeable pavers
 - o Enhanced mechanical systems
 - o Rain gardens and bioswales
 - o Rain harvesting
 - o Native landscaping
 - o Green roof

Required Approvals

To facilitate the proposed development, the following approvals are required:

- Plat of Vacation Burlington Avenue
 - The Burlington Avenue right-of-way must be vacated to facilitate the development as a portion of the DGCC building and parking lot improvements will be located within the existing Burlington Avenue right-of-way.
- Final Plat of Subdivision
 - O This will create three new lots of record.
 - Lot 1 DGCC lot
 - Lot 2 Pad ready future development lot
 - Lot 3 Existing fleet maintenance facility lot
- Planned Unit Development
 - Allows the entire site to be developed with a variety of uses with the following deviations from the VoDG Municipal Code
 - Parking Setback 4.4' from North Lot line instead of 10'
 - Telecommunications Tower Height 199' instead of 140'
 - Telecommunications Tower Setback 145' instead of 597'
- Zoning Map Amendment from DT (Downtown Transition) zoning district to DT/PUD (Downtown Transition/Planned Unit Development) zoning district

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o A zoning map amendment is required when a Planned Unit Development is granted. The map amendment changes the zoning of the property from the underlying zoning district (DT) to a designation with the PUD references (DT/PUD).

- Special Use Telecommunications Tower
 - A telecommunications tower is authorized as a special use in the Downtown Transition (DT) zoning district. The approval of the special use allows the tower to be located on the subject property.

Compliance with Village Right-of-Way Vacation Policy

The utility companies and the Village have no objections to vacating the Burlington Avenue right-of-way as new utility easements will be granted once the project is complete. Based on the proposed development, the Burlington Avenue right-of-way is no longer necessary for public use and the public interest will be served by vacating this right-of-way. The proposed vacation complies with Village policy #2003-58.

Compliance with the Subdivision Ordinance

The subject property currently consists of several lots of record. All three proposed lots comply with the Village's Subdivision Ordinance as shown in Table 1 of the Plan Commission Staff Report.

Compliance with the Zoning Ordinance

The subject property is zoned DT, Downtown Transition. The Zoning Ordinance (VoDG 28.5.010) identifies governmental facilities, such as the DGCC, as a permitted use and a telecommunications tower as an allowable Special Use. A discussion concerning the requested bulk zoning relief and compliance with the applicable bulk and parking requirements of the Zoning Ordinance are highlighted in Table 2 of the Plan Commission Staff Report. A narrative which identifies how the standards of approval for a Planned Unit Development, Zoning Map Amendment and Special Use have been met is provided in the Plan Commission Staff Report Attachments.

Compliance with Comprehensive Plan

As described in the Plan Commission Staff Report, the proposed development is compliant with the Comprehensive Plan. Specifically, the proposed development meets a vast majority of the Community Facilities recommendations found on pages 92 and 93 in the Comprehensive Plan. This includes recommendations that public buildings shall be catalyst sites and signature projects within the community, public buildings should be constructed so that they can be shared with other public bodies, such as the school district, and that the Village maintain emergency services.

Compliance with Downtown Design Guidelines

The recently updated Downtown Design Guidelines provide guidance for building and site design which will assist in creating a vibrant downtown. The proposed development meets the guidelines as demonstrated in Table 3 of the Plan Commission Staff Report.

Public Comment

During the Plan Commission public hearing, four members of the public provided comments on the project. The public spoke in opposition to the proposed relocation of the new telecommunications tower citing concerns about the location, height, setback, effect on property values and appearance.

As noted above, the existing 40 year old, 183-foot tall lattice telecommunications tower, used primarily for public safety and emergency dispatch communications, will be replaced with a new 199-foot monopole tower

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about 270 feet east of its current location. The new tower, which will have the same communication equipment as the existing tower, must be constructed on the Civic Center property to continue to provide:

- Emergency dispatch services to Downers Grove and nearby municipalities without a redesign of the regional dispatch communication network
- Residents and businesses in the neighborhood with wireless communication services.

The only location on the property that ensures that the tower will not fall on nearby residential buildings, the planned residential building and the police station is just east of the fleet maintenance garage.

Three wireless communications companies have equipment on the existing tower that provide services to residents and businesses in the area. These companies will continue to use the new tower. The Village encourages private wireless communication providers to use this tower to avoid the construction of multiple towers in neighborhoods near the Civic Center. The attached informational sheet provides additional information about the proposed relocation of the telecommunication tower.

ATTACHMENTS

Ordinances and Resolutions
Aerial Map
Telecommunication Tower Information
Staff Report with attachments dated April 4, 2022
Draft Minutes of the Plan Commission Hearing dated April 4, 2022

VILLAGE OF DOWNERS GROVE COUNCIL ACTION SUMMARY

| INITIATED: | Village Manager | DATE: _ | May 10, 2022 |
|-------------|---------------------------|-------------------------------------|---|
| | (Name) | | |
| RECOMMEND | | an Commission FI ard or Department) | LE REF: 22-PLC-0007 |
| NATURE OF A | CTION: | STEPS NEEDED | TO IMPLEMENT ACTION: |
| X Ordinanc | e | - | AN ORDINANCE AMENDING RDINANCE OF THE VILLAGE |
| Resolution | on . | OF DOWNERS G | ROVE, ILLINOIS TO E PROPERTY AT 700, 801 & 825 |
| Motion | | BURLINGTON A | ETROTERT AT 700, 801 & 823 EVENUE AND 842 CURTISS ERS GROVE CIVIC CENTER) |
| Other | | , | NIT DEVELOPMENT #66", as |
| | | presented. | CB . |
| SUMMARY OF | TITEM: | | |
| | on and 842 Curtiss as Pla | | to designate the property at 700, 801 ent #66 and authorize a governmenta |
| RECORD OF A | ACTION TAKEN: | | |
| | | | |
| | | | |
| | | | |

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| Downers Grove Civic Center PU | D#66 |
|-------------------------------|-------|
| 22-PLC | -0007 |

ORDINANCE NO. _____

AN ORDINANCE AMENDING THE ZONING ORDINANCE OF THE VILLAGE OF DOWNERS GROVE, ILLINOIS TO DESIGNATE THE PROPERTY AT 700, 801 & 825 BURLINGTON AVENUE AND 842 CURTISS STREET (DOWNERS GROVE CIVIC CENTER) AS PLANNED UNIT DEVELOPMENT #66

WHEREAS, the owner(s) of the property located directly east of the intersection of Burlington Avenue and Washington Street, commonly known as 700, 801 and 825 Burlington Avenue and 842 Curtiss Street, Downers Grove, IL (PINs 09-08-131-018, 09-08-131-019, 09-08-211-001, 09-08-211-002, 09-08-211-003, 09-08-211-014, 09-08-211-015, 09-08-211-016, 09-08-211-001, 09-08-304-008) (hereinafter referred to as the "Property" and legally described below) have requested that such real estate be designated as a Planned Unit Development to be known as "Downers Grove Civic Center Planned Unit Development #66" pursuant to the provisions of the Zoning Ordinance of the Village of Downers Grove, as set forth in Chapter 28 of the Downers Grove Municipal Code (hereinafter referred to as the "Zoning Ordinance"); and

WHEREAS, the owner(s) have also filed a written petition with the Village conforming to the requirements of the Zoning Ordinance and requesting approval of the Downers Grove Civic Center Planned Unit Development #66 as provided under the Zoning Ordinance; and,

WHEREAS, the Property is zoned "DT Downtown Transition District" pursuant to the Downers Grove Zoning Ordinance; and,

WHEREAS, the Plan Commission of the Village of Downers Grove has given the required public notice and has conducted a public hearing on April 4, 2022, respecting a governmental facility redevelopment plan for the Downers Grove Civic Center Planned Unit Development #66 on the Property in accordance with the statutes of the State of Illinois and the ordinances of the Village of Downers Grove and has reported its findings and recommendations to the Village Council of the Village of Downers Grove pursuant to the provisions of the Zoning Ordinance; and,

NOW, THEREFORE, BE IT ORDAINED by the Council of the Village of Downers Grove, in DuPage County, Illinois, as follows:

SECTION 1. That the provisions of the preamble are incorporated into this ordinance.

SECTION 2. The documents collectively referred to as "Downers Grove Civic Center planned unit development plans", are incorporated herein by reference as a part of this ordinance.

SECTION 3. That the Village Council hereby finds as follows:

- (1) That Planned Unit Development #66 meets the requirements of the Zoning Ordinance as follows:
 - a. the zoning map amendment review and approval criteria of Sec. 28.12.030.I;
 - b. the proposed PUD development plan and map amendment is consistent with the comprehensive plan and any other adopted plans for the subject area;

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c. the PUD development plan complies with the PUD overlay district provisions of Sec. 28.4.030;

- d. the proposed development will result in public benefits that are greater than or at least equal to those that would have resulted from development under conventional zoning regulations; and
- e. the appropriate terms and conditions have been imposed on the approval to protect the interests of surrounding property owners and residents, existing and future residents of the PUD and the general public.
- (2) That the proposed development conforms with the requirements of the Zoning Ordinance.

SECTION 4. The Zoning Ordinance is hereby amended by adding to the Zoning Map the boundaries of the following described real estate and by designating said real estate as a Planned Unit Development under the title and style "Downers Grove Civic Center Planned Unit Development #66" to be stated on the face of said map within the boundaries of the real estate hereinafter described, to wit:

PARCEL 1: LOTS 1 THROUGH 11, BOTH INCLUSIVE, AND THE WEST 11.3 FEET (AS MEASURED ON THE SOUTH LINE) BY 16.85 FEET (MEASURED ON THE NORTH LINE) OF LOT 12 IN STANLEY'S SUBDIVISION, ALSO THAT PART OF VACATED RAILROAD STREET LYING NORTH OF AND ADJOINING LOTS 1, 2, 3, 4 AND 5 IN SAID STANLEY'S SUBDIVISION, OF PART OF THE NORTHWEST QUARTER AND THE SOUTHWEST QUARTER OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO* THE* PLAT THEREOF RECORDED FEBRUARY 18, 1884 AS DOCUMENT 32879, IN DUPAGE COUNTY, ILLINOIS.

A PART OF LOT 12 OF THE PLAT OF STANLEY'S SUBDIVISION OF PART OF THE NORTHWEST QUARTER AND THE SOUTHWEST QUARTER OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11 EAST OF THE THIRD PRINCIPAL MERIDIAN, AS SHOWN BY THE PLAT RECORDED IN BOOK OF PLATS, ON PAGE 43, AND ALSO A PART OF LOT 1 OF THE PLAT OF THE HOMESTEAD OF THOMAS LYMAN, DECEASED, AS RECORDED IN BOOK 6 OF PLATS, ON PAGE 5, OF THE RECORDS OF DUPAGE COUNTY, ILLINOIS; THE ENTIRE TRACT HEREIN INCLUDED, BEING DESCRIBED AS FOLLOWS, TO WIT: COMMENCING AT THE SOUTHWEST CORNER OF SAID LOT 12, OF STANLEY'S SUBDIVISION, AND RUNNING THENCE EAST ALONG THE NORTH LINE OF CURTISS STREET, 11.3 FEET FOR A PLACE OF BEGINNING; THENCE NORTHERLY TO A POINT ON THE NORTH LINE OF SAID LOT 12, WHICH POINT IS 16.85 FEET EAST FROM THE NORTHWEST CORNER OF SAID LOT; THENCE EAST, ON THE NORTH LINE OF SAID LOT 12, 53.75 FEET TO THE NORTHEAST CORNER OF SAID LOT, THENCE SOUTHERLY, TO A POINT ON THE NORTH LINE OF CURTISS STREET WHICH IS 7 FEET EAST FROM THE SOUTHEAST CORNER OF SAID LOT 12; THENCE WESTERLY ALONG THE NORTH LINE OF CURTISS STREET, 50 FEET TO THE PLACE OF BEGINNING, IN THE VILLAGE OF DOWNERS GROVE, DUPAGE COUNTY, ILLINOIS.

PARCEL 2: LOTS 1-6 AND THE EAST 5 1/2 FEET OF LOT 7 IN RANDALL'S RESUBDIVISION OF BLOCK 2 IN THE TOWN OF DOWNERS GROVE, IN THE WEST HALF OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO THE PLAT OF SAID RANDALL'S RESUBDIVISION RECORDED OCTOBER 26, 1875 AS DOCUMENT 20748, IN DUPAGE COUNTY, ILLINOIS.

PARCEL 3: THE NORTH 35 FEET (AS MEASURED AT RIGHT ANGLES TO THE NORTHEASTERLY LINE OF SAID LOT) OF LOT 6, IN WALLACE'S SUBDIVISION OF PART OF LOT 5 OF THE ASSESSOR'S SUBDIVISION OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOIS.

PARCEL 4: THE NORTH 35 FEET (AS MEASURED AT RIGHT ANGLES TO THE NORTHERLY LINE OF SAID LOT) OF LOT 7, IN WALLACE'S SUBDIVISION OF PART OF LOT 5 OF THE ASSESSOR'S SUBDIVISION OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOIS.

PARCEL 5: LOT 1 OF THE PLAT OF THE HOMESTEAD OF THOMAS LYMAN, DECEASED (EXCEPT THAT PART CONVEYED TO THE CHICAGO BURLINGTON & QUINCY RAILROAD AND EXCEPT THAT PART CONVEYED TO H.J. HAWKINS ON MARCH 27, 1907) IN SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOIS.

PARCEL 6: SO MUCH OF THE SOUTHWEST QUARTER OF THE NORTHEAST QUARTER (SW1/2 NE1/2) OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11 EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOIS, DESCRIBED AS FOLLOWS:

COMMENCING AT A POINT ON THE SOUTH LINE OF 100-FOOT WIDE RIGHT OF WAY OF BURLINGTON NORTHERN, INC., 336.6 FEET EASTWARD (MEASURED ON SAID SOUTH LINE) FROM THE INTERSECTION OF THE NORTH AND SOUTH CENTER LINE OF SAID SECTION 8 WITH SAID SOUTH LINE TO THE PLACE OF BEGINNING; THENCE SOUTH 9 DEGREES WEST (MAGNETIC BEARING), 20.14 FEET; THENCE EASTWARD, PARALLEL TO SAID SOUTH RIGHT OF WAY LINE AND 20 FEET DISTANT THEREFROM, 425.9 FEET; THENCE NORTH 64 DEGREES EAST (MAGNETIC BEARING), 42.3 FEET TO SAID SOUTH RIGHT OF WAY LINE; THENCE WESTWARD ON SAID SOUTH RIGHT OF WAY LINE, 459.5 FEET TO THE PLACE OF BEGINNING.

PARCEL 7: LOTS 3, 4, 5 AND LOT 6 (EXCEPT THE WEST 50 FEET THEREOF) IN BLOCK 2 IN CURTISS ADDITION TO THE TOWN OF DOWNERS GROVE, BEING A SUBDIVISION OF PART OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO THE PLAT THEREOF RECORDED MARCH 8, 1866 AS DOCUMENT 7317, IN DUPAGE COUNTY, ILLINOIS.

PARCEL 8: LOTS 1 AND 2 IN BLOCK 2 IN CURTISS ADDITION TO THE TOWNS OF DOWNERS GROVE, BEING A SUBDIVISION OF PART OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO

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THE PLAT THEREOF RECORDED MARCH 8, 1866 AS DOCUMENT 7317, IN DUPAGE COUNTY, ILLINOIS.

BEGINNING AT THE NORTHEAST CORNER OF LOT 6 IN SAID STANLEY'S SUBDIVISION, ALSO BEING THE SOUTHWEST CORNER OF BURLINGTON AVENUE (RAILROAD STREET), VACATED PER DOCUMENT 451892; THENCE SOUTH 89 DEGREES 37 MINUTES 19 SECONDS WEST ALONG THE NORTH LINE OF SAID LOT 6 AND ALONG THE NORTH LINE OF LOTS 1 THROUGH 7, BOTH INCLUSIVE, IN SAID RANDALL'S RESUBDIVISION, 360.81 FEET TO THE WEST LINE OF THE EAST 5 1/2 FEET OF SAID LOT 7; THENCE NORTH 37 DEGREES 41 MINUTES 31 SECONDS EAST, 63.51 FEET TO THE SOUTH LINE OF THE BURLINGTON NORTHERN AND SANTA FE RAILROAD; THENCE NORTH 89 DEGREES 37 MINUTES 19 SECONDS EAST ALONG SAID SOUTH LINE, 319.20 FEET TO THE WEST LINE OF VACATED BURLINGTON AVENUE (RAILROAD STREET) RECORDED AS DOCUMENT NUMBER 451892; THENCE SOUTH 03 DEGREES 10 MINUTES 34 SECONDS EAST ALONG SAID WEST LINE, 50.06 FEET TO THE POINT OF BEGINNING, IN DUPAGE COUNTY, ILLINOIS.

Commonly known as: 700, 801 & 825 Burlington Avenue and 842 Curtiss Street

PINs: 09-08-131-018, 09-08-131-019, 09-08-211-001, 09-08-211-002, 09-08-211-003, 09-08-211-014, 09-08-211-015, 09-08-211-016, 09-08-211-001, 09-08-304-008

<u>SECTION 5</u>. The Downers Grove Civic Center Planned Unit Development #66 plans are hereby approved to permit a Planned Unit Development authorizing a governmental facility redevelopment plan, subject to the conditions and restrictions contained therein, and subject to the following:

- 1. The Special Use, Planned Unit Development and Map Amendment shall substantially conform to the staff report dated April 4, 2022, renderings, architecture plans, landscaping plans and engineering plans prepared by fgma architects, dated through March 25, 2022, and traffic plans prepared by KLOA dated March 25, 2022 except as such plans may be modified to conform to the Village codes and ordinances.
- 2. The petitioner shall subdivide the lot into the three lots of record pursuant to Section 20.5 of the Subdivision Ordinance prior to the issuance of any site development or building permits.
- 3. A development application for Lot 2 shall be submitted to the Plan Commission as a PUD Amendment, and along with any other required entitlements, with final approval by the Downers Grove Village Council.
- 4. A plat of easement is provided for public utility purposes and recorded with DuPage County once infrastructure is constructed.
- 5. A plat of easement is provided for telecommunication purposes and recorded with DuPage County once telecommunications equipment is constructed.
- 6. A plat of easement is provided for stormwater purposes and must be recorded with DuPage County once infrastructure is constructed.

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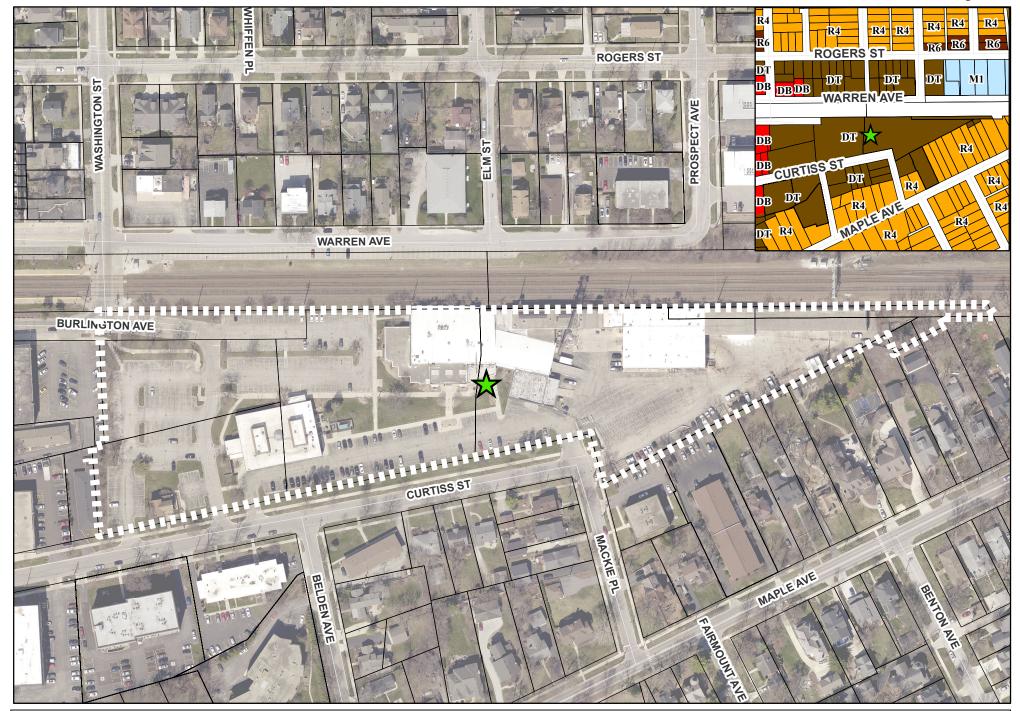
<u>SECTION 6</u>. That all ordinances or resolutions, or parts thereof, in conflict with the provisions of this ordinance be and are hereby repealed.

<u>SECTION 7</u>. That this ordinance shall be in full force and effect from and after its passage and publication in the manner provided by law.

| | Mayor |
|---------------|-------|
| Passed: | |
| Published: | |
| Attest: | |
| Village Clerk | |

 $1\\mw\\ord.22\\PUD\#66-EstVH-22-PLC-0007$

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N 0 100 200 Feet

Subject Property

★ Project Location

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Telecommunications Tower Relocation Information

As part of the construction of the new combined Police Station and Village Hall, the existing 40 year old, 183 foot tall telecommunications tower used primarily for public safety and emergency dispatch communications will be replaced with a new monopole tower about 270 feet east of its current location. The new 199 foot tall tower, which will have the same communication equipment as the existing tower, must be constructed on the Civic Center property to:

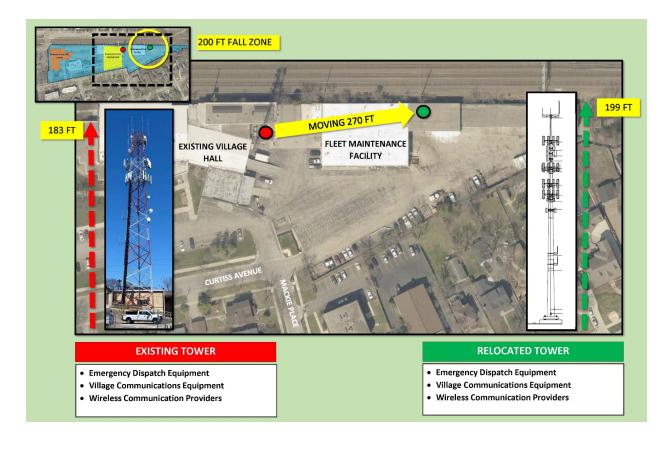
- Continue to provide emergency dispatch services to Downers Grove and nearby municipalities without a redesign of the regional dispatch communication network
- Continue to provide residents and businesses in the neighborhood with wireless communication services

The only location on the property that ensures the tower won't fall on nearby residential buildings, the planned residential building and the police station is just east of the fleet maintenance garage.

Three wireless communications companies have equipment on the existing tower that provide services to residents and businesses in the area. These companies will continue to use the new tower. The Village encourages wireless communication providers to use this tower to avoid the construction of multiple towers in neighborhoods near the Civic Center. If you would like to learn more, please see the additional information below.

- Plan Commission Presentation Slides
- Plan Commission Staff Report

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Telecommunications Tower FAQ - Updated

Questions Introduced at or following the Plan Commission Public Hearing

1. What is the site selection criteria that was used to determine the new tower site?

The telecommunications tower needs to be:

- In close proximity to the new Civic Center Facility, since the equipment on the tower needs to have a hard-wired connection to the equipment in the Police Station. The length of the connection should be minimized, so as to reduce possible failure of the connection.
- In close proximity to existing telecommunications tower to:
 - Continue to provide emergency dispatch services to Downers Grove and nearby municipalities without a redesign of the regional dispatch communication network
 - Continue to provide residents and businesses in the neighborhood with wireless communication services
- 200 feet or more from existing or proposed residential buildings and the proposed Police Station to ensure that, if the tower collapses, it won't fall on these buildings.
- Located on Village-owned property so that it can be constructed in a timely manner on land that the Village controls.
- Constructed along the railroad tracks so that the telecommunications equipment provides services demanded by commuter train riders and reduces the need for the installation of small-cell wireless equipment in nearby residential neighborhoods.

2. Were other sites considered?

Yes, and it was determined that there were no other sites that satisfied the above criteria.

3. Why does the Village host private telecommunications equipment?

Private telecommunications vendors are encouraged to locate on Village telecommunications towers to:

- Minimize the total number of telecommunications towers in the Village;
- Promote the joint use of new and existing telecommunications tower sites as a primary option rather than construction of additional single-use telecommunications towers;
- Encourage users of telecommunications towers to locate them in areas where the adverse impact on the community is minimal;
- Encourage users of telecommunications towers to design and configure them in a way that minimizes the adverse visual impact of the telecommunications towers;

- Avoid potential damage to adjacent properties from telecommunications tower failure through engineering and careful siting of telecommunications tower structures.
- 4. Why does the Village collect revenue from private telecommunications providers?

Collecting revenue from private telecommunications providers represents one of the Village's many diverse revenue streams. This reduces the tax burden on property owners and allows for essential Village services to be provided that may otherwise have to be funded with increased property taxes.

5. What is the expected revenue from the private leases on the existing tower?

Leasing terms for each of the private vendors that maintain equipment on the tower are described in the approved contracts, available at the following links:

- Verizon Wireless
- Cingular Wireless (AT&T)
- McKay Brothers
- 6. Will the Village consider options to enhance screening along the property line between the fleet maintenance property and residential properties to the south?

Yes, if directed by the Village Council.

Original Questions Included in Plan Commission Packet

1. Why is there a telecommunications tower on the site and when was it constructed?

The telecommunications tower provides required public safety (police and fire) communications, internally and with the dispatch center. The existing tower was constructed in the early 1980's.

2. What else is it used for?

In addition to the support of public safety communications, the tower also provides for wireless data & internet connections between the Village Hall and the Public Works facility. Three private telecommunications providers also access the tower for their equipment and make lease payments to the Village in exchange for this space. This practice of co-location is intended to result in fewer telecommunications towers within the Village.

3. Why is the proposed tower 190' (199' to the highest antenna)?

The height of the proposed tower was established to accommodate existing users and future users on a monopole structure. Additional tower capacity that allows for co-location of future telecommunications providers reduces the likelihood that new cellular communications towers will be constructed in close proximity to the proposed tower. The monopole design provides a narrower configuration and footprint, as opposed to the existing tower's wider lattice structure. This design allows for more vertical space to accommodate current and future equipment.

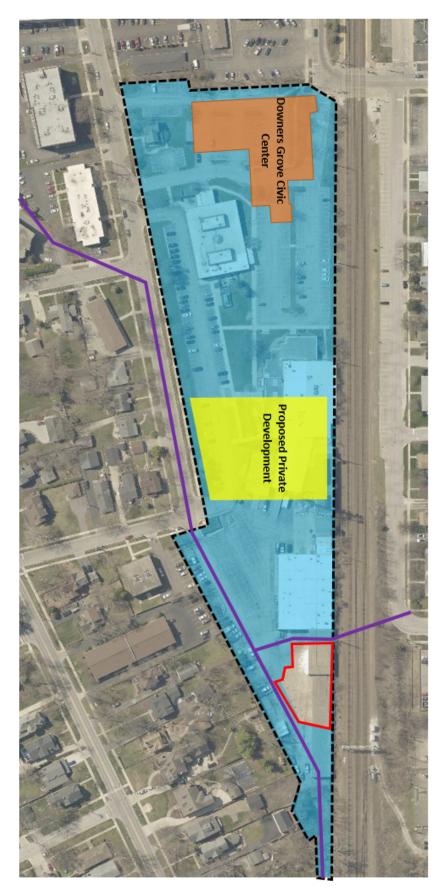
4. Why does the tower have to be relocated to the proposed site?

The proposed site is the only location on the existing Civic Center property that maintains a 200' fall-zone separation from nearby residential structures and from public safety operations in the new facility. This is to ensure that if the tower falls, it doesn't impact a residential structure or public safety facility. The proposed location also maintains access to the fleet maintenance garage and parking lot to the south. See attached tower siting location map.

5. Why can the tower not be relocated off site?

The proposed location in close proximity to the Police Station/Village Hall is critical to support public safety dispatching communications that the Village relies on for police, fire and EMS services. In addition, off-site relocation may result in construction of additional towers in the immediate vicinity by private telecommunications providers as allowed by the Federal Telecommunications Act.

Telecommunication Tower Location Map Analysis



Within 200 Feet of DGCC or residential structures

Available Location

Storm Sewer

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VILLAGE OF DOWNERS GROVE REPORT FOR THE PLAN COMMISSION APRIL 4, 2022 AGENDA

| SUBJECT: | TYPE: | SUBMITTED BY: |
|---|---|--|
| 22-PLC-0007 700 Burlington Avenue, 801 Burlington Avenue, 825 Burlington Avenue and 842 Curtiss Street | Plat of Vacation, Plat of Subdivision, Planned Unit Development, Map Amendment and Special Use | Jason Zawila, AICP Planning Manager |

REQUEST

The Village of Downers Grove (Village), as the petitioner, is requesting approval of the following items:

- Plat of Vacation
- Final Plat of Subdivision
- Planned Unit Development
- Map Amendment for DT (Downtown Transition) to DT/PUD (Downtown Transition/Planned Unit Development)
- Special Use

These approvals would permit the construction of a combined Village Hall and Police Station facility (Downers Grove Civic Center (DGCC)) and relocation of a telecommunication tower. The subject property is located at the southeast corner of Burlington Avenue and Washington Street, commonly known as 700 Burlington Avenue, 801 Burlington Avenue, 825 Burlington Avenue and 842 Curtiss Street.

NOTICE

The application has been filed in conformance with applicable procedural and public notice requirements.

GENERAL INFORMATION

OWNER/PETITIONER: Village of Downers Grove

801 Burlington Avenue Downers Grove, IL 60515

PROPERTY INFORMATION

EXISTING ZONING: DT, Downtown Transition

EXISTING LAND USE: Various Governmental Facilities including the Village Hall and Police Station

PROPERTY SIZE: 8.42 acres (366,893 square feet)

PINS: 09-08-131-018, 09-08-813-019, 09-08-211-002, 09-08-211-003, 09-08-211-014,

09-08-211-015, 09-08-211-016, 09-08-211-001, 09-08-304-008

SURROUNDING ZONING AND LAND USES

ZONING FUTURE LAND USE

NORTH: DB, Downtown Business Downtown (Business)
DT, Downtown Transition Downtown (Business)

M-1, Light Manufacturing Low-Intensity Office

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SOUTH: DT, Downtown Transition Institutional/Downtown (Transition)

EAST: DT, Downtown Transition Downtown (Transition)
R-4, Residential Detached House 4 Downtown Transition

WEST: DB, Downtown Business Downtown (Business)

ANALYSIS

SUBMITTALS

This report is based on the following documents, which are on file with the Department of Community Development:

- 1. Application/Petition for Public Hearing
- 2. Location Map
- 3. Project Narrative and Findings Response
- 4. Plat of Survey
- 5. Plat of Vacation Burlington Avenue
- 6. Final Plat of Subdivision
- 7. Downers Grove Sanitary approval
- 8. Site Plan
- 9. Engineering Site Plan
- 10. Floor Plan Combined Village Hall and Police Station
- 11. Building Elevations Combined Village Hall and Police Station
- 12. Landscape Plan
- 13. Signage Location Plan
- 14. Traffic Study
- 15. Wireless Communication Tower Plans
- 16. Washington Street Improvement Plan
- 17. Phasing Plan
- 18. Neighborhood Meeting Summary

PROJECT DESCRIPTION

The petitioner, the Village of Downers Grove (Village), is proposing to construct an approximate 79,000 square foot Downers Grove Civic Center (DGCC) building that will house the offices of Village Hall, Police Station and School District 58 (District 58), in addition to removing the existing telecommunication tower and constructing a new telecommunication tower. The DGCC project requires the following approvals:

- Plat of Vacation Burlington Avenue Right-of-Way
- Final Plat of Subdivision Three (3) Lots
- Final Planned Unit Development
- Zoning Map Amendment from DT to DT / PUD
- Special Use Telecommunications Tower

The subject property is made up of several lots of record. The subject property is currently zoned Downtown Transition (DT) and is currently improved with Village Hall that was purchased in 1968, and at that time it was anticipated to have a 20-year lifespan. The Village's Police Station, constructed in 1979, is in need of substantial system replacement and does not meet the operational requirements of the Department. A converted single family house sits on the southwest corner of the subject property. The site also includes the Village's fleet management facility and a telecommunication tower located immediately east of Village Hall.

The Village is proposing to subdivide the property into three lots of record. In conjunction with the

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subdivision, the Village will need to vacate the Burlington Avenue right-of-way to accommodate the proposed redevelopment of the site. The following is a summary of the proposed development:

DGCC (Lot 1)

On Lot 1 the Village is proposing the construction of a two story building, with a basement, that will provide for modern work spaces that will allow for efficient and effective interactions among employees and customers. The new building will be constructed immediately west of the existing Police Station and Village Hall buildings in the existing commuter parking lot (Lot L). The new facility will include two distinct lobbies, one for current Village Hall and District 58 services; and a second lobby for the Police services. The new Council Chambers will be located off of the Village Hall lobby and can be used for a variety of public meetings and community events.

The building and site design will adhere to the recently adopted Downtown Design Guidelines (further discussed below). While instantly recognizable as a government facility, the building will use traditional materials and reflect the familiar aesthetics of our Downtown to welcome the community. The site will include public plazas, pedestrian walkways and native landscaping. On-site parking will also be included for public, employee and official Village vehicles. Further detailed below, the proposed plan for the Washington Street intersection will eliminate the eastern leg of the intersection, streamlining the traffic flow and improving pedestrian walkways and bicycle connections.

A variety of environmental sustainable features are planned to be integrated into the project. As directed by the Village Council, integration of environmental sustainable design has been a critical foundation to the planning process for the DGCC. The proposed sustainable features will include: solar panels, permeable pavers, enhanced mechanical systems, rain gardens and bioswales, native landscaping, rain harvesting and the installation of a green roof.

Private Development (Lot 2)

In the middle of the subject property the Village plans to make available a development ready pad for private multi-family residential development. Plans have not been identified for Lot 2 and any proposal will be required to go through the complete public process similar to the approvals that the Village is currently seeking for the remainder of the subject property. A condition is proposed for the requested approvals that will require approval of an amendment to the PUD, in addition to other entitlements that may be required for the private development.

Fleet Management Facility and Telecommunication Tower (Lot 3)

While improvements will not be made to the fleet management facility located on the east side of the subject property, the Village's existing telecommunication tower will be demolished and a new monopole telecommunications tower will be constructed in this area. The existing tower is a critical part of the emergency services response network used by DU-COMM, the Village and adjacent communities. Certain zoning relief is being requested for the new tower, as further described below. The new tower will continue to offer critical coverage for emergency services and personal telecommunication devices.

COMPLIANCE WITH THE COMPREHENSIVE PLAN

The Comprehensive Plan designates the subject property as part of the Downtown Focus Area. Key concepts of this area include:

- Redevelopment of key sites.
- Development that is pedestrian-oriented and walkable.
- Maintains a commitment to quality architecture.
- Improvements to the BNSF Crossing and Washington Street.

The Comprehensive Plan also places the subject property within the Downtown Functional Subarea - Downtown Edge. This area should be understood as:

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• A combination of transit-oriented development, a mixed use residential and commercial area, and public uses, which seek to leverage access to public transportation.

The proposed development also meets other goals in the Comprehensive Plan. These goals include:

- New public facilities should be located, designed, and developed as focal points and "signature" projects within the community.
- The Village should actively promote cooperation, interaction and collaboration among the various agencies and organizations which serve Downers Grove.
- Promotion of a mix of uses in the Downtown.
- Construction of best management practices in order to improve stormwater management in addition to the adaptation of sustainable technologies and application of sustainable management practices to existing development throughout the Village.
- Provision of a detailed parking and traffic study which plans for current and future parking demand, access and circulation.

The petitioner has addressed how the proposed development is in compliance with the Comprehensive Plan in the attached Findings of Fact for the Map Amendment and the Planned Unit Development request.

COMPLIANCE WITH THE SUBDIVISION AND DEVELOPMENT ORDINANCE

The subject property is made up of several lots of record. The petitioner is proposing to subdivide the property into three lots of record. All three proposed lots meet the requirement as shown in the table below.

| Table 1 - | Subdivision | Compliance |
|-----------|---------------------------------|------------|
|-----------|---------------------------------|------------|

| | Lot V | Vidth | Lot I | Depth | Lot Ar | ea (s.f.) |
|-------|----------|----------|----------|----------|----------|-----------|
| | Required | Proposed | Required | Proposed | Required | Proposed |
| Lot 1 | 100' | 605' | 140' | 211' | 10,500 | 200,969 |
| Lot 2 | 100' | 211' | 140' | 263.5' | 10,500 | 58,320 |
| Lot 3 | 100' | 148.71' | 140' | 391' | 10,500 | 107,604 |

The petitioner is also requesting that Burlington Avenue be vacated to facilitate the redevelopment. The existing right-of-way is used for the eastern leg of the Burlington Avenue and Washington Street intersection as well as the parking lot for the existing Village facilities. The Burlington Avenue right-of-way is proposed to be vacated because a portion of the DGCC building and parking lot improvements will be located in the area. Per the Village's Right-of-Way Vacation Policy, staff contacted the utility companies, outside public agencies and other Village departments to determine if any rights to the public right-of-ways should be retained. The Village will coordinate with the utility providers to place the necessary easements on the redeveloped site to meet the Village's and utility companies' mutual needs. The Final Plat of Subdivision includes the initial necessary public utility and drainage easements, which are provided as required by the Village Ordinance. As a condition of approval, additional easements will be placed as plans are finalized and infrastructure is placed on the site.

COMPLIANCE WITH THE ZONING ORDINANCE

The subject property is zoned DT, Downtown Transition. Per Section 28.5.010 of the Zoning Ordinance, a government facility for the DGCC, is a permitted use in the DT zoning district. A wireless communications freestanding tower, is expressly authorized as a special use in the Downtown Transition (DT) zoning district. Compliance with the applicable bulk and parking requirements of the Zoning Ordinance are highlighted in the table below:

Table 2 – Zoning Requirements

| Civic Center Campus (Lot 1) | Downtown Transition Bulk Requirements | Proposed |
|--------------------------------------|---------------------------------------|---|
| Minimum Lot Area | 7,500 sq. ft. (min) | 200,969 sq. ft. |
| Street Setback – North property line | 10 feet | 4.4 feet (parking lot)* 17.3 feet (building) |
| Street Setback – South property line | 10 feet | 10 feet (parking lot) 84.5 feet (building) |
| Side Setback – West property line | 5 feet | 34.8 feet (building) |
| Side Setback – East property line | 5 feet | 5 feet (parking lot) 297.8 feet (building) |
| FAR | 2.5 | 0.40 |
| Building Height | 36 feet | 32. 2 feet |
| Parking Spaces | 164 | 252 |
| Future Multi-Family (Lot 2) | Downtown Transition Bulk Requirements | Proposed |

Lot and Building Regulation Compliance TBD with Final Approval

| Fleet Maintenance and Telecommunications Tower (Lot 3) | Downtown Transition Bulk Requirements | Proposed | |
|---|--|--|--|
| Minimum Lot Area | 7,500 sq. ft. (min) | 107,604 sq. ft. | |
| Street Setback – North property line | 10 feet | 2.8 feet^ | |
| Interior Setback – South/East property line | 5 feet | 23.8 feet (building) 5 feet (parking) | |
| Side Setback – West property line | 5 feet | 2.2 feet (building)^ | |
| FAR | 2.5 | 0.27 | |
| Telecommunications Tower | | | |
| Setback Requirement | 597 feet | 145 feet* | |
| Height Requirement | 140 feet | 199 feet* | |

^{*} Indicates a deviation from the Zoning Ordinance Requirements

Telecommunication Tower Special Use and Deviations

As noted above, the Village's existing telecommunication tower will be demolished and a new monopole telecommunications tower will be constructed in this area. The construction of the new tower will require approval of a special use and certain deviations for height and setback. The existing telecommunications tower is a critical part of the emergency services response network used by Du-COMM, the Village and adjacent communities. The new tower and related height is necessary in order to continue to offer critical coverage for emergency services and personal telecommunication devices. Please refer to the attached "Telecommunication Tower FAQ", which provides a summary of the tower operations and rationale for its construction and required location.

Planned Unit Development Request

A Planned Unit Development is intended to accommodate development that may be difficult to carry out under applicable zoning standards and results in public benefits that are at least commensurate with the degree of flexibility provided. Examples of development types that are appropriate for PUD approval, per Section 4.030.A.1 of the Zoning Ordinance include:

- Developments that provide energy conservation and sustainability
- Developments that provide housing variety
- Developments that provide multi-use development
- Developments that are consistent with the goals and policies of the Comprehensive Plan

[^] Indicates improvement is legal non-conforming and no additional improvements are proposed

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The proposed project will make use of sustainable design and development practices and will include permeable pavers, enhanced mechanical systems, rain gardens and bioswales, rain harvesting, green roofs and native landscaping. The incorporation of sustainable development features, including green infrastructure practices in landscaping and parking areas, will maximize the aesthetic and water quality benefits of best practices in stormwater management.

The PUD request does propose development of Lot 2 with a residential component that will be developed in the near future. The multi-family development will offer a variety of lifestyle choices to meet the needs of different age groups and household types. As it relates to the Comprehensive Plan, the response to criteria seven for the Map Amendment demonstrates the project is consistent with and will help advance the goals and policies of the Comprehensive Plan.

The PUD request will also achieve a variety of planning goals as outlined in Section 28.4.030.A.2 of the Zoning Ordinance:

- Implementation of and consistency with the comprehensive plan and other relevant plans and policies.
- Efficient and economical provision of public facilities and services
- Compact, mixed-use development patterns where residential, commercial, civic and open spaces are located in close proximity to one another.
- High-quality buildings and improvements that are compatible with surrounding areas, as determined by their arrangement, massing, form, character and landscaping.
- The incorporation of sustainable development features including green infrastructure practices in landscapes and parking area, to maximize the aesthetic and water quality benefits of best practices in stormwater management.
- Attractive, high-quality landscaping, lighting, architecture and signage, including the use of native landscaping that reflects the unique character of the Village and the surrounding area.

As part of the development plan, the petitioner is also requesting relief related the setback of proposed parking lot from the northern property line. The Zoning Ordinance requires a setback of 10 feet, and 4.4 feet will be provided. The deviation is necessary in order to maximize the available parking. The center of the site has a significant grade difference that does not allow the northern parking lot be further moved south, to meet the setback. Since this is adjacent to the BNSF railway there is minimal impact to the railroad, which will also be safely separated with the proposed fencing.

In summary, the plan is the epitome of a compact, mixed-use development pattern where residential, civic and open spaces are located in close proximity to one another.

Signage

With the PUD approval the petitioner is seeking approval of certain sign locations as demonstrated in the master sign package. The sign package includes an assortment of wall signage, ground signage and incidental signage to the primary identification signs for the DGCC.

COMPLIANCE WITH DOWNTOWN DESIGN GUIDELINES

The recently updated Downtown Design Guidelines provide guidance for building and site design which will assist in creating a vibrant downtown. The guidelines are divided into seven separate sections: site design, building design, building base, building middle, building top, utility considerations, and parking facilities. Each section describes elements which support good design and provides visual references which identify both encouraged and discouraged elements. As recommended by the Downtown Design Guidelines, the proposed development incorporates the following features:

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Table 3 – Downtown Design Guideline Compliance

| Downtown Design | Summary of Compliance | |
|---------------------------|--|--|
| Guideline Elements | у | |
| Site Design | The apparent mass and bulk of the combined facility is reduced by structural articulation, windows or other architectural and functional elements, and by landscaping. Physical gaps are necessary for the proposed project, but this is minimized by providing outdoor gathering spaces that are screened with landscaping along Curtiss Avenue. The proposed landscaping complement adjacent residential neighborhoods, providing four-season interest through inclusion of evergreen, deciduous and perennial species. | |
| Building Design | The building utilizes structural articulation through the placement of columns in addition to a roof structure that further pronounces the building. The façade is visually appealing through articulation, detailing, openings and materials of each elevation. The exterior uses natural brick and stone, which complements the hues of nearby buildings. The exterior also uses building materials such as brick, stone, manufactured stone, metal accents and wood to provide visual interest. Consistent building materials and detailing on all sides of the structure that are open to public view has been provided. Illumination will be provided to all entries to ensure customer and employee safety. | |
| Building Base | Primary building entrances to the public street have been provided that are highly visible and inviting to visitors and employees. Entries have been designed as prominent features of the base. Knee walls are provided around the perimeter of the facade, which provides a strong base. The proposed awnings are complementary to the style and color of the building. | |
| Building Middle | • The building middle reflects proportionate shapes and patterns as it relates to the rest of the building, through articulated walls and window treatments that are consistent with the rest of the building and in rhythm with the base level. | |
| Building Top | • The proposed roof has been designed to distinction to the entire building. The cornice provides detailing that is in scale with the rest of the building. | |
| Utility Considerations | The proposed trash and generator enclosure is designed with wall surfaces that complement the primary material of the principal building. The design of maintenance, utility and service areas were integrated into the overall design of the building. | |
| Parking Facilities | The plan installs a combination of landscaping, low seating walls at the edges of surface parking lots that border public walkways. The proposed surface parking lots follow the Village's landscaping and screening requirements. The plan also provides clear and safe pathways for pedestrian circulation at parking lot entrances. | |

ENGINEERING/PUBLIC IMPROVEMENTS

The subject property slopes gradually from the northwest to the southeast. There is approximately a tenfoot elevation change across the whole area. The site is currently served by a public sidewalk along Curtiss Street and Burlington Avenue. There are four existing curb cuts on Curtiss Street and access is currently provided from the eastbound approach of Burlington Avenue into and through the site.

The redevelopment of the subject property will include three vehicular access points from Curtiss Street to Lot 1. These locations will improve parking flow throughout the parking lot. Pedestrian access will

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also be available at the center and western access points that provide direct access to the main entrances of the DGCC and the pedestrian pathway described below. An emergency only secured exit will also be provided for Police vehicles at the northwest corner of the site. This exit will only be used when Police vehicles engage lights and sirens while responding to an emergency. This will allow the police a quicker response for emergency calls to the north. This exit will not be used during normal operations and will not be used as an entry point to the secured Police parking area.

The petitioner is proposing to construct a variety of public amenities including public plazas, pedestrian walkways and native landscaping. On-site parking will also be included for public, employee and official Village vehicles. The public amenities will be designed as flexible spaces:

- The south plaza will be a transition gathering space from the parking areas to the building lobbies and a place for public gatherings. This plaza will be encountered by staff and visitors arriving to the building from the neighborhood and visitor parking lots.
- The north plaza will be a transition gathering space from the Metra station/downtown to the DGCC campus. This space will be encountered by staff and visitors arriving to the building from the north, including Metra users, residents of adjacent properties, and others coming from the downtown area.
- A pedestrian pathway will be located on the west side of the site to allow improved pedestrian
 access between the train and neighborhoods to the south. This area will be planted with native
 landscaping and allow access to a secondary entrance to the DGCC.

A new water service and sanitary sewer service will be provided off of main lines located within Curtiss Street. The Downers Grove Sanitary District conceptually approved the request for sanitary sewer service to this development. Public sidewalks along Curtiss Street will be replaced. Due to the anticipated construction impacts on the existing parkway trees along Curtiss Street, the petitioner will be removing parkway trees as part of the proposed development. In their place, the petitioner will be installing new parkway trees along Curtiss Street as provided in the proposed landscaping plan.

Stormwater Management

The proposed project will meet the Village's Stormwater Ordinance requirements. Volume control best management practices (BMPs) for the site will be provided as required. There is an existing storm sewer system located throughout the parking lots. The existing storm sewers shall be removed and a new storm sewer system will be constructed. Underdrains for permeable paver, bioswales and rain gardens areas will also be installed. Roof drainage is expected to have internal piping and connect below ground. The site will have above ground cisterns to collect stormwater roof drainage for irrigation purposes.

Phasing Plan

The project will have four distinct phases, with construction taking place over the next three to four years. The phasing plan allows for the continuity of operations and allows Village staff and its services to stay on the subject property. The project will start with the demolition of the existing parking lot and former social services building on the western portion of the site and the closure of the east leg of the Washington Street crossing (Phase 1). With demolition complete, construction of the DGCC will begin (Phase 2). The existing Police Station and Village Hall will remain open throughout construction.

With Phase 3 and 4, the Village and School District 58 will move into the new building and the existing Village Hall and Police Station buildings will be demolished to make room for the rest of the parking lot and completion of the remainder of the site amenities (Phase 3). The final phase will include completion of a pad ready site for a new private development on Lot 2 (Phase 4). The final phase will necessitate an amendment to the Planned Unit Development approvals in addition to a special use for a multi-family

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development. This will require a Plan Commission review with Village Council approval. A condition is proposed with the current development request that would require approval of an amendment to the PUD, in addition to other entitlements that may be required for the private development.

TRAFFIC AND PARKING

A traffic impact study for the proposed development was completed by the petitioner. The study examined the anticipated traffic increases along the street network; the relationship of the access drives in relationship to Curtiss Street and Washington Street; and the following Washington Street intersections with Maple Avenue, Curtiss Street, Burlington Avenue, Warren Avenue and Rogers Street; and Curtiss Street intersections with Belden Avenue and Mackie Place.

As part of the proposed redevelopment, the eastern leg of the Burlington Avenue and Washington Street intersection will be eliminated. Similar to the current operations, the Washington Street northbound traffic is proposed to continue and will be under stop sign control just south of the railroad crossing. The elimination of the eastern leg of the intersection will offer the following benefits:

- It will enhance the flow of traffic along northbound Washington Street and dissipate the northbound queue of traffic quicker after a train event as the northbound traffic will no longer have to defer to the Washington Street southbound left-turn movement or contend with outbound movements from the access drive.
- It will provide more efficient and orderly pedestrian circulation as the pedestrians will have one less intersection leg to traverse and four less vehicle movements to contend with.
- The elimination of the southbound left-turn movement from Washington Street to the access drive will eliminate potential back-ups that may occur on the railroad tracks and will dissipate the southbound queue of traffic quicker after a train event.
- The provision of stopping northbound traffic just south of the tracks will meter northbound traffic which will (1) permit turning movements at the intersection of Washington Street with Warren Avenue to occur more frequently and (2) provide additional gaps in the traffic stream for pedestrians to traverse Washington Street

The proposed access drives on Curtiss Street serving the redevelopment will be adequate to accommodate the traffic estimated to be generated by the proposed redevelopment, will ensure efficient and flexible access, and will have a limited impact on Curtiss Street, Belden Avenue, and Mackie Place. The study also found that the existing roadway system will have sufficient reserve capacity to accommodate the additional traffic that will be generated by the proposed redevelopment and the redistribution of traffic due to the elimination of the existing access drive opposite Burlington Avenue.

With regards to the parking the development will provide 252 parking spaces that will be used by Village and District 58 staff. Of these 252 spaces, 80 spaces will be located in a secured parking lot. In addition to employee parking, 82 parking spaces will be provided for public and visitor parking.

PUBLIC SAFETY REQUIREMENTS

The Fire Prevention Division of the Fire Department has reviewed the application. Sufficient access for the Fire Department will be provided for the building. Fire hydrant placement will be provided as required by Village Ordinance. All floors will be equipped with fire alarms and will be sprinkled, as required by Village regulations.

NEIGHBORHOOD COMMENT

Notice was provided to all property owners 250 feet or less from the subject property in addition to posting the public hearing sign and publishing a legal notice in the *Daily Herald*. As required by the Zoning Ordinance, the petitioner held a neighborhood meeting on March 21, 2022. To advertise the neighborhood meeting, the Village mailed invitations to all property owners 250 feet or less from the

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property in addition to placing a video on the Village website inviting the community to the neighborhood meeting (http://www.downers.us/facilities).

Approximately 50 residents attended with various comments and questions. The comments provided at the neighborhood meeting, in addition to the petitioner's responses are provided in the attached Neighborhood Meeting Summary.

STANDARDS OF APPROVAL

To permit the construction of a combined Village Hall and Police Station facility and relocation of a telecommunication tower, the petitioner is requesting approval of the following items:

- Plat of Vacation
- Plat of Subdivision
- Planned Unit Development
- Map Amendment for DT (Downtown Transition) to DT/PUD (Downtown Transition/Planned Unit Development)
- Special Use

The review and approval criterion for each request is listed below.

The petitioner has submitted a narrative that attempts to address all the standards of approval. The Plan Commission should consider the petitioner's documentation, the staff report and the discussion at the Plan Commission meeting in determining whether the standards for approval have been met:

Planned Unit Development

Section 28.12.040.C.6 Review and Approval Criteria

The decision to amend the zoning map to approve a PUD development plan and to establish a PUD overlay district are matters of legislative discretion that are not controlled by any single standard. In making recommendations and decisions regarding approval of planned unit developments, review and decision-making bodies must consider at least the following factors:

- a. The zoning map amendment review and approval criteria of Sec. 28.12.030.I.
- b. Whether the proposed PUD development plan and map amendment would be consistent with the comprehensive plan and any other adopted plans for the subject area.
- c. Whether PUD development plan complies with the PUD overlay district provisions of Sec. 28.4.030.
- d. Whether the proposed development will result in public benefits that are greater than or at least equal to those that would have resulted from development under conventional zoning regulations.
- e. Whether appropriate terms and conditions have been imposed on the approval to protect the interests of surrounding property owners and residents, existing and future residents of the PUD and the general public.

Zoning Map Amendment

Section 12.030.I. Zoning Map Amendment Review and Approval Criteria

The decision to amend the zoning map is a matter of legislative discretion that is not controlled by any single standard. In making recommendations and decisions about zoning map amendments, review and decision-making bodies must consider at least the following factors:

- 1. The existing use and zoning of nearby property.
- 2. The extent to which the particular zoning restrictions affect property values.
- 3. The extent to which any diminution in property value is offset by an increase in the public health, safety and welfare.
- 4. The suitability of the subject property for the zoned purposes.

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- 5. The length of time that the subject property has been vacant as zoned, considering the context of land development in the vicinity.
- 6. The value to the community of the proposed use.
- 7. The comprehensive plan.

Special Use

Section 28.12.050.H Approval Criteria – Special Uses

No special use may be recommended for approval or approved unless the respective review or decisionmaking body determines that the proposed special use is constituent with and in substantial compliance with all Village Council policies and plans and that the petitioner has presented evidence to support each of the following conclusions:

- 1. That the proposed use is expressly authorized as a Special Use in the district in which it is to be located;
- 2. That the proposed use at the proposed location is necessary or desirable to provide a service or a facility that is in the interest of public convenience and will contribute to the general welfare of the neighborhood or community.
- 3. That the proposed use will not, in the particular case, be detrimental to the health, safety or general welfare of persons residing or working in the vicinity or be injurious to property values or improvements in the vicinity.

DRAFT MOTION

Staff recommends approval at the April 4, 2022 meeting. Should the Plan Commission find that the request meets the standards of approval for a Planned Unit Development, accompanying Map Amendment, Special Use, Plat of Vacation and Plat of Subdivision, staff has prepared a draft motion that the Plan Commission may make for the recommended approval of 22-PLC-0007:

Based on the petitioner's submittal, the staff report, and the testimony presented, I find that the petitioner has met the standards of approval for a Planned Unit Development, accompanying Rezoning, and Special Use, Plat of Vacation and Plat of Subdivision as required by the Village of Downers Grove Zoning Ordinance and is in the public interest and therefore, I move that the Plan Commission recommend to the Village Council approval of 22-PLC-0007, subject to the following conditions:

- 1. The Special Use, Planned Unit Development and Map Amendment shall substantially conform to the staff report, renderings, architecture plans, landscaping plans and engineering plans prepared by fgma architects, dated through March 25, 2022, and traffic plans prepared by KLOA dated March 25, 2022 except as such plans may be modified to conform to the Village codes and ordinances.
- 2. The petitioner shall subdivide the lot into the three lots of record pursuant to Section 20.5 of the Subdivision Ordinance prior to the issuance of any site development or building permits.
- 3. A development application for Lot 2 shall be submitted to the Plan Commission as a PUD Amendment, and along with any other required entitlements, with final approval by the Downers Grove Village Council.
- 4. A plat of easement is provided for public utility purposes and recorded with DuPage County once infrastructure is constructed.
- 5. A plat of easement is provided for telecommunication purposes and recorded with DuPage County once telecommunications equipment is constructed.
- 6. A plat of easement is provided for stormwater purposes and must be recorded with DuPage County once infrastructure is constructed.

Staff Report Approved By:

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22-PLC-0007, Downers Grove Civic Center PUD April 4, 2022

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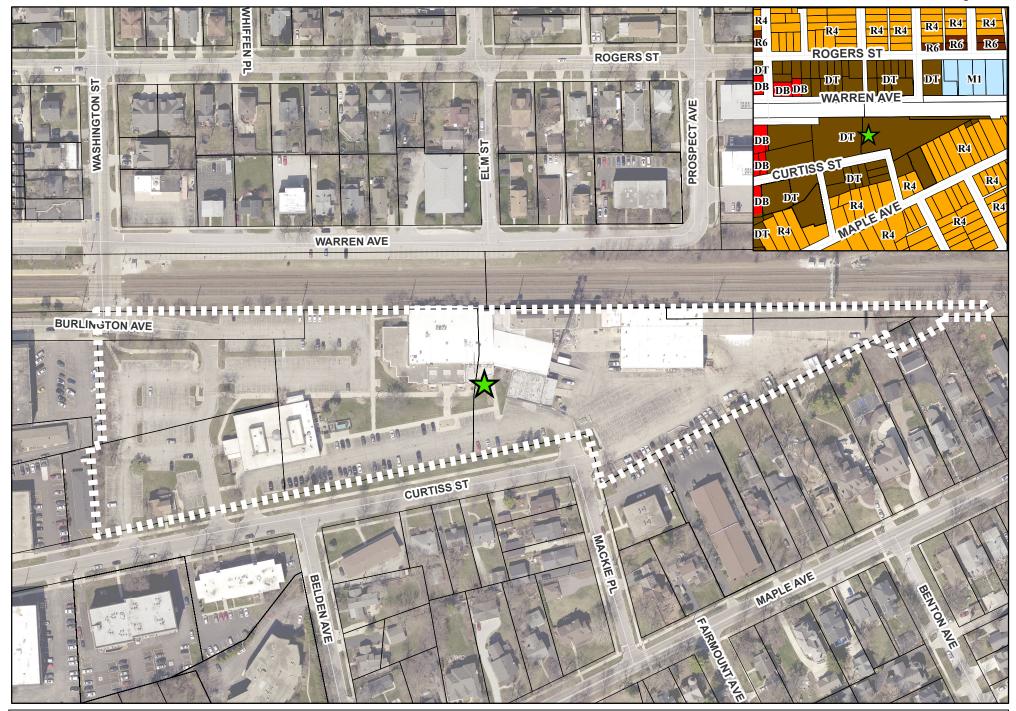
Stan Popovich, AICP

Director of Community Development

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N 0 100 200 Feet

Subject Property

★ Project Location

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www.downers.us

Village Hall 801 Burlington Ave. Downers Grove, IL 60515-4782 630.434.5500

Fire Department Administration

5420 Main St. Downers Grove, IL 60515-4834 630.434.5980

Police Department

825 Burlington Ave. Downers Grove, IL 60515-4783 630.434.5600

Public Works

5101 Walnut Ave. Downers Grove, IL 60515-4046 630.434.5460

PROJECT SUMMARY AND NARRATIVE

Background

The Village owns and operates seven major buildings that must be properly maintained. Five buildings are in a maintainable condition. The 43 year-old Police Station and 90+ year-old Village Hall are in Below Average condition and do not provide modern work spaces that allow for efficient and effective interactions among employees and customers. Both facilities have outdated major building systems and are in need of replacement.

In order to address the conditions of these facilities, the Village has developed a Facility Replacement & Sustainability Plan (FRSP). More information regarding the plan is available here: http://www.downers.us/facilities A video overview of the project is available here: https://www.youtube.com/watch?v=Ewpvf5ot-r8&t=1s

The plan includes the replacement these buildings with a new combined Police Station and Village Hall to be located on the Civic Center property, which will also provide space for District 58 administrative offices.

The plan is summarized below:

| What will be constructed? | New Police Station, Village Hall, District 58 Admin. Offices & Apartment Building; Relocation of Telecommunications Tower |
|----------------------------------|---|
| Where will construction occur? | Civic Center Property |
| When will construction occur? | August 2022 to December 2024 |
| How much will it cost? | Current Total Project Budget of \$59 million |
| | Cash on Hand (\$24 million) and Bond |
| How will the Village pay for it? | Issuance (\$35 million). No increase in |
| | property taxes required. |

Development Request

In order to achieve the development of the site as summarized below, the Village of Downers Grove is requesting the following entitlements to construct a 79,000 square foot building that will house the offices of Village Hall, Police Station and School District 58:

- Plat of Vacation Burlington Avenue Right-of-Way
- Final Plat of Subdivision Three (3) Lots
- Final Planned Unit Development
- Zoning Map Amendment from DT to DT / PUD
- Special Use Telecommunications Tower

Final Plat of Subdivision

The subject property is made up of several lots of record. The Village is proposing to subdivide the property into three lots of record. In conjunction with the subdivision, the Village will need to vacate the Burlington Avenue right-of-way to accommodate the proposed redevelopment of the site.

Village Facility Plan (Lot 1)

On Lot 1, the Village is proposing the construction of a two story building, with a basement, that will provide for modern work spaces that will allow for efficient and effective interactions among employees and customers. The new building will be constructed immediately west of

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Public Works 5101 Walnut Ave. Downers Grove, IL 60515-4046 630.434.5460 the existing Police Station and Village Hall buildings in the existing commuter parking lot (Lot L). The new facility will include two distinct lobbies, one for Village Hall and District 58 services; and a second lobby for the Police services. The new Council Chambers will be located off of the Village Hall lobby and can be used for a variety of public meetings and community events.

The building and site design will adhere to the recently adopted Downtown Design Guidelines. While instantly recognizable as a government facility, the building will use traditional materials and reflect the familiar aesthetics of our Downtown to welcome the community. The site will include public plazas, pedestrian walkways, improved bicycle infrastructure and native landscaping. On-site parking will also be included for public, employee, commuter and official Village vehicles. Further detailed below, the proposed plan for the Washington Street intersection will eliminate the eastern leg of the intersection, streamlining the traffic flow and improving pedestrian walkways and bicycle connections.

A variety of environmental sustainable features are planned to be integrated into the project. As directed by the Village Council, integration of environmental sustainable design has been a critical foundation to the planning process for the Civic Center property. The proposed sustainable features will include: solar panels, permeable pavers, enhanced mechanical systems, rain gardens and bioswales, native landscaping, rain harvesting and the installation of a green roof.

Private Development (Lot 2)

In the middle of the civic center property, the Village plans to make available a development ready pad for private multi-family residential development. The introduction of this private investment, is in line with the Village's Comprehensive Plan, which recommends capitalizing on the redevelopment potential of the site and supports an appropriate and well-designed transit-oriented development. Private investment and the resulting property tax revenues will assist in the Village's efforts for paying for the project through the Tax Increment Financing District that is proposed to be placed over the Civic Center property.

Plans have not been identified for Lot 2 and any proposal will be required to go through the complete public process similar to the approvals that the Village is currently seeking for the remainder of the subject property.

Telecommunication Tower (Lot 3)

A new Village telecommunications tower will be constructed east of the existing fleet maintenance facility. Once constructed and operational, the existing tower will be demolished. More information regarding the tower can be found in the FAQ document, attached to this narrative.

Washington Street Improvements

The Washington Street intersection experiences a variety of vehicles, pedestrians, commuters, cyclists and downtown visitors converging in a confusing intersection with multiple different turning movements. The proposed plan addresses this issue by eliminating the eastern leg of the intersection, south of the train tracks, while better defining pedestrian and vehicular zones. The Village conducted a traffic study that demonstrates the plan will enhance the flow of traffic along northbound Washington Street and dissipate the northbound queue of traffic quicker after a train event as the northbound traffic will no longer have to defer to the Washington Street southbound left-turn movement or contend with outbound movements from the access drive.

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Phasing Plan

The project will have many different facets, with construction taking place over two and a half years years. The project will start with the demolition of the existing parking lot and small building on the western portion of the site and the closure of the east leg of the Washington Street crossing (Phase 1). With demolition complete, construction of the new facility will begin (Phase 2). The existing Police Station and Village Hall will remain open throughout construction.

With Phase 3 and 4, Village Hall and Police operations will move into the new building and the existing Village Hall and Police Station buildings will be demolished to make room for the rest of the parking lot and completion of the remainder of the site amenities (Phase 3). The final phase will also include completion of a pad ready site for a new private development (Phase 4).

Document Submission

The submitted plans and exhibits have been submitted for Village review:

- Plat of Survey
- Plat of Vacation Burlington Avenue
- Final Plat of Subdivision
- Downers Grove Sanitary approval
- Site Plan
- Engineering Site Plan
- Floor Plan Combined Village Hall and Police Station
- Building Elevations Combined Village Hall and Police Station
- Landscape Plan
- Signage Location Plan
- Traffic Study
- Wireless Communication Tower Plans
- Washington Street Improvement Plan
- Phasing Plan
- Neighborhood Meeting Summary

Requested Relief

| VoDG Code Section | Relief Request | Rationale |
|----------------------|---|---|
| 28.6.170(g)(3) | Requirement: Setback of Telecommunication Tower: 597 Feet Relief Request: 145 Feet | The existing telecommunications tower is a critical part of the emergency services response network used by Du-COMM, the Village and adjacent communities. With this project, the current tower will be demolished and a new tower will continue to offer critical coverage for emergency services and personal telecommunication devices. There is no location on the site |

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| | | that can be provided without the requested relief. See attached FAQ document for more information. |
|----------------|--|---|
| 28.6.170(k)(4) | Requirement: Height of Telecommunication Tower: 183 feet (height of highest antenna) Relief Request: 199 Feet | The existing telecommunications tower is a critical part of the emergency services response network used by Du-COMM, the Village and adjacent communities. The new tower and related height is necessary in order to continue to offer critical coverage for emergency services and colocate equipment from private telecommunications providers. |
| 28.7.070(c) | Requirement: Setback for parking area: 10 feet Relief Request: 4.3 Feet | The deviation is necessary in order to maximize the available parking. The center of the site has a significant grade difference that does not allow the northern parking lot be further moved south, to meet the setback. Since this is adjacent to the BNSF railway there is minimal impact to the railroad, which will also be safely separated with the proposed fencing. |

Review and Approval Criteria/Developer's Statement of Intent

The following attachments have been provided that demonstrate compliance with the review and approval criteria for the requested entitlements for the subject property.

- Zoning Map Amendment
- Planned Unit Development
- Special Use -Telecommunications Tower

As demonstrated in the response to the standards for a PUD the proposed development provides greater benefits to the Village than would a development carried out in accordance with otherwise applicable zoning ordinance standards.

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Review and Approval Criteria PLANNED UNIT DEVELOPMENT

Section 28.12.040.C.6
Review and Approval Criteria (Planned Unit Development)

The decision to amend the zoning map to approve a PUD development plan and to establish a PUD overlay district are matters of legislative discretion that are not controlled by any single standard. In making recommendations and decisions regarding approval of planned unit developments, review and decision-making bodies must consider at least the following factors:

- **1.** The zoning map amendment review and approval criteria of Sec. 12.030.I. See the analysis of zoning map amendment review and approval criteria in a separate document (*Review and Approval Criteria Map Amendment*). This criteria is met.
- 2. Whether the proposed PUD development plan and map amendment would be consistent with the Comprehensive Plan and any other adopted plans for the subject area.

The proposed PUD and project are consistent with the Comprehensive Plan as detailed in the response to criteria seven in a separate document (*Review and Approval Criteria - Map Amendment*). This criteria is met.

3. Whether PUD development plan complies with the PUD overlay district provisions of Sec. 28.4.030.

Section 28.4.030 provides for general guidance for examples of the types of development that may be appropriate for PUD approval including those developments that further the following categories: energy conservation and sustainability, housing variety, multi-use developments and Comprehensive Plan development recommendations.

The project will make use of sustainable design and development practices and will include permeable pavers, enhanced mechanical systems, rain gardens and bioswales, rain harvesting, green roofs and native landscaping. The incorporation of sustainable development features, including green infrastructure practices in landscaping and parking areas, will maximize the aesthetic and water quality benefits of best practices in stormwater management.

The PUD request does propose development of the subject property for a portion of the site with a residential component that will be developed in the near future. The multi-family development will offer a variety of lifestyle choices to meet the needs of different age groups and household types.

As it relates to the Comprehensive Plan, the response to criteria seven in a separate document (*Review and Approval Criteria - Map Amendment*) demonstrates the project is consistent with and will help advance the goals and policies of the Comprehensive Plan.

Section 28.4.030 also provides that PUDs should include elements that further certain objectives as provided in the section of the ordinance. In summary, the plan

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is the epitome of a compact, mixed-use development pattern where residential, civic and open spaces are located in close proximity to one another. The building and site design will adhere to the recently adopted Downtown Design Guidelines. While instantly recognizable as a government facility, the building will use traditional materials and reflect the familiar aesthetics of our Downtown to welcome the community. Furthermore the site will be designed with attractive, high-quality landscaping, lighting, architecture and signage, including the use of native landscaping that reflects the unique character of the Village and the surrounding area. As it relates to efficient and economical provision of public facilities and services, the Village will share the facility with School District 58. This shared use of community facilities will maximize efficiency, tax dollars and land. This criteria is met.

4. Whether the proposed development will result in public benefits that are greater than or at least equal to those that would have resulted from development under conventional zoning regulations.

The PUD request will result in a public benefit to the Village that will allow shared facilities between the Village and School District 58, which will maximize efficiency, tax dollars and public land. The combined facility will provide for modern work spaces that will allow for efficient and effective interactions among employees and customers. Furthermore, the Village's Police Department will be able utilize modern technologies and space layout that more effectively allows them to provide their services.

The site will include public plazas, pedestrian walkways and native landscaping. On-site parking will also be included for public, employee and official Village vehicles. A variety of environmental sustainable features are planned to be integrated into the project. As directed by the Village Council, integration of environmental sustainable design has been a critical foundation to the planning process for the subject property.

The existing telecommunications tower is a critical part of the emergency services response network used by Du-COMM, the Village and adjacent communities. With this project, the current tower will be demolished and a new tower will continue to offer critical coverage for emergency services and personal telecommunication devices.

The Washington Street intersection has had its challenges over the years, with vehicles, pedestrians, commuters and downtown visitors converging in a confusing intersection with multiple different turning movements. The proposed plan addresses this issue by eliminating the eastern leg of the intersection, south of the train tracks, while better defining pedestrian and vehicular zones, enhancing the safety of the site. Although deviations are being requested for the site, there will be benefits, as stated herein, provided to the public if the development is constructed. This criteria is met.

5. Whether appropriate terms and conditions have been imposed on the approval to protect the interests of surrounding property owners and residents, existing and future residents of the PUD and the general public.

There are several requirements that the Village will have to accomplish in order to both construct the proposed project and protect the interests of the surrounding

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property owners, residents, future residents and the general public. The conditions placed on the development of the civic campus will ensure that the proposed development satisfies all applicable codes and requirements as well as ensure that the development continues to provide benefits to the public after the building is occupied. The impact of the project will result in numerous improvements to the immediate area including providing various plazas and making connections to existing land uses that have historically been disconnected. This project will advance many goals and objectives laid out in several adopted documents and the conditions below will ensure that those goals and objectives are advanced. This standard has been met.

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Review and Approval Criteria ZONING MAP AMENDMENTS

Section 28.12.030.I. Review and Approval Criteria (Zoning Map Amendments)

The decision to amend the zoning map is a matter of legislative discretion that is not controlled by any single standard. In making recommendations and decisions about zoning map amendments, review and decision making bodies must consider at least the following factors.

(1) The existing uses and zoning of nearby property.

To the immediate south of the property are a mixture of single-family homes and multi-family developments that are zoned Downtown Transition (DT). To the immediate west of the subject property are an assortment of mixed use developments and multiple family developments that are zoned Downtown Business (DB). Immediately north of the subject property is the BNSF Railway. To the north of the BNSF property, are a mix of single-family homes and multi-family developments that are zoned DT.

The Village property is currently zoned Downtown Transition (DT). The property is currently improved with the Village Hall that was purchased in 1968, and at that time it was anticipated to have a 20-year lifespan. The Village's Police Station, constructed in 1979, is in need of substantial system replacement and does not meet the operational requirements of the Department. The site also includes the Village's fleet management facility, telecommunications tower and converted residential home that was previously used for social services.

The redevelopment plan is to replace these buildings with a new combined Police Station and Village Hall (Downers Grove Civic Center (DGCC)) on the subject property (on a newly created Lot 1). The DGCC will also include School District 58 administrative offices. The middle lot (Lot 2) is planned for multi-family development that will be required to seek Village Council approval in the near future. While improvements will not be made to the fleet management facility located on the east side of the subject property, the Village's telecommunication tower will be relocated into this area (on a newly created Lot 3).

The redevelopment plan is in line with the Village's Comprehensive Plan, which capitalizes on the redevelopment potential of the site and supports an appropriate and well-designed transit-oriented development. This is compatible with surrounding uses, both existing and recommended in the Comprehensive Plan. This criteria is met.

(2) The extent to which the particular zoning restrictions affect property values.

The proposed map amendment to change the zoning to DT/PUD allows the replacement of outdated Village facilities. The PUD approval and conditions will ensure the site will be developed to the highest standards and in accordance with the Village's Comprehensive Plan. That allotment of future private investment on Lot 2 will actually increase the value of the subject property as currently the subject property does not remit any property tax revenue to the Village.

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The redevelopment plan will improve the property with modern, high quality buildings which, in turn, should raise property values. This project will include PUD conditions and restrictions which will not negatively affect property values but should protect property values. This criteria is met.

(3) The extent to which any diminution in property value is offset by an increase in the public health, safety and welfare.

It is not expected that there will be any diminution in property value, and in fact, the development will be an enhancement to the public health, safety and welfare of the community. The Village will share the facility with School District 58, which will maximize efficiency, tax dollars and public land. The combined facility will provide for modern work spaces that will allow for efficient and effective interactions among employees and customers. Furthermore the Village's police department will be afforded a substantial system replacement that will meet the operational requirements of the department.

The site will include public plazas, pedestrian walkways and native landscaping. On-site parking will also be included for public, employee and official Village vehicles. A variety of environmental sustainable features are planned to be integrated into the project. As directed by the Village Council, integration of environmental sustainable design has been a critical foundation to the planning process for the subject property.

The existing telecommunications tower is a critical part of the emergency services response network used by Du-COMM (DuPage Public Safety Communications) the Village and adjacent communities. With this project, the current tower will be demolished and a new tower will continue to offer critical coverage for emergency services and personal telecommunication devices.

The Washington Street intersection experiences a variety of vehicle, pedestrian, commuter and downtown visitor interactions converging in a confusing intersection with multiple different turning movements. The proposed plan addresses this issue by eliminating the eastern leg of the intersection, south of the train tracks, while better defining pedestrian and vehicular zones, enhancing the safety of the site. This criteria is met.

(4) The suitability of the subject property for the zoned purposes.

Currently, the property is zoned DT with the request for a map amendment to change the zoning to DT/PUD. The proposed plan for the subject property is in line with the zoning which permits governmental facilities, the telecommunications tower and multi-family development. The introduction of this private investment and a modern shared community facility is in line with the Village's Comprehensive Plan, which capitalizes on the redevelopment potential of the site and supports an appropriate and well-designed transit-oriented development. This criteria is met.

(5) The length of time that the subject property has been vacant as zoned, considering the context of land development in the vicinity.

The property is not vacant, but underutilized in its current configuration. This criteria is met.

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(6) The value to the community of the proposed use.

Public sites and buildings are viewed as potential catalysts for improvement and/or redevelopment of an area. The new combined facility will be located, designed, and developed as a focal point of the downtown and the community. The redevelopment of the site will add value to the downtown and the community. Rezoning the property with the PUD overlay will permit the Village to create a development that will advance several other goals and objectives identified in the Comprehensive Plan as well as provide public benefits that do not currently exist. This criteria is met.

(7) The Comprehensive Plan.

As noted in the Comprehensive Plan, public sites and buildings should be viewed as potential catalysts for improvement and/or redevelopment of an area. New facilities should be located, designed, and developed as focal points and "signature" projects within the community. The PUD request capitalizes on the redevelopment potential of the site, and supports an appropriate and well-designed transit-oriented development. The building and site design will adhere to the recently adopted Downtown Design Guidelines. While instantly recognizable as a government facility, the building will use traditional materials and reflect the familiar aesthetics of our Downtown to welcome the community. The DGCC will "lead by example" and will include sustainable design and development practices. The project will include permeable pavers, enhanced mechanical systems, rain gardens, bioswales, rain harvesting, green roofs and native landscaping.

As noted in the Comprehensive Plan, the importance of public uses (churches, Village Hall, parks, library, post office, and social services) cannot be overstated for the continued success of the Downtown. The proposed plan preserves the subject property for public uses and in conjunction with the rest of the Downtown encourages visitors to make several stops during a trip to Downtown and encourages them to stay longer.

The Comprehensive Plan states that as new development or redevelopment occurs, infrastructure improvements, such as the BNSF crossing at Washington Street, may be necessary to accommodate increased residential density. The Washington Street intersection experiences vehicles, pedestrians, commuters and downtown visitors converging in a confusing intersection with multiple different turning movements. The proposed plan addresses this issue by eliminating the eastern leg of the intersection, south of the train tracks, while better defining pedestrian and vehicular zones.

The Village's Comprehensive Plan Key Focus Area for Downtown identifies that the Village should consider a range of uses including multi-family for the subject property. The plan does identify Lot 2 for multi-family development. The introduction of this private investment in addition to a modern shared community facility, is in line with the Village's Comprehensive Plan, which capitalizes on the redevelopment potential of the site and supports an appropriate and well-designed transit-oriented development.

Lastly, as noted in the Comprehensive Plan, the Village should actively promote cooperation, interaction, and collaboration among the various agencies and

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organizations which serve Downers Grove. The Village will share the facility with School District 58. The Village should encourage the use of shared facilities (i.e. between the Village, Park District and schools) in order to maximize efficiency, tax dollars and land. This criteria is met.

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Review and Approval Criteria SPECIAL USE Telecommunications Tower

Section 28.12.050.H Approval Criteria (Special Uses)

No special use may be recommended for approval or approved unless the respective review or decision-making body determines that the proposed special use is constituent with and in substantial compliance with all Village Council policies and plans and that the applicant has presented evidence to support each of the following conclusions:

1. That the proposed use is expressly authorized as a Special Use in the district in which it is to be located.

The use, a wireless communications freestanding tower, is expressly authorized by the Village in the Downtown Transition (DT) zoning district. This criteria is met.

2. That the proposed use at the proposed location is necessary or desirable to provide a service or a facility that is in the interest of public convenience and will contribute to the general welfare of the neighborhood or community.

The proposed monopole will include: Downers Grove emergency communications equipment and microwave wide-area network radios, DuPage Public Safety Communications (DU-COMM) emergency communications equipment and several wireless carriers' equipment (AT&T, Verizon, and McKay Brother). Specifically DU-COMM equipment will continue to provide emergency communication services to police, fire, and emergency medical services. The proposed telecommunications tower is necessary to provide a service that is in the interest of the public and will greatly contribute to the general welfare of the neighborhood, community and DuPage County. This criteria is met.

3. That the proposed use will not, in the particular case, be detrimental to the health, safety or general welfare of persons residing or working in the vicinity or be injurious to property values or improvements in the vicinity.

The proposed monopole and associated equipment will be in compliance with the Federal Communications Commission (FCC) and Federal Aviation Administration (FAA) regulations. Both entities govern construction requirements, technical standards, interference protection, power and height limitations, and radio frequency standards. Specifically, the FCC regulates the operation of all telecommunications equipment and devices and maintains authority over any health and environmental effects of radio frequency emissions. Furthermore, the monopole will provide for vital communications in emergency situations to be used by local residents and emergency personnel to protect the general public's health, safety, and welfare. Routine maintenance visits by wireless carriers will not interfere with any police, fire, or emergency communications. This criteria is met.

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NEIGHBORHOOD MEETING SUMMARY

Outreach

As required by the Zoning Ordinance, the petitioner (the Village of Downers Grove (Village)) held a neighborhood meeting on March 21, 2022. To advertise the neighborhood meeting, the Village mailed invitations (attached) to all property owners 250 feet or less from the property. Additionally, the Village created a top story on the Village website inviting the general public to the meeting and included a video as well. The video was also placed on the facilities page of the Village website (http://www.downers.us/facilities).

Meeting Format

The meeting was held in an open house format. The video of the project played in the Council Chambers during the event, while the Committee Room offered stations featuring poster boards and information about the following topics, with staff available to receive comments and answer questions:

- Washington Street Crossing
- Floor Plan
- Site Plan
- Landscaping Plan
- Building Elevations and Renderings
- Project Financing
- Sustainability Elements

Feedback

Approximately 50 residents attended with various comments and questions. The following is a summary of comments that were made in support of the project, followed by stated concerns and responses from the petitioner. Comments provided were either summarized from verbal comments made at the meeting (Meeting Feedback), through the Village's facilities home page (Website Feedback) or emailed to a Village representative (Email Feedback).

Meeting Feedback:

- The building design is well thought out.
- It's time for a new facility.
- The Washington Street changes appear to improve pedestrian safety and vehicle conflicts.
- Support for additional multi-family opportunities in the downtown.
- There was an appreciation for the sustainable elements provided in the project.

Website Feedback

• Looks great. Will improve the village's properties. Will be wonderful work environments for its law enforcement and village staff.

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5101 Walnut Ave. Downers Grove, IL 60515-4046 630.434.5460 Telecommunication Tower/Fleet Maintenance Lot (Lot 3)

| Residents' Concerns | Petitioner's Response | |
|--------------------------------------|---|--|
| Meeting Feedback: Location of the | Please see attached Telecommunications Tower FAQ document for | |
| proposed telecommunication tower | information about the tower and its location. | |
| Meeting Feedback: Why does the | The fleet maintenance facility is in maintainable condition and the | |
| plan not include development of the | FRSP prioritized the facility needs of the Police Station and Village | |
| Fleet Maintenance facility? | Hall facilities. | |
| Meeting Feedback: Lights located | The Village will evaluate options for reducing the light level in the | |
| on the existing garages on the east | area of the fleet garage. | |
| side of the Fleet Garage are glaring | | |
| and shining into the rear yards of | | |
| the adjacent houses. | | |

Private Development Lot (Lot 2)

| Residents' Concerns | Petitioner's Response | |
|------------------------------------|--|--|
| Meeting Feedback: Objection to | The introduction of this private investment and a modern shared | |
| selling any portion of the Civic | community facility is in line with the Village's Comprehensive | |
| Center property for private | Plan, which capitalizes on the redevelopment potential of the site | |
| development. The public land | and supports an appropriate and well-designed transit-oriented | |
| should remain under Village | development. Furthermore, the inclusion of private investment on | |
| ownership and used for future | the Civic Center property is a key component of the financial | |
| public services. | strategy for funding the project. | |
| Meeting Feedback: What is the | The height and number of stories has not been determined yet. | |
| height limit on the proposed | Plans have not been identified for Lot 2 and any proposal will be | |
| apartment building? How many | required to go through the complete public process similar to the | |
| stories will it be? | approvals that the Village is currently seeking for the remainder of | |
| | the subject property. A condition is proposed for the approvals that | |
| | would require approval of a PUD amendment, in addition to other | |
| | entitlements that may be required for the private development. | |
| Meeting Feedback: Concern that the | Located in the downtown, multi-family development is required to | |
| apartment building will not be | comply with the recently adopted Downtown Design Guidelines. | |
| aesthetically pleasing and not | | |
| thought out enough. | | |

Commuter Parking

| Residents' Concerns | Petitioner's Response |
|--|---|
| Meeting Feedback: Concern about the removal of Commuter Parking from the site | The Village is responsible for providing and administering commuter parking near all three Metra stations within the Village. The Village will continue to monitor commuter parking activity and demand throughout the system and make adjustments as necessary. Commuter parking is not required for this project. |
| Email Feedback: Lot L utilization survey was used at a time when many people were not going to work due to omicron variant surge and the winter/holiday season. | The Village will continue to monitor commuter parking activity and demand through the commuter parking system and make adjustments as necessary. |
| Email Feedback: A large number of spots are assigned as public parking (72), I wonder where this number comes from? Is there a plan to allow for dual-purpose public and commuter spots? Wouldn't primary use of public parking be used for council chamber meetings after normal business hours when commuters have likely vacated the lot? | The parking spaces provided to support the new facility accommodate the requirements of the Village Hall, Police Department and District 58. This includes parking for customers who access the building for services and for visitors who attend training events and meetings during regular business hours. |
| Email Feedback: \$1M in water main work on Curtiss is cited as reason to not add commuter parking. How much has the village collected in Lot L parking fees over the prior 2-plus | Parking fees are collected and used to maintain existing parking infrastructure. Water main replacement at this location is not anticipated in the next five years. |

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Village Hall 801 Burlington Ave. Downers Grove, IL 60515-4782 630.434.5500

Fire Department Administration 5420 Main St. Downers Grove, IL 60515-4834 630.434.5980

Police Department 825 Burlington Ave. Downers Grove, IL 60515-4783 630.434.5600

Public Works 5101 Walnut Ave. Downers Grove, IL 60515-4046 630.434.5460

| decades to offset this amount? Would | |
|---------------------------------------|--|
| the water main work be required to be | |
| completed in the next 5yrs if not | |
| addressed with this project? | |

Washington Street/BNSF Intersection

| Residents' Concerns | Petitioner's Response |
|--|---|
| Meeting Feedback: Concern about the | See project narrative for detailed description of Washington |
| removal of the eastern leg of | crossing improvements and related benefits. |
| Washington Street and the limit of | |
| vehicle access through the site | |
| Website Feedback: I am 100% against | The path located west side of the new facility will have sufficient |
| the elimination of the eastern leg of | width and signage to encourage use by bicycles. Additional safety |
| the Washington Street intersection. I | enhancements related to this crossing are described in the project |
| use it to safely cross the tracks on my | narrative. |
| bike several times per week. It is far | |
| safer for cyclists than the western leg. | |
| The Western leg of the Washington | |
| Street intersection is a pedestrian and | |
| cyclist incident waiting to happen. | |
| Meeting Feedback: Concern that the | Traffic modeling was completed for the proposed change that |
| plan eliminates the drive aisle that | identified the improvement will enhance the flow of traffic along |
| connects Curtiss Street to Washington | northbound Washington Street and dissipate the northbound queue |
| Street through the Civic Center. | of traffic quicker after a train event as the northbound traffic will |
| Concern that this will cause traffic | no longer have to defer to the Washington Street southbound left- |
| back-ups at the intersection of | turn movement or contend with outbound movements from the |
| Washington and Curtiss. | access drive. |
| | |
| | Furthermore, the elimination of the southbound left-turn |
| | movement from Washington Street to the access drive will |
| | eliminate potential back-ups that may occur on the railroad tracks |
| | and will dissipate the southbound queue of traffic quicker after a |
| | train event. |

Project Financing/Taxes

| Residents' Concerns | Petitioner's Response |
|--|---|
| Website Feedback: Bottom line - how | This project will require no increase in real estate taxes to the |
| much will this raise RE taxes for a | Village's residents and property owners. The March 8, 2022 |
| home taxed at \$400k? Dist. 58 already | Village Council discussion on the budget provides a more detailed |
| wants to raise out taxes \$300/ year | overview of the project funding. |
| and the park district wants to build | |
| another \$\$\$ rec center. What is the | |
| total cost for taxpayers from all taxing | |
| bodies?? | |
| Website Feedback: I would like to | This project will require no increase in real estate taxes to the |
| know what effect this will have on my | Village's residents and property owners. The March 8, 2022 |
| RE taxes. How much will they have to | Village Council discussion on the budget provides a more detailed |
| increase to build this plan | overview of the project funding. |

General/Site Plan

| Residents' Concerns | Petitioner's Response |
|--|--|
| Meeting Feedback: What will occur | The petitioner will consider this request. |
| with all the landscaping on the | |
| existing site and will there be an | |
| opportunity for residents and | |
| businesses to relocate plantings (i.e. | |
| shrubs, bulbs) | |
| Meeting Feedback: The outdoor civic | The petitioner will consider this request. |
| plaza should be designed with | |
| audio/visual capabilities for future | |
| community events. | |

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825 Burlington Ave. Downers Grove, IL 60515-4783 630.434.5600

Public Works

5101 Walnut Ave. Downers Grove, IL 60515-4046 630.434.5460

Telecommunications Tower FAQ

1. Why is there a telecommunications tower on the site and when was it constructed? The telecommunications tower provides required public safety (police and fire) communications, internally and with the dispatch center. The existing tower was constructed in the early 1980's.

2. What else is it used for?

In addition to the support of public safety communications, the tower also provides for wireless data & internet connections between the Village Hall and the Public Works facility. Three private telecommunications providers also access the tower for their equipment and make lease payments to the Village in exchange for this space. This practice of co-location is intended to result in fewer telecommunications towers within the Village.

3. Why is the proposed tower 190' (199' to the highest antenna)?

The height of the proposed tower was established to accommodate existing users and future users on a monopole structure. Additional tower capacity that allows for colocation of future telecommunications providers reduces the likelihood that new cellular communications towers will be constructed in close proximity to the proposed tower. The monopole design provides a narrower configuration and footprint, as opposed to the existing tower's wider lattice structure. This design allows for more vertical space to accommodate current and future equipment.

4. Why does the tower have to be relocated to the proposed site?

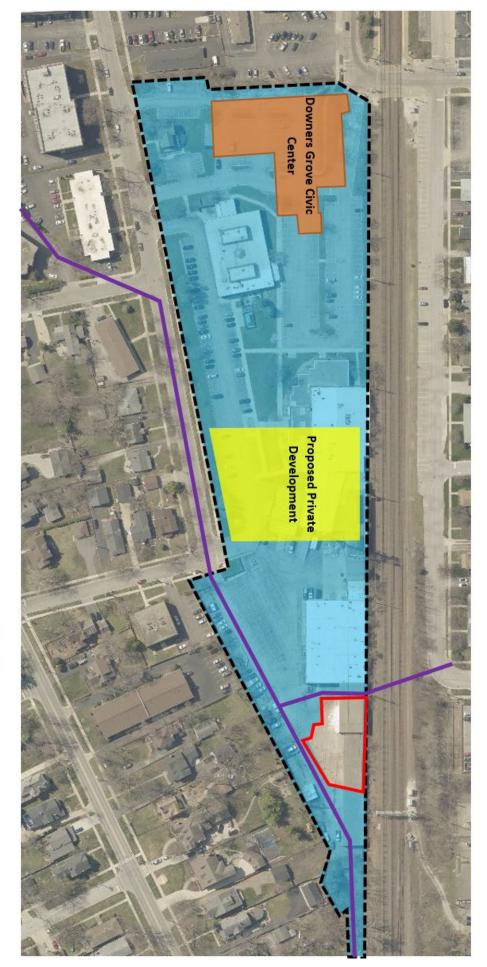
The proposed site is the only location on the existing Civic Center property that maintains a 200' fall-zone separation from nearby residential structures and from public safety operations in the new facility. This is to ensure that if the tower falls, it doesn't impact a residential structure or public safety facility. The proposed location also maintains access to the fleet maintenance garage and parking lot to the south. See attached tower siting location map.

5. Why can the tower not be relocated off site?

The proposed location in close proximity to the Police Station/Village Hall is critical to support public safety dispatching communications that the Village relies on for police, fire and EMS services. In addition, off-site relocation may result in construction of additional towers in the immediate vicinity by private telecommunications providers as allowed by the Federal Telecommunications Act.

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Telecommunication Tower Location Map Analysis



Available Location

Storm Sewer

Within 200 Feet of DGCC or residential structures

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NEIGHBORHOOD OPEN HOUSE

MONDAY, MARCH 21, 2022 7:00 P.M. - 8:30 P.M.

VILLAGE HALL 801 BURLINGTON AVENUE DOWNERS GROVE, IL 60515



Dear Neighbor,

The Village is preparing for the construction of a combined Civic Center facility, which will be the new home of Village Hall, the Police Department and the administrative offices of Downers Grove Grade School District 58.

We invite you to an Open House that will feature information about the project in advance of the April 4, 2022, Plan Commission meeting. Guests may arrive at any time during the Open House hours to view project information and offer comments. Village staff will be available to answer questions.

Please note that your property is not part of the proposed redevelopment. You are receiving this invitation because you are located within 250 feet of the Civic Center Campus.

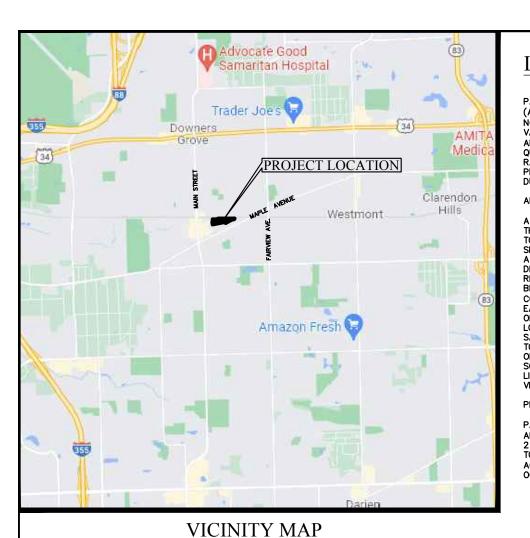
Questions? Contact Jason Zawila at 630.434.5520 or email jzawila@downers.us

For additional information on the Facilities Project, visit downers.us/facilities



Architectural rendering of proposed facility.

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LEGAL DESCRIPTION

PARCEL 1: LOTS 1 THROUGH 11, BOTH INCLUSIVE, AND THE WEST 11.3 FEET (AS MEASURED ON THE SOUTH LINE) BY 16.85 FEET (MEASURED ON THE NORTH LINE) OF LOT 12 IN STANLEY'S SUBDIVISION, ALSO THAT PART OF VACATED RAILROAD STREET LYING NORTH OF AND ADJOINING LOTS 1, 2, 3, 4 AND 5 IN SAID STANLEY'S SUBDIVISION, OF PART OF THE NORTHWEST QUARTER AND THE SOUTHWEST QUARTER OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO* THE* PLAT THEREOF RECORDED FEBRUARY 18, 1884 AS DOCUMENT 32879, IN

A PART OF LOT 12 OF THE PLAT OF STANLEY'S SUBDIVISION OF PART OF THE NORTHWEST QUARTER AND THE SOUTHWEST QUARTER OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11 EAST OF THE THIRD PRINCIPAL MERIDIAN, AS SHOWN BY THE PLAT RECORDED IN BOOK OF PLATS, ON PAGE 43, AND ALSO A PART OF LOT 1 OF THE PLAT OF THE HOMESTEAD OF THOMAS LYMAN, DECEASED, AS RECORDED IN BOOK 6 OF PLATS, ON PAGE 5, OF THE RECORDS OF DUPAGE COUNTY, ILLINOIS; THE ENTIRE TRACT HEREIN INCLUDED, BEING DESCRIBED AS FOLLOWS, TO WIT: COMMENCING AT THE SOUTHWEST COMMENCE OF SUBDIVISION AND PUNISHING THEREOF BEING DESCRIBED AS FOLLOWS, TO WIT: COMMENCING AT THE SOUTHWEST CORNER OF SAID LOT 12, OF STANLEY'S SUBDIVISION, AND RUNNING THENCE EAST ALONG THE NORTH LINE OF CURTISS STREET, 11.3 FEET FOR A PLACE OF BEGINNING; THENCE NORTHERLY TO A POINT ON THE NORTH LINE OF SAID LOT 12, WHICH POINT IS 16.85 FEET EAST FROM THE NORTHWEST CORNER OF SAID LOT; THENCE EAST, ON THE NORTH LINE OF SAID LOT 12, 53.75 FEET TO THE NORTHEAST CORNER OF SAID LOT, THENCE SOUTHERLY, TO A POINT ON THE NORTH LINE OF CURTISS STREET WHICH IS 7 FEET EAST FROM THE SOUTHEAST CORNER OF SAID LOT 12; THENCE WESTERLY ALONG THE NORTH LINE OF CURTISS STREET, 50 FEET TO THE PLACE OF BEGINNING, IN THE VILLAGE OF DOWNERS GROVE, DUPAGE COUNTY, ILLINOIS.

AND THE EAST 5 1/2 FEET OF LOT 7 IN RANDALL'S RESUBDIVISION OF BLOCK 2 IN THE TOWN OF DOWNERS GROVE, IN THE WEST HALF OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN,

P.U. & D.E. = PUBLIC UTILITY AND

DRAINAGE EASEMENT

२०२२ O OVERHEAD TRAFFIC

——— BUILDING SETBACK LINE

———— SECTION LINE

PARCEL 3: THE NORTH 35 FEET (AS MEASURED AT RIGHT ANGLES TO THE NORTHEASTERLY LINE OF SAID LOT) OF LOT 6, IN WALLACE'S SUBDIVISION OF PART OF LOT 5 OF THE ASSESSOR'S SUBDIVISION OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE

PARCEL 4: THE NORTH 35 FEET (AS MEASURED AT RIGHT ANGLES TO THE NORTHERLY LINE OF SAID LOT) OF LOT 7, IN WALLACE'S SUBDIVISION OF PART OF LOT 5 OF THE ASSESSOR'S SUBDIVISION OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE

DECEASED (EXCEPT THAT PART CONVEYED TO THE CHICAGO BURLINGTON & QUINCY RAILROAD AND EXCEPT THAT PART CONVEYED TO H.J. HAWKINS ON MARCH 27, 1907) IN SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOIS. PIN: 09-08-211-001

PARCEL 5: LOT 1 OF THE PLAT OF THE HOMESTEAD OF THOMAS LYMAN.

QUARTER (SW1/2 NE1/2) OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11 EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOIS, DESCRIBED AS FOLLOWS: COMMENCING AT A POINT ON THE SOUTH LINE OF 100-FOOT WIDE RIGHT OF WAY OF BURLINGTON NORTHERN, INC., 336.6 FEET EASTWARD (MEASURED ON SAID SOUTH LINE) FROM THE INTERSECTION OF THE NORTH AND SOUTH CENTER LINE OF SAID SECTION 8 WITH SAID SOUTH LINE TO THE PLACE OF

PARCEL 7: LOTS 3, 4, 5 AND LOT 6 (EXCEPT THE WEST 50 FEET THEREOF) IN BLOCK 2 IN CURTISS ADDITION TO THE TOWN OF DOWNERS GROVE, BEING A SUBDIVISION OF PART OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO THE PLAT THEREOF RECORDED MARCH 8, 1866 AS DOCUMENT 7317, IN DUPAGE COUNTY, ILLINOIS.

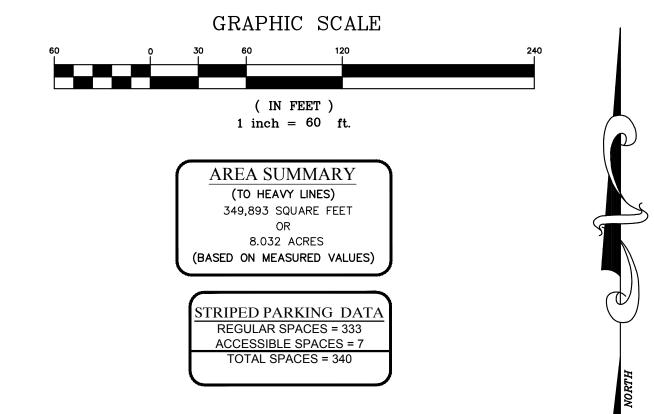
PARCEL 8: LOTS 1 AND 2 IN BLOCK 2 IN CURTISS ADDITION TO THE TOWNS OF DOWNERS GROVE, BEING A SUBDIVISION OF PART OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO THE PLAT THEREOF RECORDED MARCH 8, 1866 AS DOCUMENT 7317, IN

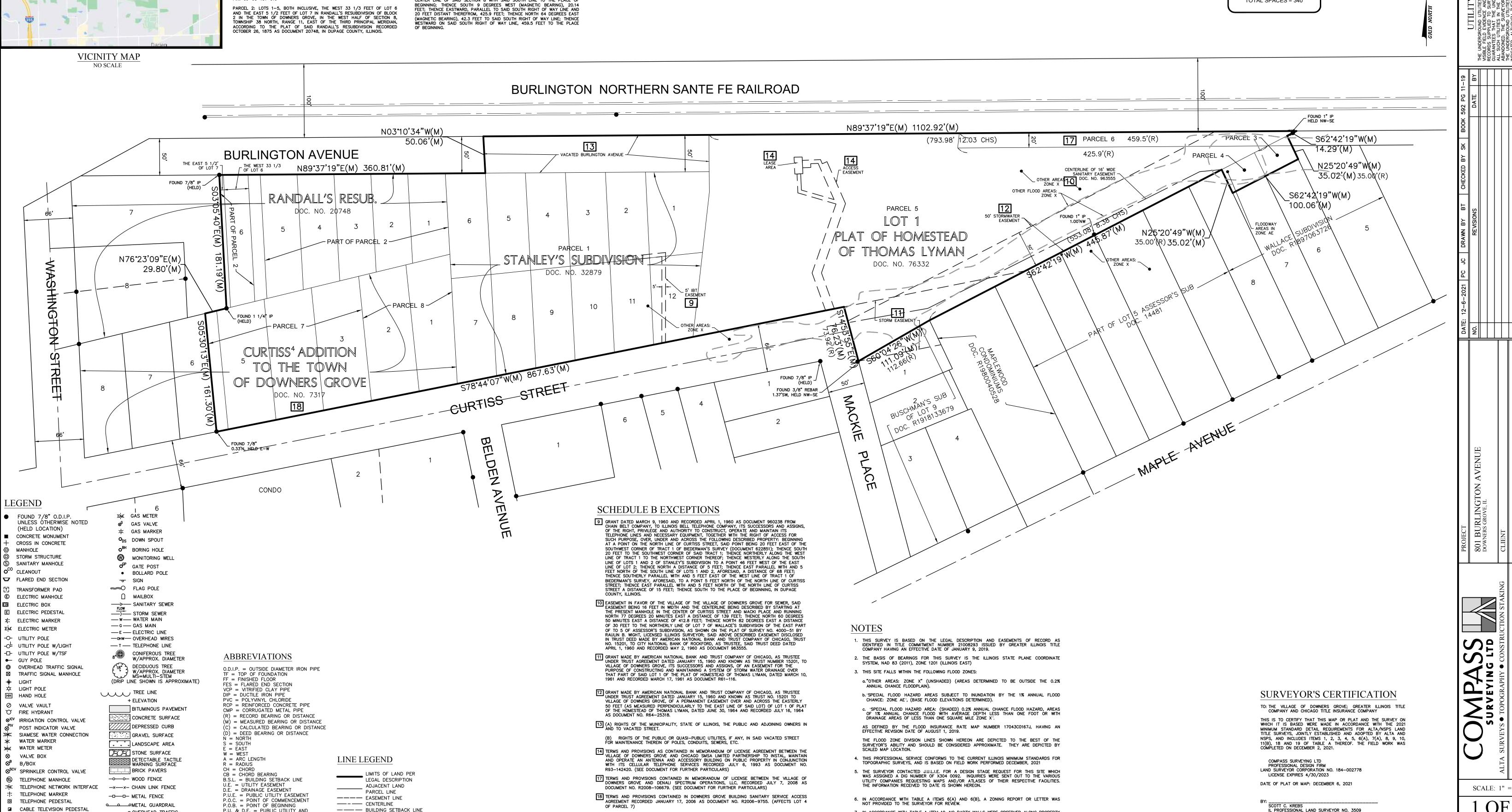
*TYPOGRAPHICAL ERROR CORRECTED BY SURVEYOR

ALTA/NSPS LAND TITLE AND TOPOGRAPHIC SURVEY

7. IN ACCORDANCE WITH TABLE A ITEM 10, NO PARTY WALLS WERE OBSERVED ALONG PROPERTY

LINES CONTIGUOUS WITH ADJOINING PROPERTIES.





SCHEDULE B EXCEPTIONS 1, 2, 3, 4, 5, 6, 7, 8, 15, AND 16 ARE NOT SURVEY RELATED AND THEREFORE NOT SHOWN



Call before you did

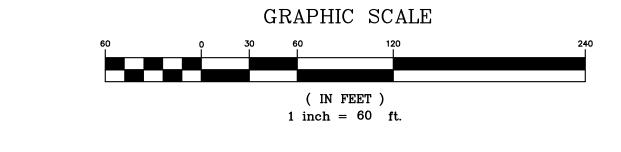
SCALE: 1'' = 60'

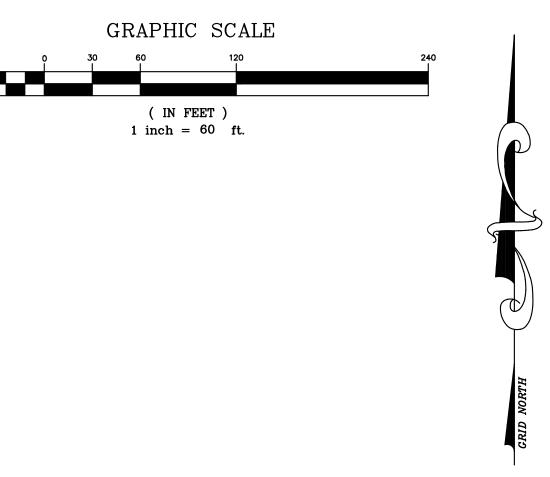
IL PROFESSIONAL LAND SURVEYOR NO. 3509

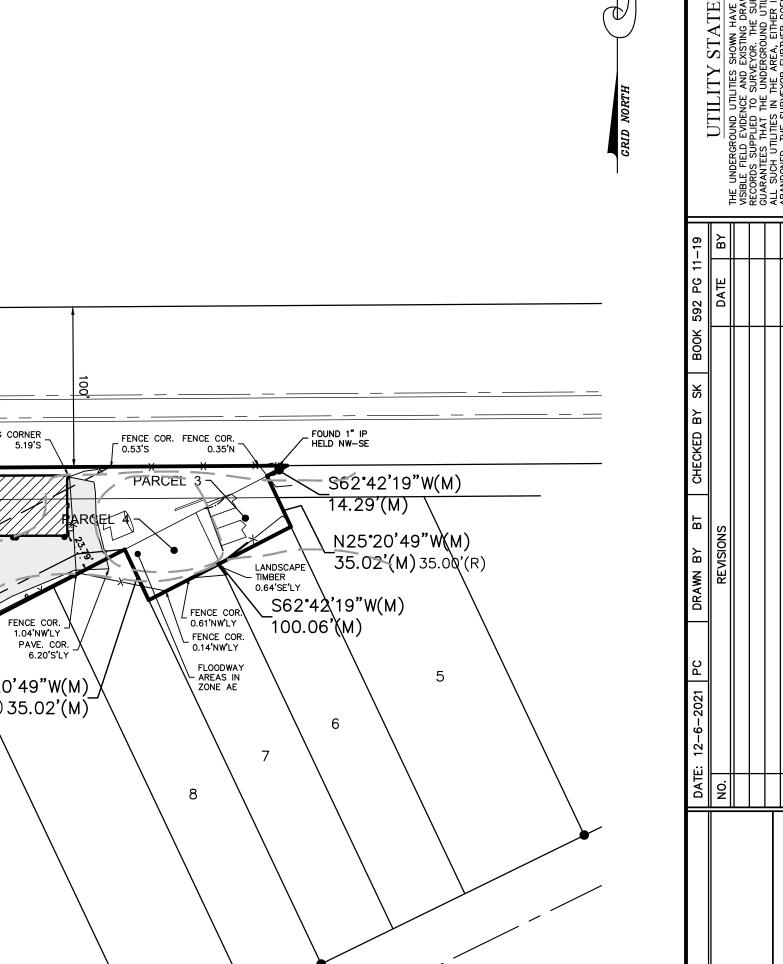
J: \PSDATA\2021 PROJECTS\21.0319\21.0319 LTS.DWG

LICENSE EXPIRES 11/30/22

ALTA/NSPS LAND TITLE AND TOPOGRAPHIC SURVEY





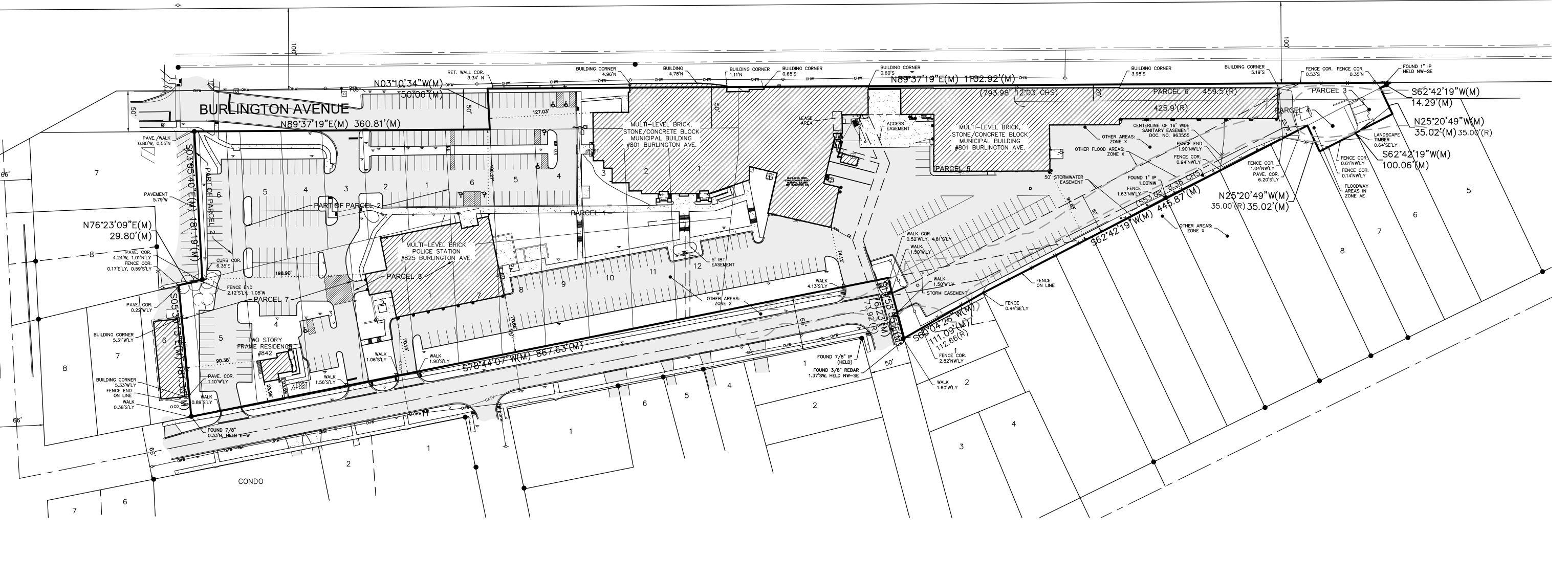




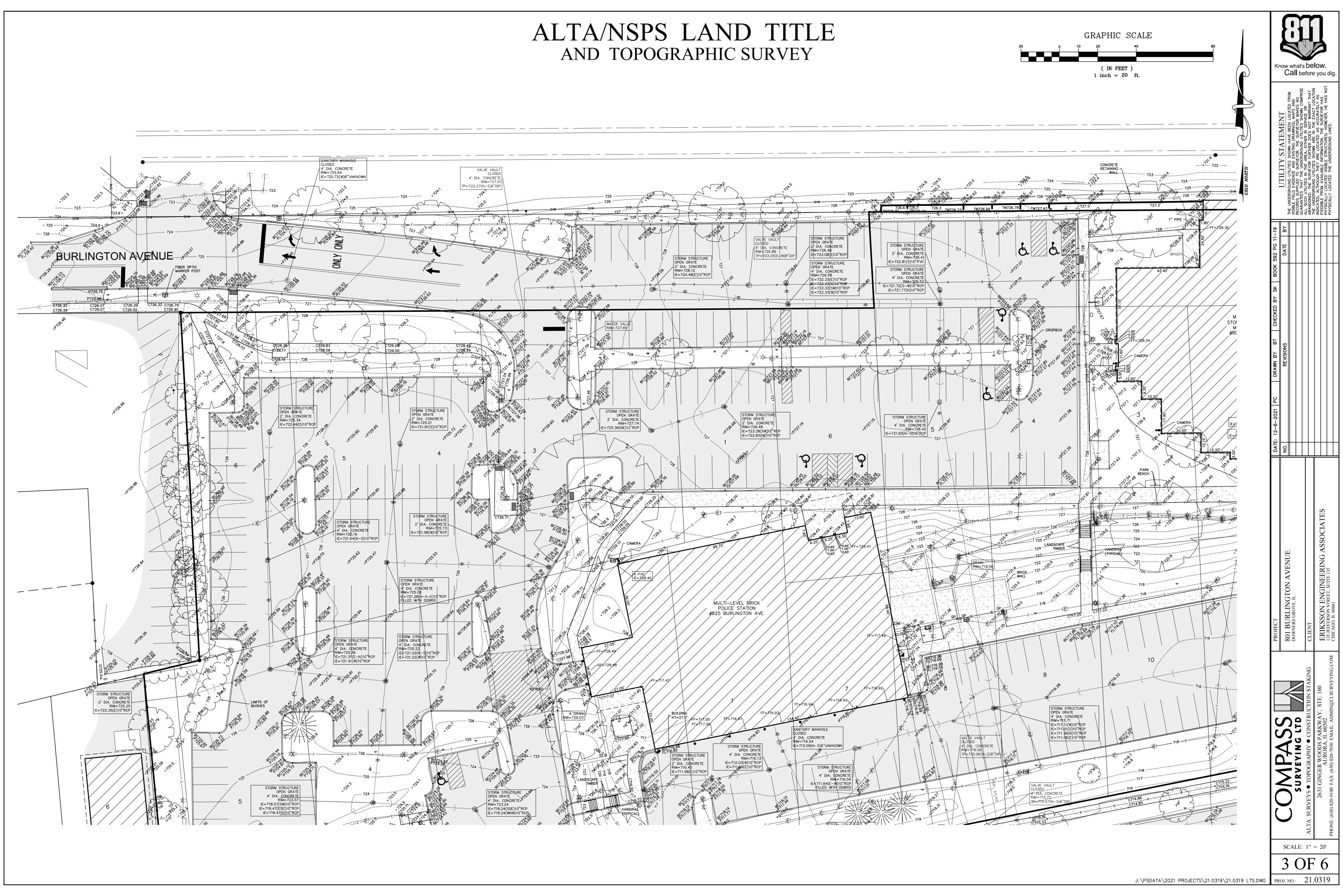
Know what's **below. Call** before you dig.

2 OF 6

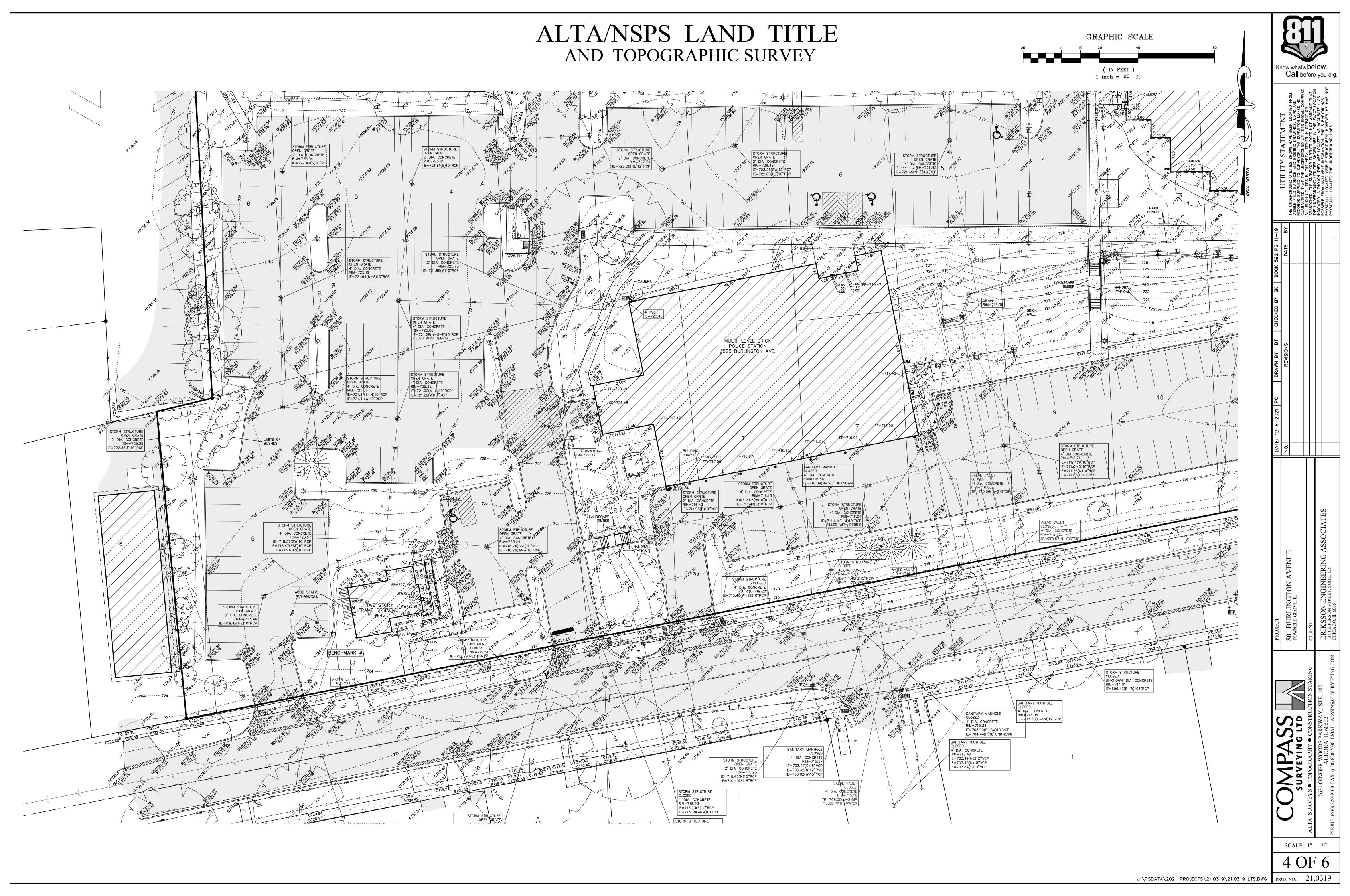
J:\PSDATA\2021 PROJECTS\21.0319\21.0319 LTS.DWG



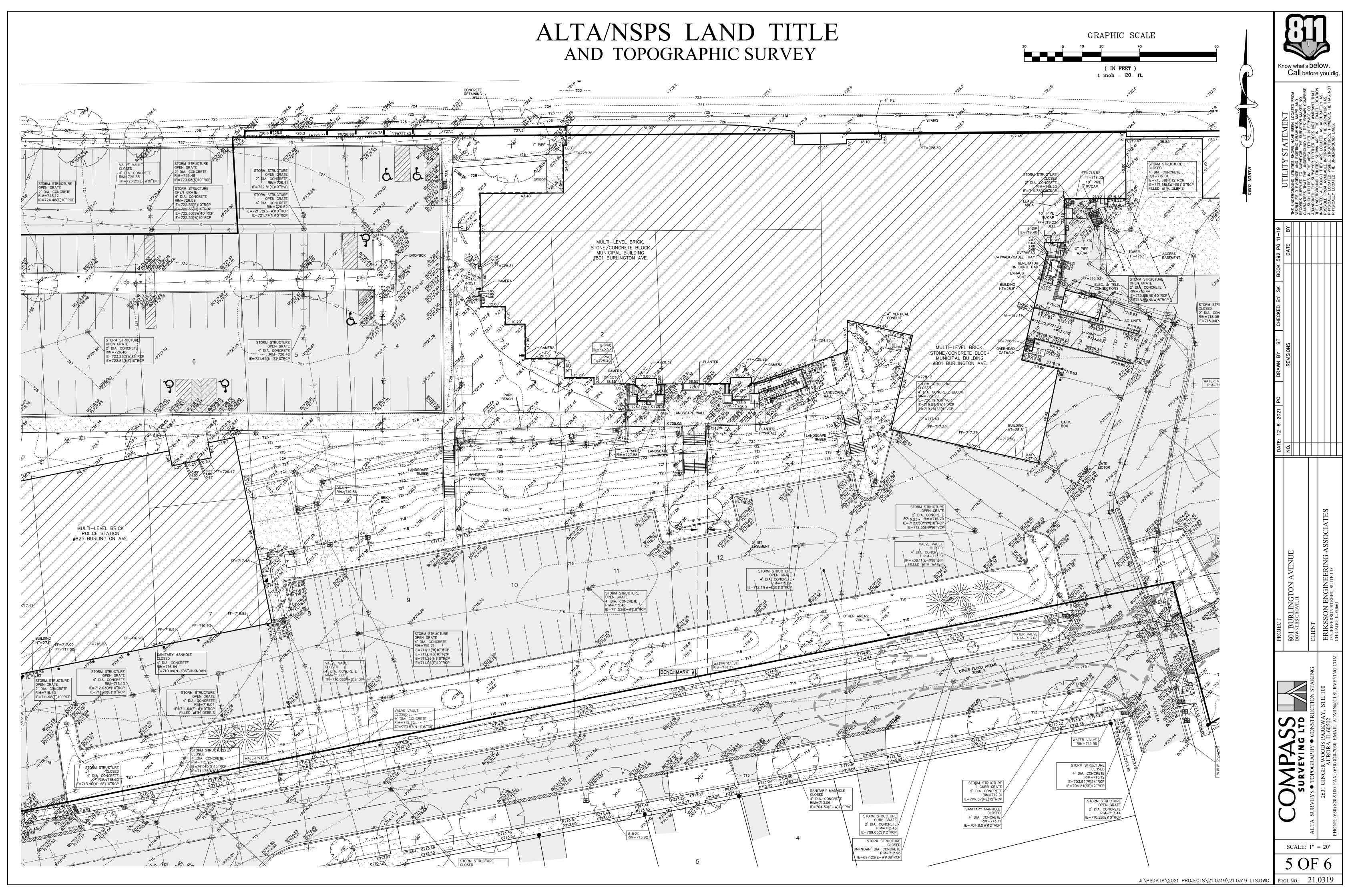
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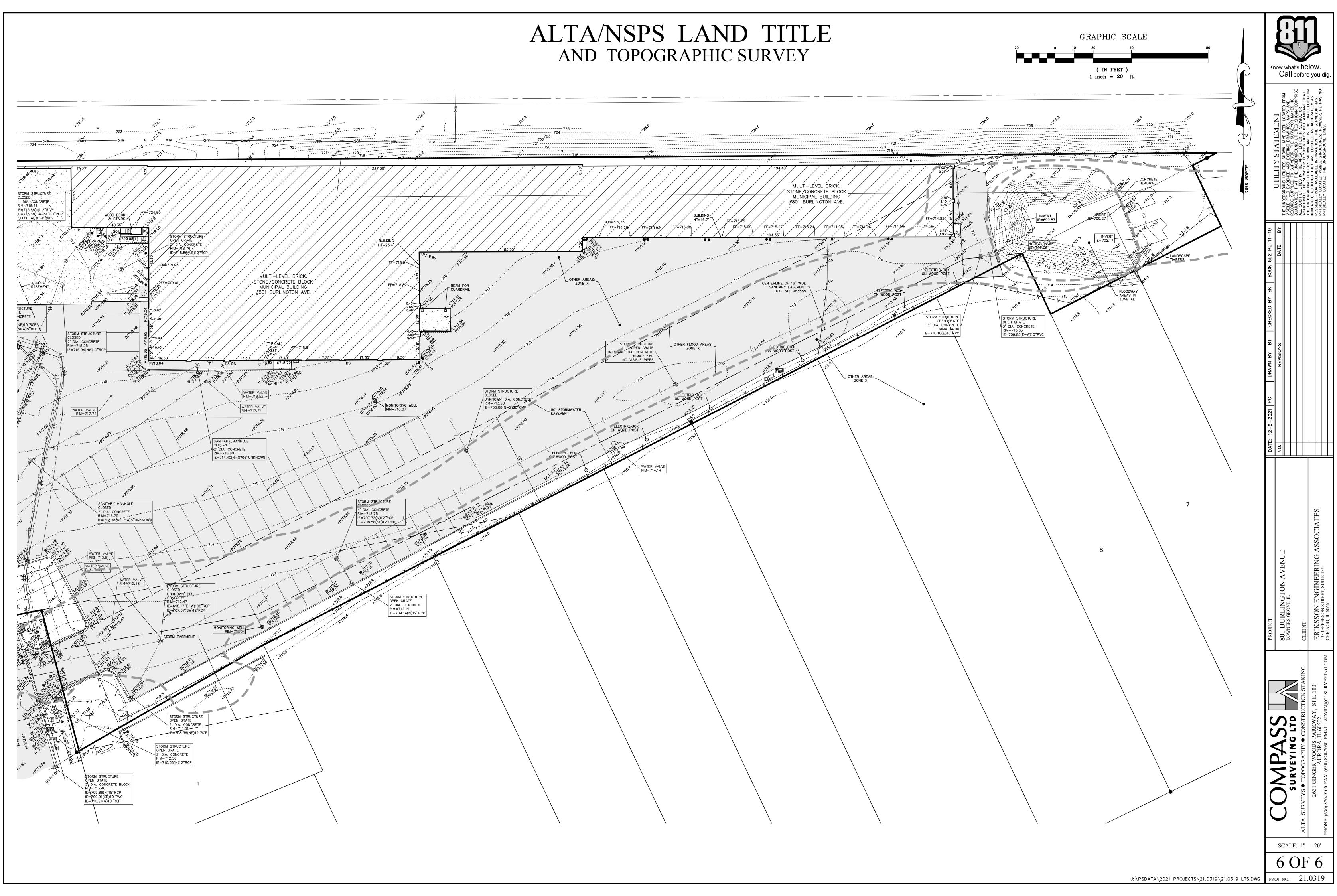
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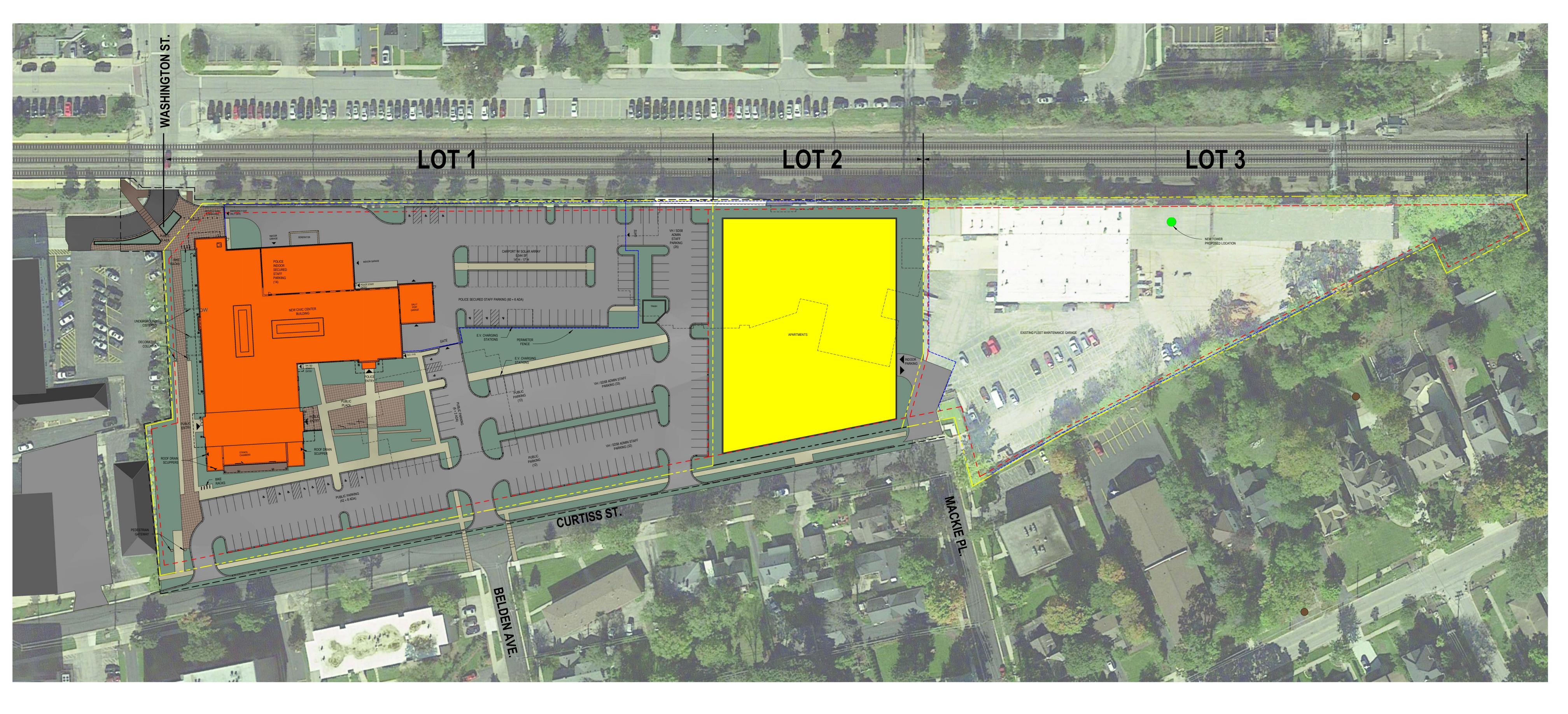


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Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515



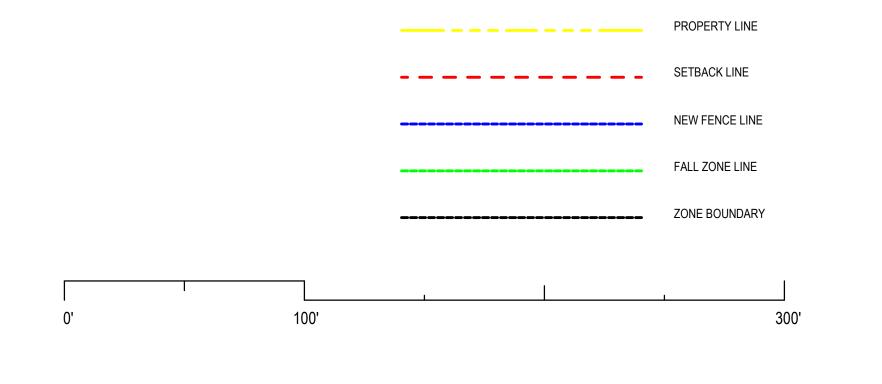
NEW VHPS BUILDING + SCHOOL DISTRICT 58 ADMINISTRATION

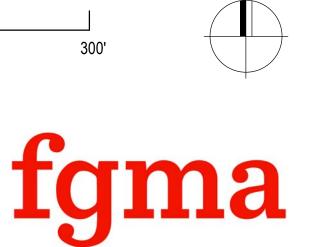
• NEW CIVIC CENTER BUILDING = 80,000 SF +/- (2 STORY + BASEMENT)

 SECURE POLICE STAFF PARKING = 80 SPACES (14 INTERIOR + 24 CARPORT + 42 EXTERIOR)

• VILLAGE HALL STAFF PARKING = 65 EXTERIOR SPACES SD58 STAFF PARKING = 25 EXTERIOR SPACES • PUBLIC VISITOR PARKING = 82 EXTERIOR SPACES

TOTAL PARKING SPACES = 252 SPACES



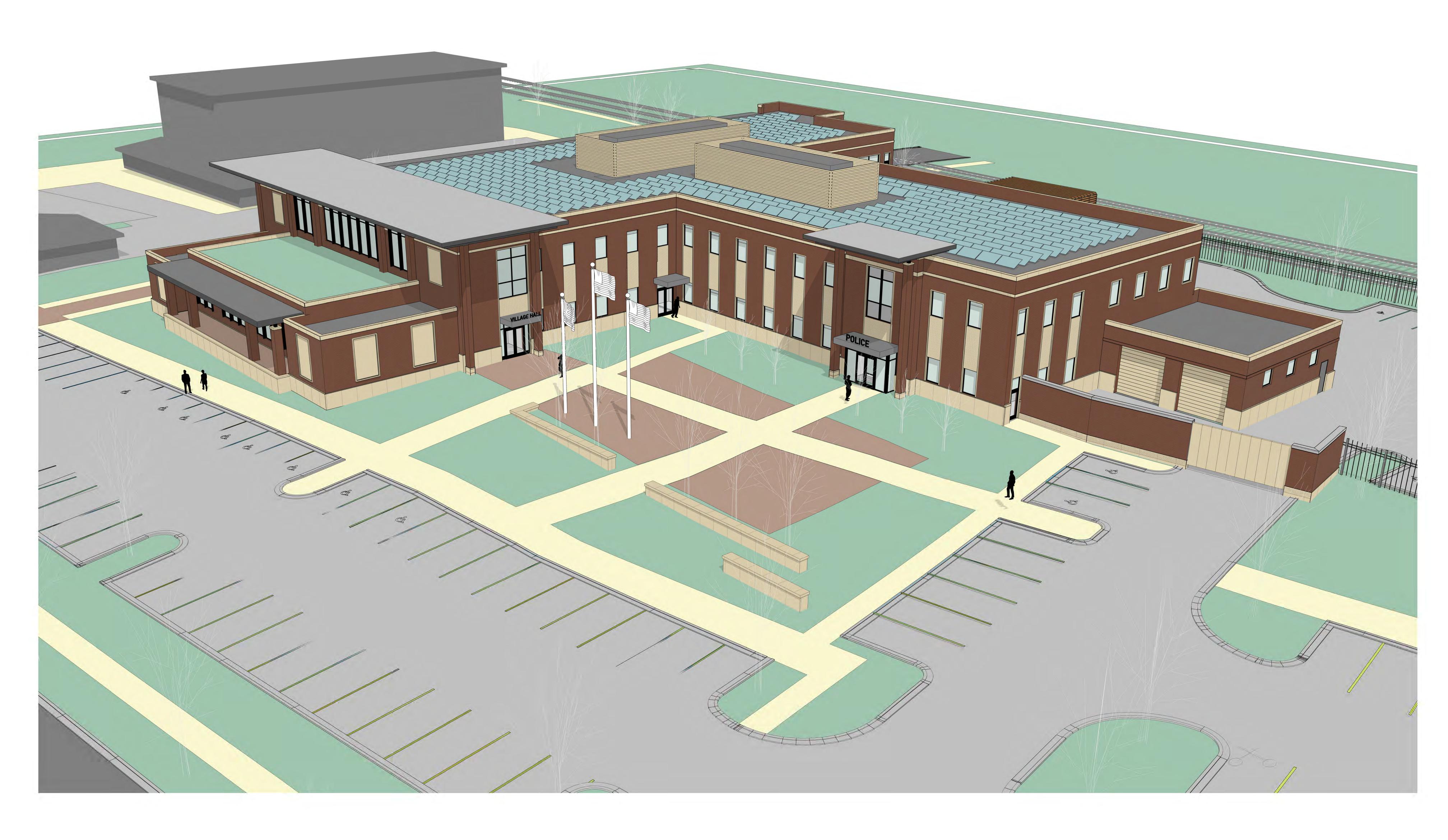




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Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515



BIRDSEYE VIEW FROM SOUTHEAST



Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515





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Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515





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Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515



BIRDSEYE VIEW FROM SOUTHWEST



RD 2022-9422

Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515





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Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515





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Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515



2 SOUTH ELEVATION
1/8" = 1'-0"



BUILDING ELEVATIONS

ESTABLISHED GRADE CALCULATION

742.2' @ NW CORNER STAIR TOWER
724.5' @ SW CORNER OF COUNCIL CHAMBERS
724.0' @ SE CORNER OF COUNCIL CHAMBERS
725.1' @ SE CORNER OF POLICE WING
SUM OF GRADES / 4 = ESTABLISHED GRADE



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Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515





BUILDING ELEVATIONS

ESTABLISHED GRADE CALCULATION

742.2' @ NW CORNER STAIR TOWER
724.5' @ SW CORNER OF COUNCIL CHAMBERS
724.0' @ SE CORNER OF COUNCIL CHAMBERS
725.1' @ SE CORNER OF POLICE WING

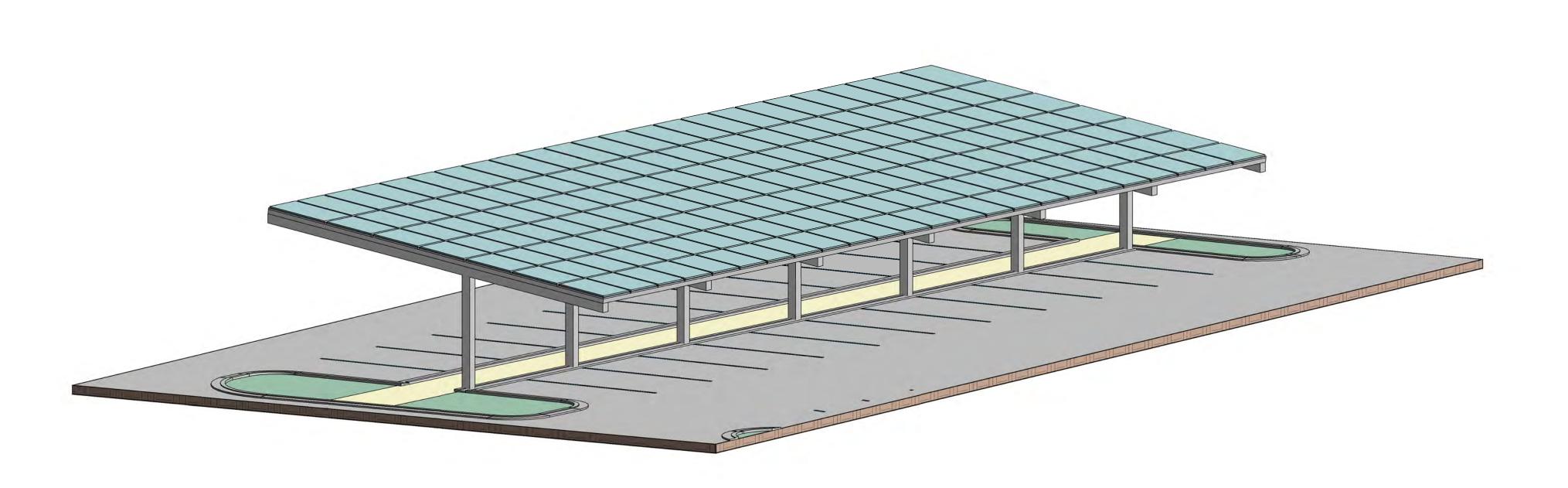
SUM OF GRADES / 4 = ESTABLISHED GRADE

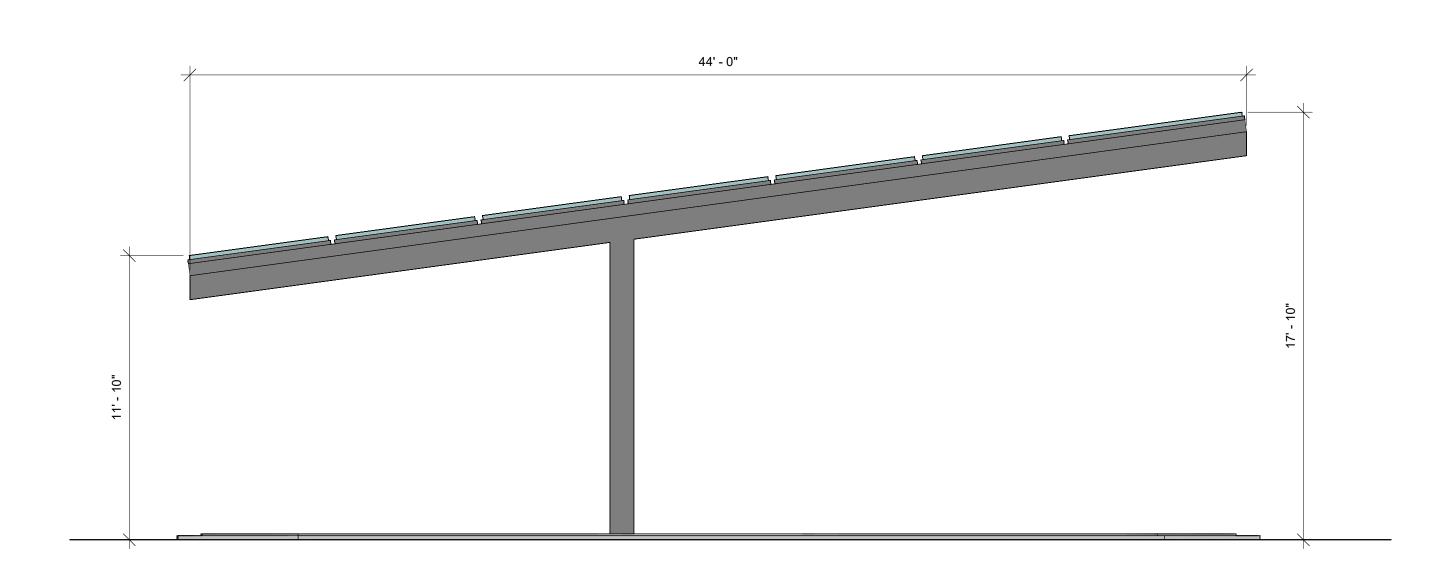


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Downers Grove Civic Center

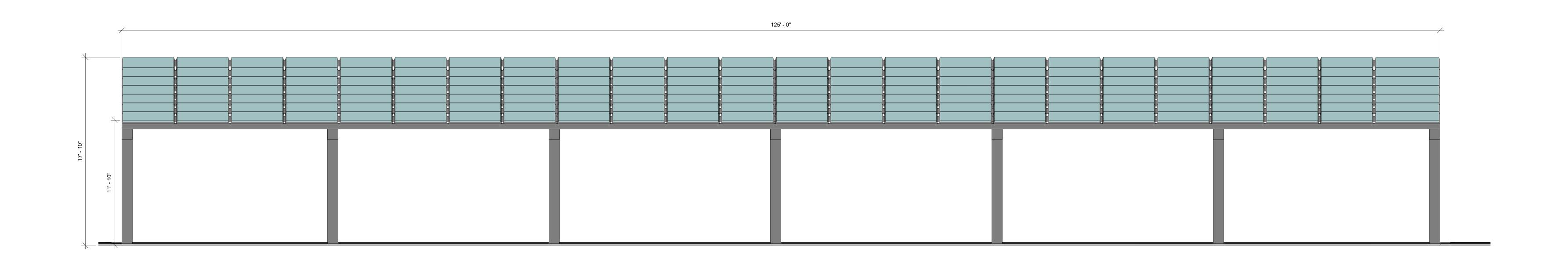
825 Burlington Ave, Downers Grove, IL 60515





1 CARPORT - 3D AXONOMETRIC VIEW

2 CARPORT - EAST ELEVATION
1/4" = 1'-0"



3 CARPORT - SOUTH ELEVATION
1/4" = 1'-0"

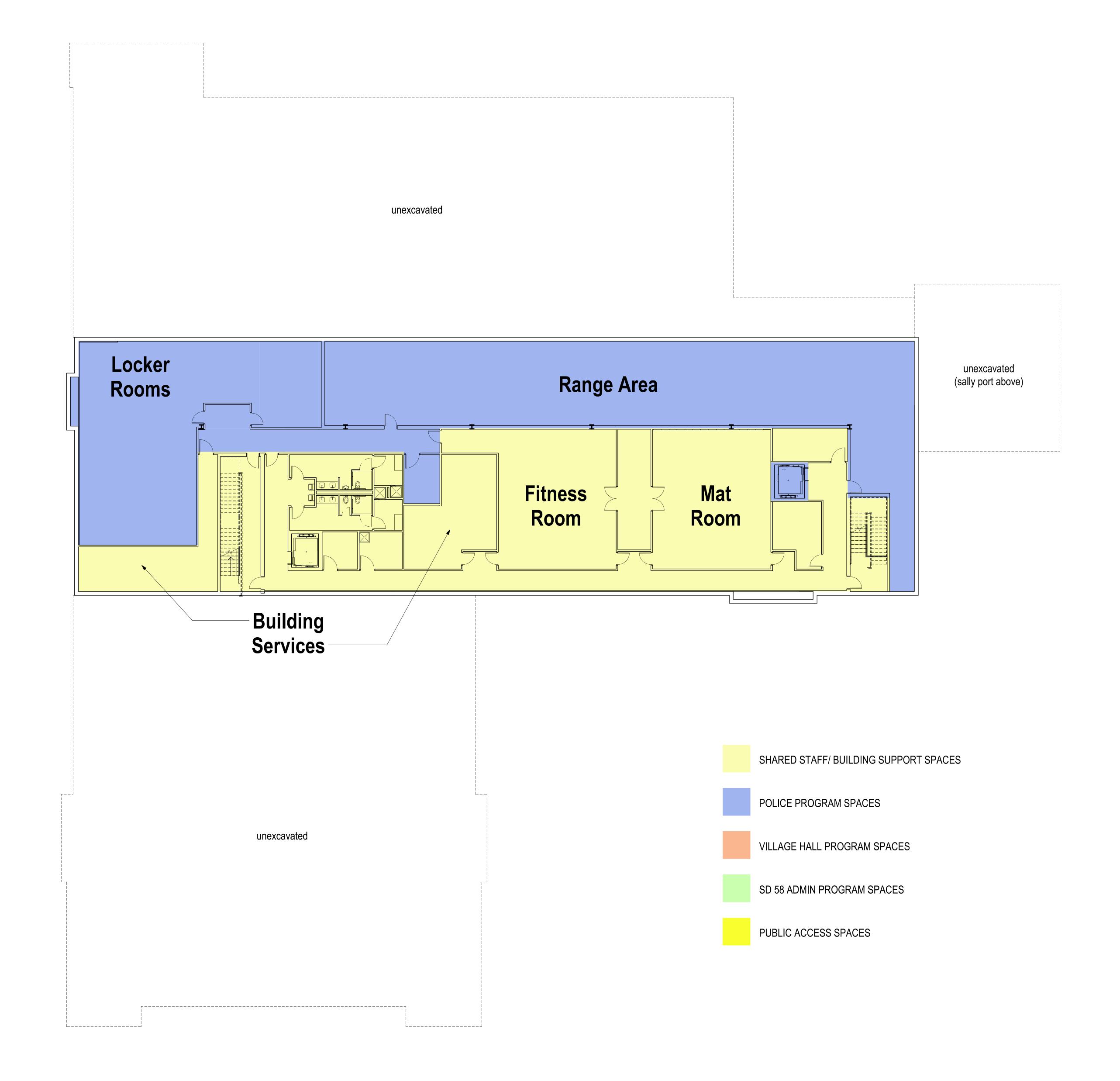




ORD 2022-9422

Downers Grove Civic Center

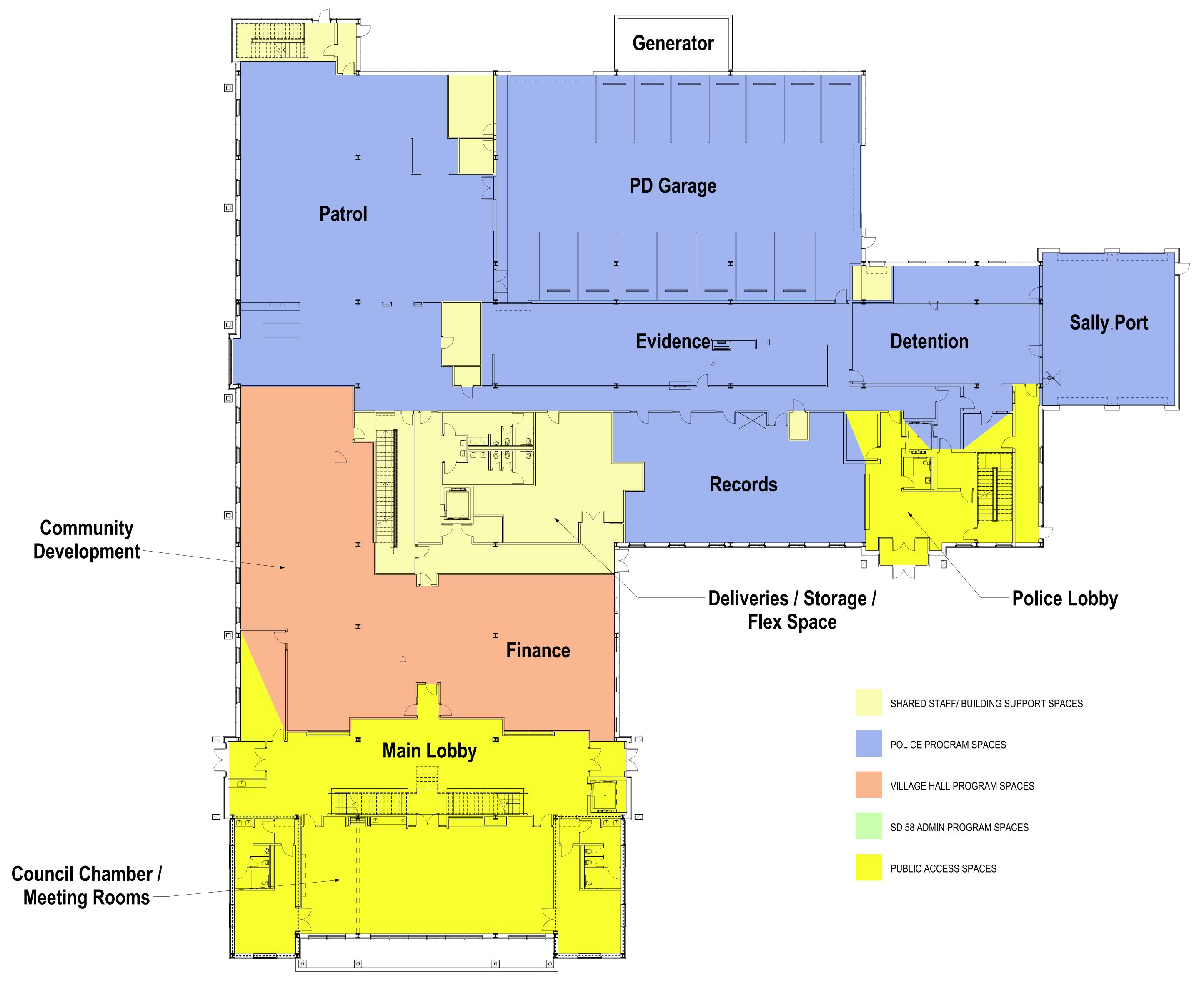
825 Burlington Ave, Downers Grove, IL 60515





Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515





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Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515



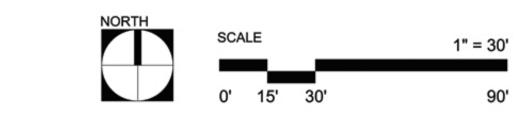


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Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515

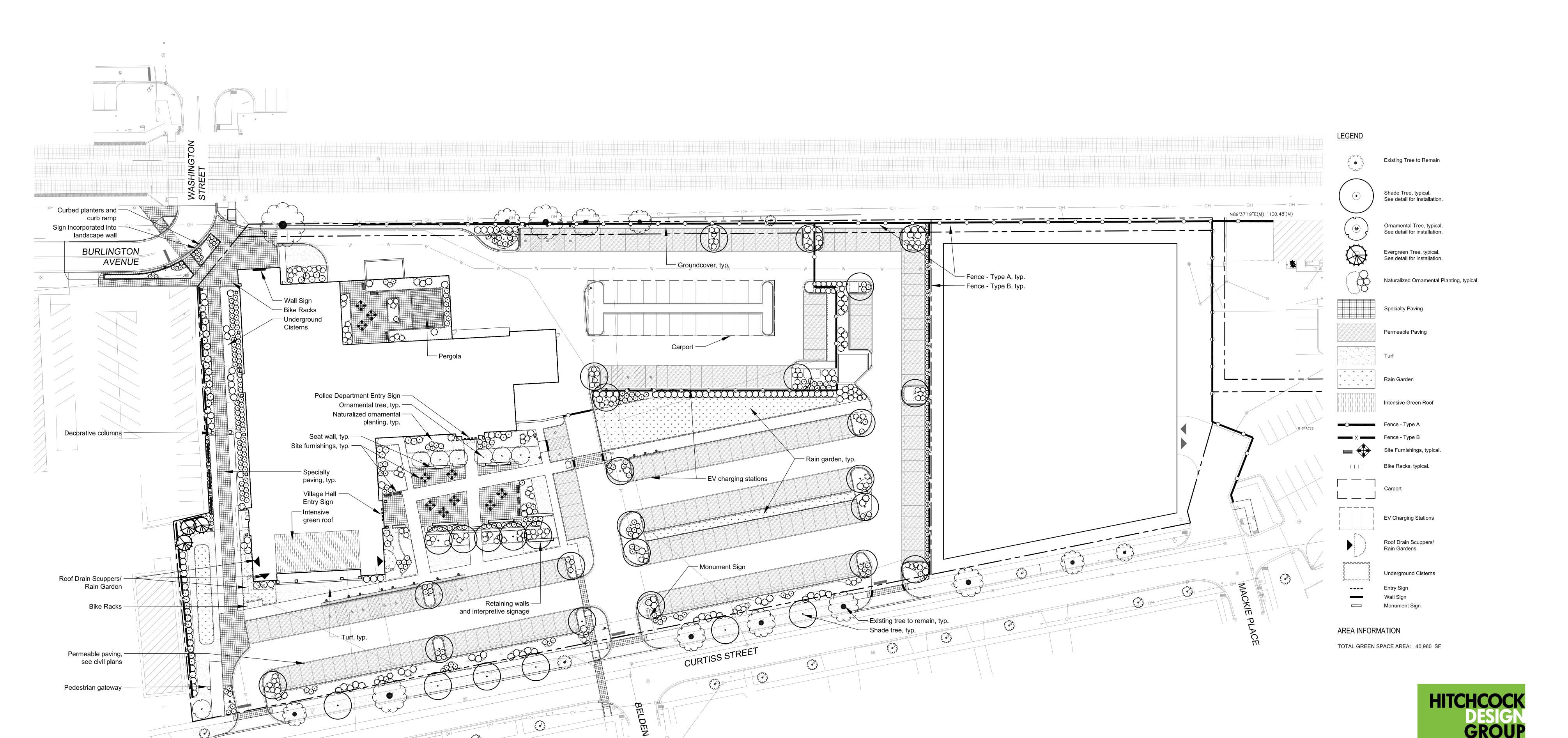


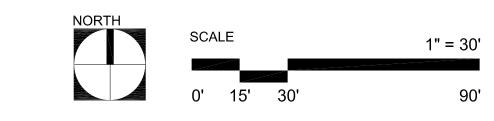




Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515







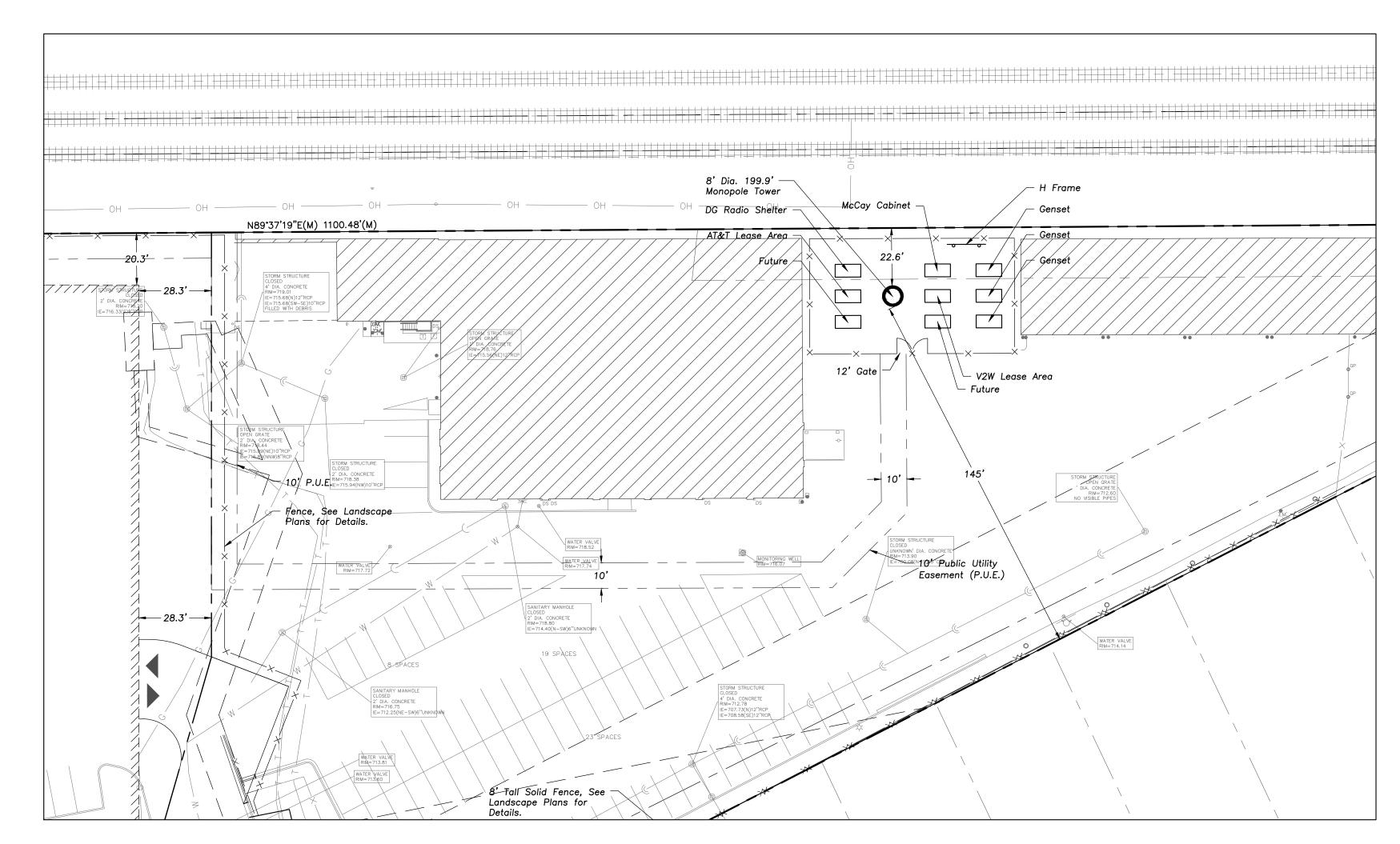
SCHEMATIC LANDSCAPE & SIGNAGE PLAN

Village of Downers Grove Job No. 22-3818.01 Published 03/29/2022 FGM Architects Inc.

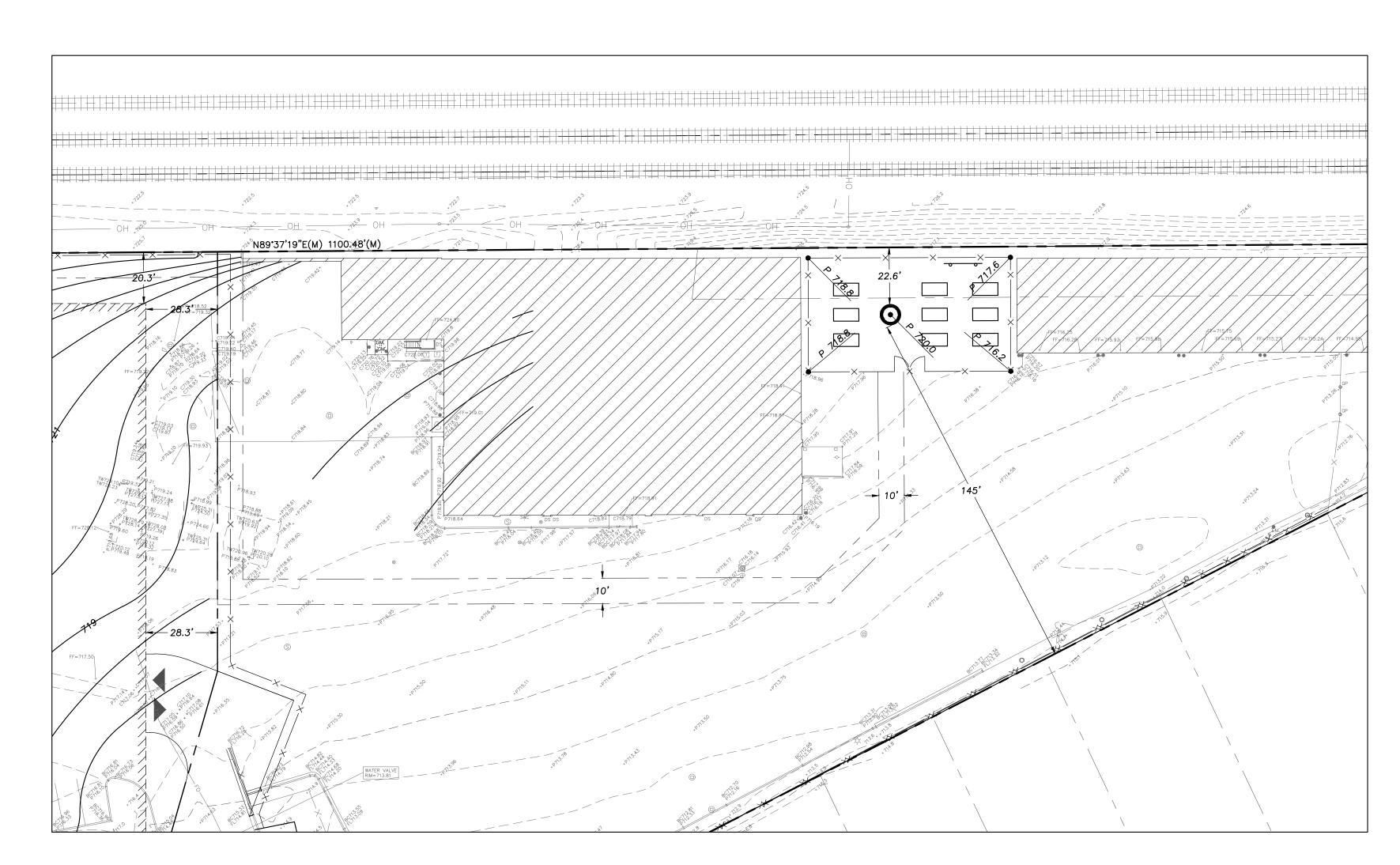


Downers Grove Civic Center

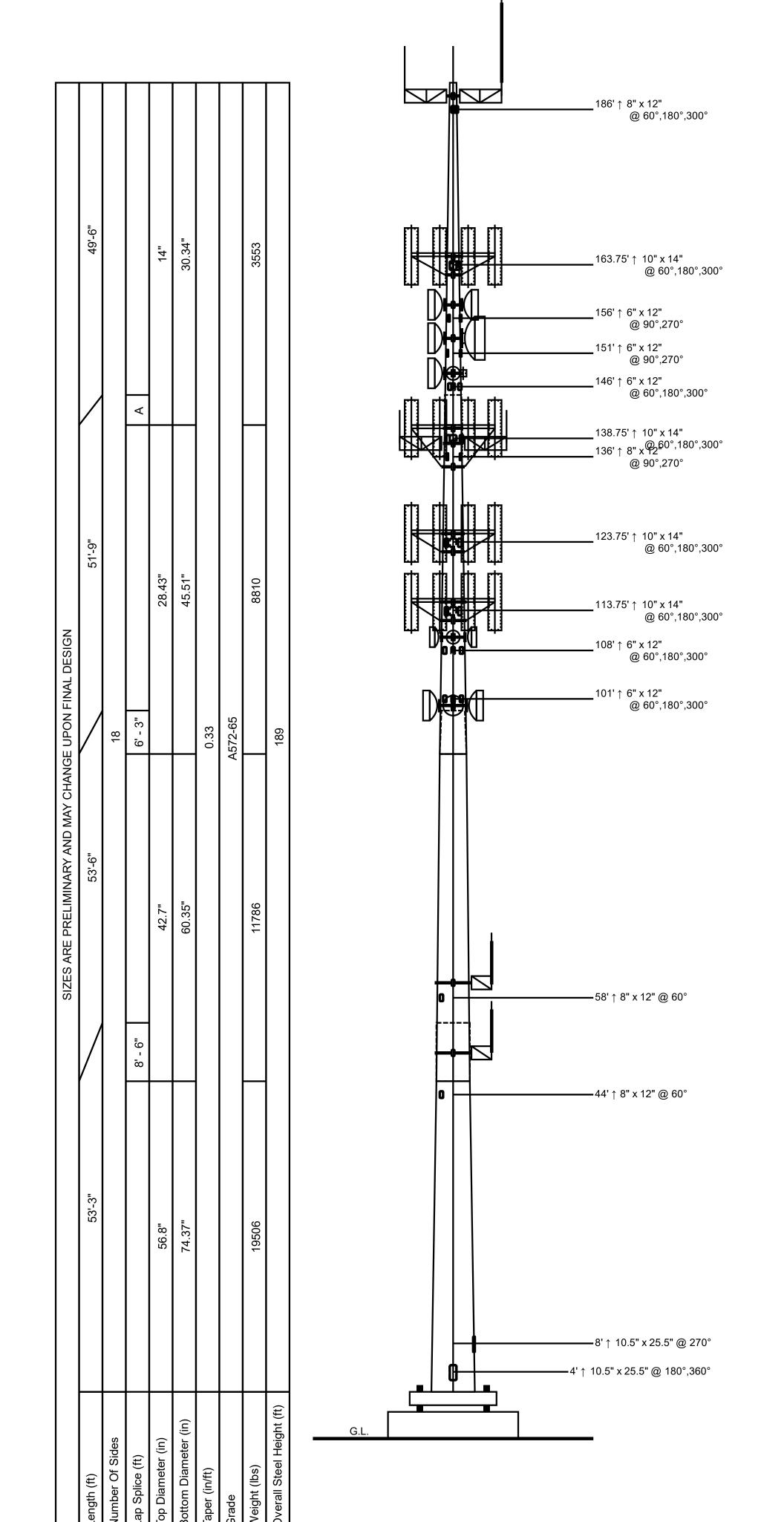
825 Burlington Ave, Downers Grove, IL 60515



MONOPOLE GEOMETRY AND UTILITY PLAN



MONOPOLE GRADING PLAN



GEOMETRY NOTES

- 1. All Dimensions Contained Herein Reference Back Of Curb, Face Of Retaining Wall, Edge Of Pavement, Center of Structure And Outside Face Of Building Foundation Unless
- 2. All Pavement Striping Shall Be 4" Wide Yellow Paint Per Specifications, **Two Coats for Latex Paints**. All Cross Hatch Striping Shall Be 45° At 2'-0" Centers.
- 3. All Accessible Parking Signs (R7-8) Must Be Placed at the Center of the Space and Within 5 Feet of the Space.
- 4. Refer to Architectural Drawings for Locations and Details of All Permanent Site Fencing.
- 5. Traffic Sign Posts Shall Be Breakaway Green U-Channel Posts, 2-lb/ft, 11 Gauge Steel, Embedded 42" Minimum

| | Walliole | |
|--|------------------------------------|-------------------------|
| \oplus | Catch Basin | |
| | Inlet | |
| \triangle | Area Drain | A |
| O _{c.o.} | Clean Out | O _{c.o.} |
| | Flared End Section | |
| —————————————————————————————————————— | Storm Sewer | |
| ——)—— | Sanitary Sewer | |
| | Combined Sewer | |
| W | Water Main | ——w— |
| ——— G ——— | Gas Line | ——G—— |
| OH | Overhead Wires | ——ОН—— |
| ——E—— | Electrical Cable (Buried) | ——Е—— |
| T | Telephone Line | ——т— |
| 2 | Fire Hydrant | A |
| \otimes | Valve Vault | • |
| \bigotimes_{B} | Buffalo Box | $oldsymbol{\Theta}_{B}$ |
| O_{DS} | Downspout | O_DS |
| \bigcirc_{BOL} | Bollard | O _{BOL} |
| $\otimes_{_{\mathbb{G}}}$ | Gas Valve | |
| \mathbb{M}_{G} | Gas Meter | |
| \mathbb{M}_{E} | Electric Meter | |
| (i) | ComEd Manhole | |
| E [H] | Hand Hole | |
| Ä | Light Pole | * |
| | Light Pole w/ | |
| 0 Q | Mast Arm | |
| -0- | Utility Pole | - |
| \boxtimes_{T} | Telephone Pedestal | |
| ○ _T | Telephone Manhole | |
| d | Sign | • |
| ×× | Fence | xx |
| G | Accessible Parking Stall | 5 . |
| | Curb & Gutter | |
| | Depressed Curb | |
| x C 782.50 | Curb Elevation | C 782.50 |
| x G 782.00 | Gutter Elevation | G 782.00 |
| x P 783.25 | Pavement Elevation | P 783.25 |
| × W 782.10 | Sidewalk Elevation | <u> W 782.10</u> |
| × 784.0 | Ground Elevation | _x 784.0 |
| × T/W 785.20 | Top of Retaining Wall Elevation | T/W 785.20 |
| • • • • • | Swale | ••••• |
| -781 | | -781 |

LEGEND

EXISTING

UTILITY NOTES

- 1. Utility Service Lines as Shown Hereon are Approximate. Coordinate The Exact Locations With The Plumbing Drawings. Coordinate The Locations With The Plumbing Contractor and/or the Owner's Construction Representative Prior to Installation of Any New Utilities.
- Refer to Plumbing Drawings for Continuation of All Utilities Within 5 Feet of Building Face. Field Verify Invert & Locations of Existing Utility Mains Prior to Installing Any On—Site Utilities or Structures. All Elevations and Inverts Referencing Said Utility Shall Be
- Field Verified Prior To Installation Of Any New Structures Or Utilities, and Adjustments Shall Be Made as Necessary. Contact Éngineer Prior to Installation if Discrepancy Exists With These Drawings. 4. Coordinate the Relocation Of Any Utilities Encountered And
- Replacement Of Any Utilities Damaged Within Influence Zone Of New Construction. Contact Engineer If The Existing Utilities Vary Appreciably From The Plans. 5. All Water Main and Services Shall Be Installed at a
- Elevation to Top of Main. 6. Protection of water supplies shall be as described in Section 370.350 of the Illinois Recommended Standards

Specifications for Water and Sewer Main Construction in

Minimum Depth of 5.5' From Top of Finished Ground

7. Clean Out All Existing and Proposed Storm Inlets and Catch Basins at the Completion of Construction.

Illinois, latest edition.

- 8. Provide Adequate Coupling Device and/or Oversized Concrete Flared—End Section to Accommodate HDPE Storm
- 9. The "Standard Specifications for Water and Sewer Main Construction in Illinois", Current Edition Shall Govern Work Where Applicable.
- Rebuild Existing Structures and Adjust Rim Elevations to Match Proposed Ground Elevations.

GENERAL NOTES

Deciduous Tree

1. The Location of Existing Underground Utilities, Such As Watermains, Sewers, Gas Lines, Etc., As Shown On The Plans, Has Been Determined From The Best Available Information and Is Given For The Convenience of The Contractor. However, The Owner and The Engineer Do Not Assume Responsibility In The Event That During Construction, Utilities Other Than Those Shown May Be Encountered, and That The Actual Location of Those Which Are Shown May Be Different From The Location As Shown On The Drawings. Contact Engineer Immediately If Surface and/or Subsurface Features Are Different Than Shown On

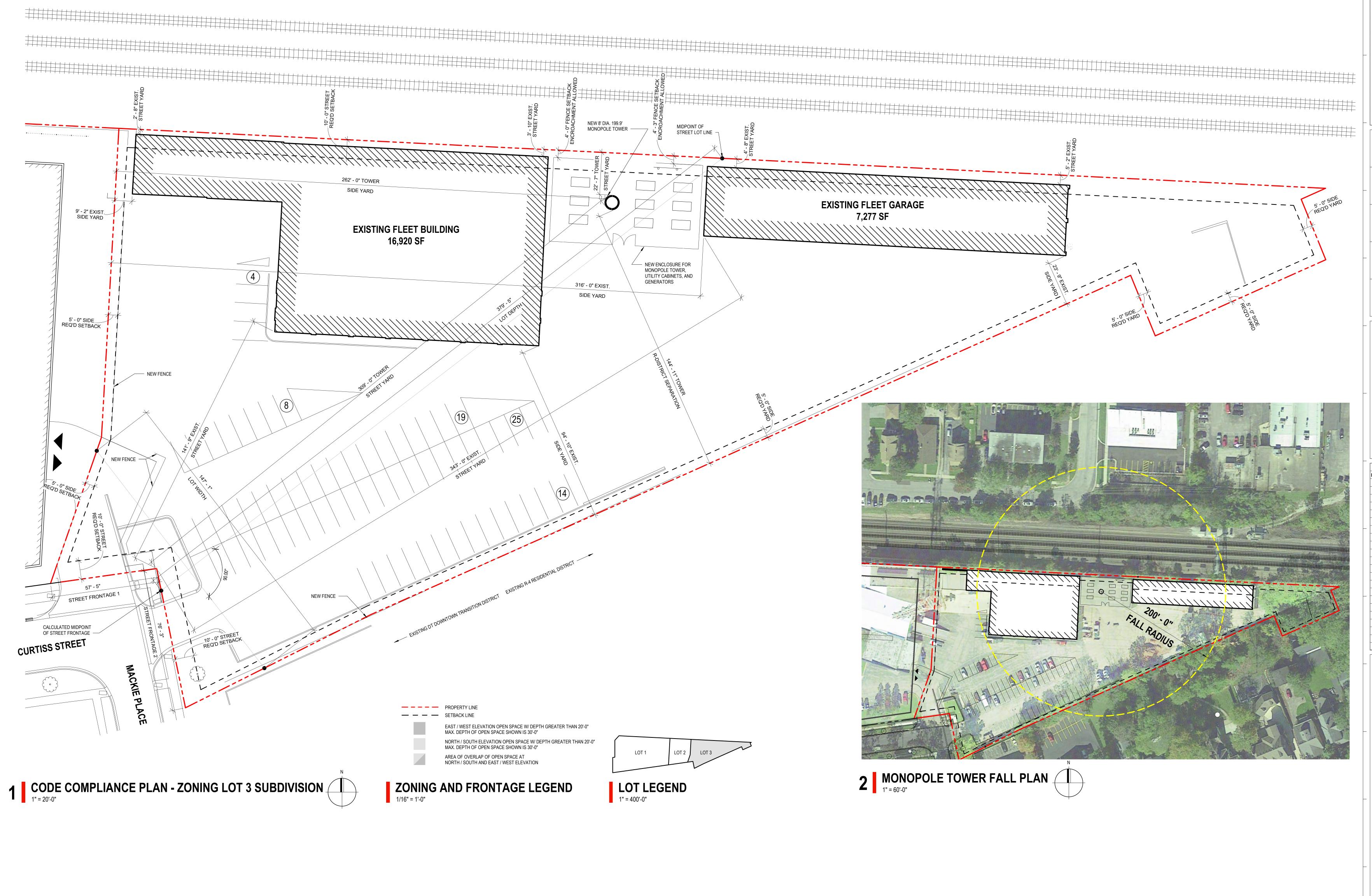
Fencing at Drip Line

- 2. Notify The Engineer Without Delay of Any Discrepancies Between the Drawings and Existing Field Conditions.
- 3. Notify The Owner, Engineer and The Village of Downers Grove A Minimum of 48 Hours In Advance of Performing
- 4. All Areas, On or Off Site, Disturbed During Construction Operations and Not Part of the Work As Shown Hereon Shall Be Restored To Original Condition to the Satisfaction of the Owner at No Additional Cost to the Owner. It is Incumbent Upon Contractor to Show That Damaged Areas Were Not Disturbed By Construction Operations.
- Electronic Drawing File (DWG) and Stake All Site Improvements Accordingly.

5. These Drawings Assume That The Contractor Will Utilize An

- 6. No Person May Utilize The Information Contained Within These Drawing's Without Written Approval From Eriksson Engineering Associates, Ltd.
- 7. The Engineer Is Furnishing These Drawings For Construction Purposes As A Convenience To The Owner, Architect, Surveyor, or Contractor. Prior To The Use Of These Drawings For Construction Purposes, The User Of This Media Shall Verify All Dimensions And Locations Of Buildings With The Foundation Drawings And Architectural Site Plan, and Coordinate All Dimensions and Locations of All Site Items. If Conflicts Exist The User Of This Information Shall Contact The Engineer Immediately.
- 8. Provide An As-built Survey Prepared By A Licensed Professional Land Surveyor In Accordance With The Authorities Having Jurisdiction Which Shall Include As a Minimum All Detention Basins and Best Management Practices, Include All Storm and Sanitary Sewers, Structure Locations, Sizes, Rim and Invert Elevations, Final Detention Volume Calculations For The Basin(s), Watermain and Valve and Appurtenance Locations.
- 9. The Illinois Department Of Transportation Standard Specifications For Road And Bridge Construction Latest Earthwork And Paving Work Under This Contract Unless Noted Otherwise.

MONOPOLE ELEVATION



1211 W 22nd St, Suite 700 Oak Brook, Illinois 60523 630.574.8300 OFFICE

ILLINOIS PROFESSIONAL DESIGN FIRM #184-000350

630.574.7070 FAX

ERIKSSON ENGINEERING ASSOCIATES GRAYSLAKE, IL 60030 847.223.4804 (O)

LANDSCAPE HITCHCOCK DESIGN GROUP NAPERVILLE, ILLINOIS 60540 P.O. BOX 5126 (60567-5126) 630.961.1787 (O)

STRUCTURAL IMEG CORP.
1100 WARRENVILLE ROAD, SUITE 400W

NAPERVILLE, ILLINOIS 60563 630.527.2320 (O)

NAPERVILLE, ILLINOIS 60563

630.527.2320 (O)

847.783.3000 (O)

M.E.P. & FP IMEG CORP. 1100 WARRENVILLE ROAD, SUITE 400W

LEOPARDO 5200 PRAIRIE STONE PARKWAY HOFFMAN ESTATES, ILLINOIS 60192

NOT FOR CONSTRUCTION

FOR REVIEW ONLY

construction, recording purposes, or

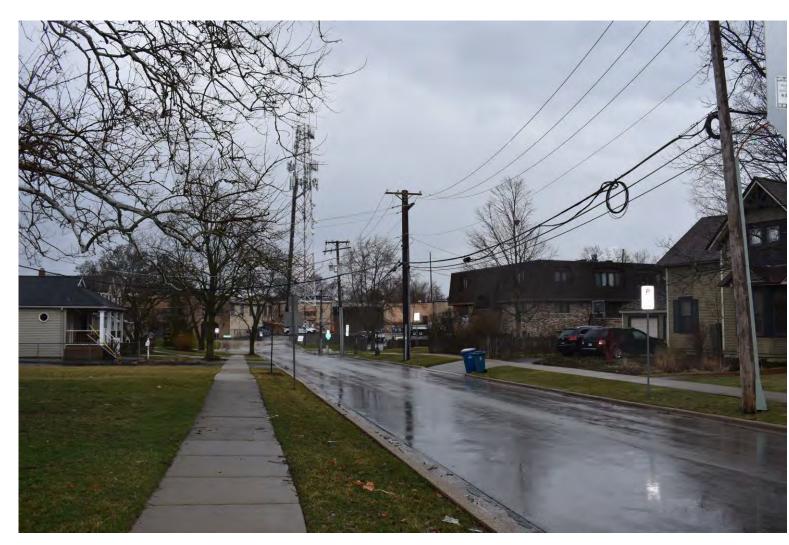
NO DATE DESCRIPTION

CODE COMPLIANCE PLAN -ZONING SITE PLAN

G1.5A

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DGCC | Communications Tower Relocation



Current | View looking north along Mackie Place



Proposed | View looking north along Mackie Place





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DGCC | Communications Tower Relocation



Current | View looking north from front yard of 706 Maple Avenue



Proposed | View looking north from front yard of 706 Maple Avenue



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DGCC | Communications Tower Relocation



Current | View looking north from rear yard of 704 Maple Avenue



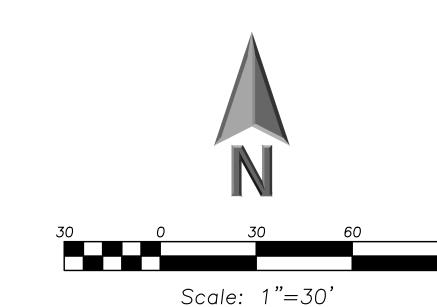
Proposed | View looking north from rear yard of 704 Maple Avenue

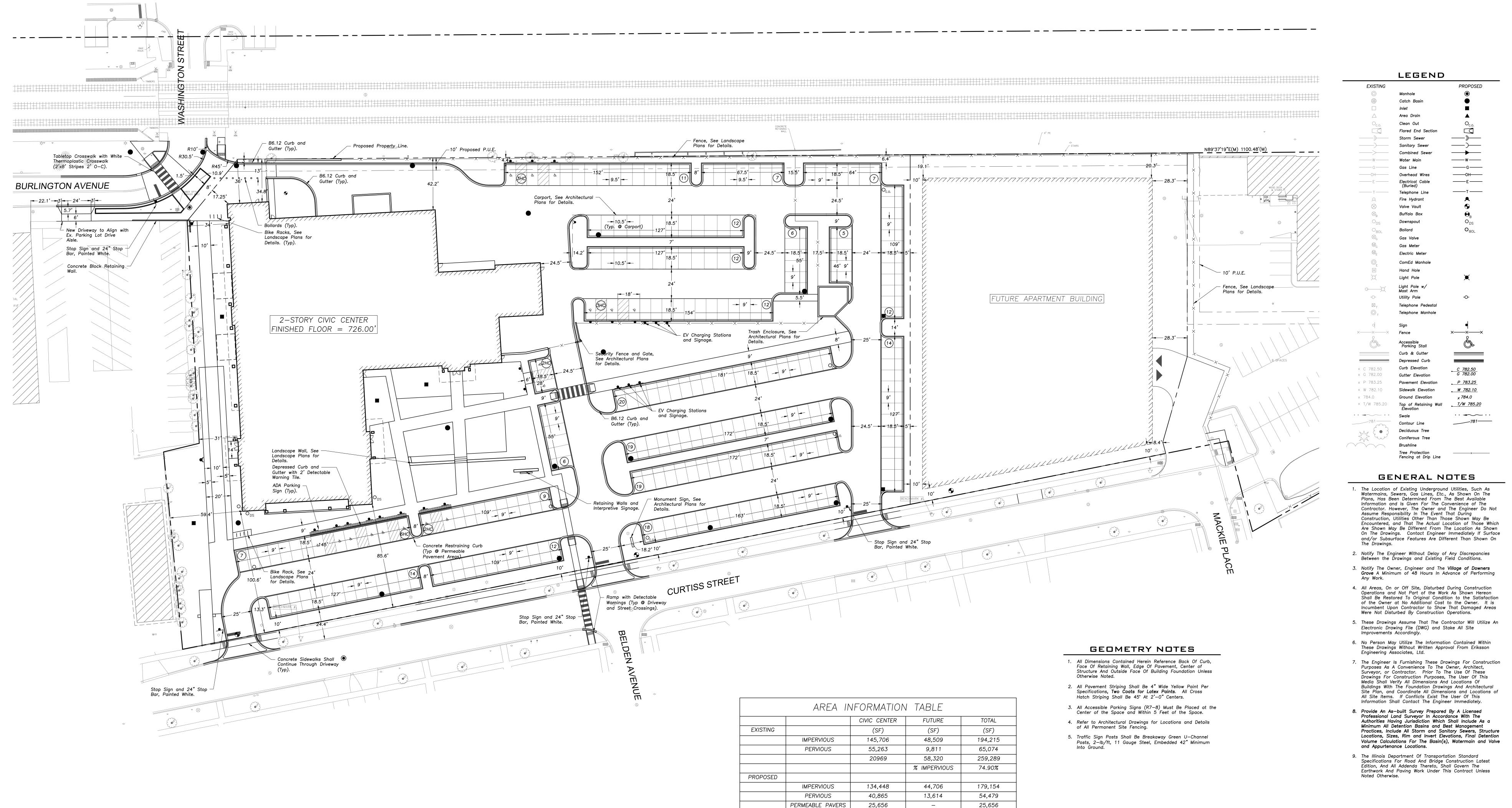




Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515





58,320

% IMPERVIOUS

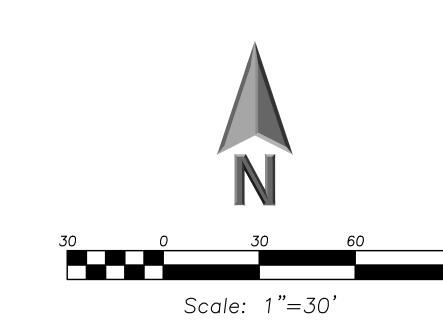
259,289

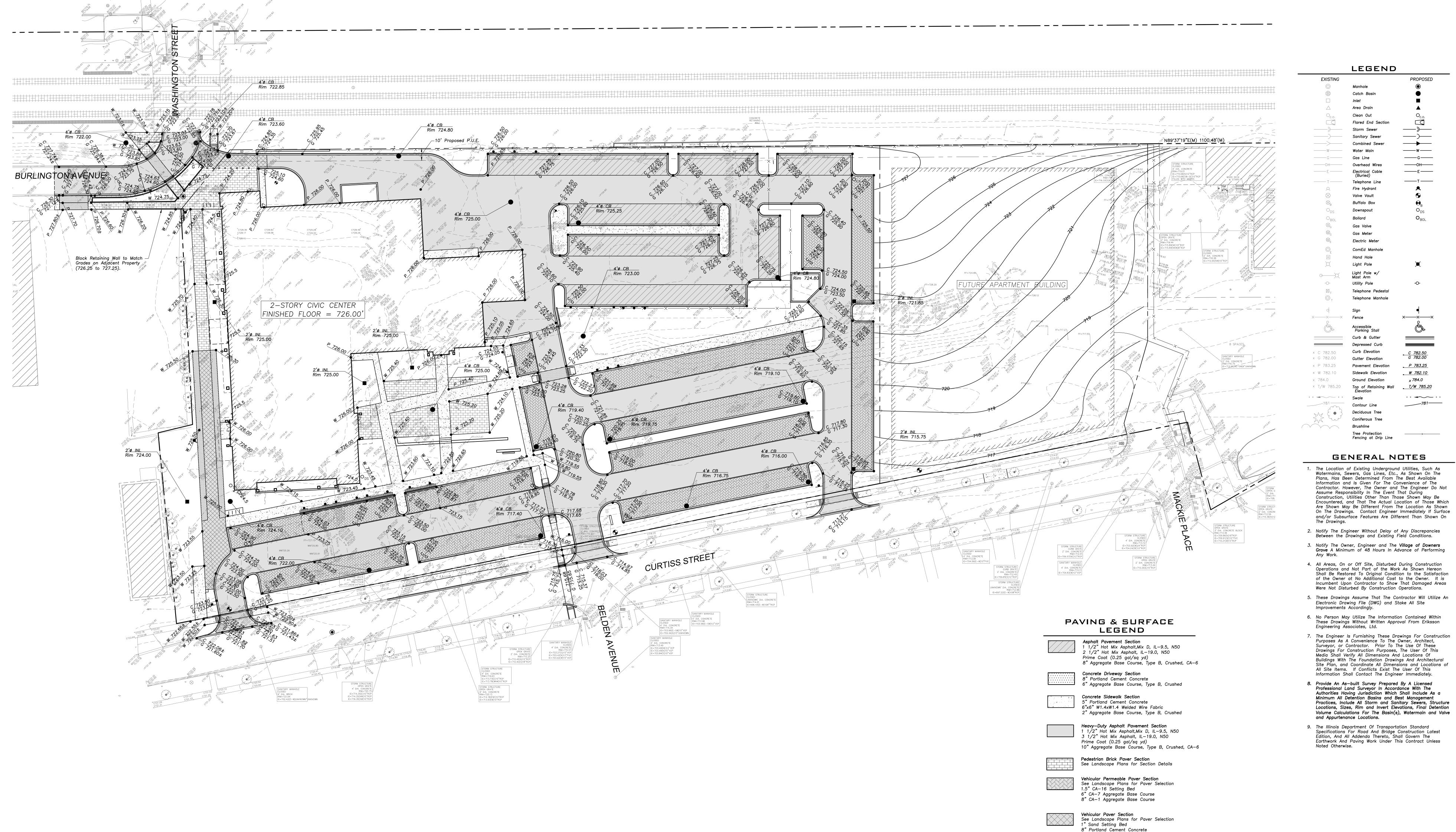
SITE GEOMETRY PLAN



Downers Grove Civic Center

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GRADING AND PAVING PLAN

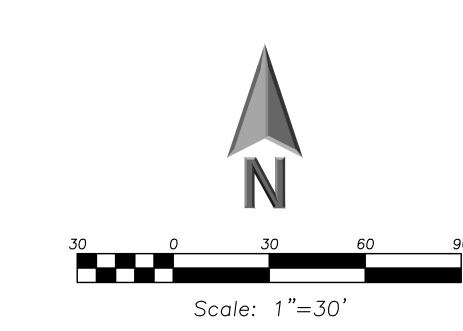
6" Aggregate Base Course, Type B, Crushed

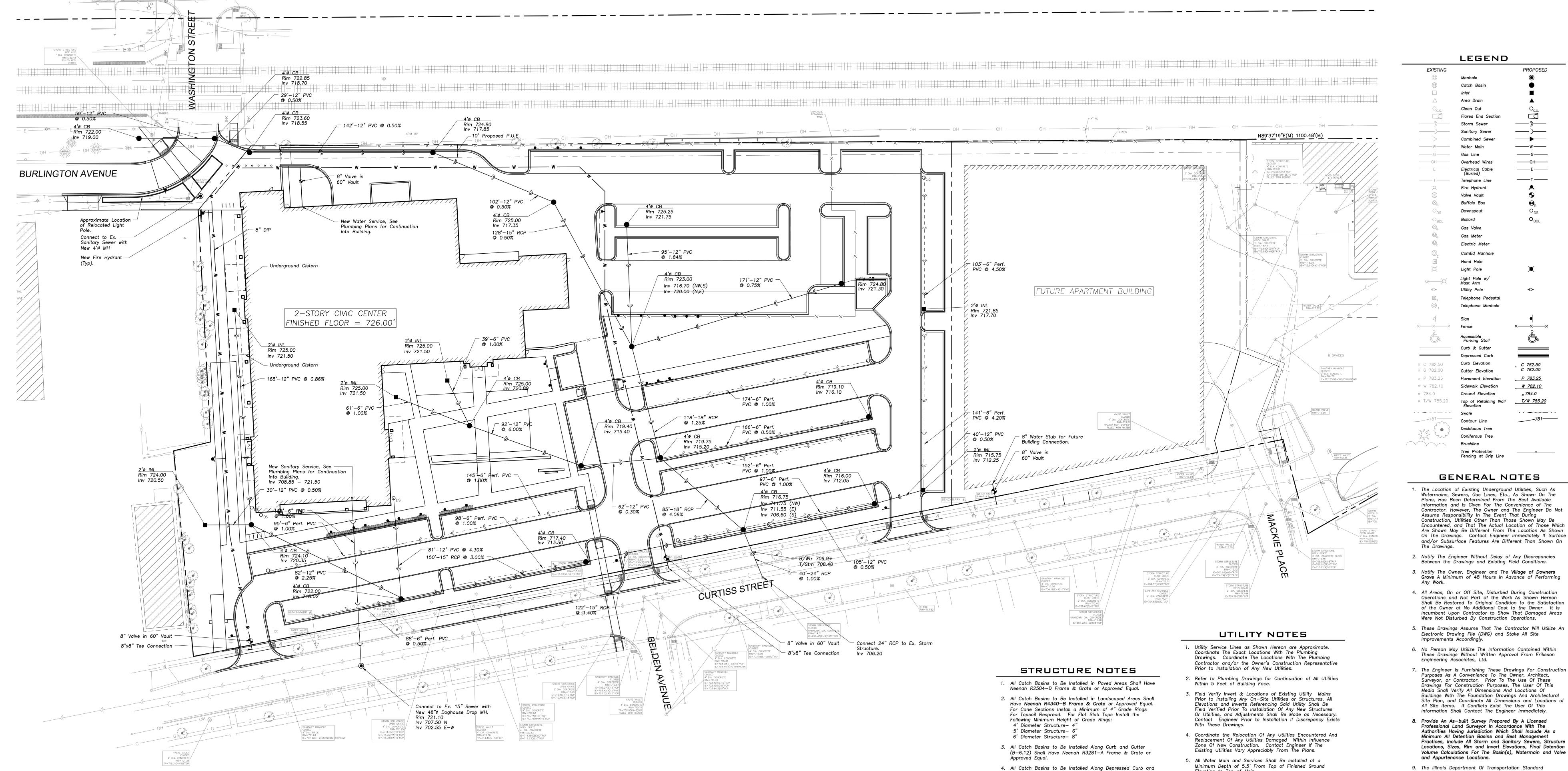
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Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515





SITE UTILITY PLAN

Rebuild Existing Structures and Adjust Rim Elevations to Match Proposed Ground Elevations.

Gutter (Dep B-6.12) Shall Have East Jordan Iron Works 5120 Catch Basin Inlet Frame and Grate, or Approved

Specifically Stated Otherwise, It Is Intended That the Frame

5. Where Structures are Shown Along the Curbline, Unless

of the Structure Is To Fall Within the Flowline Of The Gutter or at the Pavement Edge Where No Gutter Exists.

6. All Manholes Shall Have Neenah R1713-B Frame & Closed Lid or Approved Equal, with "Storm" or "Sanitary"

7. For All Structures Indicated to be Adjusted, Remove and Install Adjusting Rings, Cone Section, Barrel Sections, or

8. All Sanitary Manholes Shall Include a Chimney Seal.

Imprinted as Appropriate.

Flat Slab Top as Necessary.

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6. Protection of water supplies shall be as described in

7. Clean Out All Existing and Proposed Storm Inlets and Catch Basins at the Completion of Construction.

9. The "Standard Specifications for Water and Sewer Main

8. Provide Adequate Coupling Device and/or Oversized

Illinois, latest edition.

Where Applicable.

Section 370.350 of the Illinois Recommended Standards

for Sewage Works or Section 41-2.01 of the Standard

Specifications for Water and Sewer Main Construction in

Concrete Flared-End Section to Accommodate HDPE Storm

Construction in Illinois", Current Edition Shall Govern Work



- 1. The Location of Existing Underground Utilities, Such As Watermains, Sewers, Gas Lines, Etc., As Shown On The Plans, Has Been Determined From The Best Available Information and Is Given For The Convenience of The Contractor. However, The Owner and The Engineer Do Not Assume Responsibility In The Event That During Construction, Utilities Other Than Those Shown May Be Encountered, and That The Actual Location of Those Which Are Shown May Be Different From The Location As Shown On The Drawings. Contact Engineer Immediately If Surface and/or Subsurface Features Are Different Than Shown On
- 3. Notify The Owner, Engineer and The Village of Downers Grove A Minimum of 48 Hours In Advance of Performing
- Incumbent Upon Contractor to Show That Damaged Areas Were Not Disturbed By Construction Operations.
- 6. No Person May Utilize The Information Contained Within These Drawings Without Written Approval From Eriksson
- 7. The Engineer Is Furnishing These Drawings For Construction Purposes As A Convenience To The Owner, Architect, Surveyor, or Contractor. Prior To The Use Of These Drawings For Construction Purposes, The User Of This
- Media Shall Verify All Dimensions And Locations Of Buildings With The Foundation Drawings And Architectural Site Plan, and Coordinate All Dimensions and Locations of All Site Items. If Conflicts Exist The User Of This Information Shall Contact The Engineer Immediately. 8. Provide An As-built Survey Prepared By A Licensed Professional Land Surveyor In Accordance With The
- Authorities Having Jurisdiction Which Shall Include As a Minimum All Detention Basins and Best Management Practices. Include All Storm and Sanitary Sewers, Structure Locations, Sizes, Rim and Invert Elevations, Final Detention Volume Calculations For The Basin(s), Watermain and Valve
- 9. The Illinois Department Of Transportation Standard Specifications For Road And Bridge Construction Latest Edition, And All Addenda Thereto, Shall Govern The Earthwork And Paving Work Under This Contract Unless Noted Otherwise.

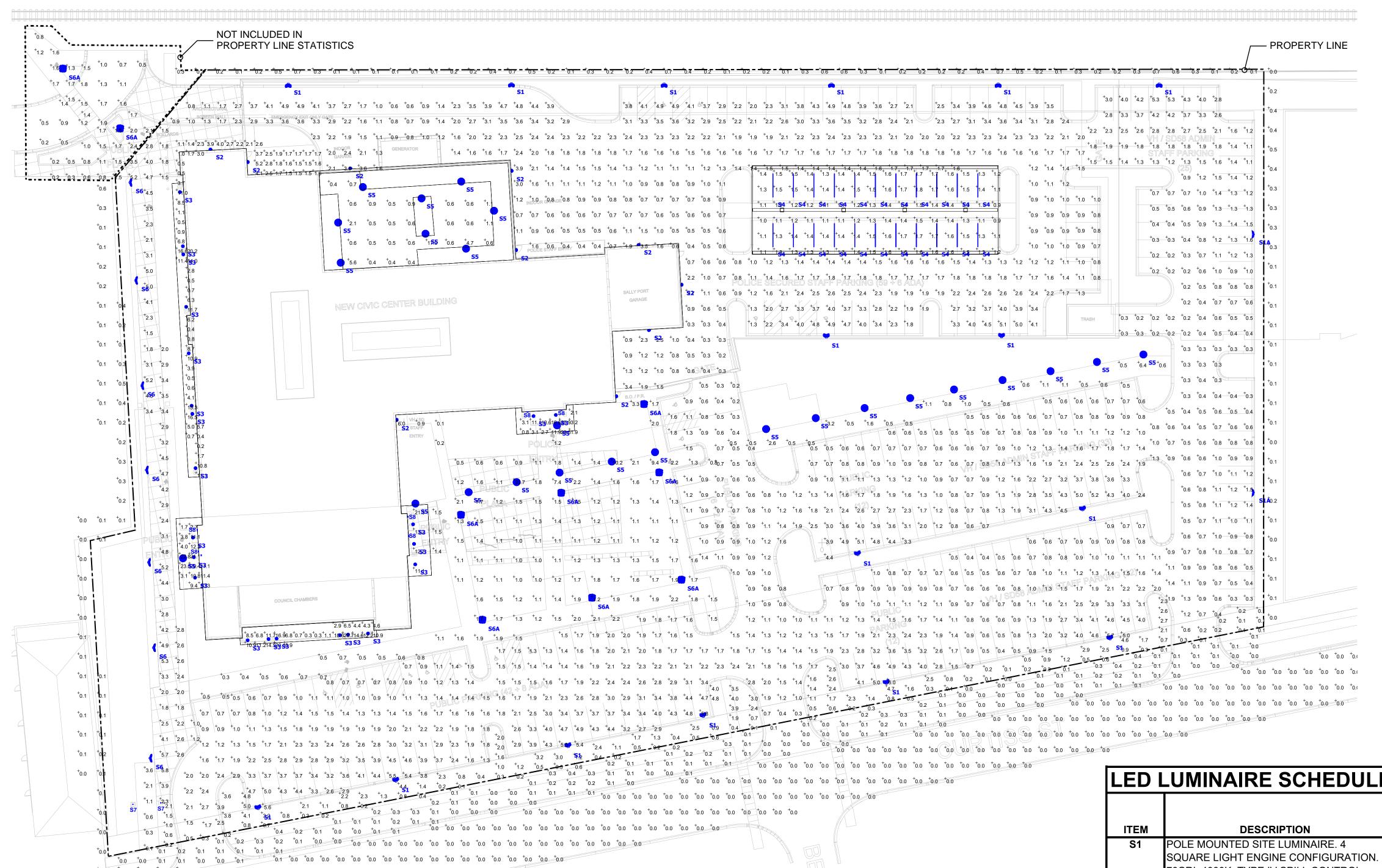
Downers Grove Civic Center fgma & MEG

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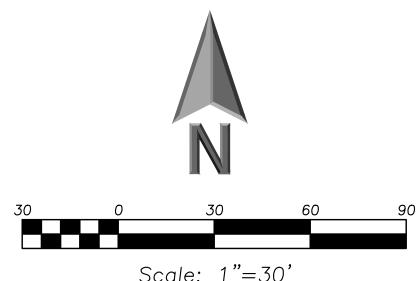


825 Burlington Ave, Downers Grove, IL 60515



| STATISTICS | | | | | | | | | |
|-----------------------------|---------|---------|--------|---------|--|--|--|--|--|
| DESCRIPTION | AVG | MAX | MIN | AVG/MIN | | | | | |
| CARPORT | 1.4 FC | 1.8 FC | 0.9 FC | 1.6:1 | | | | | |
| EAST PUBLIC ENTRANCE | 9.0 FC | 21.3 FC | 1.4 FC | 6.4:1 | | | | | |
| EAST STAFF PARKING | 1.1 FC | 5.3 FC | 0.2 FC | 5.5:1 | | | | | |
| GREEN ROOF | 1.5 FC | 17.0 FC | 0.4 FC | 4.0:1 | | | | | |
| PLAZA & WALKWAYS | 1.7 FC | 32.7 FC | 0.1 FC | 17.0:1 | | | | | |
| POLICE LOBBY ENTRANCE | 9.5 FC | 29.6 FC | 0.8 FC | 11.9:1 | | | | | |
| SECURED STAFF DRIVE/PARKING | 1.9 FC | 5.1 FC | 0.2 FC | 9.5:1 | | | | | |
| SOUTH PUBLIC/ADMIN PARKING | 1.8 FC | 6.0 FC | 0.2 FC | 9.0:1 | | | | | |
| WEST PUBLIC ENTRANCE | 11.2 FC | 23.0 FC | 1.7 FC | 6.6:1 | | | | | |
| WEST PUBLIC PLAZA | 2.6 FC | 6.0 FC | 0.2 FC | 13.5:1 | | | | | |

- LETTERING AND IS EXCLUDED FROM PHOTOMETRICS.
- 3. ALL FLAG POLES SHALL BE INTERNALLY ILLUMINATED AND ARE **EXCLUDED FROM PHOTOMETRICS**



| DOWNERS GROVE MUNICIPAL CODE |
|--------------------------------|
| SEC 28.10.030 OUTDOOR LIGHTING |
| |

28.10.030(d) - THE PLAN MUST INCLUDE AT LEAST THE FOLLOWING INFORMATION: 1) A PHOTOMETRIC STUDY AND DATA ON THE TYPES OF LIGHTING FIXTURES TO BE COMPLIANCE: REFER TO SITE LIGHTING

PHOTOMETRIC PLAN AND LUMINAIRE SCHEDULE ON THIS SHEET.

2) LOCATION OF AND CATALOG CUT-SHEET DATA FOR ALL PROPOSED LIGHT FIXTURES INCLUDING THOSE USED FOR SITE LIGHTING, CANOPY LIGHTING, AND EXTERIOR BUILDING LIGHTING **COMPLIANCE: REFER TO SITE LIGHTING** PHOTOMETRIC PLAN ON THIS SHEET FOR FIXTURE LOCATIONS. REFER TO ENCLOSED **CUT SHEETS FOR FIXTURE DATA.**

3) CALCULATIONS SHOWING CONTOURS OF INDIVIDUAL ILLUMINATION VALUES IN FOOT-CANDLES. THE CONTOURS MUST INCLUDE PROPERTY LINES AND AT LEAST FIVE FEET (5') BEYOND. **COMPLIANCE: REFER TO ENCLOSED** CONTOUR PHOTOMETRIC PLAN.

4) SUMMARY CALCULATION TABLE SHOWING AVERAGE FOOT-CANDLES, MINIMUM FOOT-CANDLES, MAXIMUM FOOT-CANDLES AVERAGE-MINIMUM UNIFORMITY RATION. AND FOOT-CANDLES AT PROPERTY LINES AND AT LEAST FIVE FEET (5') BEYOND. COMPLIANCE: REFER TO STATISTICS TABLE ON THIS SHEET

28.10.030(f) GLARE. ALL OUTDOOR LIGHTING MUST BE DIRECTED, SHADED, SHIELDED, OR OTHERWISE LOCATED AND DESIGNED TO MINIMIZE PERCEIVED GLARE ON ADJACENT PROPERTIES AND STREETS. COMPLIANCE: ALL FIXTURES ARE FULL **CUT-OFF TYPE. PARKING LOT POLE LIGHTS** ARE PROVIDED WITH HOUSE SIDE SHIELDS AND SPILL CONTROL DISTRIBUTION OPTICS

28.10.030(g) LIGHT TRESPASS. ALL OUTDOOR LIGHTING MUST BE DESIGNED, INSTALLED AND MAINTAINED TO AVOID THE ADVERSE IMPACTS OF GLARE AND LIGHT TRESPASS ASSOCIATED WITH POORLY SHIELDED OR INAPPROPRIATELY DIRECTED LIGHTING FIXTURES. AVERAGE MAINTAINED FOOT-CANDLES AT THE PROPERTY LINE MAY NOT EXCEED 0.10 COMPLIANCE: REFER TO PROPERTY LINE PHOTOMETRIC STATISTICS. THE AVERAGE MAINTAINED FOOT-CANDLES AT THE PROPERTY LINE IS 0.10 PER CODE REQUIREMENTS.

28.10.030(h) LIGHT FIXTURE MOUNTING HEIGHT. PARKING LOT LIGHT FIXTURES IN R ZONING DISTRICTS MAY NOT EXCEED TWENTY FEET (20') IN HEIGHT. COMPLIANCE: PARKING LOT POLE LIGHTS ARE MOUNTED AT TWENTY FEET (20')

28.10.030(i) HOURS OF ILLUMINATION. OUTDOOR LIGHTING FOR NONRESIDENTIAL **USES ADJACENT TO RESIDENTIAL** DISTRICTS MUST BE EXTINGUISHED OR REDUCED TO SECURITY - LEVEL INTENSITY NO LATER THAN THIRTY (30) MINUTES AFTER THE CLOSE OF BUSINESS OF THE USE. COMPLIANCE: ALL EXTERIOR LIGHTING WILL BE DIMMABLE TO ACHIEVE SECURITY LIGHT LEVELS. CONTROLS WILL BE PROVIDED TO REDUCE AND/OR TURN OFF **ZONES OF LIGHTING AT SELECTABLE**

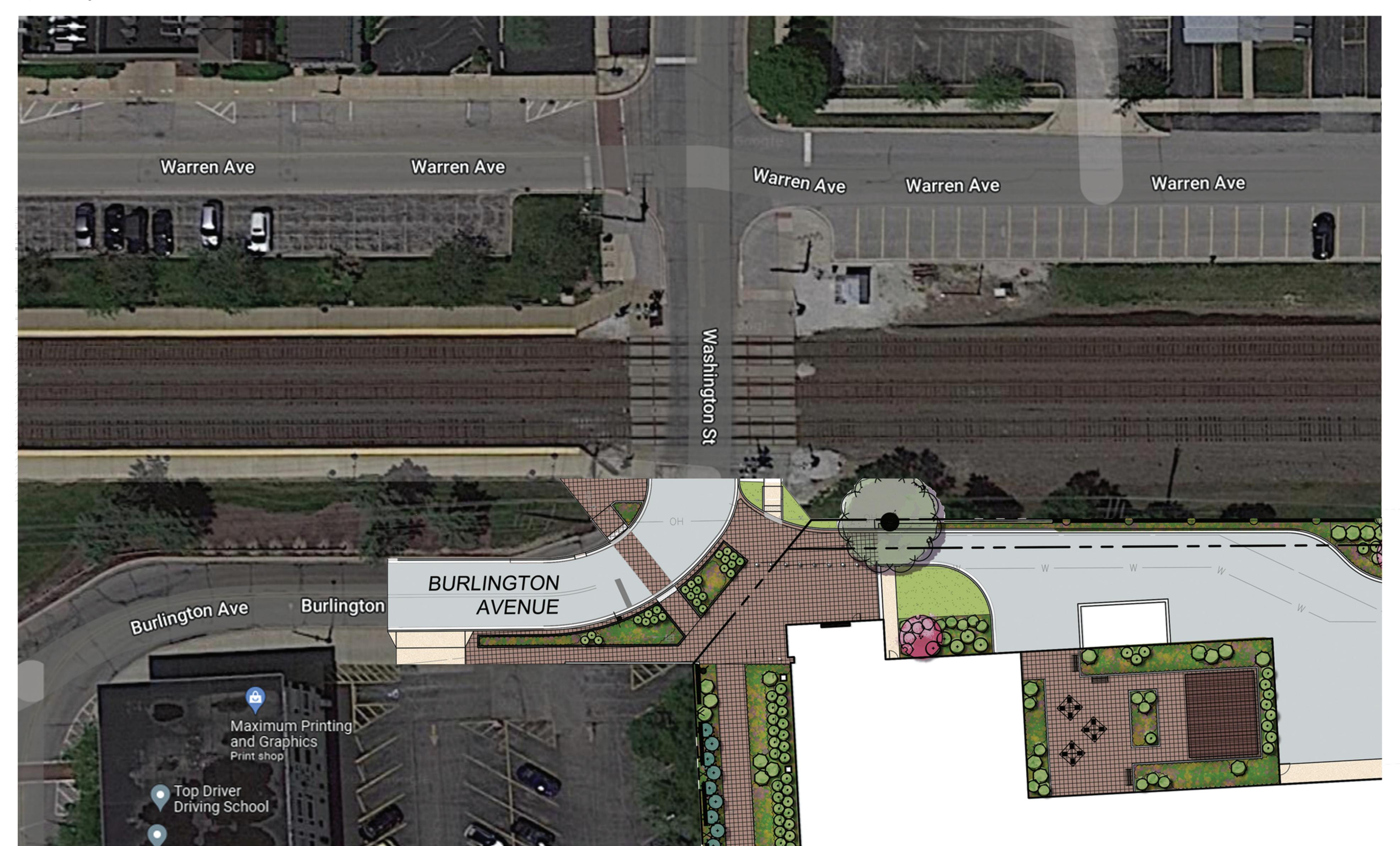
LED LUMINAIRE SCHEDULE WATT **DRIVER** LED ABSOLUTE APPROVED **WATTS LUMENS (MIN)** QTY VOLTS TYPE MTG **MANUFACTURER** MVOLT 0-10V 129 W 17863 LM SQUARE LIGHT ENGINE CONFIGURATION GLEON GALLEON 70CRI. 4000K. TYPE IV SPILL CONTROL WITH HOUSE SIDE SHIELD. MOUNT AT 20-FEET ABOVE GRADE ON SQUARE STRAIGHT ALUMINUM POLE. POLE MOUNTED SITE LUMINAIRE. SINGLE LED **GLEON GALLEON** SQUARE LIGHT ENGINE CONFIGURATION 70CRI. 4000K. TYPE II SPILL CONTROL WITH HOUSE SIDE SHIELD. MOUNT AT 20-FEET ABOVE GRADE ON SQUARE STRAIGHT ALUMINUM POLE. WALL MOUNTED BUILDING LUMINAIRE. MVOLT 0-10V BEGA 22 360 LED DOWNWARD ORIENTATED LIGHT OUTPUT DIRECTION, 80CRI OR LESS, 4000K 6" WET LOCATION LISTED ROUND 15 W DOWNLIGHT. MEDIUM DISTRIBUTION (1.0 SC). 85CRI, 4000K. MVOLT 0-10V ACCLAIM LIGHTING LED OUTDOOR FLEXIBLE STRIP. DURABLE 215 LM/FT LEX TUBE SC SILICONE BODY, ALUMINUM MOUNTING CHANNEL. IP68 AND HIGH IMPACT RATING. 80CRI, 4000K. 120 DEGREE BEAM ANGLE. REMOTE POWER SUPPLY. SITE ILLUMINATED BOLLARD. 360 DEGREE MVOLT 0-10V BEGA 99 856 LED FIX LIGHT DISTRIBUTION, IP65, 80CRI OR LESS. 4000K. WOOD BOLLARD TUBE. POST TOP PEDESTRIAN LUMINAIRE. LED 4116 LM 277V 0-10V LUMEC MPTC FIX ROUNDED TOP WITH FLAT LENS. 4000K. TYPE II OPTICS. MOUNT TO 12-FOOT MPTC-APR4 POLE. 0-10V LUMEC MPTC POST TOP PEDESTRIAN LUMINAIRE. ROUNDED TOP WITH FLAT LENS. 4000K. YPE V OPTICS, MOUNT TO 12-FOOT IPTC-APR4 POLE LED EXTERIOR BUILDING SHALLOW SCONCE MVOLT VISA LIGHTING VISAGE 0W1606 UMINAIRE. ACRYLIC DIFFUSER. 80CRI,

SITE LIGHTING PHOTOMETRIC PLAN

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Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515



WASHINGTON STREET UPGRADES



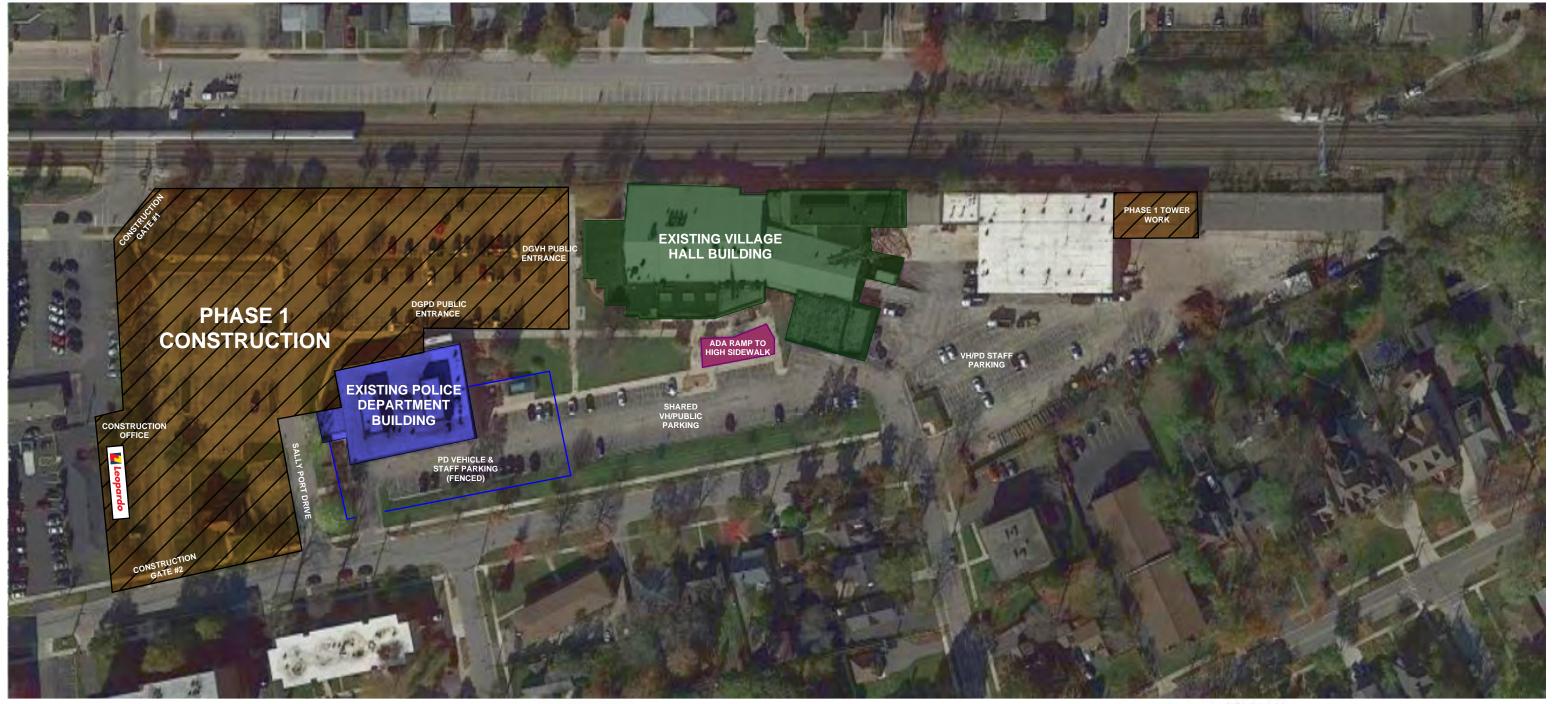
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Downers Grove Civic Center

PHASE 1 LOGISTICS

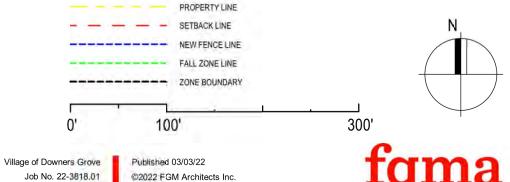


825 Burlington Ave, Downers Grove, IL 60515



PHASE 1 CONSTRUCTION NOTES:

- METRA LOT "L" CLOSED TO PUBLIC
- PD VEHICLE/STAFF PARKING SEPERATED FROM VH/PUBLIC PARKING WITH TEMPORARY FENCING
- VD/PD OVERFLOW PARKING ESTABLISHED AT PUBLIC WORKS PARKING LOT
- CONSTRUCTION FENCING ERECTED AROUND PHASE 1 CONSTRUCTION PERIMETER
- ABATEMENT, DEMOLITION, SITE DEMO, SITE GRADING & SITE UTILITIES COMMENCE
- RADIO TOWER BUILDING DEMOLITION TO COMMENCE



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Downers Grove Civic Center

PHASE 2 LOGISTICS

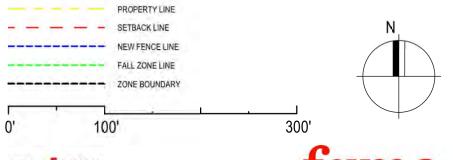


825 Burlington Ave, Downers Grove, IL 60515



PHASE 2 CONSTRUCTION NOTES:

- CONSTRUCTION OF NEW CIVIC CENTER BUILDING TO COMMENCE PHASE 2
- ACCESS TO EXISTING POLICE DEPARTMENT AND VILLAGE HALL REMAIN THE SAME AS PHASE 1
- EXCESS SPOILS FROM MASS GRADING STOCKPILED BETWEEN NEW CIVIC CENTER AND EXISTING VILLAGE HALL BUILDING
- CIVIC CENTER SIDEWALKS AND PARTIAL PARKING LOTS DEVELOPED
- WASHINGTON STREET UTILITY WORK AND STREETSCAPE IMPROVEMENTS INSTALLED



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Downers Grove Civic Center

PHASE 3 LOGISTICS

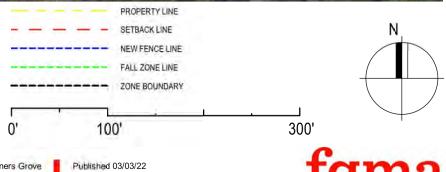


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PHASE 3 CONSTRUCTION NOTES:

- VILLAGE AND POLICE STAFF MOVE TO NEW CIVIC CENTER BUILDING
- UTILITY DISCONNECTS AND PARTIAL DEMO OF EXISTING BUILDINGS MAY START TO COMMENCE PHASE 3
- SITE IMPROVEMENTS CONTINUE ON SOUTH SIDE OF PROPERTY
- PD SECURED PARKING STRIPED TO MAXIMIZE CAPACITY OF VEHICLES
- LIMITED ADA/VISITOR PARKING AVAILABLE SOUTH OF NEW CIVIC CENTER



Village of Downers Grove Job No. 22-3818.01

Published 03/03/22

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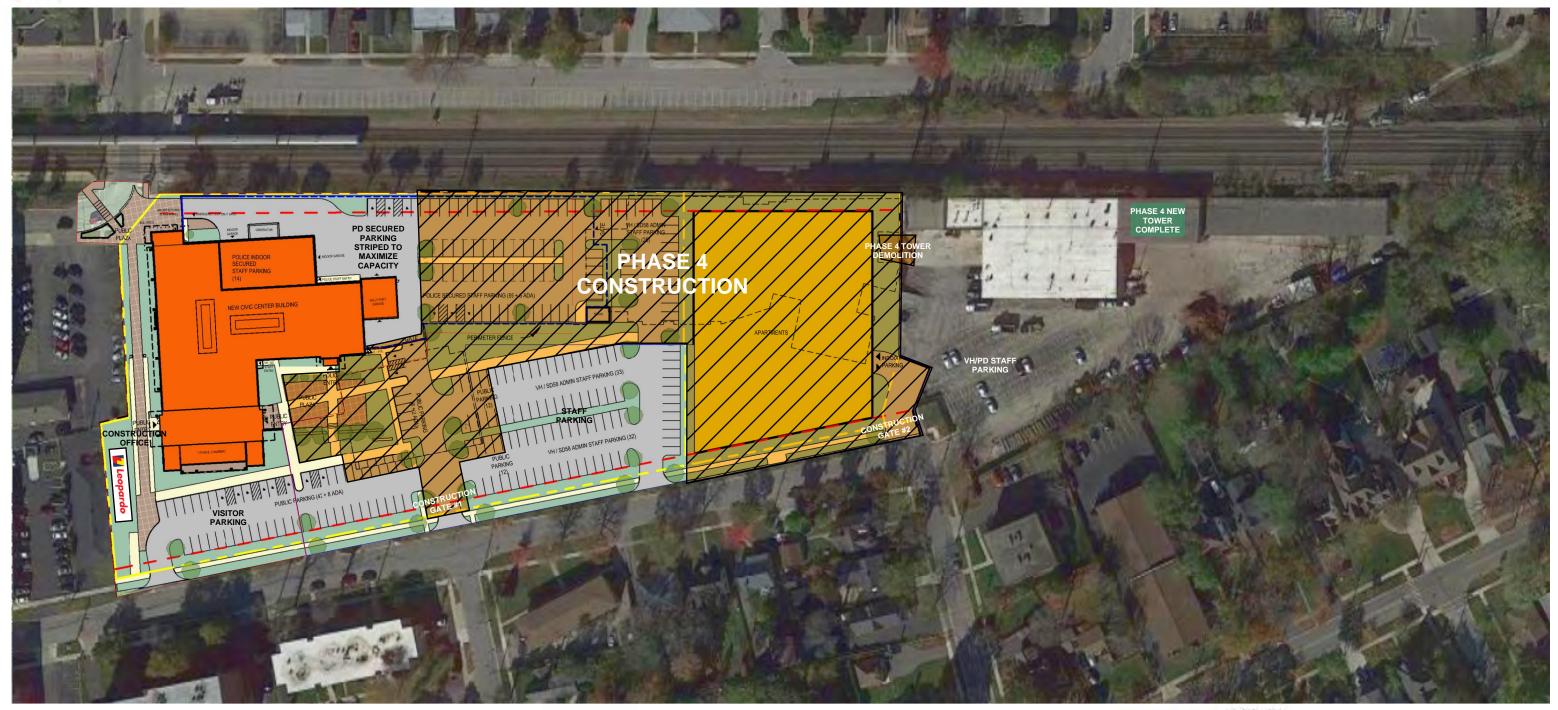
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Downers Grove Civic Center

PHASE 4 LOGISTICS

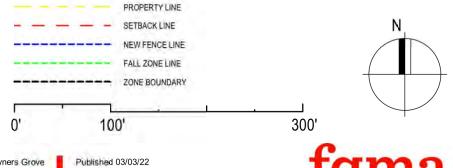


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PHASE 4 CONSTRUCTION NOTES:

- EXISTING VILLAGE HALL AND POLICE DEPARTMENT DEMOLISHED
- REMAINING PARKING LOTS COMPLETED
- BUILDING PAD & UTILITIES FOR NEW DEVELOPMENT INSTALLED
- PUBLIC PLAZAS INSTALLED
- FINAL LANDSCAPING, SURFACE ASPHALT AND OTHER SITE FINISHES COMPLETED
- CONSTRUCTION FENCING REMOVED



Village of Downers Grove Job No. 22-3818.01

Published 03/03/22

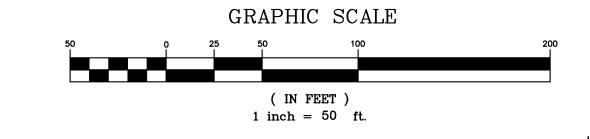
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PLAT OF VACATION OF

THAT PART OF BURLINGTON AVENUE LYING NORTH OF AND ADJOINING LOT 6 IN STANLEY'S SUBDIVISION, RECORDED FEBRUARY 18, 1884 AS DOCUMENT 32879, AND LYING NORTH OF AND ADJOINING LOTS 1 THROUGH 6, BOTH INCLUSIVE, AND THE EAST 5 1/2 FEET OF LOT 7 IN RANDALL'S RESUBDIVISION, RECORDED OCTOBER 26, 1875 AS DOCUMENT 20748, BEING PART OF THE WEST HALF OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, DESCRIBED AS FOLLOWS:

BEGINNING AT THE NORTHEAST CORNER OF LOT 6 IN SAID STANLEY'S SUBDIVISION, ALSO BEING THE SOUTHWEST CORNER OF BURLINGTON AVENUE (RAILROAD STREET), VACATED PER DOCUMENT 451892; THENCE SOUTH 89 DEGREES 37 MINUTES 19 SECONDS WEST ALONG THE NORTH LINE OF SAID LOT 6 AND ALONG THE NORTH LINE OF LOTS 1 THROUGH 7, BOTH INCLUSIVE, IN SAID RANDALL'S RESUBDIVISION, 360.81 FEET TO THE WEST LINE OF THE EAST 5 1/2 FEET OF SAID LOT 7; THENCE NORTH 37 DEGREES 41 MINUTES 31 SECONDS EAST, 63.51 FEET TO THE SOUTH LINE OF THE BURLINGTON NORTHERN AND SANTA FE RAILROAD; THENCE NORTH 89 DEGREES 37 MINUTES 19 SECONDS EAST ALONG SAID SOUTH LINE, 319.20 FEET TO THE WEST LINE OF VACATED BURLINGTON AVENUE (RAILROAD STREET) RECORDED AS DOCUMENT NUMBER 451892; THENCE SOUTH 03 DEGREES 10 MINUTES 34 SECONDS EAST ALONG SAID WEST LINE, 50.06 FEET TO THE POINT OF BEGINNING, IN DUPAGE COUNTY, ILLINOIS.



| GRID NORTH | -19 | à | I D | T Y | |
|------------|----------------------|-----|-----------------------------------|--|--|
| | BOOK 592 PG 11-19 | 74 | 3/20/22 | 77/67/6 | |
| AD . | BOOK | | | | |
| | CHECKED BY SK | ш | | 7 | |
| 50, | DRAWN BY MRA | ┨ | REVISIONS DEP EMAII DATED 3/08/00 | EK EMAIL DAIED J/20/4 | |
| | DATE: 3/18/2022 PC | -1I |] | 1. | |
| | DATE | 2 | <u></u> | <u>- </u> | |
| | | 11 | | | |

BURLINGTON NORTHERN AND SANTAFE RAILROA N37°41'31"E 63.51'~ N89°37'19"E 319.20' ~S03°10'34"E 50.06' BURLINGTON AVENUE BURLINGTON & AVENUE VACATED BURLINGTON AVENUE PER DOCUMENT 451892 HEREBY VACATED 17,000 SQUARE FEET S89°37'19"W 360.81' `-P.O.B. THE EAST 5½'_/ OF LOT 7 RANDALL'S RESUB. STANLEY'S SUBDIVISION ___ DOC. NO. 20748 DOC. NO. 32879 11 10 CURTISS ADDITION TO THE TOWN OF DOWNERS GROVE DQC. NO. 7317

VILLAGE COUNCIL CERTIFICATE

STATE OF ILLINOIS)
) SS
COUNTY OF DUPAGE)

APPROVED THIS _____ DAY OF ______A.D. 20____

BY THE COUNCIL OF THE VILLAGE OF DOWNERS GROVE.

BY: _____ ATTEST: _____

COUNTY RECORDER CERTIFICATION

STATE OF ILLINOIS)
)SS

COUNTY OF DUPAGE)

THIS PLAT WAS FILED FOR RECORD IN THE RECORDER'S OFFICE OF DUPAGE

COUNTY, ILLINOIS, ON

THE _____ DAY OF _____ A.D. 20 ____ ,

AT ____ O'CLOCK __ M. AS DOCUMENT NUMBER _____

ABBREVIATIONS

COUNTY RECORDER

O.D.I.P. = OUTSIDE DIAMETER IRON PIPE N = NORTHS = SOUTHE = EASTW = WEST(R) = RECORD BEARING OR DISTANCE(M) = MEASURED BEARING OR DISTANCE (C) = CALCULATED BEARING OR DISTANCE(D) = DEED BEARING OR DISTANCE3.S.L. = BUILDING SETBACK LINE U.E. = UTILITY EASEMENT D.E. = DRAINAGE EASEMENT P.U.E. = PUBLIC UTILITY EASEMENT P.O.C. = POINT OF COMMENCEMENT P.O.B. = POINT OF BEGINNING P.U. & D.E. = PUBLIC UTILITY AND DRAINAGE EASEMENT

LINE LEGEND

---- LAND PARCEL LINE
---- EASEMENT LINE
---- CENTERLINE
---- BUILDING SETBACK LINE
---- SECTION LINE

= ROADWAY HEREBY VACATED

SURVEYOR'S STATEMENT

THE ABOVE PLAT WAS PREPARED UNDER MY DIRECT SUPERVISION FROM EXISTING RECORDS, MAPS AND PLATS.

COMPASS SURVEYING LTD
PROFESSIONAL DESIGN FIRM
LAND SURVEYING CORPORATION NO. 184-002778
LICENSE EXPIRES 4/30/2023

BY: _____ DATE: _____ SCOTT KREBS

ILLINOIS PROFESSIONAL LAND SURVEYOR NO. 3509 LICENSE EXPIRES: 11/30/2022

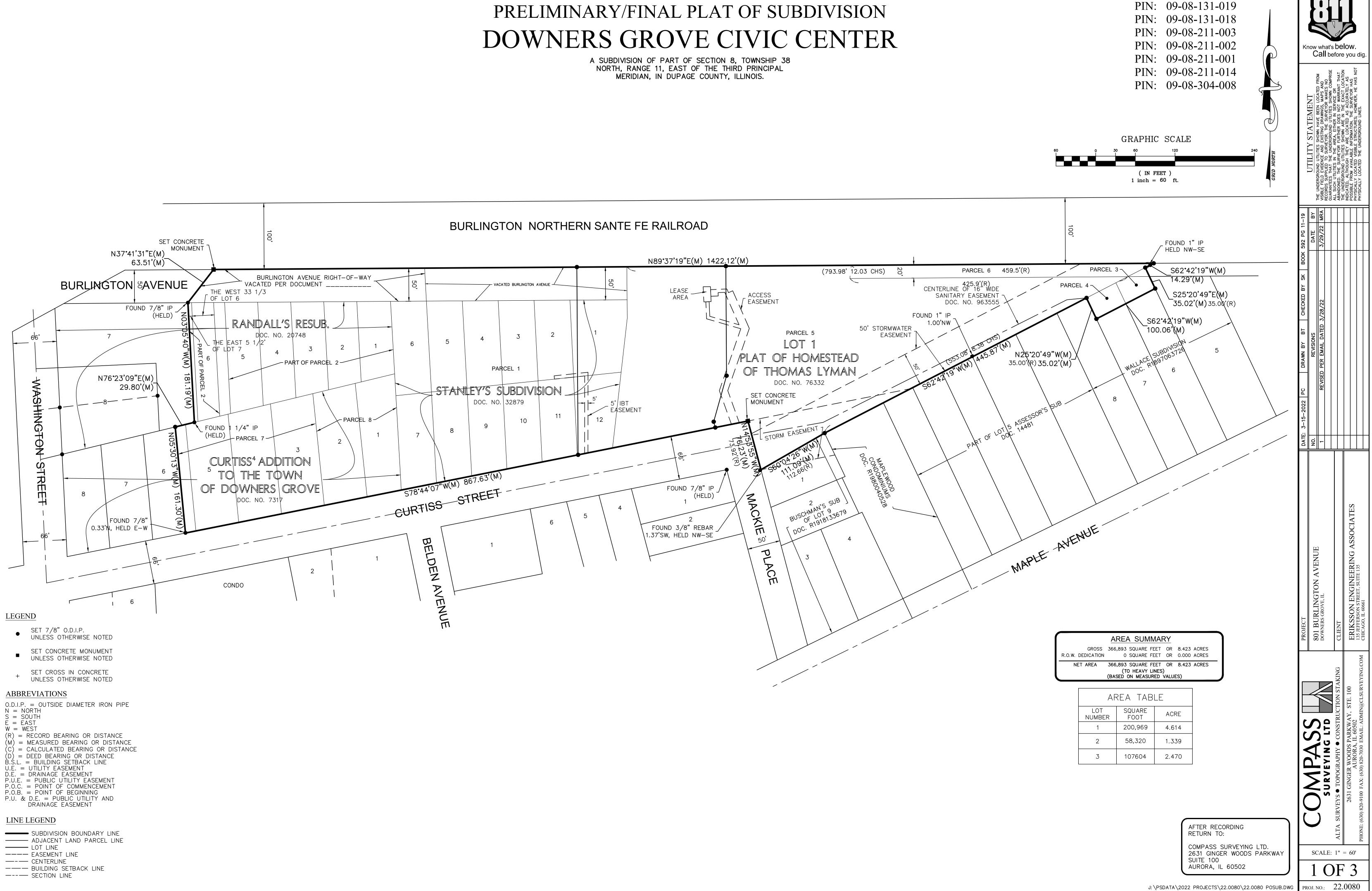
1 OF 1

J:\PSDATA\2022 PROJECTS\22.0080\22.0080-01 VACATION\22.0080-01 POVAC.DWG

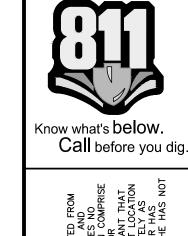
SCALE: 1'' = 50'

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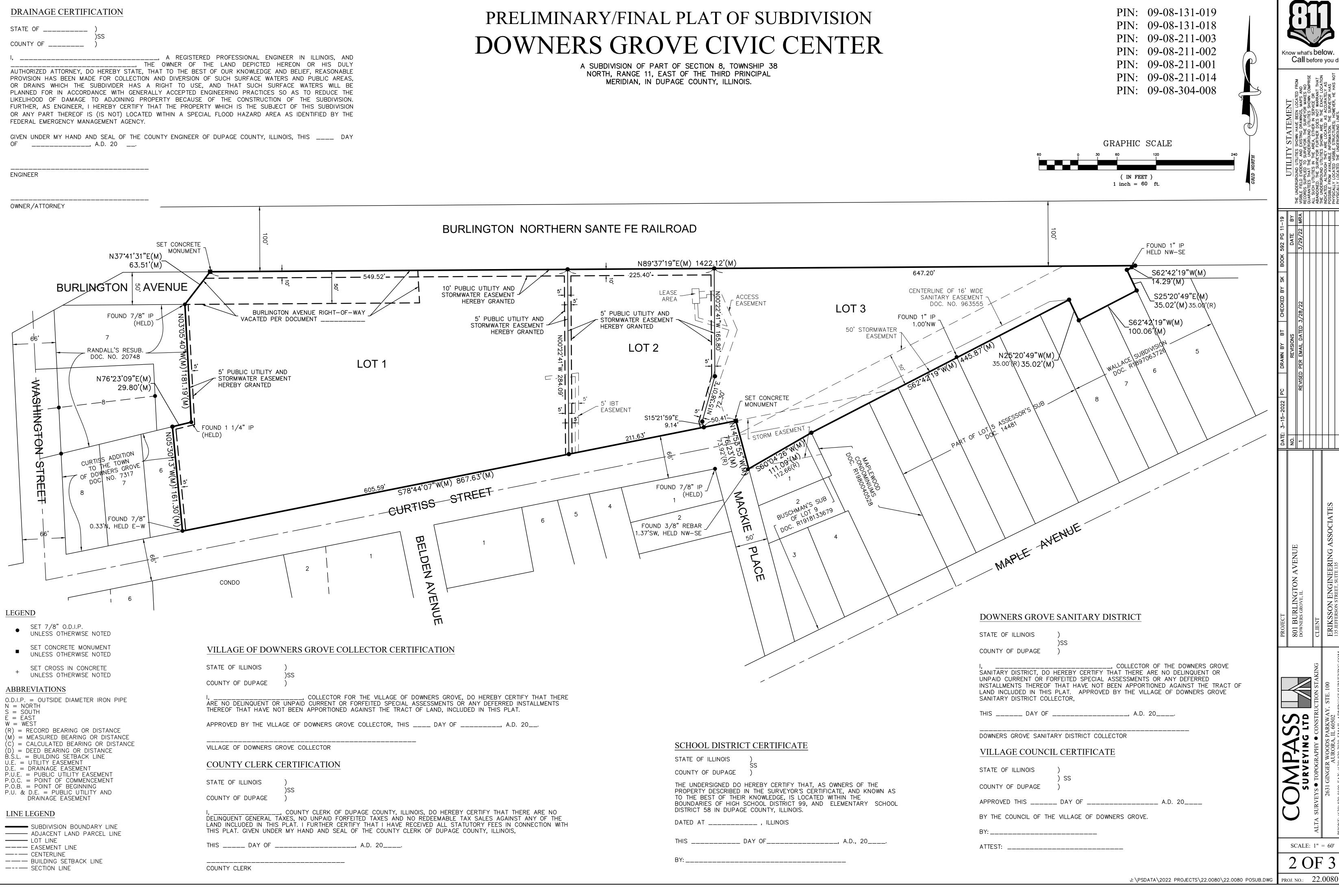
PRELIMINARY/FINAL PLAT OF SUBDIVISION



PIN: 09-08-131-019 PIN: 09-08-131-018 PIN: 09-08-211-003 PIN: 09-08-211-002 PIN: 09-08-211-001 PIN: 09-08-211-014 PIN: 09-08-304-008



SCALE: 1'' = 60'



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PRELIMINARY/FINAL PLAT OF SUBDIVISION DOWNERS GROVE CIVIC CENTER

A SUBDIVISION OF PART OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOIS.

OWNER'S CERTIFICATE

COUNTY OF DUPAGE)

THIS IS TO CERTIFY THAT VILLAGE OF DOWNERS GROVE, AN ILLINOIS MUNICIPAL CORPORATION, IS THE OWNER OF THE PROPERTY DESCRIBED HEREON AND BY THE DULY ELECTED OFFICERS HAS CAUSED THE SAME TO BE SURVEYED AND PLATTED AS INDICATED HEREON FOR THE USES AND PURPOSES THEREIN SET FORTH AS ALLOWED AND PROVIDED BY STATUTE AND HEREBY ACKNOWLEDGES AND ADOPTS THE SAME UNDER THE STYLE

ALSO, THE UNDERSIGNED DO HEREBY CERTIFY THAT, AS OWNERS OF THE PROPERTY DESCRIBED IN THE SURVEYOR'S CERTIFICATE, TO THE BEST OF THEIR KNOWLEDGE, IS LOCATED WITHIN THE BOUNDARIES OF THE DOWNERS NORTH HIGH SCHOOL DISTRICT 99, HERRICK MIDDLE SCHOOL AND LESTER ELEMENTARY SCHOOL DISTRICT IN DUPAGE COUNTY, ILLINOIS.

ALSO, THE UNDERSIGNED OWNER HEREBY DECLARES THAT THE REAL PROPERTY DESCRIBED IN AND DEPICTED ON THIS PLAT OF SUBDIVISION SHALL BE HELD, TRANSFERRED, SOLD, CONVEYED AND OCCUPIED SUBJECT TO THE FOLLOWING COVENANT AND RESTRICTION:

ALL PUBLIC UTILITY STRUCTURES AND FACILITIES, WHETHER LOCATED ON PUBLIC OR PRIVATE PROPERTY SHALL BE CONSTRUCTED WHOLLY UNDERGROUND. EXCEPT FOR TRANSFORMERS. TRANSFORMER PADS. LIGHT POLES, REGULATORS, VALVES, MARKERS AND SIMÍLAR STRUCTURES APPROVED BY THE VILLAGE ENGINÉER OF THE VILLAGE OF DOWNERS GROVE PRIOR TO RECORDING OF THIS PLAT OF SUBDIVISION.

| DATED AT | , THIS DAY |
|----------|------------|
| OF | A.D., 20 |
| BY: | ATTEST: |
| TITI C. | TITI C. |

NOTARY'S CERTIFICATE

| STATE OF |) |
|-----------|---|
| COUNTY OF |) |

______, A NOTARY PUBLIC IN THE COUNTY AND STATE AFORESAID, DO HEREBY CERTIFY THAT

| | , |
|-------------|-------|
| (TITLE) AND | |

WHO ARE PERSONALLY KNOWN TO ME TO BE THE SAME PERSONS WHO ARE SUBSCRIBED TO THE FOREGOING CERTIFICATE OF OWNERSHIP, APPEARED BEFORE ME THIS DAY IN PERSON AND ACKNOWLEDGED THE EXECUTION OF THIS INSTRUMENT IN THEIR CAPACITY FOR THE USES AND PURPOSES THEREIN SET FORTH AS THE FREE AND VOLUNTARY ACT AND DEED OF SAID CORPORATION.

| GIVEN | UNDER | MY | HAND | AND | NOTARIAL | SEAL | THIS _ | | D |
|-------|-------|----|------|-----|----------|------|--------|------|---|
| OF | | | | | | | A.D. | , 20 | |
| | | | | | | | | | |

NOTARY PUBLIC

EASEMENT PROVISIONS

AN EASEMENT FOR SERVING THE SUBDIVISION AND OTHER PROPERTY WITH ELECTRIC AND COMMUNICATION SERVICE IS HEREBY RESERVED FOR AND GRANTED TO

COMMONWEALTH EDISON COMPANY AND AT&T TELEHOLDINGS INCORPORATED, ILLINOIS A.K.A. ILLINOIS BELL TELEPHONE COMPANY, GRANTEES,

THEIR RESPECTIVE LICENSEES, SUCCESSORS AND ASSIGNS JOINTLY AND SEVERALLY, TO CONSTRUCT, OPERATE, REPAIR, MAINTAIN, MODIFY, RECONSTRUCT, REPLACE, SUPPLEMENT, RELOCATE AND REMOVE. FROM TIME TO TIME, POLES GUYS, ANCHORS, WIRES, CABLES, CONDUITS, MANHOLES, TRANSFORMERS, PEDESTALS, EQUIPMENT CABINETS OR OTHER FACILITIES USED IN CONNECTION WITH OVERHEAD AND UNDERGROUND TRANSMISSION AND DISTRIBUTION OF ELECTRICITY, COMMUNICATIONS, SOUNDS AND SIGNALS IN, OVER, UNDER, ACROSS, ALONG AND UPON THE SURFACE OF THE PROPERTY SHOWN WITHIN THE DASHED OR DOTTED LINES (OR SIMILAR DESIGNATION) ON THE PLAT AND MARKED "EASEMENT", "UTILITY EASEMENT", "PUBLIC UTILITY EASEMENT", "P.U.E." (OR SIMILAR DESIGNATION), THE PROPERTY DESIGNATED IN THE DECLARATION OF CONDOMINIUM AND/OR ON ÌHIS PLAT AS "COMMON ÉLEMENTS", AND THE PROPERTY DESIGNATED ON THE PLAT AS "COMMON AREA OR AREAS", AND THE PROPERTY DESIGNATED ON THE PLAT FOR STREETS AND ALLEYS, WHETHER PUBLIC OR PRIVATE. TOGETHER WITH THE RIGHTS TO INSTALL REQUIRED SERVICE CONNECTIONS OVER OR UNDER THE SURFACE OF EACH LOT AND COMMON AREA OR AREAS TO SERVE IMPROVEMENTS THEREON. OR ON ADJACENT LOTS, AND COMMON AREA OR AREAS, THE RIGHT TO CUT, TRIM OR REMOVE TREES, BUSHES, ROOTS AND SAPLINGS AND TO CLEAR OBSTRUCTIONS FROM THE SURFACE AND SUBSURFACE AS MAY BE REASONABLY REQUIRED INCIDENT TO THE RIGHTS HEREIN GIVEN, AND THE RIGHT TO ENTER UPON THE SUBDIVIDED PROPERTY FOR ALL SUCH PURPOSES. OBSTRUCTIONS SHALL NOT BE PLACED OVER GRANTEES' FACILITIES OR IN, UPON OR OVER THE PROPERTY WITHIN THE DASHED OR DOTTED LINES (OR SIMILAR DESIGNATION) MARKED "EASEMENT", "UTILITY EASEMENT", "PUBLIC UTILITY EASEMENT", "P.U.E." (OR SIMILAR DESIGNATION) WITHOUT THE PRIOR WRITTEN CONSENT OF GRANTEES. AFTER INSTALLATION OF ANY SUCH FACILITIES, THE GRADE OF THE SUBDIVIDED PROPERTY SHALL NOT BE ALTERED IN A MANNER SO AS TO INTERFERE WITH THE PROPER OPERATION AND MAINTENANCE THEREOF

THE TERM "COMMON ELEMENTS" SHALL HAVE THE MEANING SET FORTH FOR SUCH TERM IN THE "CONDOMINIUM PROPERTY ACT", CHAPTER 765 ILCS 605/2, AS AMENDED FROM TIME TO TIME.

THE TERM "COMMON AREA OR AREAS" IS DEFINED AS A LOT, PARCEL OR AREA OF REAL PROPERTY, THE BENEFICIAL USE AND ENJOYMENT OF WHICH IS RESERVED IN WHOLE OR AS AN APPURTENANCE TO THE SEPARATELY OWNED LOTS, PARCELS OR AREAS WITHIN THE PLANNED DEVELOPMENT, EVEN THOUGH SUCH BE OTHERWISE DESIGNATED ON THE PLAT BY TERMS SUCH AS "OUTLOTS", "COMMON ELEMENTS", "OPEN SPACE", "OPEN AREA", "COMMON GROUND", "PARKING" AND "COMMON AREA". THE TERM "COMMON AREA OR AREAS", AND "COMMON ELEMENTS" INCLUDE REAL PROPERTY SURFACED WITH INTERIOR DRIVEWAYS AND WALKWAYS, BUT EXCLUDES REAL PROPERTY PHYSICALLY OCCUPIED BY A BUILDING, SERVICE BUSINESS DISTRICT OR STRUCTURES SUCH AS A POOL, RETENTION POND OR MECHANICAL EQUIPMENT.

RELOCATION OF FACILITIES WILL BE DONE BY GRANTEES AT COST OF THE GRANTOR/LOT OWNER, UPON WRITTEN REQUEST.

DECLARATION OF RESTRICTIVE COVENANTS

THE UNDERSIGNED OWNER HEREBY DECLARES THAT THE REAL PROPERTY DESCRIBED IN AND DEPICTED ON THIS PLAT OF SUBDIVISION SHALL BE HELD, TRANSFERRED, SOLD, CONVEYED AND OCCUPIED SUBJECT TO THE FOLLOWING COVENANTS AND RESTRICTIONS:

(A) ALL PUBLIC UTILITY STRUCTURES AND FACILITIES, WHETHER LOCATED ON PUBLIC OR PRIVATE PROPERTY SHALL BE CONSTRUCTED WHOLLY UNDERGROUND, EXCEPT FOR TRANSFORMERS, TRANSFORMER PADS, LIGHT POLES, REGULATORS, VALVES, MARKERS AND SIMILAR STRUCTURES APPROVED BY THE VILLAGE ENGINEER OF THE VÍLLAGE OF DOWNERS GROVE PRIOR TO RECORDING OF THIS PLAT OF SUBDIVISION.

(B)AN EASEMENT FOR SERVING THE SUBDIVISION, AND OTHER PROPERTY WITH STORM DRAINAGE, SANITARY SEWER, STREET LIGHTING, POTABLE WATER SERVICE, AND OTHER PUBLIC UTILITY SERVICES, IS HEREBY RESERVED FOR AND GRANTED TO THE VILLAGE OF DOWNERS GROVE AND DOWNERS GROVE SANITARY DISTRICT, THEIR RESPECTIVE SUCCESSORS AND ASSIGNS, JOINTLY AND SEPARATELY, TO INSTALL, OPERATE AND MAINTAIN, AND REMOVE, FROM TIME TO TIME, FACILITIES AND EQUIPMENT USED IN CONNECTION WITH THE PUBLIC WATER SUPPLY, TRANSMISSION LINES, SANITARY SEWERS, STORM DRAINAGE SYSTEM, STREET LIGHTING SYSTEM, OR OTHER PUBLIC UTILITY SERVICE, AND THEIR APPURTENANCES, EITHER ON, OVER, ACROSS, BELOW OR THROUGH THE GROUND SHOWN WITHIN THE DOTTED LINES ON THE PLAT MARKED "PUBLIC UTILITY AND/OR DRAINAGE EASEMENT. "OR SIMILAR LANGUAGE DESIGNATING A STORMWATER OR SEWER EASEMENT. AND THÉ PROPERTY DESIGNATED ON THE PLAT FOR STREETS AND ALLEYS, TOGETHER WITH THE RIGHT TO CUT, TRIM OR REMOVE TREES. BUSHES AND ROOTS AS MAY BE REASONABLY REQUIRED INCIDENT TO THE RIGHTS HEREIN GIVEN, AND THE RIGHT TO ENTER UPON THE SUBDIVIDED PROPERTY FOR ALL SUCH PURPOSES. OBSTRUCTIONS SHALL NOT BE PLACED OVER GRANTEES' FACILITIES OR IN, UPON OR OVER, THE PROPERTY WITHIN THE STORMWATER OR SEWER EASEMENT WITHOUT THE PRIOR WRITTEN CONSENT OF GRANTEES. AFTER INSTALLATION OF ANY SUCH FACILITIES, THE GRADE OF THE SUBDIVIDED PROPERTY SHALL NOT BE ALTERED IN A MANNER SO AS TO INTERFERE WITH THE PROPER OPERATION AND MAINTENANCE THEREOF.

WHEREAS, SAID LOTS WILL BE CONVEYED TO PURCHASERS SUBJECT TO THIS DECLARATION TO THE END THAT THE RESTRICTIONS IMPOSED SHALL INURE TO THE BENEFIT OF EACH AND ALL OF THE PURCHASERS OF SUCH LOTS WHETHER THEY SHALL HAVE BECOME SUCH BEFORE OR AFTER THE DATE THEREOF, AND THEIR RESPECTIVE HEIRS AND ASSIGNS, AND

WHEREAS, THE AFORESAID PROPERTY DESCRIBED ON THE ATTACHED PLAT IS LOCATED ENTIRELY WITHIN THE CORPORATE LIMITS OF THE VILLAGE OF DOWNERS GROVE, ILLINOIS, AND

WHEREAS, ALL OF THE PROVISIONS, RESTRICTIONS, CONDITIONS, COVENANTS, AGREEMENTS, AND CHARGES HEREIN CONTAINED SHALL RUN WITH AND BIND ALL OF SAID LOTS AND LAND AND SHALL INURE TO THE BENEFIT OF. AND BE ENFORCEABLE BY THE VILLAGE OF DOWNERS GROVE, ILLINOIS, AND THE OWNERS OR OWNER OF ANY OF THE LOTS OF LAND COMPRISED WITHIN SAID PLAT, AND THEIR RESPECTIVE HEIRS, EXECUTORS, ADMINISTRATORS, SUCCESSORS, GRANTEES AND ASSIGNS.

NOW, THEREFORE, ALL PERSONS, FIRMS OR CORPORATIONS NOW OWNING THE AFORESAID PROPERTY DO COVENANT AND AGREE THAT THEY OR ANY PERSON, FIRM OR CORPORATION HEREAFTER ACQUIRING ANY PROPERTY OR LOTS SHOWN UPON THE ATTACHED PLAT OF SUBDIVISION ARE HEREBY SUBJECTED TO THE FOLLOWING RESTRICTIONS RUNNING WITH SAID PROPERTY TO WHOMSOEVER OWNED. TO WIT:

OWNER HEREBY GRANTS TO THE VILLAGE OF DOWNERS GROVE A STORMWATER MANAGEMENT EASEMENT FOR THE USE AND BENEFIT OF THE VILLAGE. OVER THE STORMWATER FACILITIES WITHIN THE PROPERTY AND A RIGHT OF ACCESS TO PRIVATELY-OWNED LAND FOR THE REASONABLE EXERCISE OF THE RIGHTS GRANTED TO THE VILLAGE.

EACH OWNER OR PURCHASER SHALL BE RESPONSIBLE TO INSPECT AND MAINTAIN THE STORMWATER FACILITIES ON THEIR LOT. NO BUILDINGS OR STRUCTURES OF ANY KIND SHALL BE PLACED ON SAID EASEMENT NOR SHALL ANY OTHER CHANGE BE MADE ON THE PROPERTY THAT MIGHT MATERIALLY AFFECT THE PROPER MANAGEMENT. OPERATION OR CONTINUED MAINTENANCE OF ANY STORMWATER FACILITY: IMPEDE STORMWATER DRAINAGE IN OR ON THE PROPERTY; NEGATIVELY IMPACT THE WATER QUALITY OF THE STORMWATER FACILITIES; OR MATERIALLY REDUCE THE STORMWATER DETENTION OR RETENTION CAPACITY THEREOF AS PROVIDED IN THE APPROVED PLANS.

IN THE EVENT THE VILLAGE DETERMINES, IN ITS SOLE AND ABSOLUTE DISCRETION, THAT THE PROHIBITIONS OF THE PRECEDING PARAGRAPH HAVE BEEN VIOLATED OR THAT PROPER MAINTENANCE OF THE STORMWATER FACILITIES IS NOT BEING PERFORMED OR THAT PROPER OPERATION OF THE STORMWATER FACILITIES IS NOT OCCURRING ON THE PROPERTY AT ANY TIME. THE VILLAGE OR ITS CONTRACTORS OR AGENTS. AFTER TEN (10) DAYS PRIOR WRITTEN NOTICE TO THE OWNER. MAY, BUT SHALL NOT BE OBLIGATED TO, ENTER UPON ANY OR ALL OF THE PROPERTY FOR THE PURPOSES OF (A) CORRECTING ANY VIOLATION AND (B) PERFORMING MAINTENANCE WORK ON AND TO THE STORMWATER FACILITIES.

IN THE EVENT THAT THE VILLAGE SHALL PERFORM, OR CAUSE TO BE PERFORMED, ANY WORK PURSUANT TO THE STORMWATER MANAGEMENT EASEMENT, THE VILLAGE SHALL HAVE THE RIGHT TO CHARGE THE OWNER AN AMOUNT SUFFICIENT TO DEFRAY THE ENTIRE COST OF SUCH WORK, INCLUDING ADMINISTRATIVE COSTS, EITHER BEFORE OR AFTER SUCH COST IS INCURRED. IF THE AMOUNT SO CHARGED IS NOT PAID BY THE OWNER WITHIN THIRTY (30) DAYS FOLLOWING A DEMAND IN WRITING BY THE VILLAGE FOR SUCH PAYMENT, SUCH CHARGE, TOGETHÈR WITH INTEREST AND COSTS OF COLLECTION, SHALL BECOME A LIEN UPON THE PROPERTY AND THE VILLAGE SHALL HAVE THE RIGHT TO COLLECT SUCH CHARGE, WITH INTEREST AND COSTS, AND TO ENFORCE SUCH LIEN AS IN FORECLOSURE PROCEEDINGS AS PERMITTED BY LAW.

IN WITNESS WHEREOF, THE OWNERS HAVE SET THEIR HANDS UPON THE ATTACHED PLAT THE DAY AND DATE FIRST WRITTEN THERON.

| C | OATED THIS, DAY OF, 20 |
|---|------------------------|
| E | OWNER |
| Е | SY: NOTARY PUBLIC |

| COUNTY RECORDER CERTIFICATION |
|---|
| STATE OF ILLINOIS))SS |
| COUNTY OF DUPAGE) |
| THIS PLAT WAS FILED FOR RECORD IN THE RECORDER'S OFFICE OF DUPAGE |
| COUNTY, ILLINOIS, ON THE DAY OF A.D. 20, AT |
| O'CLOCKM. AS DOCUMENT NUMBER |
| |
| COUNTY RECORDER |

SURVEYOR'S AUTHORIZATION TO RECORD

STATE OF ILLINOIS COUNTY OF KANE AND/OR REPRESENTATIVES THEREOF, TO RECORD THIS PLAT, A TRUE COPY OF WHICH HAS BEEN RETAINED BY ME TO ASSURE NO CHANGES HAVE BEEN MADE TO SAID PLAT. DATED THIS _____ DAY OF______, 20___, AT AURORA, KANE COUNTY, ILLINOIS COMPASS SURVEYING LTD

PROFESSIONAL DESIGN FIRM

LAND SURVEYOR CORPORATION NO. 184-002778

LICENSE EXPIRES 4/30/2023

SCOTT C. KREBS ILLINOIS PROFESSIONAL LAND SURVEYOR NO. 3509 EXPIRES 11/30/2022

SURVEYOR'S CERTIFICATE

STATE OF ILLINOIS COUNTY OF KANE

I, SCOTT C. KREBS, ILLINOIS PROFESSIONAL LAND SURVEYOR NO. 3509, HAVE SURVEYED AND SUBDIVIDED THE FOLLOWING

PARCEL 1: LOTS 1 THROUGH 11, BOTH INCLUSIVE, AND THE WEST 11.3 FEET (AS MEASURED ON THE SOUTH LINE) BY 16.85 FEET (MEASURED ON THE NORTH LINE) OF LOT 12 IN STANLEY'S SUBDIVISION, ALSO THAT PART OF VACATED RAILROAD STREET LYING NORTH OF AND ADJOINING LOTS 1, 2, 3, 4 AND 5 IN SAID STANLEY'S SUBDIVISION, OF PART OF THE NORTHWEST QUARTER AND THE SOUTHWEST QUARTER OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO THE PLAT THEREOF RECORDED FEBRUARY 18, 1884 AS DOCUMENT 32879, IN DUPAGE COUNTY, ILLINOIS.

A PART OF LOT 12 OF THE PLAT OF STANLEY'S SUBDIVISION OF PART OF THE NORTHWEST QUARTER AND THE SOUTHWEST QUARTER OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11 EAST OF THE THIRD PRINCIPAL MERIDIAN, AS SHOWN BY THE PLAT RECORDED IN BOOK OF PLATS, ON PAGE 43, AND ALSO A PART OF LOT 1 OF THE PLAT OF THE HOMESTEAD OF THOMAS LYMAN, DECEASED, AS RECORDED IN BOOK 6 OF PLATS, ON PAGE 5, OF THE RECORDS OF DUPAGE COUNTY, ILLINOIS; THE ENTIRE TRACT HEREIN INCLUDED, BEING DESCRIBED AS FOLLOWS, TO WIT: COMMENCING AT THE SOUTHWEST CORNER OF SAID LOT 12. OF STANLEY'S SUBDIVISION, AND RUNNING THENCE EAST ALONG THE NORTH LINE OF CURTISS STREET. 11.3 FEET FOR A PLACE OF BEGINNING; THENCE NORTHERLY TO A POINT ON THE NORTH LINE OF SAID LOT 12, WHICH POINT IS 16.85 FEET EAST FROM THE NORTHWEST CORNER OF SAID LOT; THENCE EAST, ON THE NORTH LINE OF SAID LOT 12, 53.75 FEET TO THE NORTHEAST CORNER OF SAID LOT, THENCE SOUTHERLY, TO A POINT ON THE NORTH LINE OF CURTISS STREET WHICH IS 7 FEET EAST FROM THE SOUTHEAST CORNER OF SAID LOT 12; THENCE WESTERLY ALONG THE NORTH LINE OF CURTISS STREET, 50 FEET TO THE PLACE OF BEGINNING, IN THE VILLAGE OF DOWNERS GROVE, DUPAGE COUNTY, ILLINOIS.

PARCEL 2: LOTS 1-6 BOTH INCLUSIVE, AND THE EAST 5 1/2 FEET OF LOT 7 IN RANDALL'S RESUBDIVISION OF BLOCK 2 IN THE TOWN OF DOWNERS GROVE, IN THE WEST HALF OF SECTION 8. TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO THE PLAT OF SAID RANDALL'S RESUBDIVISION RECORDED OCTOBER 26, 1875 AS DOCUMENT 20748, IN DUPAGE COUNTY, ILLINOIS.

PARCEL 3: THE NORTH 35 FEET (AS MEASURED AT RIGHT ANGLES TO THE NORTHEASTERLY LINE OF SAID LOT) OF LOT 6, IN WALLACE'S SUBDIVISION OF PART OF LOT 5 OF THE ASSESSOR'S SUBDIVISION OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOIS.

PARCEL 4: THE NORTH 35 FEET (AS MEASURED AT RIGHT ANGLES TO THE NORTHERLY LINE OF SAID LOT) OF LOT 7. IN WALLACE'S SUBDIVISION OF PART OF LOT 5 OF THE ASSESSOR'S SUBDIVISION OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOIS.

PARCEL 5: LOT 1 OF THE PLAT OF THE HOMESTEAD OF THOMAS LYMAN, DECEASED (EXCEPT THAT PART CONVEYED TO THE CHICAGO BURLINGTON & QUINCY RAILROAD AND EXCEPT THAT PART CONVEYED TO H.J. HAWKINS ON MARCH 27, 1907) IN SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOIS.

PARCEL 6: SO MUCH OF THE SOUTHWEST QUARTER OF THE NORTHEAST QUARTER (SW1/2 NE1/2) OF SECTION 8. TOWNSHIP 38 NORTH, RANGE 11 EAST OF THE THIRD PRINCIPAL MERIDIAN, IN DUPAGE COUNTY, ILLINOÍS, DESCRÍBED AS FOLLOWS:

COMMENCING AT A POINT ON THE SOUTH LINE OF 100-FOOT WIDE RIGHT OF WAY OF BURLINGTON NORTHERN, INC., 336.6 FEET EASTWARD (MEASURED ON SAID SOUTH LINE) FROM THE INTERSECTION OF THE NORTH AND SOUTH CENTER LINE OF SAID SECTION 8 WITH SAID SOUTH LINE TO THE PLACE OF BEGINNING; THENCE SOUTH 9 DEGREES WEST (MAGNETIC BEARING), 20.14 FEET; THENCE EASTWARD, PARALLEL TO SAID SOUTH RIGHT OF WAY LINE AND 20 FEET DISTANT THEREFROM, 425.9 FEET; THENCE NORTH 64 DEGREES EAST (MAGNETIC BEARING), 42.3 FEET TO SAID SOUTH RIGHT OF WAY LINE: THENCE WESTWARD ON SAID SOUTH RIGHT OF WAY LINE. 459.5 FEET TO THE PLACE OF BEGINNING.

PARCEL 7: LOTS 3. 4. 5 AND LOT 6 (EXCEPT THE WEST 50 FEET THEREOF) IN BLOCK 2 IN CURTISS ADDITION TO THE TOWN OF DOWNERS GROVE, BEING A SUBDIVISION OF PART OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO THE PLAT THEREOF RECORDED MARCH 8, 1866 AS DOCUMENT 7317, IN DUPAGE COUNTY, ILLINOIS.

PARCEL 8: LOTS 1 AND 2 IN BLOCK 2 IN CURTISS ADDITION TO THE TOWNS OF DOWNERS GROVE, BEING A SUBDIVISION OF PART OF SECTION 8, TOWNSHIP 38 NORTH, RANGE 11, EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO THE PLAT THEREOF RECORDED MARCH 8, 1866 AS DOCUMENT 7317, IN DUPAGE COUNTY, ILLINOIS.

AS SHOWN BY THE ANNEXED PLAT WHICH IS A CORRECT REPRESENTATION OF SAID SURVEY AND SUBDIVISION. ALL DISTANCES ARE SHOWN IN FEET AND DECIMALS THEREOF. THIS SUBDIVISION IS WITHIN THE __ AN OFFICIAL COMPREHENSIVE PLAN AND IS EXERCISING THE SPECIAL POWERS AUTHORIZED BY THE STATE OF ILLINOIS ACCORDING TO 65 ILCS 5/11-12-6 AS HERETOFORE AND HEREAFTER AMENDED AND THIS SITE FALLS WITHIN THE FOLLOWING FLOOD ZONES:

- a. "OTHER AREAS: ZONE X" (UNSHADED) (AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN).
- b. "SPECIAL FLOOD HAZARD AREAS SUBJECT TO INUNDATION BY THE 1% ANNUAL FLOOD CHANCE: ZONE AE", (BASE FLOOD ELEVATIONS DETERMINED).
- c. "SPECIAL FLOOD HAZARD AREA: (SHADED) 0.2% ANNUAL CHANCE FLOOD HAZARD, AREAS OF 1% ANNUAL CHANCE FLOOD WITH AVERAGE DEPTH LESS THAN ONE FOOT OR WITH DRAINAGE AREAS OF LESS THAN ONE SQUARE MILE ZONE X".

AS DEFINED BY THE FLOOD INSURANCE RATE MAP NUMBER 17043C0167J, HAVING AN EFFECTIVE REVISION DATE OF AUGUST 1,

GIVEN UNDER MY HAND AND SEAL AT AURORA, ILLINOIS THIS_____ DAY OF _____, 20____, 20____.

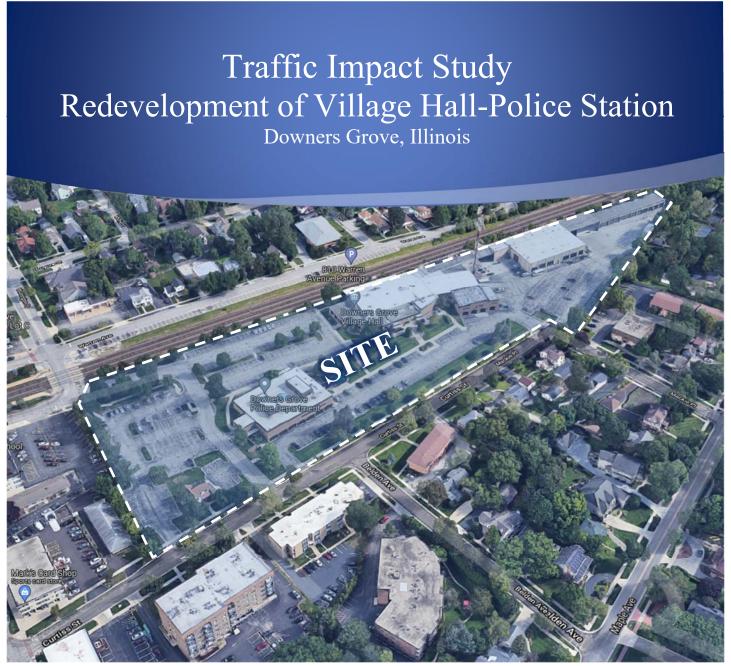
COMPASS SURVEYING LTD PROFESSIONAL DESIGN FIRM LAND SURVEYOR CORPORATION NO. 184-002778 LICENSE EXPIRES 4/30/2023

| BY: | | |
|-------------------------------------|-----|------|
| SCOTT C. KREBS | | |
| ILLINOIS PROFESSIONAL LAND SURVEYOR | NO. | 3509 |
| LICENSE EXPIRES 11/30/2022 | | |

J:\PSDATA\2022 PROJECTS\22.0080\22.0080 POSUB.DWG

SCALE: 1'' = 60'

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Prepared For:



Village of Downers Grove



March 29, 2022

1. Introduction

This report summarizes the methodologies, results, and findings of a traffic impact study conducted by Kenig, Lindgren, O'Hara, Aboona, Inc. (KLOA, Inc.) for the proposed redevelopment of the Village Hall and Police Station in Downers Grove, Illinois. The site, which is currently occupied by the existing Downers Grove Village Hall and Police Station and the Downers Grove Metra Station Commuter Lot L, is bounded by the Burlington Northern Santa Fe (BNSF) Metra Commuter railroad tracks on the north, the Village of Downers Grove fleet maintenance garage on the east, Curtiss Street on the south, and two multifamily buildings on the west.

As proposed, the site will be redeveloped with a new combined civic center building containing the Village Hall, Police Station, and the relocation of the School District 58 offices. The site will provide a total of approximately 252 parking spaces of which 80 will be gated police staff parking spaces, 65 will be Village Hall staff parking, 25 will be School District 58 staff parking, and 82 will be public parking spaces. Access to the site will be provided via five full movement access drives on Curtiss Street. As part of the proposed redevelopment, the existing connection within the site that extended between Curtiss Street and Washington Street/Burlington Avenue will be eliminated.

The purpose of this study was to examine background traffic conditions, assess the impact that the proposed redevelopment will have on traffic conditions in the area, and determine if any roadway or access improvements are necessary to accommodate the additional traffic generated by the proposed redevelopment. **Figure 1** shows the location of the site in relation to the area roadway system. **Figure 2** shows an aerial view of the site. The sections of this report present the following:

- Existing roadway conditions
- A description of the proposed redevelopment
- Directional distribution of the redevelopment traffic
- Vehicle trip generation for the redevelopment
- Future traffic conditions including access to the redevelopment
- Traffic analyses for the weekday morning and weekday evening peak hours
- Recommendations with respect to adequacy of the site access and adjacent roadway system
- Evaluation of the adequacy of the proposed parking supply

Traffic capacity analyses were conducted for the weekday morning and weekday evening peak hours for the following conditions:

- 1. Existing Conditions Analyze the capacity of the existing roadway system using existing peak hour traffic volumes in the surrounding area.
- 2. Projected Conditions Analyze the capacity of the future roadway system using the projected traffic volumes that include the existing traffic volumes, ambient traffic growth, the redistribution of the existing traffic volumes due to the roadway modifications, and the additional traffic estimated to be generated by the proposed redevelopment.



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Site Location Figure 1



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Aerial View of Site

Figure 2



2. Existing Conditions

Existing traffic and roadway conditions were documented based on field visits and traffic counts conducted by KLOA, Inc. The following provides a detailed description of the physical characteristics of the roadways including geometry and traffic control, adjacent land uses, and peak hour traffic flows along area roadways.

Site Location

The site, which is currently occupied by the existing Downers Grove Village Hall and Police Station and the Downers Grove Metra Station Lot L, is bounded by Curtiss Street on the north, the Burlington Northern Santa Fe (BNSF) Metra Commuter railroad on the north, the Village of Downers Grove fleet maintenance garage on the east, and two multi-family buildings on the west and is located just east of downtown Downers Grove. Land uses in the vicinity of the site include residential uses to the north, east and south and commercial, residential, and mixed-use uses to the west. The Downers Grove Metra station is located just west of the site.

Existing Roadway System Characteristics

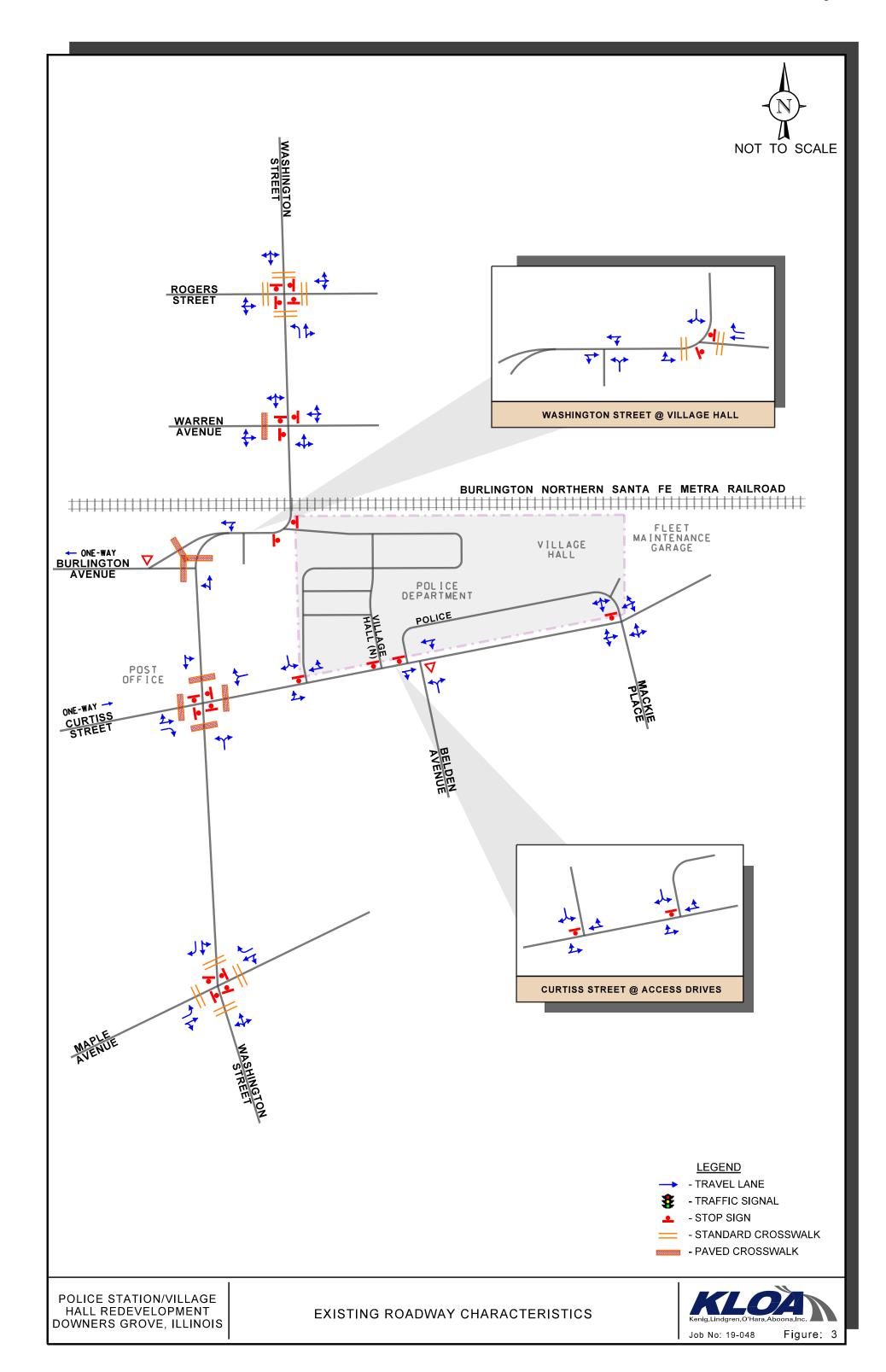
The characteristics of the existing roadways that surround the proposed redevelopment are illustrated in **Figure 3** and described below.

Washington Street is a north-south roadway that provides one travel lane in each direction and is classified as a collector roadway between Maple Avenue and Rogers Street. Parking is generally permitted on both sides of the road. Washington Street is under all-way stop sign control at its intersections with Maple Avenue, Curtiss Street, and Rogers Street. At its intersection with Warren Avenue, all legs of the intersection are under stop sign control except northbound Washington Street. Washington Street provides an exclusive right-turn lane on the southbound approach at its intersection with Maple Street. It should be noted that Washington Street has an at-grade railroad crossing with the that BNSF/Metra railroad tracks that is located between Burlington Avenue and Warren Avenue.

Curtiss Street is an east-west roadway that is classified as a collector roadway west of Washington Street and a local roadway east of Washington Street. West of Washington Street, Curtiss Street is a one-way eastbound roadway that has a single eastbound travel lane with parking generally permitted on both sides of the road. East of Washington Street, Curtiss Street is a two-way roadway with a single travel lane in each direction with parking generally permitted on both sides of the road. At is intersection with Washington Street, Curtiss Street is under all-way stop-sign control and the eastbound approach provides an exclusive right-turn lane.



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Burlington Avenue is an east-west, local roadway that extends from the Village Hall access drive west to its terminus at Forest Avenue. Between the Washington Street segments, Burlington Avenue provides a single travel lane in each direction with parking prohibited on both sides of the road. Between Washington Street and Forest Avenue, Burlington Avenue is a one-way westbound road that provides one travel lane with parking generally permitted on both sides of the road. Burlington Avenue and the Village Hall access drive are under stop sign control at their intersection with Washington Street.

Maple Avenue is generally a northeast-to-southwest, collector roadway that provides a single travel lane in each direction with parking is generally permitted on the north side of the road only. At its interaction with Washington Street, Maple Street is under all-way stop sign control and provides an exclusive left-turn lane on the eastbound approach and an exclusive right-turn lane on the westbound approach.

Warren Avenue is an east-west, local roadway that provides a single travel lane in each direction with parking generally permitted on both sides of the road. At its intersection with Washington Street. all legs of the intersection are under stop sign control except northbound Washington Street.

Rogers Street is an east-west, collector roadway that provides a single travel lane in each direction with parking generally permitted on the south side of the road only. At its intersection with Washington Street, Rogers Street is under all-way stop sign control.

Belden Avenue is a north-south, local roadway that provides a single travel lane in each direction with parking generally permitted on the east side of the road only. No traffic control is provided at its intersection with Curtiss Street. However, given the nature of the intersection, the Belden Avenue approach operates in the yield condition.

Mackie Place is a north-south, local roadway that provides a single travel lane in each direction with parking generally permitted on the east side of the road only. No traffic control is provided at its intersection with Curtiss Street and the access drives to the Village Hall and the public works facility. However, given the nature of the intersection, the eastbound right turns from Curtiss Street onto Mackie Place and northbound left turns from Mackie Place onto Belden Avenue operate under free flow conditions, with outbound movements from the access drives operating under yield control.



Existing Traffic Volumes

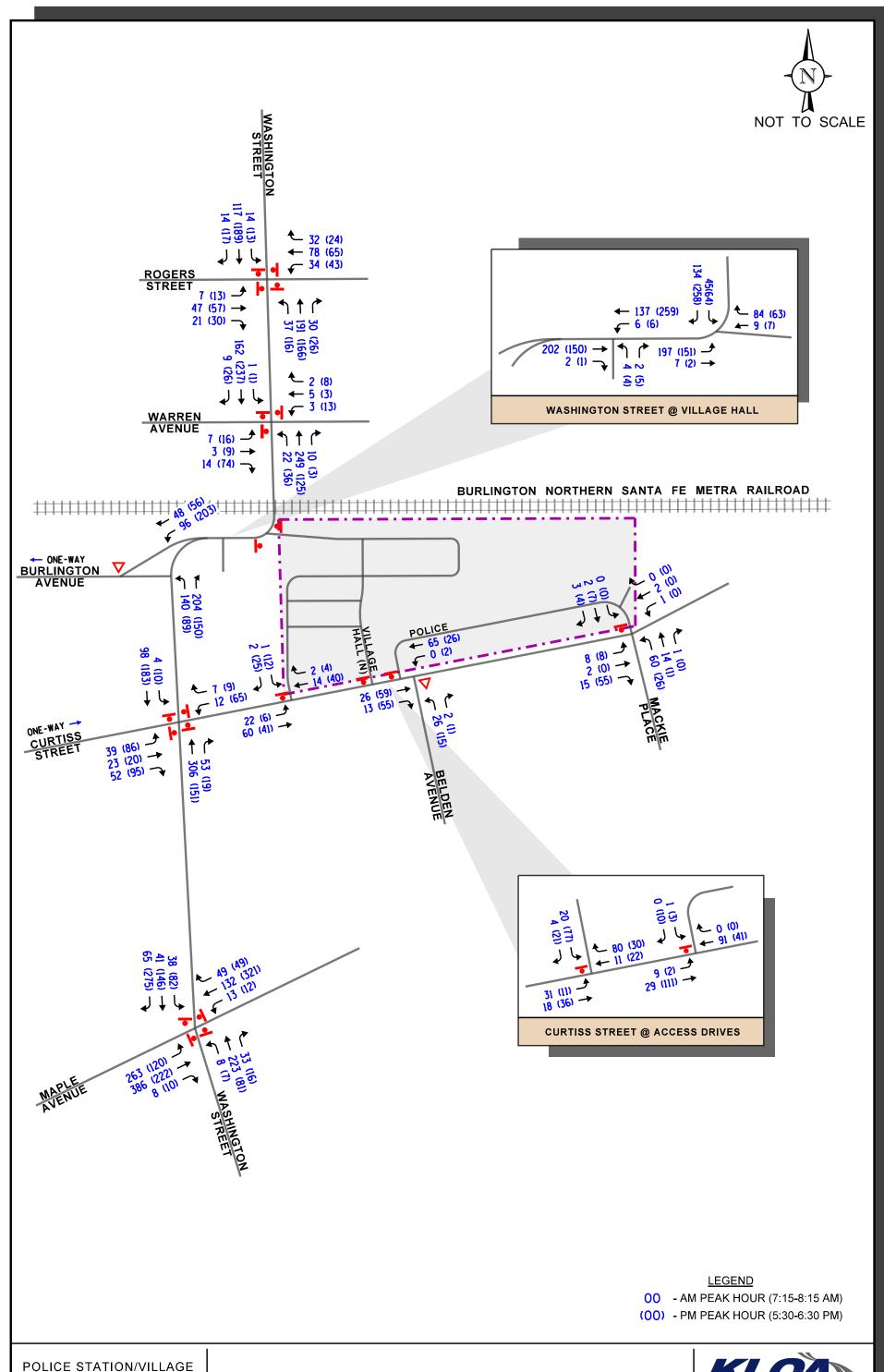
In order to determine current traffic conditions in the vicinity of the site, KLOA, Inc. conducted peak period vehicle, pedestrian, and bicycle movement traffic counts utilizing Miovision Scout Video Collection Units on Wednesday, February 13, 2019 and on Thursday, February 14, 2019 during the weekday morning (7:00 to 9:00 A.M.) and evening (4:30 to 6:30 P.M.) peak periods at the following intersections:

- Washington Street with Maple Avenue
- Washington Street with Curtiss Street
- Washington Street with Burlington Avenue
- Washington Street with Commercial Access Drive
- Washington Street with Burlington Avenue/Existing Village Hall Access Drive
- Washington Street with Warren Avenue
- Washington Street with Rogers Street
- Curtiss Street with Metra Commuter Parking Lot Access Drive
- Curtiss Street with Belden Avenue
- Curtiss Street with Village Hall Access Drive
- Curtiss Street with Police Department West Access Drive
- Curtiss Street with Mackie Place/Police Department East Access Drive

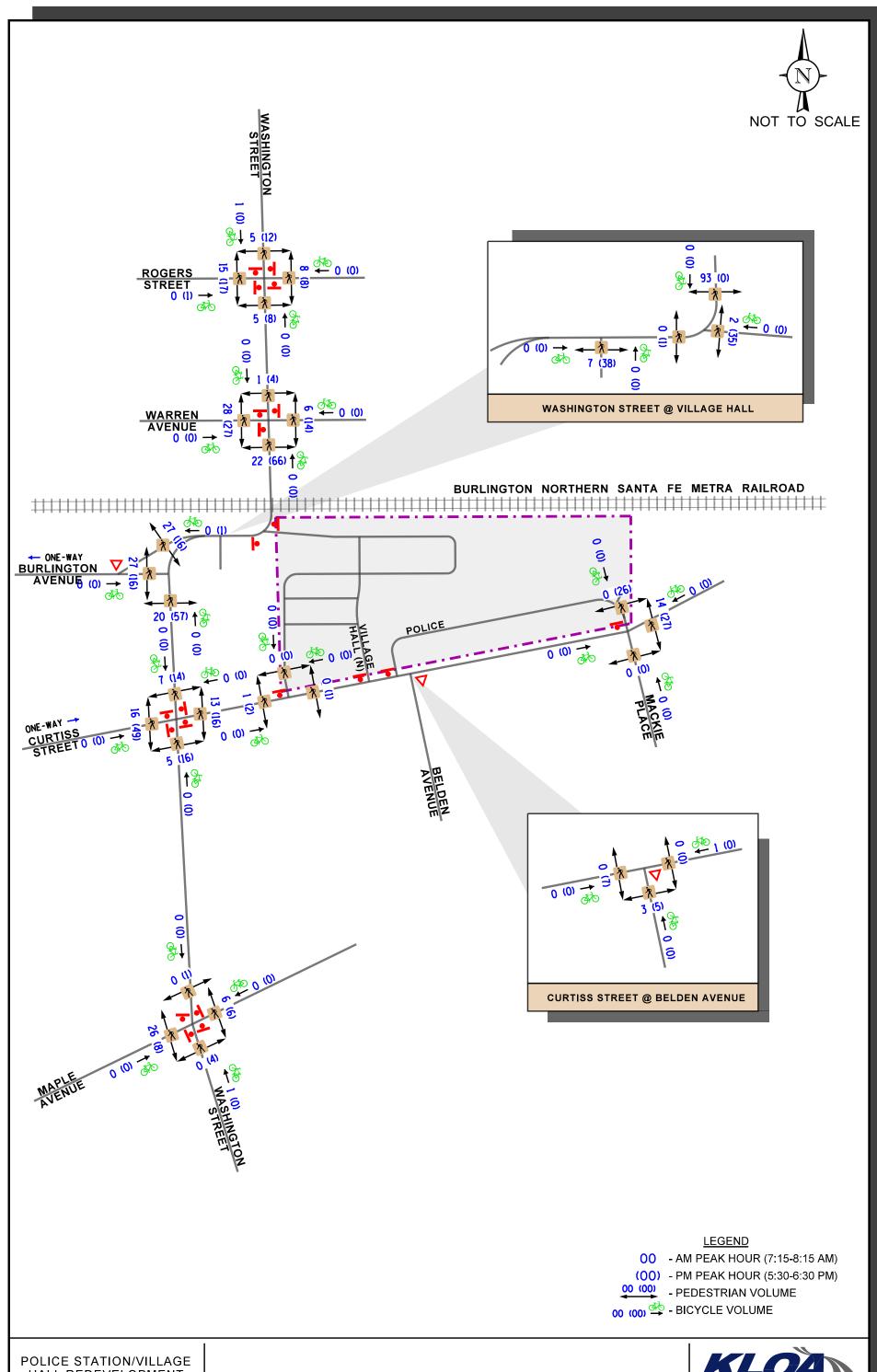
The results of the traffic counts showed that the weekday morning peak hour of traffic occurs from 7:15 A.M. to 8:15 A.M. and the weekday evening peak hour of traffic occurs from 5:30 P.M. to 6:30 P.M. **Figure 4** illustrates the existing peak hour traffic volumes. **Figure 5** illustrates the existing pedestrian and bicycle traffic volumes. Copies of the traffic count summary sheets are included in the Appendix.



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Train Crossing Observations

In order to determine the existing operations of the Washington Street at-grade railroad crossing with the BSFN/Metra railroad tracks and its impacts on the intersections of Washington Street with Burlington Avenue and Washington Street with Warren Avenue, observations of train activities were conducted during the weekday morning and weekday evening peak periods. These observations indicated the following:

- Four eastbound train events and four westbound train events occurred during the weekday morning peak hour. During eastbound train events, the gates were down for approximately three to four minutes and during westbound train events, the gates were down for approximately one to two minutes.
- Three eastbound train events and seven westbound train events occurred during the evening peak hour. During eastbound train events, the gates were down for approximately three and a half to five and a half minutes and during westbound train events, the gates were down for one to two minutes.
- Northbound queues on Washington Street typically extend to or just past Burlington Avenue with a peak queue that is several vehicles past Burlington Avenue and the northbound queues in the Village Hall parking lot typically extend to several vehicles along the norths-south section of the circulation road. During the weekday morning peak hour, the queues clear within two to three minutes and during the weekday evening peak hour, the queues clear within one to two minutes.
- Southbound queues on Washington Street typically extend to and past Rogers Street, especially during the weekday evening peak hour, and along Warren Avenue. During the weekday morning peak hour, the queues clear within one to two minutes and during the weekday evening peak hour, the queues clear within three to five minutes.

Crash Data Analysis

KLOA, Inc. obtained crash data¹ for the most recent available five years (2016 to 2020) for the study area intersections. **Table 1** summarizes the crash data for the intersection of Washington Street with Maple Avenue. A review of the crash data indicated the following:

- The intersection of Washington Street with Curtiss Street experienced zero crashes in 2016, 2018 and 2020, and one crash in 2017 and 2019.
- The easterly intersection of Washington Street with Burlington Avenue experienced one crash in 2016 and zero crashes between 2017 and 2020.

¹ IDOT DISCLAIMER: The motor vehicle crash data referenced herein was provided by the Illinois Department of Transportation. Any conclusions drawn from analysis of the aforementioned data are the sole responsibility of the data recipient(s). Additionally, for coding years 2015 to present, the Bureau of Data Collection uses the exact latitude/longitude supplied by the investigating law enforcement agency to locate crashes. Therefore, location data may vary in previous years since data prior to 2015 was physically located by bureau personnel.



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• The westerly intersection of Washington Street with Burlington Avenue experienced one crash in 2016 and zero crashes between 2017 and 2020.

- The intersection of Washington Street with Warren Avenue experienced one crash in 2016 and zero crashes between 2017 and 2020.
- The intersection of Washington Street with Rogers Street experienced one crash in 2016 and 2017 and zero crashes between 2018 and 2020.
- The intersections of Curtiss Street with Belden Avenue and Mackie Place experienced zero crashes between 2016 and 2017.
- The study area intersections did not experience any fatalities between 2016 and 2020.

Table 1 WASHINGTON STREET WITH MAPLE AVENUE – CRASH SUMMARY

| | Type of Crash Frequency | | | | | | | | | |
|---------|-------------------------|----------|----------|----------|-----------|----------|----------|----------|--|--|
| Year | Angle | Head On | Object | Rear End | Sideswipe | Turning | Other | Total | | |
| 2016 | 3 | 0 | 0 | 2 | 0 | 0 | 0 | 5 | | |
| 2017 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | | |
| 2018 | 3 | 0 | 0 | 0 | 0 | 1 | 0 | 4 | | |
| 2019 | 2 | 1 | 0 | 1 | 0 | 1 | 0 | 5 | | |
| 2020 | <u>0</u> | <u>0</u> | <u>0</u> | <u>1</u> | <u>0</u> | <u>0</u> | <u>0</u> | <u>1</u> | | |
| Total | 10 | 1 | 0 | 4 | 0 | 2 | 0 | 17 | | |
| Average | 2 | <1 | 0 | <1 | 0 | < 1 | 0 | 3.4 | | |



3. Traffic Characteristics of the Proposed Redevelopment

To evaluate the impact of the subject redevelopment on the area roadway system, it was necessary to quantify the number of vehicle trips the site will generate during the peak hours and then determine the directions from which the proposed traffic will approach and depart the site.

Proposed Site and Redevelopment Plan

As proposed, the subject site will be redeveloped with a new combined civic center building containing the Village Hall and Police Station. Furthermore, the new building will accommodate the relocation of the School District 58 offices. The redeveloped site will provide a total of approximately 252 parking spaces of which 80 will be gated police staff parking spaces, 65 will be Village Hall staff parking, 25 will be School District 58 staff parking, and 82 will be public parking spaces.

It should be noted that the site contains Downers Grove Metra Station Commuter Lot L, which has a total of 86 parking spaces. As part of the proposed development, the quarterly parking permits in this lot will be relocated to the various public parking locations with the downtown area. However, based on parking surveys performed by the Village, the COVID-19 pandemic has resulted in lower and more sporadic use of the quarterly permit commuter parking spaces and thus, these spaces are currently underutilized. Further, sufficient parking is available to accommodate the relocation of the commuter parking.

Redevelopment Access System

Access to the redeveloped site will be provided via the following five access drives:

- A full movement access drive on Curtiss Street located approximately 305 feet west of Belden Avenue. As proposed, the access drive will provide one inbound lane and one outbound lane with the outbound movements under stop sign control. This access drive will be located in the approximate location of the existing westerly access drive and will primarily serve 50 public parking spaces.
- A full movement access drive on Curtiss Street aligned opposite Belden Avenue. As
 proposed, the access drive will provide one inbound lane and one outbound lane with the
 outbound movements under stop sign control. This access drive will serve all of the
 proposed on-site parking locations.
- A full movement access drive on Curtiss Street located approximately 200 feet east of Belden Avenue. As proposed, the access drive will provide one inbound lane and one outbound lane with the outbound movements under stop sign control. This access drive will primarily serve the Village Hall and School District 58 staff parking spaces.



A full movement access drive on Curtiss Street aligned opposite Mackie Place which will
form the north leg of the intersection of Curtiss Street and Mackie Place. As proposed, the
access drive will provide one inbound lane and one outbound lane with the outbound
movements under stop sign control. This access drive will primarily serve the existing fleet
maintenance garage.

• A full movement access drive that will be aligned opposite Curtiss Street and will form the fourth (east) leg of the intersection of Curtiss Street with Mackie Place/fleet maintenance garage access drive. As proposed, the access drive will provide one inbound lane and one outbound lane with the outbound movements under stop sign control. This access drive will continue to serve the existing fleet maintenance garage.

Proposed Roadway Modifications

As part of the proposed redevelopment, the existing access drive to the subject site aligned opposite Burlington Avenue at its intersection with Washington Street will be eliminated. The area that contained the access drive will be converted to a public plaza that will also provide emergency access from the police station to Washington Street/Burlington Avenue via a gated connection. Similar to the current operations, the Washington Street northbound traffic is proposed to continue to be under stop sign control south of the railroad crossing. The removal of the access drive will convert the existing three-legged intersection of Washington Street/Burlington Avenue/access drive located just south of the BNSF/Metra at-grade railroad crossing to a two-legged intersection, which will eliminate any turning conflicts at this intersection and provide the following benefits:

- It will enhance the flow of traffic along northbound Washington Street and dissipate the northbound queue of traffic quicker after a train event as the northbound traffic will no longer have to defer to the Washington Street southbound left-turn movement or contend with outbound movements from the access drive.
- It will improve the pedestrian safety at this intersection as the pedestrians will have one less intersection leg to traverse and four less vehicle movements to contend with. Further, several pedestrian enhancements are proposed as part of the redevelopment, including a direct north-south pedestrian connection between Curtiss Street and Burlington Avenue/atgrade railroad crossing.
- The elimination of the southbound left-turn movement from Washington Street to the access drive will eliminate potential back-ups that may occur on the railroad tracks and allow the southbound queue of traffic to dissipate quicker after a train event.
- The provision of stopping northbound traffic south of the tracks, similar to existing conditions, will meter northbound traffic which will (1) permit turning movements at the intersection of Washington Street with Warren Avenue to occur more frequently and (2) provide additional gaps in the traffic stream for pedestrians to traverse Washington Street.



It should be noted that the elimination of the access drive will result in the redistribution of the Village Hall/Police Station traffic as well as the area traffic traveling through the subject site to area roadways, particularly Washington Street and Curtiss Street. However, the additional travel is limited and, as the traffic study will show, the area roadways and intersections have sufficient reserve capacity to accommodate the additional traffic. Further, the redistribution of the traffic to Washington Street will increase the queue of traffic along northbound Washington Street associated with the at-grade railroad crossing. However, the queue of traffic is anticipated to dissipate quicker than compared to existing conditions given the enhancement to the flow of the Washington Street northbound traffic.

Additionally, the westerly intersection of Washington Street with Burlington Avenue is proposed to be restriped to provide an exclusive northbound left-turn lane, a single northbound through lane, and a single southbound receiving lane along the Washington Street northbound approach. In order to accommodate left-turn lane, approximately two on-street parking spaces on the west side of Washington Street will need to be removed. It should be noted that no on-street parking on the east side of the roadway will be impacted. The northbound left-turn lane will (1) ensure Washington Street northbound left-turn movements do not block northbound through movements and (2) potentially reduce the Washington Street northbound queue due to the at-grade railroad crossing by providing a dedicated lane for the Washington Street northbound left-turn traffic to by-pass the northbound queue.

A copy of the site plan and a conceptual exhibit of the restriping of the westerly Washington Street/Burlington Avenue intersection is located in the Appendix.

Directional Distribution of Site Traffic

The directional distribution of how traffic approaches and departs the site was estimated based on the general travel patterns through the study area derived from the peak hour traffic volumes. Approximately 15 percent of traffic travels to/from the north, 15 percent of traffic travels to/from the east, 20 percent travels to/from the south, and 50 percent travels to/from the west.

Development Traffic Generation

The volume of existing peak hour traffic traveling to, from, and through the subject site was determined based on the results of the traffic counts conducted at the five existing access drives. The results of the traffic counts indicated that a total of 322 trips during the weekday morning peak hour (201 in and 121 out) and a total of 357 trips during the weekday evening peak hour (128 in and 229 out) traveled to, from, or though the subject site. It is important to note a portion of the traffic traveling to and from the subject site consisted of (1) area traffic that was traversing the site to travel between Washington Street/Burlington Avenue and Curtiss Street and (2) vehicles that were dropping off or picking up Metra commuters. The percentage of the traffic traveling through the subject site was estimated based on (1) the existing traffic volumes and (2) the trip generation characteristics of the existing uses occupying the subject site (Village Hall, Police Station, and Commuter Lot L).



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As previously indicated, in addition to the replacing the Village Hall and Police Station, the new civic center building will also accommodate the relocation of the School District 58 offices. According to Village officials, the School District 58 offices will have a total of approximately 22 employees and several visitors per day. In order to provide a conservative (worst-case) analysis, it was estimated that the School District offices will generate 20 inbound trips during the weekday morning peak hour and 20 outbound trips during the weekday evening peak hour. For the purposes of these analyses, it was assumed that any vehicle trips generated by visitors of the School District 58 offices would occur outside of the weekday morning and weekday evening peak hours. **Table 2** summarizes the (1) existing area traffic traveling through the site, (2) the traffic generated by the existing uses (Village Hall, Police Station, and Commuter Lot L), and (3) the trips estimated to be generated by the relocation of the School District 58 offices. It should be noted that to provide a conservative (worst-case) analysis, the traffic traveling to and from the Downers Grove Metra Station Lot L (86 spaces) was not removed from the traffic generated by the existing uses (Village Hall, Police Station, and Commuter Lot L).

Table 2
PEAK HOUR VEHICLE TRIP GENERATION

| Land Use | | day Mo | U | Weekday Evening Peak Hour | | | |
|--|-----|--------|-------|------------------------------|-----|-------|--|
| | In | Out | Total | In | Out | Total | |
| Existing Traffic Volumes | | | | | | | |
| Area through traffic and Metra commuter drop-off/pick-up activity | 90 | 90 | 180 | 95 | 95 | 190 | |
| • Traffic generated by the existing Village Hall, Police Station, and commuter lot | 111 | 31 | 142 | 33 | 134 | 167 | |
| Total | 201 | 121 | 322 | 128 | 229 | 357 | |
| New/Additional Traffic Volumes | | | | | | | |
| School District 58 Offices | 20 | 0 | 20 | 0 | 20 | 20 | |
| Total Redevelopment Trips | 131 | 31 | 162 | 33 | 154 | 187 | |



4. Projected Traffic Conditions

The total projected traffic volumes take into consideration the existing traffic volumes, increase in background traffic due to growth, and the traffic estimated to be generated by the proposed subject redevelopment.

New Redevelopment Traffic Assignment

The estimated additional weekday morning and evening peak hour traffic volumes that will be generated by the School District 58 offices were assigned to the roadway system in accordance with the previously described directional distribution. **Figure 6** illustrates the traffic assignment of the additional vehicle trips.

Existing Traffic Reassignment

The existing weekday morning and evening peak hour traffic volumes generated by the current Village Hall, Police Department, and Downers Grove Metra Commuter Lot L as well as the traffic that was traveling through the subject site were reassigned to the roadway network, taking into consideration the proposed access system and elimination of the connection between Curtiss Street and Washington Street/Burlington Avenue. **Figure 7** illustrates the traffic reassignment of the existing vehicle trips. It should be noted that this reassignment is conservative, as it maintains the trips generated by the existing commuter parking spaces. As previously indicated, these parking spaces will be relocated to other various public parking locations within the downtown area and that the utilization of these parking spaces has decreased with the COVID-19 pandemic. However, in order to provide a worst-case scenario, these trips were not removed/reduced from the area roadway network.

Year 2025 No-Build Traffic Conditions

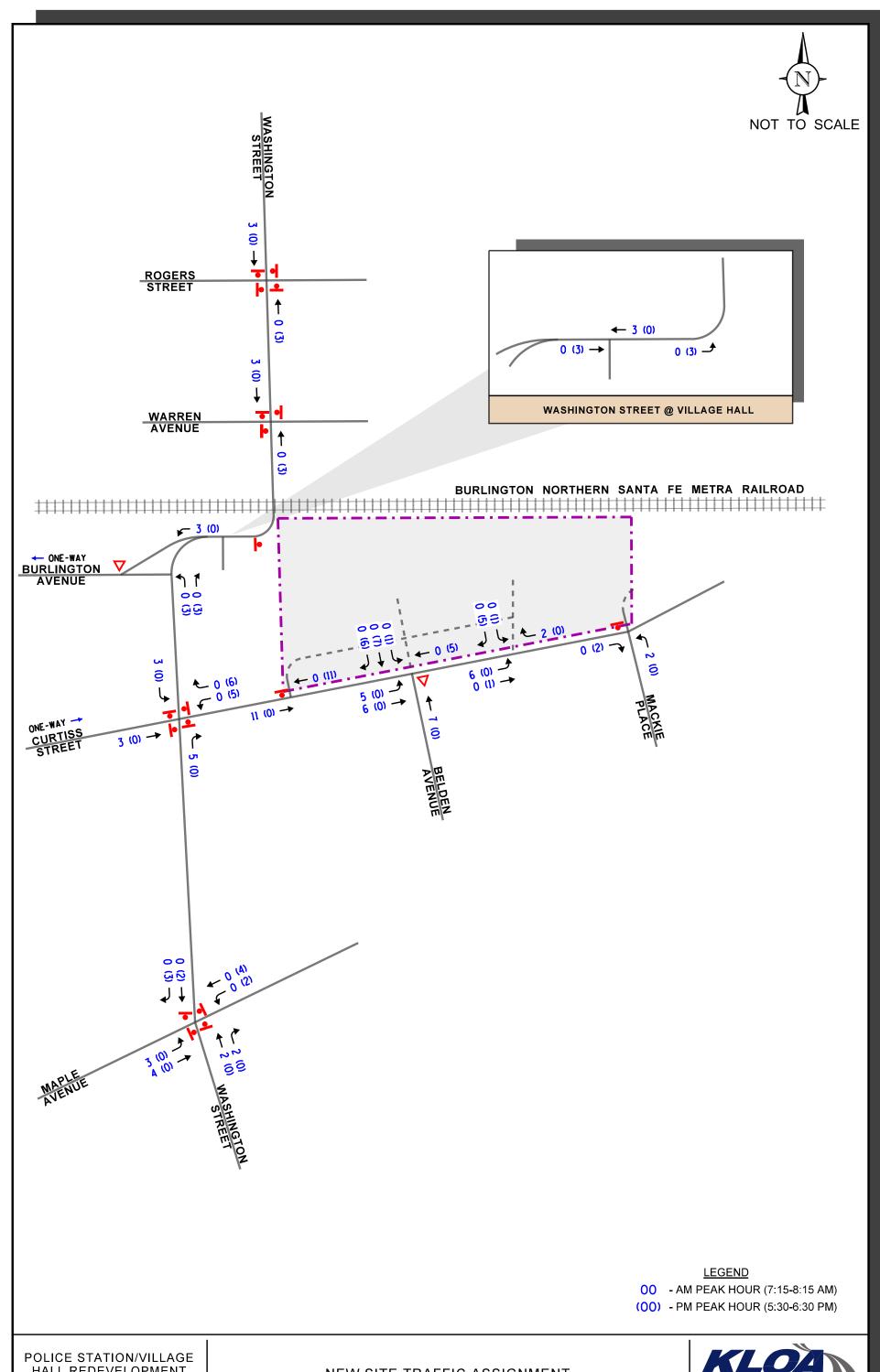
The existing traffic volumes (Figure 4) were increased by a regional growth factor to account for the increase in existing traffic related to regional growth in the area (i.e., not attributable to any planned development). Based on AADT projections provided by the Chicago Metropolitan Agency for Planning (CMAP), the existing traffic volumes are projected to increase by an annual growth rate of approximately 0.5 percent per year. As such, traffic volumes were increased by approximately three percent total to represent Year 2025 no-build conditions.

Year 2025 Total Projected Traffic Conditions

The assignment of new redevelopment-generated traffic (Figure 6) and reassignment of existing traffic volumes (Figure 7) was combined with the no-build traffic volumes to determine the Year 2025 total projected traffic volumes, illustrated in **Figure 8**.



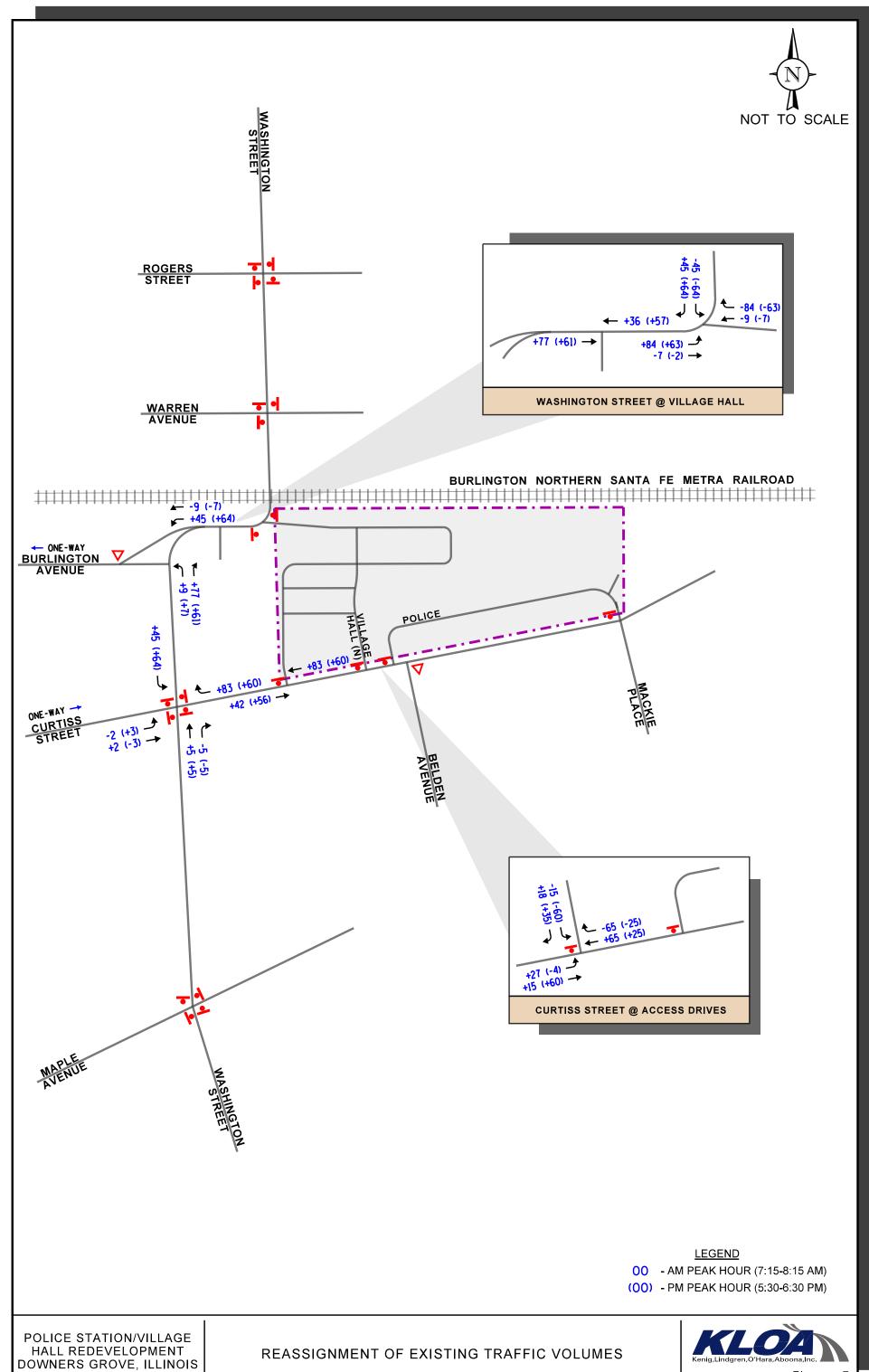
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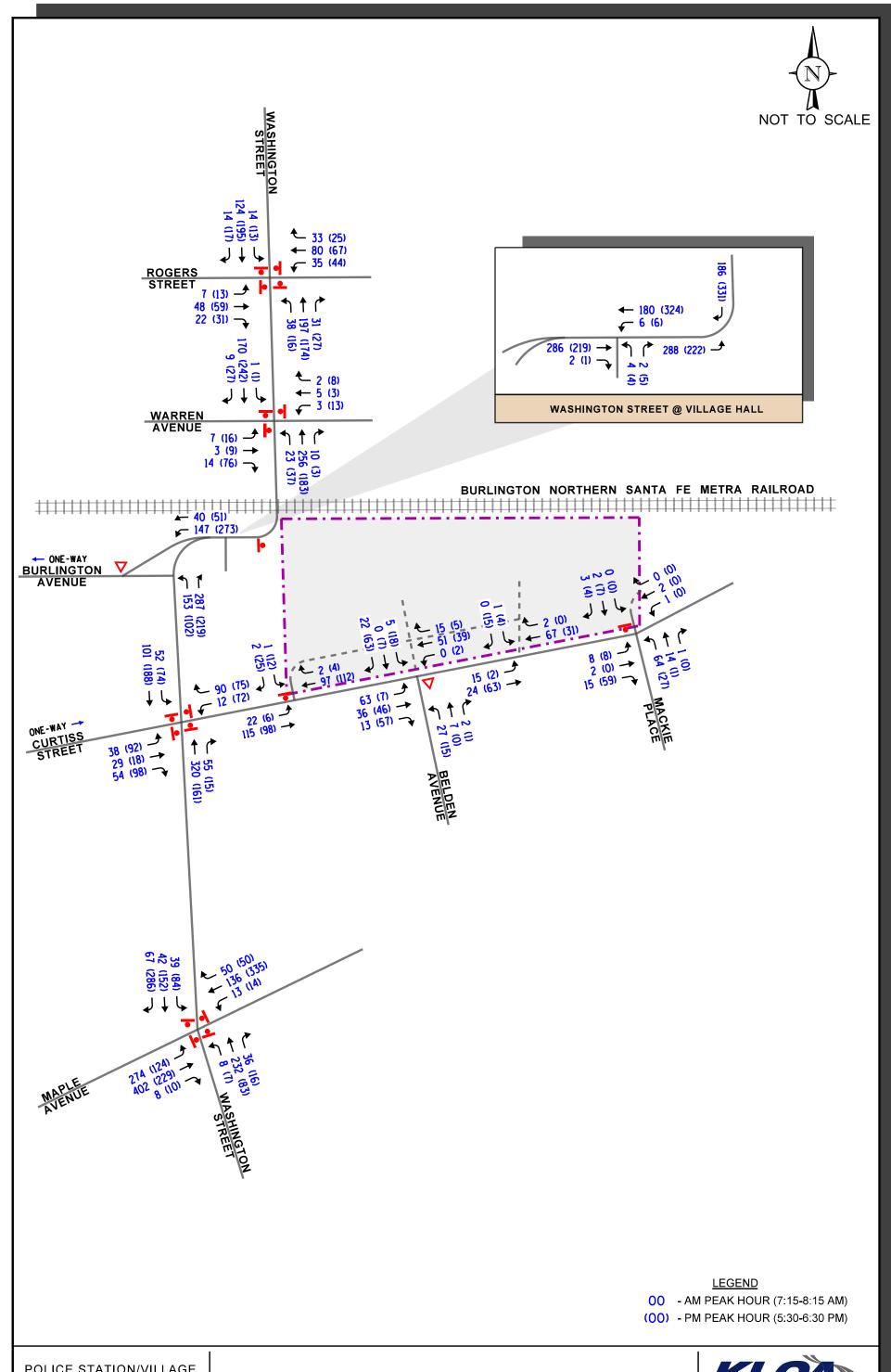
POLICE STATION/VILLAGE HALL REDEVELOPMENT DOWNERS GROVE, ILLINOIS

Figure: 6 Job No: 19-048

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POLICE STATION/VILLAGE HALL REDEVELOPMENT DOWNERS GROVE, ILLINOIS Kenig,Lindgren,O'Hara,Aboona,Inc.

Job No: 19-048 Figure: 8

5. Traffic Analysis and Recommendations

The following provides an evaluation conducted for the weekday morning and weekday evening peak hours. The analysis includes conducting capacity analyses to determine how well the roadway system and access drives are projected to operate and whether any roadway improvements or modifications are required.

Traffic Analyses

Roadway and adjacent or nearby intersection analyses were performed for the weekday morning and weekday evening peak hours for the existing (Year 2019) and Year 2025 total projected traffic volumes, taking into consideration the proposed Washington Street geometric improvements.

The traffic analyses were performed using the methodologies outlined in the Transportation Research Board's *Highway Capacity Manual (HCM)*, 6th Edition and analyzed using Synchro/SimTraffic 11 software.

The analyses for the unsignalized intersections determine the average control delay to vehicles at an intersection. Control delay is the elapsed time from a vehicle joining the queue at a stop sign (includes the time required to decelerate to a stop) until its departure from the stop sign and resumption of free flow speed. The methodology analyzes each intersection approach controlled by a stop sign and considers traffic volumes on all approaches and lane characteristics.

The ability of an intersection to accommodate traffic flow is expressed in terms of level of service, which is assigned a letter from A to F based on the average control delay experienced by vehicles passing through the intersection. The *Highway Capacity Manual* definitions for levels of service and the corresponding control delay for signalized intersections and unsignalized intersections are included in the Appendix of this report.

Summaries of the traffic analysis results showing the level of service and overall intersection delay (measured in seconds) for the existing and total projected conditions are presented in **Tables 3** and 4, respectively. A discussion of each intersection follows. Summary sheets for the capacity analyses are included in the Appendix.



Table 3 CAPACITY ANALYSIS RESULTS – EXISTING CONDITIONS

| | Intersection | • | Morning Hour | | y Evening Hour |
|--------------------|---|-------------|-------------------|-----|-------------------|
| | | LOS | Delay | LOS | Delay |
| Washi | ngton Street with Maple Avenue ¹ | | | | |
| • | Overall | C | 21.1 | C | 18.7 |
| • | Eastbound Approach | D | 25.9 | C | 16.2 |
| • | Westbound Approach | В | 13.0 | C | 24.4 |
| • | Northbound Approach | C | 20.1 | В | 14.0 |
| • | Southbound Approach | В | 12.0 | C | 17.0 |
| Washi | ngton Street with Curtiss Street ¹ | | | | |
| • | Overall | В | 11.8 | В | 10.2 |
| • | Eastbound Approach | A | 9.6 | В | 10.3 |
| • | Westbound Approach | A | 8.8 | A | 9.3 |
| • | Northbound Approach | В | 13.5 | В | 10.1 |
| • | Southbound Approach | A | 9.0 | В | 10.4 |
| Washi | ngton Street with Rogers Street ¹ | | | | |
| • | Overall | A | 9.7 | В | 10.1 |
| • | Eastbound Approach | A | 8.8 | A | 9.2 |
| • | Westbound Approach | A | 9.5 | A | 9.7 |
| • | Northbound Approach | В | 10.4 | В | 10.4 |
| • | Southbound Approach | A | 9.3 | В | 10.4 |
| Curtis | s Street with Belden Avenue ² | | | | |
| • | Northbound Approach | A | 9.8 | A | 9.4 |
| • | Westbound Left Turn | | | A | 7.5 |
| Curtis | s Street with Commuter Parking L | ot Access D | rive ² | | |
| • | Southbound Approach | A | 9.1 | A | 8.9 |
| • | Eastbound Left Turn | A | 7.3 | A | 7.3 |
| Delay is 1 – All-v | Level of Service measured in seconds. way stop sign control way stop sign control | | | | |



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Table 3 – Cont. CAPACITY ANALYSIS RESULTS – EXISTING CONDITIONS

| | Intersection | | Morning Hour | | y Evening x Hour |
|-------|--|--------------------------|-------------------------|--------------|---------------------|
| | | LOS | Delay | LOS | Delay |
| Curti | ss Street with Village Hall Access D | Prive ² | | | |
| • | Southbound Approach | A | 9.8 | A | 9.5 |
| • | Eastbound Left Turn | A | 7.6 | A | 7.3 |
| Curti | ss Street with Police Department A | ccess Drive ² | | | |
| • | Southbound Approach | A | 9.7 | A | 8.8 |
| • | Eastbound Left Turn | A | 7.5 | A | 7.3 |
| Wash | ington Street with Apartment Acce | ess Drive ² | | | |
| • | Northbound Approach | В | 11.0 | В | 11.2 |
| • | Westbound Left Turn | A | 7.8 | A | 7.8 |
| Wash | ington Street with Burlington Stree | et (West Inte | ersection) ² | | |
| • | Northbound Left Turn | A | 3.8 | A | 3.6 |
| Curti | ss Street with Mackie Place ² | | | | |
| • | Overall | A | 7.7 | A | 7.0 |
| • | Eastbound Approach | A | 7.3 | A | 6.8 |
| • | Westbound Approach | A | 7.7 | | |
| • | Northbound Approach | A | 7.8 | A | 7.5 |
| • | Southbound Approach | A | 7.5 | A | 6.9 |
| Wash | ington Street with Burlington Aver | nue/Village H | Iall Access Dr | ive (East In | ntersection) |
| • | Eastbound Approach | С | 16.1 | С | 17.7 |
| • | Westbound Approach | A | 8.9 | A | 9.0 |
| • | Southbound Left Turn | A | 2.0 | A | 1.8 |
| Wash | ington Street with Warren Avenue | 3 | | | |
| • | ICU Level of Service | A | 44.7% | A | 49.4% |

LOS = Level of Service

Delay is measured in seconds.

^{3 –} The operation of this intersection is based on a critical volume to saturation flow (v/s) evaluation also known as the Intersection Capacity Utilization (ICU) method.



^{1 –} All-way stop sign control

^{2 –} Two way stop sign control

Table 4
CAPACITY ANALYSIS RESULTS – PROJECTED CONDITIONS

| | Intersection | | Morning Hour | | y Evening Hour |
|---------|--|-----------------------|-----------------|-----|-------------------|
| | | LOS | Delay | LOS | Delay |
| Wash | ington Street with Maple Avenue ¹ | | | | |
| • | Overall | C | 23.6 | C | 20.5 |
| • | Eastbound Approach | D | 29.8 | C | 17.1 |
| • | Westbound Approach | В | 13.4 | D | 28.1 |
| • | Northbound Approach | C | 21.6 | В | 14.4 |
| • | Southbound Approach | В | 12.3 | C | 18.2 |
| Wash | ington Street with Curtiss Street ¹ | | | | |
| • | Overall | В | 14.3 | В | 12.0 |
| • | Eastbound Approach | В | 10.7 | В | 11.7 |
| • | Westbound Approach | A | 10.1 | В | 10.7 |
| • | Northbound Approach | C | 18.0 | В | 11.4 |
| • | Southbound Approach | В | 11.0 | В | 13.3 |
| Wash | ington Street with Rogers Street ¹ | | | | |
| • | Overall | A | 9.9 | В | 10.3 |
| • | Eastbound Approach | A | 8.9 | A | 9.3 |
| • | Westbound Approach | A | 9.6 | A | 9.9 |
| • | Northbound Approach | В | 10.5 | В | 10.7 |
| • | Southbound Approach | A | 9.5 | В | 10.7 |
| Curti | ss Street with Belden Avenue/Acces | ss Drive ² | | | |
| • | Northbound Approach | В | 13.1 | В | 10.3 |
| • | Southbound Approach | A | 9.5 | A | 9.4 |
| • | Eastbound Left Turn | A | 7.6 | A | 7.3 |
| • | Westbound Left Turn | | | A | 7.5 |
| Delay i | Level of Service is measured in secondsway stop sign control o way stop sign control | | | | |



Table 4 – Cont. CAPACITY ANALYSIS RESULTS – PROJECTED CONDITIONS

| | Intersection | | Morning Hour | | y Evening Hour |
|-------|--|------------------|-------------------------|-----|-------------------|
| | | LOS | Delay | LOS | Delay |
| Curti | ss Street with Westerly Access Driv | Ve^2 | | | |
| • | Southbound Approach | В | 10.4 | A | 9.5 |
| • | Eastbound Left Turn | A | 7.7 | A | 7.5 |
| Curti | ss Street with Easterly Access Driv | e^2 | | | |
| • | Southbound Approach | A | 9.2 | A | 8.6 |
| • | Eastbound Left Turn | A | 7.4 | A | 7.3 |
| Wash | ington Street with Apartment Acco | ess ² | | | |
| • | Northbound Approach | В | 12.3 | В | 12.5 |
| • | Westbound Left Turn | A | 8.0 | A | 8.0 |
| Wash | ington Street with Burlington Stre | et (West Inte | ersection) ² | | |
| • | Northbound Left Turn | A | 8.0 | A | 8.6 |
| Curti | ss Street with Mackie Place ² | | | | |
| • | Overall | A | 7.7 | A | 7.0 |
| • | Eastbound Approach | A | 7.4 | A | 6.9 |
| • | Westbound Approach | A | 7.7 | | |
| • | Northbound Approach | A | 7.9 | A | 7.5 |
| • | Southbound Approach | A | 7.5 | A | 7.0 |
| Wash | ington Street with Burlington Aver | nue (East Int | ersection) | | |
| • | Eastbound Approach | В | 10.4 | A | 9.9 |
| Wash | ington Street with Warren Avenue | 3 | | | |
| • | ICU Level of Service | A | 45.8% | A | 50.0% |

LOS = Level of Service

Delay is measured in seconds.



^{1 –} All-way stop sign control

^{2 –} Two way stop sign control

^{3 –} The operation of this intersection is based on a critical volume to saturation flow (v/s) evaluation also known as the Intersection Capacity Utilization (ICU) method.

Discussion and Recommendations

The following is an evaluation of the analyzed intersections based on the projected traffic volumes and the capacity analyses performed.

Washington Street with Maple Avenue

The results of the capacity analysis indicate that overall, this intersection currently operates at Level of Service (LOS) C during the weekday morning and weekday evening peak hours. Furthermore, all of the approaches currently operate at LOS D or better during the peak hours. Under future conditions, this intersection overall is projected to continue operating at LOS C during the peak hours with increase in delay of approximately two seconds or less. Furthermore, all of the approaches are projected to continue operating at LOS D or better with increases in delay of approximately four seconds or less. As such, this intersection has sufficient reserve capacity to accommodate the additional traffic estimated to be generated by the proposed redevelopment and no roadway or traffic control improvements will be required.

Washington Street with Curtiss Street

The results of the capacity analysis indicate that overall, this intersection currently operates at LOS B during the weekday morning and weekday evening peak hours. Furthermore, all of the approaches currently operate at LOS B or better during the peak hours. Under future conditions, this intersection is projected to continue operating at LOS B during the peak hours with increases in delay of approximately three seconds or less. All of the approaches are projected to operate at the acceptable LOS C or better during the peak hours with increases in delay of approximately five seconds or less. As such, this intersection has sufficient reserve capacity to accommodate the additional traffic estimated to be generated by the redevelopment and the redistribution of traffic due to the elimination of the existing access drive opposite Burlington Avenue and no roadway or traffic control improvements will be required.

Washington Street with Burlington Avenue (West Intersection)

The results of the capacity analysis indicate that northbound left-turn movements from Washington Street onto Burlington Avenue currently operate at LOS A during the weekday morning and weekday evening peak hours. As part of the redevelopment, Washington Street is proposed to be restriped to provide an exclusive northbound left-turn serving Burlington Avenue. The proposed geometric improvements will (1) ensure Washington Street northbound left-turn movements do not block northbound through movements and (2) potentially reduce the Washington Street northbound queue due to the at-grade railroad crossing by providing a dedicated lane for the Washington Street northbound left-turn traffic to by-pass the northbound queue. Under future conditions, assuming the northbound left-turn lane, northbound left-turn movements are projected to continue operating at LOS A during the peak hours. As such, this intersection will have sufficient reserve capacity to accommodate the additional traffic to be generated by the redevelopment and the redistribution of traffic due to the elimination of the existing access drive opposite Burlington Avenue.



Washington Street with Burlington Avenue and Access Drive (East Intersection)

The results of the capacity analysis indicate that the eastbound approach of Burlington Avenue currently operate at LOS C during the weekday morning and weekday evening peak hours. Under future conditions, with the proposed redevelopment and elimination of access drive aligned opposite Burlington Avenue, the eastbound approach of Burlington Avenue is projected to operate at LOS B during the weekday morning peak hour and at LOS A during the weekday evening peak hour.

It should be noted that the proposed elimination of the existing access drive subject will eliminate the existing southbound left-turn movements and the outbound movements from the access drive. By removing these movements, southbound vehicles will be able to operate under free flow conditions and the northbound movements will not have to defer to the southbound left-turn movement or contend with the access drive movements which will only improve the flow of traffic along Washington Street and allow queues following train events to dissipate quicker. Additionally, maintaining the stop sign control for the eastbound Burlington Avenue approach will meter northbound traffic, permitting turning movements at the intersection of Washington Street with Warren Avenue to occur more frequently and provide additional gaps in the traffic stream for pedestrians to traverse Washington Street. Further, the elimination of the access drive will result in one less intersection leg that pedestrians will have to traverse and four less vehicle movements pedestrians will have to contend with. Overall, this intersection has sufficient reserve capacity to accommodate the additional traffic to be generated by the redevelopment and the redistribution of traffic due to the proposed elimination of Washington Street access.

Washington Street with Warren Avenue

Because of the traffic control configuration of this intersection where the northbound traffic is free flow and the other three approaches are under stop sign control, the intersection could not be analyzed using HCM procedures. This intersection's traffic control is designed to allow northbound movements to operate under free flow conditions in order to keep southbound queues from extending onto the at-grade railroad crossing. Given this traffic control configuration and the limitations of the HCM procedures, the intersection was analyzed using the Intersection Capacity Utilization (ICU) level of service. The ICU indicates how much reserve capacity is available or how much an intersection is over capacity. Based on the ICU analysis, the intersection currently utilizes approximately 45 to 50 percent of its capacity during the weekday morning and weekday evening peak hours. Under projected conditions, it is projected that the intersection will continue to utilize approximately 45 to 50 percent of its capacity during the weekday morning and weekday evening peak hours. As a result, this intersection has sufficient reserve capacity to accommodate the additional traffic estimated to be generated by the proposed redevelopment and no roadway or traffic control improvements will be required.



Washington Street with Rogers Street

The results of the capacity analysis indicate that overall, this intersection currently operates at LOS A during the weekday morning peak hour and at LOS B during the weekday evening peak hour. Furthermore, all of the approaches currently operate at LOS B or better during the peak hours. Under future conditions, this intersection overall and all of the approaches are projected to continue operating at existing levels of service during the peak hours with increase in delay of less than one second. As such, this intersection has sufficient reserve capacity to accommodate the additional traffic estimated to be generated by the proposed redevelopment and no roadway or traffic control improvements will be required.

Curtiss Street with Belden Avenue/Proposed Access Drive

The results of the capacity analysis indicate that the northbound approach currently operates at LOS A during the weekday morning and weekday evening peak hours. Under future conditions, the northbound approach is projected to operate at an acceptable LOS B during the peak hours with increases in delay of approximately four seconds or less. Westbound left-turn movements are projected to continue operating at LOS A during the peak hours.

Outbound movements from the proposed access drive onto Curtiss Street and eastbound left-turn movements from Curtiss Street onto the proposed access drive are projected to operate at LOS A during the peak hours. As such, this access drive will be adequate in accommodating the traffic estimated to be generated by the proposed redevelopment, will ensure efficient and flexible access is provided, and will have a limited impact on the operations of Curtiss Street and Belden Avenue.

Curtiss Street with Mackie Place/Access Drives

The results of the capacity analysis indicate that this intersection overall and all of the approaches currently operate at LOS A during the weekday morning and weekday evening peak hours. Under future conditions, this intersection overall and all of the approaches are projected to continue operating at LOS A during the peak hours with increases in delay of less than one second. As such, this intersection has sufficient reserve capacity to accommodate the traffic estimated to be generated by the proposed redevelopment and no roadway or traffic control improvements will be required.

Curtiss Street with Access Drives

The results of the capacity analysis indicate that outbound movements from the westerly and easterly access drives onto Curtiss Street are projected to operate at LOS B or better during the weekday morning and weekday evening peak hours. Furthermore, eastbound left-turn movements from Curtiss Street onto the access drives are projected to operate at LOS A during the peak hours. As such, these access drives will be adequate in accommodating the traffic estimated to be generated by the proposed redevelopment and will ensure efficient and flexible access is provided.



6. Conclusion

Based on existing conditions and the traffic capacity analyses for the full buildout of the redevelopment, the findings and recommendations of this study are outlined below:

- As part of the proposed redevelopment, the existing access drive to the subject site aligned opposite Burlington Avenue at its intersection with Washington Street will be eliminated. Similar to the current operations, the Washington Street northbound traffic is proposed to continue to be under stop sign control south of the railroad crossing. The elimination of the access drive will provide the following benefits:
 - o It will enhance the flow of traffic along northbound Washington Street and dissipate the northbound queue of traffic quicker after a train event as the northbound traffic will no longer have to defer to the Washington Street southbound left-turn movement or contend with outbound movements from the access drive.
 - O It will provide more efficient and orderly pedestrian circulation as the pedestrians will have one less intersection leg to traverse and four less vehicle movements to contend with. Further, several pedestrian enhancements are proposed as part of the redevelopment, including a direct pedestrian connection between Curtiss Street and Burlington Avenue/at-grade crossing.
 - O The elimination of the southbound left-turn movement from Washington Street to the access drive will eliminate potential back-ups that may occur on the railroad tracks and will dissipate the southbound queue of traffic quicker after a train event.
 - O The provision of stopping northbound traffic south of the tracks will meter northbound traffic which will (1) permit turning movements at the intersection of Washington Street with Warren Avenue to occur more frequently and (2) provide additional gaps in the traffic stream for pedestrians to traverse Washington Street.
- In addition, Washington Street is proposed to be restriped at its intersection with Burlington Avenue to provide a northbound left-turn lane. The northbound left-turn lane will (1) ensure Washington Street northbound left-turn movements do not block northbound through movements and (2) potentially reduce the Washington Street northbound queue due to the at-grade railroad crossing by providing a dedicated lane for the Washington Street northbound left-turn traffic to by-pass the northbound queue.
- The results of the capacity analyses indicate that the existing roadway system assuming the proposed modifications will have sufficient reserve capacity to accommodate the additional traffic that will be generated by the proposed redevelopment and the redistribution of traffic due to the elimination of the existing access drive opposite Burlington Avenue.



• The proposed access drives on Curtiss Street serving the redevelopment will be adequate to accommodate the traffic estimated to be generated by the proposed redevelopment, will ensure efficient and flexible access, and will have a limited impact on Curtiss Street, Belden Avenue, and Mackie Place.

- The elimination of the existing access drive opposite Burlington Avenue will result in the redistribution of the Village Hall/Police Station/Commuter Lot traffic as well as the area traffic traveling through the subject site to the area roadways, particularly Washington Street and Curtiss Street. However, the additional travel is limited and, as the traffic study has shown, the area roadways and intersections have sufficient reserve capacity to accommodate the additional traffic generated by the redevelopment and the redistribution of the existing traffic.
- The elimination of the existing access drive opposite Burlington Avenue will increase the queue of traffic along northbound Washington Street associated with the at-grade railroad crossing. However, the queue of traffic is anticipated to dissipate quicker compared to existing conditions given the enhancement to the flow of the Washington Street northbound traffic.



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Appendix

Traffic Count Summary Sheets
Site Plan
Washington Street Geometric Improvements
Level of Service Criteria
Capacity Analysis Summary Sheets

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Traffic Count Summary Sheets

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Maple Avenue with Washington Street Site Code: Start Date: 02/14/2019 Page No: 1

| | | | Maple A | Avenue | | | | | | Avenue bound | 9 | | | | | ton Street | | | | | | ton Street | | | |
|-------------------------|--------|------|---------|--------|------|---------------|--------|-------|------|-----------------|------|---------------|--------|------|------|------------|------|---------------|--------|------|------|------------|------|---------------|------------|
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 7:00 AM | 0 | 70 | 76 | 1 | 2 | 147 | 0 | 0 | 23 | 14 | 1 | 37 | 0 | 0 | 38 | 2 | 0 | 40 | 0 | 7 | 9 | 12 | 2 | 28 | 252 |
| 7:15 AM | 0 | 81 | 94 | 0 | 8 | 175 | 0 | 3 | 27 | 11 | 3 | 41 | 0 | 3 | 82 | 10 | 0 | 95 | 0 | 12 | 8 | 17 | 0 | 37 | 348 |
| 7:30 AM | 0 | 74 | 103 | 1 | 5 | 178 | 0 | 3 | 41 | 14 | 0 | 58 | 0 | 2 | 48 | 9 | 0 | 59 | 0 | 9 | 12 | 21 | 0 | 42 | 337 |
| 7:45 AM | 0 | 55 | 101 | 1 | 5 | 157 | 0 | 3 | 28 | 17 | 2 | 48 | 0 | 2 | 57 | . 7 | 0 | 66 | 0 | 7 | . 8 | 13 | 0 | 28 | 299 |
| Hourly Total | 0 | 280 | 374 | 3 | 20 | 657 | 0 | 9 | 119 | 56 | 6 | 184 | 0 | 7 | 225 | 28 | 0 | 260 | 0 | 35 | 37 | 63 | 2 | 135 | 1236 |
| 8:00 AM | 0 | 53 | 88 | 6 | 8 | 147 | 0 | 4 | 36 | 7 | 1 | 47 | 0 | 1 | 37 | 7 | 0 | 45 | 0 | 10 | 13 | 14 | 0 | 37 | 276 |
| 8:15 AM | 0 | 52 | 98 | 3 | 1 | 153 | 0 | 2 | 46 | 11 | 0 | 59 | 0 | 6 | 31 | 11 | 0 | 48 | 0 | 9 | 8 | 18 | 0 | 35 | 295 |
| 8:30 AM | 0 | 42 | 88 | 2 | 1 | 132 | 0 | 2 | 32 | 3 | 1 | 37 | 0 | 2 | 17 | 6 | 1 | 25 | 0 | 4 | 5 | 18 | 0 | 27 | 221 |
| 8:45 AM | 0 | 17 | 68 | 0 | 2 | 85 | 0 | 2 | 32 | 12 | 0 | 46 | 0 | 1 | 17 | 6 | 0 | 24 | 0 | 5 | 10 | 29 | 0 | 44 | 199 |
| Hourly Total | 0 | 164 | 342 | 11 | 12 | 517 | 0 | 10 | 146 | 33 | 2 | 189 | 0 | 10 | 102 | 30 | 1 | 142 | 0 | 28 | 36 | 79 | 0 | 143 | 991 |
| *** BREAK *** | - | - | _ | - | - | - | - | - | - | _ | - | _ | - | - | _ | | - | _ | - | - | _ | _ | - | - | - |
| 4:30 PM | 0 | 27 | 49 | 2 | 1 | 78 | 0 | 4 | 74 | 13 | 0 | 91 | 0 | 1 | 24 | 2 | 0 | 27 | 0 | 18 | 29 | 52 | 0 | 99 | 295 |
| 4:45 PM | 0 | 34 | 48 | 2 | 1 | 84 | 0 | 4 | 63 | 10 | 0 | 77 | 0 | 1 | 10 | 4 | 0 | 15 | 0 | 17 | 37 | 49 | . 1 | 103 | 279 |
| Hourly Total | 0 | 61 | 97 | 4 | 2 | 162 | 0 | 8 | 137 | 23 | 0 | 168 | 0 | 2 | 34 | 6 | 0 | 42 | 0 | 35 | 66 | 101 | 1 | 202 | 574 |
| 5:00 PM | 0 | 36 | 52 | 2 | 3 | 90 | 0 | 1 | 100 | 18 | 0 | 119 | 0 | 1 | 19 | 3 | 0 | 23 | 0 | 21 | 34 | 66 | 1 | 121 | 353 |
| 5:15 PM | 0 | 27 | 60 | 6 | 0 | 93 | 0 | 2 | 93 | 12 | 0 | 107 | 0 | 1 | 9 | 5 | 0 | 15 | 0 | 25 | 35 | 83 | 0 | 143 | 358 |
| 5:30 PM | 0 | 34 | 57 | 1 | 4 | 92 | 0 | 3 | 83 | 15 | 1 | 101 | 0 | 1 | 19 | 4 | 2 | 24 | 0 | 14 | 43 | 83 | 0 | 140 | 357 |
| 5:45 PM | 0 | 33 | 63 | 0 | 1 | 96 | 0 | 2 | 76 | 10 | 0 | 88 | 0 | 3 | 22 | 5 | 2 | 30 | 0 | 12 | 20 | 45 | 0 | 77 | 291 |
| Hourly Total | 0 | 130 | 232 | 9 | 8 | 371 | 0 | 8 | 352 | 55 | 1 | 415 | 0 | 6 | 69 | 17 | 4 | 92 | 0 | 72 | 132 | 277 | . 1 | 481 | 1359 |
| 6:00 PM | 0 | 25 | 51 | 3 | 2 | 79 | 0 | 4 | 75 | 7 | 4 | 86 | 0 | 1 | 19 | 1 | 0 | 21 | 0 | 31 | 52 | 89 | 1 | 172 | 358 |
| 6:15 PM | 0 | 28 | 51 | 6 | 1 | 85 | 0 | 3 | 87 | 17 | 1 | 107 | 0 | 2 | 21 | 6 | 0 | 29 | 0 | 25 | 31 | 58 | 0 | 114 | 335 |
| Grand Total | 0 | 688 | 1147 | 36 | 45 | 1871 | 0 | 42 | 916 | 191 | 14 | 1149 | 0 | 28 | 470 | 88 | 5 | 586 | 0 | 226 | 354 | 667 | 5 | 1247 | 4853 |
| Approach % | 0.0 | 36.8 | 61.3 | 1.9 | _ | - | 0.0 | 3.7 | 79.7 | 16.6 | - | _ | 0.0 | 4.8 | 80.2 | 15.0 | - | _ | 0.0 | 18.1 | 28.4 | 53.5 | - | _ | - |
| Total % | 0.0 | 14.2 | 23.6 | 0.7 | - | 38.6 | 0.0 | 0.9 | 18.9 | 3.9 | - | 23.7 | 0.0 | 0.6 | 9.7 | 1.8 | - | 12.1 | 0.0 | 4.7 | 7.3 | 13.7 | - | 25.7 | - |
| Lights | 0 | 667 | 1136 | 35 | - | 1838 | 0 | 42 | 904 | 190 | - | 1136 | 0 | 27 | 462 | 88 | - | 577 | 0 | 224 | 349 | 663 | - | 1236 | 4787 |
| % Lights | - | 96.9 | 99.0 | 97.2 | - | 98.2 | - | 100.0 | 98.7 | 99.5 | - | 98.9 | - | 96.4 | 98.3 | 100.0 | - | 98.5 | - | 99.1 | 98.6 | 99.4 | - | 99.1 | 98.6 |
| Buses | 0 | 17 | 2 | 1 | - | 20 | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 5 | 0 | - | 5 | 0 | 0 | 2 | 0 | - | 2 | 28 |
| % Buses | - | 2.5 | 0.2 | 2.8 | | 1.1 | - | 0.0 | 0.1 | 0.0 | - | 0.1 | - | 0.0 | 1.1 | 0.0 | - | 0.9 | - | 0.0 | 0.6 | 0.0 | - | 0.2 | 0.6 |
| Single-Unit Trucks | 0 | 2 | 9 | 0 | _ | 11 | 0 | 0 | 10 | 1 | - | 11 | 0 | 1 | 2 | 0 | - | 3 | 0 | 2 | 2 | 3 | - | 7 | 32 |
| % Single-Unit Trucks | - | 0.3 | 0.8 | 0.0 | - | 0.6 | - | 0.0 | 1.1 | 0.5 | - | 1.0 | - | 3.6 | 0.4 | 0.0 | - | 0.5 | - | 0.9 | 0.6 | 0.4 | - | 0.6 | 0.7 |
| Articulated Trucks | 0 | 2 | 0 | 0 | - | 2 | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 1 | - | 1 | 4 |
| % Articulated Trucks | - | 0.3 | 0.0 | 0.0 | - | 0.1 | - | 0.0 | 0.1 | 0.0 | - | 0.1 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.1 | - | 0.1 | 0.1 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 1 | 0 | _ | 1 | 2 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.2 | 0.0 | - | 0.2 | - | 0.0 | 0.3 | 0.0 | - | 0.1 | 0.0 |
| Pedestrians | - | - | - | - | 45 | - | - | - | - | - | 14 | - | - | - | - | - | 5 | - | - | - | - | - | 5 | - | - |

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Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Maple Avenue with Washington Street Site Code: Start Date: 02/14/2019 Page No: 3

| | | | Maple . Eastl | Avenue | | | | | • | Avenue bound | | | | | Washing | ton Street bound | | | | | Washing South | ton Street bound | | | |
|-------------------------|--------|-------|------------------|--------|-------|---------------|--------|-------|-------|-----------------|-------|---------------|--------|-------|---------|---------------------|------|---------------|--------|-------|------------------|---------------------|------|---------------|------------|
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 7:15 AM | 0 | 81 | 94 | 0 | 8 | 175 | 0 | 3 | 27 | 11 | 3 | 41 | 0 | 3 | 82 | 10 | 0 | 95 | 0 | 12 | 8 | 17 | 0 | 37 | 348 |
| 7:30 AM | 0 | 74 | 103 | 1 | 5 | 178 | 0 | 3 | 41 | 14 | 0 | 58 | 0 | 2 | 48 | 9 | 0 | 59 | 0 | 9 | 12 | 21 | 0 | 42 | 337 |
| 7:45 AM | 0 | 55 | 101 | 1 | 5 | 157 | 0 | 3 | 28 | 17 | 2 | 48 | 0 | 2 | 57 | 7 | 0 | 66 | 0 | 7 | 8 | 13 | 0 | 28 | 299 |
| 8:00 AM | 0 | 53 | 88 | 6 | 8 | 147 | 0 | 4 | 36 | 7 | 1 | 47 | 0 | 1 | 37 | . 7 | 0 | 45 | 0 | 10 | 13 | 14 | 0 | 37 | 276 |
| Total | 0 | 263 | 386 | 8 | 26 | 657 | 0 | 13 | 132 | 49 | 6 | 194 | 0 | 8 | 224 | 33 | 0 | 265 | 0 | 38 | 41 | 65 | 0 | 144 | 1260 |
| Approach % | 0.0 | 40.0 | 58.8 | 1.2 | - | - | 0.0 | 6.7 | 68.0 | 25.3 | - | - | 0.0 | 3.0 | 84.5 | 12.5 | - | - | 0.0 | 26.4 | 28.5 | 45.1 | - | - | - |
| Total % | 0.0 | 20.9 | 30.6 | 0.6 | - | 52.1 | 0.0 | 1.0 | 10.5 | 3.9 | - | 15.4 | 0.0 | 0.6 | 17.8 | 2.6 | - | 21.0 | 0.0 | 3.0 | 3.3 | 5.2 | - | 11.4 | - |
| PHF | 0.000 | 0.812 | 0.937 | 0.333 | - | 0.923 | 0.000 | 0.813 | 0.805 | 0.721 | - | 0.836 | 0.000 | 0.667 | 0.683 | 0.825 | - | 0.697 | 0.000 | 0.792 | 0.788 | 0.774 | - | 0.857 | 0.905 |
| Lights | 0 | 258 | 381 | 8 | - | 647 | 0 | 13 | 131 | 48 | - | 192 | 0 | 8 | 219 | 33 | - | 260 | 0 | 38 | 40 | 65 | - | 143 | 1242 |
| % Lights | - | 98.1 | 98.7 | 100.0 | - | 98.5 | - | 100.0 | 99.2 | 98.0 | - | 99.0 | - | 100.0 | 97.8 | 100.0 | - | 98.1 | - | 100.0 | 97.6 | 100.0 | - | 99.3 | 98.6 |
| Buses | 0 | 4 | 1 | 0 | - | 5 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 4 | 0 | _ | 4 | 0 | 0 | 1 | 0 | - | 1 | 10 |
| % Buses | - | 1.5 | 0.3 | 0.0 | - | 8.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 1.8 | 0.0 | - | 1.5 | - | 0.0 | 2.4 | 0.0 | - | 0.7 | 0.8 |
| Single-Unit Trucks | 0 | 1 | 4 | 0 | - | 5 | 0 | 0 | 0 | 1 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 6 |
| % Single-Unit Trucks | - | 0.4 | 1.0 | 0.0 | - | 0.8 | - | 0.0 | 0.0 | 2.0 | - | 0.5 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.5 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 1 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.8 | 0.0 | - | 0.5 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.1 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 1 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.4 | 0.0 | - | 0.4 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.1 |
| Pedestrians | - | - | - | - | 26 | _ | - | - | - | - | 6 | - | - | - | - | - | 0 | _ | - | - | - | - | 0 | - | - |
| % Pedestrians | - | - | | _ | 100.0 | _ | - | - | - | | 100.0 | - | - | - | - | - | - | _ | - | - | _ | | - | _ | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

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Count Name: Maple Avenue with Washington Street Site Code: Start Date: 02/14/2019 Page No: 4

| | | | | | | | | | | | | | 1001 | | (0.00 | , | | | | | | | | | i |
|-------------------------|--------|-------|-------|--------|-------|---------------|--------|-------|-------|--------|-------|---------------|--------|-------|---------|------------|-------|---------------|--------|-------|----------|-----------|-------|---------------|------------|
| | | | Maple | Avenue | | | | | Maple | Avenue | | | | | Washing | ton Street | | | | | Washingt | on Street | | | |
| | | | East | oound | | | | | West | bound | | | | | North | bound | | | | | South | bound | | | |
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 5:30 PM | 0 | 34 | 57 | 1 | 4 | 92 | 0 | 3 | 83 | 15 | 1 | 101 | 0 | 1 | 19 | 4 | 2 | 24 | 0 | 14 | 43 | 83 | 0 | 140 | 357 |
| 5:45 PM | 0 | 33 | 63 | 0 | 1 | 96 | 0 | 2 | 76 | 10 | 0 | 88 | 0 | 3 | 22 | 5 | 2 | 30 | 0 | 12 | 20 | 45 | 0 | 77 | 291 |
| 6:00 PM | 0 | 25 | 51 | 3 | 2 | 79 | 0 | 4 | 75 | 7 | 4 | 86 | 0 | 1 | 19 | 1 | 0 | 21 | 0 | 31 | 52 | 89 | 1 | 172 | 358 |
| 6:15 PM | 0 | 28 | 51 | 6 | 1 | 85 | 0 | 3 | 87 | 17 | . 1 | 107 | 0 | 2 | 21 | 6 | 0 | 29 | 0 | 25 | 31 | 58 | 0 | 114 | 335 |
| Total | 0 | 120 | 222 | 10 | 8 | 352 | 0 | 12 | 321 | 49 | 6 | 382 | 0 | 7 | 81 | 16 | 4 | 104 | 0 | 82 | 146 | 275 | 1 | 503 | 1341 |
| Approach % | 0.0 | 34.1 | 63.1 | 2.8 | - | - | 0.0 | 3.1 | 84.0 | 12.8 | - | - | 0.0 | 6.7 | 77.9 | 15.4 | - | - | 0.0 | 16.3 | 29.0 | 54.7 | - | - | - |
| Total % | 0.0 | 8.9 | 16.6 | 0.7 | _ | 26.2 | 0.0 | 0.9 | 23.9 | 3.7 | - | 28.5 | 0.0 | 0.5 | 6.0 | 1.2 | _ | 7.8 | 0.0 | 6.1 | 10.9 | 20.5 | - | 37.5 | - |
| PHF | 0.000 | 0.882 | 0.881 | 0.417 | - | 0.917 | 0.000 | 0.750 | 0.922 | 0.721 | - | 0.893 | 0.000 | 0.583 | 0.920 | 0.667 | - | 0.867 | 0.000 | 0.661 | 0.702 | 0.772 | - | 0.731 | 0.936 |
| Lights | 0 | 114 | 221 | 10 | - | 345 | 0 | 12 | 319 | 49 | - | 380 | 0 | 7 | 80 | 16 | - | 103 | 0 | 82 | 144 | 274 | - | 500 | 1328 |
| % Lights | - | 95.0 | 99.5 | 100.0 | _ | 98.0 | - | 100.0 | 99.4 | 100.0 | - | 99.5 | - | 100.0 | 98.8 | 100.0 | - | 99.0 | - | 100.0 | 98.6 | 99.6 | - | 99.4 | 99.0 |
| Buses | 0 | 4 | 0 | 0 | - | 4 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 4 |
| % Buses | - | 3.3 | 0.0 | 0.0 | - | 1.1 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.3 |
| Single-Unit Trucks | 0 | 0 | 1 | 0 | _ | 1 | 0 | 0 | 2 | 0 | - | 2 | 0 | 0 | 1 | 0 | _ | 1 | 0 | 0 | 2 | 0 | - | 2 | 6 |
| % Single-Unit Trucks | - | 0.0 | 0.5 | 0.0 | - | 0.3 | - | 0.0 | 0.6 | 0.0 | - | 0.5 | - | 0.0 | 1.2 | 0.0 | - | 1.0 | - | 0.0 | 1.4 | 0.0 | - | 0.4 | 0.4 |
| Articulated Trucks | 0 | 2 | 0 | 0 | - | 2 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 1 | - | 1 | 3 |
| % Articulated Trucks | - | 1.7 | 0.0 | 0.0 | - | 0.6 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.4 | - | 0.2 | 0.2 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Pedestrians | - | _ | - | - | 8 | - | - | - | - | _ | 6 | - | - | - | - | - | 4 | _ | - | _ | - | - | 1 | - | - |
| % Pedestrians | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | - | - |
| | | | | | | | | | | | | | | | | | | | | | | | | | |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Curtiss Street Site Code: Start Date: 02/13/2019 Page No: 1

| | | | Curtiss East | Street oound | | | | | | s Street tbound | 9 | | | | - | ton Street bound | | | | | | ton Street bound | | | |
|-------------------------|--------|------|-----------------|-----------------|------|---------------|--------|------|------|--------------------|------|---------------|--------|------|------|---------------------|------|---------------|--------|-------|------|---------------------|------|---------------|------------|
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 7:00 AM | 0 | 7 | 3 | 8 | 7 | 18 | 0 | 1 | 0 | . 4 | 6 | 5 | 0 | 0 | 66 | 5 | 4 | 71 | 0 | 0 | 16 | 0 | 5 | 16 | 110 |
| 7:15 AM | 0 | 9 | 5 | 10 | 6 | 24 | 0 | 2 | 0 | 0 | 3 | 2 | 0 | 0 | 88 | 6 | 0 | 94 | 0 | 2 | 15 | 0 | 1 | 17 | 137 |
| 7:30 AM | 0 | 10 | 4 | 11 | 4 | 25 | 0 | 2 | 0 | 3 | 3 | 5 | 0 | 0 | 70 | 11 | 2 | 81 | 0 | 1 | 30 | 0 | 2 | 31 | 142 |
| 7:45 AM | 0 | 7 | 11 | 15 | 6 | 33 | 0 | 7 | 0 | 2 | 5 | 9 | 0 | 0 | 95 | 27 | 3 | 122 | 0 | 0 | 30 | 0 | 4 | 30 | 194 |
| Hourly Total | 0 | 33 | 23 | 44 | 23 | 100 | 0 | 12 | 0 | 9 | 17 | 21 | 0 | 0 | 319 | 49 | 9 | 368 | 0 | 3 | 91 | 0 | 12 | 94 | 583 |
| 8:00 AM | 0 | 13 | 3 | 16 | 0 | 32 | 0 | 1 | 0 | 2 | 2 | 3 | 0 | 0 | 53 | 9 | 0 | 62 | 1 | 0 | 23 | 0 | 0 | 24 | 121 |
| 8:15 AM | 0 | 11 | . 7 | 9 | 2 | 27 | 0 | 4 | 0 | 4 | . 4 | . 8 | 0 | 0 | 78 | . 7 | 1 | 85 | 0 | 1 | 22 | 0 | 0 | 23 | 143 |
| 8:30 AM | 0 | 14 | 2 | 20 | . 1 | 36 | 0 | 3 | 0 | 2 | 0 | 5 | 0 | 0 | 45 | 2 | 0 | 47 | 0 | 5 | 30 | 0 | 2 | 35 | 123 |
| 8:45 AM | 0 | 15 | 4 | 12 | 0 | 31 | 0 | 2 | 0 | 1 | 2 | 3 | 0 | 0 | 50 | 8 | 1 | 58 | 0 | 1 | 21 | 0 | 5 | 22 | 114 |
| Hourly Total | 0 | 53 | 16 | 57 | 3 | 126 | 0 | 10 | 0 | 9 | . 8 | 19 | 0 | 0 | 226 | 26 | 2 | 252 | 1 | 7 | 96 | 0 | . 7 | 104 | 501 |
| *** BREAK *** | - | - | | - | - | - | - | - | | - | - | | - | - | - | _ | - | | - | - | - | | - | - | - |
| 4:30 PM | 0 | 17 | 1 | 34 | 3 | 52 | 0 | 9 | 0 | 1 | 2 | 10 | 0 | 0 | 24 | 2 | 3 | 26 | 0 | 3 | 62 | 0 | 4 | 65 | 153 |
| 4:45 PM | 0 | 12 | 7 | 30 | 5 | 49 | 0 | 5 | 0 | 2 | 5 | 7 | 0 | 0 | 30 | 7 | 2 | 37 | 0 | 1 | 46 | 0 | 5 | 47 | 140 |
| Hourly Total | 0 | 29 | 8 | 64 | 8 | 101 | 0 | 14 | 0 | 3 | 7 | 17 | 0 | 0 | 54 | 9 | 5 | 63 | 0 | 4 | 108 | 0 | 9 | 112 | 293 |
| 5:00 PM | 0 | 14 | 6 | 25 | 1 | 45 | 0 | 12 | 0 | 6 | 0 | 18 | 0 | 0 | 40 | 4 | 1 | 44 | 0 | 1 | 51 | 0 | 1 | 52 | 159 |
| 5:15 PM | 0 | 20 | 7 | 25 | 1 | 52 | 0 | 6 | 0 | 2 | 4 | 8 | 0 | 0 | 44 | 7 | 2 | 51 | 0 | 3 | 47 | 0 | 5 | 50 | 161 |
| 5:30 PM | 0 | 17 | 4 | 28 | 17 | 49 | 0 | 15 | 0 | 0 | 3 | 15 | 0 | 0 | 46 | 4 | 4 | 50 | 1 | 2 | 27 | 0 | 8 | 30 | 144 |
| 5:45 PM | 0 | 23 | 8 | 30 | 10 | 61 | 0 | 11 | 0 | 3 | 5 | 14 | 0 | 0 | 38 | 8 | 5 | 46 | 0 | 3 | 49 | 0 | 3 | 52 | 173 |
| Hourly Total | 0 | 74 | 25 | 108 | 29 | 207 | 0 | 44 | 0 | 11 | 12 | 55 | 0 | 0 | 168 | 23 | 12 | 191 | 1 | 9 | 174 | 0 | 17 | 184 | 637 |
| 6:00 PM | 0 | 18 | 4 | 20 | 9 | 42 | 0 | 15 | 0 | 2 | 4 | 17 | 0 | 0 | 33 | 3 | 3 | 36 | 0 | 2 | 38 | 0 | 1 | 40 | 135 |
| 6:15 PM | 0 | 28 | 4 | 17 | 13 | 49 | 0 | 24 | 0 | 4 | 4 | 28 | 0 | 0 | 34 | 4 | 4 | 38 | 0 | 2 | 70 | 0 | 2 | 72 | 187 |
| Grand Total | 0 | 235 | 80 | 310 | 85 | 625 | 0 | 119 | 0 | 38 | 52 | 157 | 0 | 0 | 834 | 114 | 35 | 948 | 2 | 27 | 577 | 0 | 48 | 606 | 2336 |
| Approach % | 0.0 | 37.6 | 12.8 | 49.6 | - | - | 0.0 | 75.8 | 0.0 | 24.2 | - | - | 0.0 | 0.0 | 88.0 | 12.0 | - | - | 0.3 | 4.5 | 95.2 | 0.0 | - | - | - |
| Total % | 0.0 | 10.1 | 3.4 | 13.3 | - | 26.8 | 0.0 | 5.1 | 0.0 | 1.6 | - | 6.7 | 0.0 | 0.0 | 35.7 | 4.9 | - | 40.6 | 0.1 | 1.2 | 24.7 | 0.0 | - | 25.9 | |
| Lights | 0 | 232 | 75 | 308 | - | 615 | 0 | 115 | 0 | 38 | - | 153 | 0 | 0 | 808 | 113 | - | 921 | 2 | 27 | 573 | 0 | - | 602 | 2291 |
| % Lights | - | 98.7 | 93.8 | 99.4 | - | 98.4 | - | 96.6 | | 100.0 | - | 97.5 | - | - | 96.9 | 99.1 | - | 97.2 | 100.0 | 100.0 | 99.3 | | - | 99.3 | 98.1 |
| Buses | 0 | 1 | 0 | 0 | _ | 1 | 0 | 1 | 0 | 0 | - | 1 | 0 | 0 | 22 | 0 | - | 22 | 0 | 0 | 0 | 0 | - | 0 | 24 |
| % Buses | - | 0.4 | 0.0 | 0.0 | _ | 0.2 | - | 0.8 | | 0.0 | - | 0.6 | - | - | 2.6 | 0.0 | - | 2.3 | 0.0 | 0.0 | 0.0 | - | _ | 0.0 | 1.0 |
| Single-Unit Trucks | 0 | 2 | 5 | 1 | - | 8 | 0 | 3 | 0 | 0 | - | 3 | 0 | 0 | 2 | 1 | - | 3 | 0 | 0 | 3 | 0 | - | 3 | 17 |
| % Single-Unit Trucks | - | 0.9 | 6.3 | 0.3 | - | 1.3 | - | 2.5 | - | 0.0 | - | 1.9 | - | - | 0.2 | 0.9 | - | 0.3 | 0.0 | 0.0 | 0.5 | - | - | 0.5 | 0.7 |
| Articulated Trucks | 0 | 0 | 0 | 1 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 2 | 0 | - | 2 | 0 | 0 | 0 | 0 | - | 0 | 3 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.3 | - | 0.2 | - | 0.0 | - | 0.0 | - | 0.0 | - | - | 0.2 | 0.0 | - | 0.2 | 0.0 | 0.0 | 0.0 | - | - | 0.0 | 0.1 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 1 | 0 | - | 1 | 1 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.2 | - | - | 0.2 | 0.0 |
| Pedestrians | | | - | _ | 85 | - | - | - | - | - | 52 | - | - | - | - | - | 35 | - | - | - | - | - | 48 | | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Curtiss Street Site Code: Start Date: 02/13/2019 Page No: 3

| | | | | Street bound | | | | | Curtiss | Street bound | | | | | Washing | ton Street bound | | | | | Washingt South | | | | |
|-------------------------|--------|-------|-------|-----------------|-------|---------------|--------|-------|---------|-----------------|-------|---------------|--------|-------|---------|---------------------|-------|---------------|--------|-------|-------------------|-------|-------|---------------|------------|
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 7:15 AM | 0 | 9 | 5 | 10 | 6 | 24 | 0 | 2 | 0 | 0 | 3 | 2 | 0 | 0 | 88 | 6 | 0 | 94 | 0 | 2 | 15 | 0 | 1 | 17 | 137 |
| 7:30 AM | 0 | 10 | 4 | 11 | 4 | 25 | 0 | 2 | 0 | 3 | 3 | 5 | 0 | 0 | 70 | 11 | 2 | 81 | 0 | 1 | 30 | 0 | 2 | 31 | 142 |
| 7:45 AM | 0 | 7 | 11 | 15 | 6 | 33 | 0 | 7 | 0 | 2 | 5 | 9 | 0 | 0 | 95 | 27 | 3 | 122 | 0 | 0 | 30 | 0 | 4 | 30 | 194 |
| 8:00 AM | 0 | 13 | 3 | 16 | 0 | 32 | 0 | 1 | 0 | 2 | 2 | 3 | 0 | 0 | 53 | 9 | 0 | 62 | 1 | 0 | 23 | 0 | 0 | 24 | 121 |
| Total | 0 | 39 | 23 | 52 | 16 | 114 | 0 | 12 | 0 | 7 | 13 | 19 | 0 | 0 | 306 | 53 | 5 | 359 | 1 | 3 | 98 | 0 | 7 | 102 | 594 |
| Approach % | 0.0 | 34.2 | 20.2 | 45.6 | - | - | 0.0 | 63.2 | 0.0 | 36.8 | - | - | 0.0 | 0.0 | 85.2 | 14.8 | - | - | 1.0 | 2.9 | 96.1 | 0.0 | - | - | - |
| Total % | 0.0 | 6.6 | 3.9 | 8.8 | - | 19.2 | 0.0 | 2.0 | 0.0 | 1.2 | - | 3.2 | 0.0 | 0.0 | 51.5 | 8.9 | - | 60.4 | 0.2 | 0.5 | 16.5 | 0.0 | - | 17.2 | - |
| PHF | 0.000 | 0.750 | 0.523 | 0.813 | - | 0.864 | 0.000 | 0.429 | 0.000 | 0.583 | - | 0.528 | 0.000 | 0.000 | 0.805 | 0.491 | - | 0.736 | 0.250 | 0.375 | 0.817 | 0.000 | - | 0.823 | 0.765 |
| Lights | 0 | 37 | 19 | 52 | - | 108 | 0 | 11 | 0 | 7 | - | 18 | 0 | 0 | 296 | 52 | - | 348 | 1 | 3 | 97 | 0 | - | 101 | 575 |
| % Lights | - | 94.9 | 82.6 | 100.0 | - | 94.7 | - | 91.7 | - | 100.0 | - | 94.7 | - | - | 96.7 | 98.1 | - | 96.9 | 100.0 | 100.0 | 99.0 | - | - | 99.0 | 96.8 |
| Buses | 0 | 1 | 0 | 0 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 9 | 0 | - | 9 | 0 | 0 | 0 | 0 | - | 0 | 10 |
| % Buses | - | 2.6 | 0.0 | 0.0 | - | 0.9 | - | 0.0 | - | 0.0 | - | 0.0 | - | - | 2.9 | 0.0 | - | 2.5 | 0.0 | 0.0 | 0.0 | - | - | 0.0 | 1.7 |
| Single-Unit Trucks | 0 | 1 | 4 | 0 | - | 5 | 0 | 1 | 0 | 0 | - | 1 | 0 | 0 | 1 | . 1 | - | 2 | 0 | 0 | 1 | 0 | - | . 1 | 9 |
| % Single-Unit Trucks | - | 2.6 | 17.4 | 0.0 | - | 4.4 | - | 8.3 | - | 0.0 | - | 5.3 | - | - | 0.3 | 1.9 | - | 0.6 | 0.0 | 0.0 | 1.0 | - | - | 1.0 | 1.5 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | - | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | - | - | 0.0 | 0.0 |
| Pedestrians | - | - | - | - | 16 | _ | - | - | - | - | 13 | - | - | - | - | - | 5 | _ | - | - | - | - | 7 | - | - |
| % Pedestrians | - | - | _ | _ | 100.0 | _ | - | - | - | | 100.0 | - | - | - | | _ | 100.0 | _ | - | - | _ | - | 100.0 | _ | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Curtiss Street Site Code: Start Date: 02/13/2019 Page No: 4

| | i | | | | | | | | | | | | | | (0.00 | , | | | | | | | | | i |
|-------------------------|--------|-------|---------|--------|-------|---------------|--------|-------|--------|--------|-------|---------------|--------|-------|---------|------------|-------|---------------|--------|-------|----------|-----------|-------|---------------|------------|
| | | | Curtiss | Street | | | | | Curtis | Street | | | | | Washing | ton Street | | | | | Washingt | on Street | | | |
| | | | East | oound | | | | | West | bound | | | | | North | bound | | | | | South | bound | | | |
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 5:30 PM | 0 | 17 | 4 | 28 | 17 | 49 | 0 | 15 | 0 | 0 | 3 | 15 | 0 | 0 | 46 | 4 | 4 | 50 | 1 | 2 | 27 | 0 | 8 | 30 | 144 |
| 5:45 PM | 0 | 23 | 8 | 30 | 10 | 61 | 0 | 11 | 0 | 3 | 5 | 14 | 0 | 0 | 38 | 8 | 5 | 46 | 0 | 3 | 49 | 0 | 3 | 52 | 173 |
| 6:00 PM | 0 | 18 | 4 | 20 | 9 | 42 | 0 | 15 | 0 | 2 | 4 | 17 | 0 | 0 | 33 | 3 | 3 | 36 | 0 | 2 | 38 | 0 | 1 | 40 | 135 |
| 6:15 PM | 0 | 28 | 4 | 17 | 13 | 49 | 0 | 24 | 0 | 4 | 4 | 28 | 0 | 0 | 34 | 4 | 4 | 38 | 0 | 2 | 70 | 0 | 2 | 72 | 187 |
| Total | 0 | 86 | 20 | 95 | 49 | 201 | 0 | 65 | 0 | 9 | 16 | 74 | 0 | 0 | 151 | 19 | 16 | 170 | 1 | 9 | 184 | 0 | 14 | 194 | 639 |
| Approach % | 0.0 | 42.8 | 10.0 | 47.3 | - | _ | 0.0 | 87.8 | 0.0 | 12.2 | - | - | 0.0 | 0.0 | 88.8 | 11.2 | - | - | 0.5 | 4.6 | 94.8 | 0.0 | - | - | - |
| Total % | 0.0 | 13.5 | 3.1 | 14.9 | - | 31.5 | 0.0 | 10.2 | 0.0 | 1.4 | - | 11.6 | 0.0 | 0.0 | 23.6 | 3.0 | - | 26.6 | 0.2 | 1.4 | 28.8 | 0.0 | - | 30.4 | T - |
| PHF | 0.000 | 0.768 | 0.625 | 0.792 | - | 0.824 | 0.000 | 0.677 | 0.000 | 0.563 | - | 0.661 | 0.000 | 0.000 | 0.821 | 0.594 | - | 0.850 | 0.250 | 0.750 | 0.657 | 0.000 | | 0.674 | 0.854 |
| Lights | 0 | 86 | 20 | 94 | - | 200 | 0 | 65 | 0 | 9 | - | 74 | 0 | 0 | 144 | 19 | - | 163 | 1 | 9 | 183 | 0 | - | 193 | 630 |
| % Lights | - | 100.0 | 100.0 | 98.9 | - | 99.5 | - | 100.0 | - | 100.0 | - | 100.0 | - | - | 95.4 | 100.0 | - | 95.9 | 100.0 | 100.0 | 99.5 | - | - | 99.5 | 98.6 |
| Buses | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 5 | 0 | - | 5 | 0 | 0 | 0 | 0 | - | 0 | 5 |
| % Buses | - | 0.0 | 0.0 | 0.0 | - | 0.0 | _ | 0.0 | - | 0.0 | - | 0.0 | - | - | 3.3 | 0.0 | - | 2.9 | 0.0 | 0.0 | 0.0 | - | | 0.0 | 0.8 |
| Single-Unit Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | | 0 | 0 |
| % Single-Unit Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | - | - | 0.0 | 0.0 |
| Articulated Trucks | 0 | 0 | 0 | 1 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 2 | 0 | - | 2 | 0 | 0 | 0 | 0 | - | 0 | 3 |
| % Articulated Trucks | - | 0.0 | 0.0 | 1.1 | - | 0.5 | - | 0.0 | - | 0.0 | - | 0.0 | - | - | 1.3 | 0.0 | - | 1.2 | 0.0 | 0.0 | 0.0 | - | - | 0.0 | 0.5 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 1 | 0 | - | 1 | 1 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.5 | - | - | 0.5 | 0.2 |
| Pedestrians | - | - | - | - | 49 | - | - | - | - | - | 16 | - | - | - | - | - | 16 | - | - | - | - | - | 14 | - | - |
| % Pedestrians | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | | - |
| | | | | | • | | • | | | | | - | • | | | • | | | • | | | | | | |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Burlington

Avenue
Site Code:
Start Date: 02/13/2019
Page No: 1

| Q: 17 | | ı | Burlington Avenue Eastbound | Э | | | 3 | Washington Stree | t | | | ١ | Washington Stree | t | | |
|----------------------|--------|------|--------------------------------|-------|------------|--------|------|------------------|-------|------------|--------|------|------------------|-------|------------|------------|
| Start Time | U-Turn | Left | Right | Peds | App. Total | U-Turn | Left | Thru | Peds | App. Total | U-Turn | Thru | Right | Peds | App. Total | Int. Total |
| 7:00 AM | 0 | 0 | 0 | 16 | 0 | 0 | 45 | 30 | 4 | 75 | 0 | 14 | 8 | 2 | 22 | 97 |
| 7:15 AM | 0 | 0 | 0 | 8 | 0 | 0 | 43 | 52 | 5 | 95 | 0 | 19 | 12 | 1 | 31 | 126 |
| 7:30 AM | 0 | 0 | 0 | 5 | 0 | 0 | 30 | 53 | 5 | 83 | 0 | 27 | 12 | 2 | 39 | 122 |
| 7:45 AM | 0 | 0 | 0 | 12 | 0 | 0 | 50 | 42 | 10 | 92 | 0 | 28 | 18 | 1 | 46 | 138 |
| Hourly Total | 0 | 0 | 0 | 41 | 0 | 0 | 168 | 177 | 24 | 345 | 0 | 88 | 50 | 6 | 138 | 483 |
| 8:00 AM | 0 | 0 | 0 | 2 | 0 | 0 | 17 | 57 | 0 | 74 | 0 | 22 | 6 | 0 | 28 | 102 |
| 8:15 AM | 0 | 0 | 0 | 4 | 0 | 0 | 34 | 54 | 5 | 88 | 0 | 25 | 11 | 1 | 36 | 124 |
| 8:30 AM | 0 | 0 | 0 | 0 | 0 | 0 | 19 | 41 | 1 | 60 | 0 | 30 | 11 | 0 | 41 | 101 |
| 8:45 AM | 0 | 0 | 0 | 0 | 0 | 0 | 16 | 45 | 0 | 61 | 0 | 20 | 7 | 0 | 27 | 88 |
| Hourly Total | 0 | 0 | 0 | 6 | 0 | 0 | 86 | 197 | 6 | 283 | 0 | 97 | 35 | 1 | 132 | 415 |
| *** BREAK *** | - | - | | - | - | - | - | | - | - | - | - | | - | - | - |
| 4:30 PM | 0 | 0 | 0 | 0 | 0 | 0 | 16 | 26 | 0 | 42 | 0 | 53 | 10 | 0 | 63 | 105 |
| 4:45 PM | 0 | 0 | 0 | 0 | 0 | 1 | 12 | 28 | 8 | 41 | 0 | 46 | 18 | 0 | 64 | 105 |
| Hourly Total | 0 | 0 | 0 | 0 | 0 | 1 | 28 | 54 | 8 | 83 | 0 | 99 | 28 | 0 | 127 | 210 |
| 5:00 PM | 0 | 0 | 0 | 1 | 0 | 0 | 28 | 33 | 0 | 61 | 0 | 53 | 15 | 0 | 68 | 129 |
| 5:15 PM | 0 | 0 | 0 | 0 | 0 | 0 | 21 | 38 | 2 | 59 | 0 | 46 | 14 | 0 | 60 | 119 |
| 5:30 PM | 0 | 0 | 0 | 4 | 0 | 0 | 32 | 30 | 37 | 62 | 0 | 31 | 8 | 0 | 39 | 101 |
| 5:45 PM | 0 | 0 | 0 | 2 | 0 | 0 | 24 | 39 | 4 | 63 | 0 | 60 | 19 | 0 | 79 | 142 |
| Hourly Total | 0 | 0 | 0 | 7 | 0 | 0 | 105 | 140 | 43 | 245 | 0 | 190 | 56 | 0 | 246 | 491 |
| 6:00 PM | 0 | 0 | 0 | 3 | 0 | 0 | 16 | 29 | 12 | 45 | 0 | 34 | 6 | 0 | 40 | 85 |
| 6:15 PM | 0 | 0 | 0 | 7 | 0 | 0 | 17 | 52 | 4 | 69 | 0 | 79 | 23 | 2 | 102 | 171 |
| Grand Total | 0 | 0 | 0 | 64 | 0 | 1 | 420 | 649 | 97 | 1070 | 0 | 587 | 198 | 9 | 785 | 1855 |
| Approach % | 0.0 | 0.0 | 0.0 | - | - | 0.1 | 39.3 | 60.7 | - | - | 0.0 | 74.8 | 25.2 | - | - | - |
| Total % | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.1 | 22.6 | 35.0 | - | 57.7 | 0.0 | 31.6 | 10.7 | - | 42.3 | - |
| Lights | 0 | 0 | 0 | - | 0 | 1 | 398 | 640 | - | 1039 | 0 | 579 | 197 | - | 776 | 1815 |
| % Lights | - | - | | - | - | 100.0 | 94.8 | 98.6 | - | 97.1 | - | 98.6 | 99.5 | - | 98.9 | 97.8 |
| Buses | 0 | 0 | 0 | - | 0 | 0 | 18 | 4 | - | 22 | 0 | 0 | 0 | - | 0 | 22 |
| % Buses | - | - | | - | | 0.0 | 4.3 | 0.6 | - | 2.1 | - | 0.0 | 0.0 | - | 0.0 | 1.2 |
| Single-Unit Trucks | 0 | 0 | 0 | - | 0 | 0 | 4 | 5 | - | 9 | 0 | 7 | . 1 | - | 8 | 17 |
| % Single-Unit Trucks | - | - | - | - | - | 0.0 | 1.0 | 0.8 | - | 0.8 | - | 1.2 | 0.5 | - | 1.0 | 0.9 |
| Articulated Trucks | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | - | - | <u>-</u> | - | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 | 1 | 0 | - | 1 | 1 |
| % Bicycles on Road | - | - | - | - | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.2 | 0.0 | - | 0.1 | 0.1 |
| Pedestrians | - | - | <u>-</u> | 64 | - | - | - | - | 97 | - | - | - | - | 9 | - | - |
| % Pedestrians | - | - | <u>-</u> | 100.0 | - | - | - | <u>-</u> | 100.0 | - | - | - | <u>-</u> | 100.0 | - | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Burlington Avenue Site Code: Start Date: 02/13/2019 Page No: 2

| | 1 | | | | | ייי כי יייי ק | | | • | , , | ı | | | | | l . |
|----------------------|--------|-------|------------------|-------|------------|---------------|-------|------------------|-------|------------|--------|-------|------------------|-------|------------|------------|
| | | E | Burlington Avenu | ie | | | \ | Nashington Stree | et | | | \ | Vashington Stree | et | | |
| Ot and Time a | | | Eastbound | | | | | Northbound | | | | | Southbound | | | |
| Start Time | U-Turn | Left | Right | Peds | App. Total | U-Turn | Left | Thru | Peds | App. Total | U-Turn | Thru | Right | Peds | App. Total | Int. Total |
| 7:15 AM | 0 | 0 | 0 | 8 | 0 | 0 | 43 | 52 | 5 | 95 | 0 | 19 | 12 | 1 | 31 | 126 |
| 7:30 AM | 0 | 0 | 0 | 5 | 0 | 0 | 30 | 53 | 5 | 83 | 0 | 27 | 12 | 2 | 39 | 122 |
| 7:45 AM | 0 | 0 | 0 | 12 | 0 | 0 | 50 | 42 | 10 | 92 | 0 | 28 | 18 | 1 | 46 | 138 |
| 8:00 AM | 0 | 0 | 0 | 2 | 0 | 0 | 17 | 57 | 0 | 74 | 0 | 22 | 6 | 0 | 28 | 102 |
| Total | 0 | 0 | 0 | 27 | 0 | 0 | 140 | 204 | 20 | 344 | 0 | 96 | 48 | 4 | 144 | 488 |
| Approach % | 0.0 | 0.0 | 0.0 | - | - | 0.0 | 40.7 | 59.3 | - | - | 0.0 | 66.7 | 33.3 | - | - | - |
| Total % | 0.0 | 0.0 | 0.0 | _ | 0.0 | 0.0 | 28.7 | 41.8 | - | 70.5 | 0.0 | 19.7 | 9.8 | - | 29.5 | - |
| PHF | 0.000 | 0.000 | 0.000 | - | 0.000 | 0.000 | 0.700 | 0.895 | - | 0.905 | 0.000 | 0.857 | 0.667 | - | 0.783 | 0.884 |
| Lights | 0 | 0 | 0 | - | 0 | 0 | 134 | 197 | - | 331 | 0 | 95 | 47 | - | 142 | 473 |
| % Lights | - | - | - | _ | - | - | 95.7 | 96.6 | - | 96.2 | - | 99.0 | 97.9 | - | 98.6 | 96.9 |
| Buses | 0 | 0 | 0 | - | 0 | 0 | 5 | 4 | - | 9 | 0 | 0 | 0 | - | 0 | 9 |
| % Buses | - | - | - | - | - | - | 3.6 | 2.0 | - | 2.6 | - | 0.0 | 0.0 | - | 0.0 | 1.8 |
| Single-Unit Trucks | 0 | 0 | 0 | _ | 0 | 0 | 1 | 3 | - | 4 | 0 | 1 | 1 | - | 2 | 6 |
| % Single-Unit Trucks | - | - | - | - | - | - | 0.7 | 1.5 | - | 1.2 | - | 1.0 | 2.1 | - | 1.4 | 1.2 |
| Articulated Trucks | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | - | - | - | - | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Bicycles on Road | - | - | - | - | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Pedestrians | - | - | _ | 27 | - | - | - | - | 20 | - | - | - | _ | 4 | _ | - |
| % Pedestrians | - | - | - | 100.0 | - | - | - | - | 100.0 | - | - | - | - | 100.0 | - | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Burlington Avenue Site Code: Start Date: 02/13/2019 Page No: 3

| | | | | | 9 | | | - 4.4 (0. | , | | | | | | |
|--------|--|--|---------|---|---|--|------------------|--|--|--------|--|--|---|------------|--|
| | E | Burlington Avenu | ie | | | \ | Washington Stree | et | | | ١ | Vashington Stree | et | | |
| | | Eastbound | | | | | Northbound | | | | | Southbound | | | |
| U-Turn | Left | Right | Peds | App. Total | U-Turn | Left | Thru | Peds | App. Total | U-Turn | Thru | Right | Peds | App. Total | Int. Total |
| 0 | 0 | 0 | 4 | 0 | 0 | 32 | 30 | 37 | 62 | 0 | 31 | . 8 | 0 | 39 | 101 |
| 0 | 0 | 0 | 2 | 0 | 0 | 24 | 39 | 4 | 63 | 0 | 60 | 19 | 0 | 79 | 142 |
| 0 | 0 | 0 | 3 | 0 | 0 | 16 | 29 | 12 | 45 | 0 | 34 | 6 | 0 | 40 | 85 |
| 0 | 0 | 0 | 7 | 0 | 0 | 17 | 52 | 4 | 69 | 0 | 79 | 23 | 2 | 102 | 171 |
| 0 | 0 | 0 | 16 | 0 | 0 | 89 | 150 | 57 | 239 | 0 | 204 | 56 | 2 | 260 | 499 |
| 0.0 | 0.0 | 0.0 | - | - | 0.0 | 37.2 | 62.8 | - | - | 0.0 | 78.5 | 21.5 | - | - | - |
| 0.0 | 0.0 | 0.0 | _ | 0.0 | 0.0 | 17.8 | 30.1 | - | 47.9 | 0.0 | 40.9 | 11.2 | - | 52.1 | - |
| 0.000 | 0.000 | 0.000 | - | 0.000 | 0.000 | 0.695 | 0.721 | - | 0.866 | 0.000 | 0.646 | 0.609 | - | 0.637 | 0.730 |
| 0 | 0 | 0 | - | 0 | 0 | 84 | 150 | - | 234 | 0 | 201 | 56 | - | 257 | 491 |
| - | - | _ | _ | - | - | 94.4 | 100.0 | - | 97.9 | - | 98.5 | 100.0 | - | 98.8 | 98.4 |
| 0 | 0 | 0 | _ | 0 | 0 | 5 | 0 | - | 5 | 0 | 0 | 0 | - | 0 | 5 |
| - | - | - | - | - | - | 5.6 | 0.0 | - | 2.1 | - | 0.0 | 0.0 | - | 0.0 | 1.0 |
| 0 | 0 | 0 | _ | 0 | 0 | 0 | 0 | - | 0 | 0 | 2 | 0 | - | 2 | 2 |
| - | - | | _ | _ | - | 0.0 | 0.0 | - | 0.0 | - | 1.0 | 0.0 | - | 0.8 | 0.4 |
| 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| - | - | _ | _ | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 | 1 | 0 | - | 1 | 1 |
| - | - | - | - | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.5 | 0.0 | - | 0.4 | 0.2 |
| - | - | _ | 16 | - | - | | _ | 57 | <u>-</u> | - | - | _ | 2 | - | - |
| - | - | - | 100.0 | - | - | _ | - | 100.0 | - | - | - | _ | 100.0 | - | - |
| | 0 0 0 0 0 0 0.0 0.00 0 0 0 - 0 - 0 | U-Turn Left 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | Company | U-Turn Left Right Peds 0 0 0 4 0 0 0 2 0 0 0 3 0 0 0 7 0 0 0 0 7 0 0 0 0 - 0.00 0.00 0.00 - - 0.000 0.000 0.000 - - 0 0 0 0 - - - - - - 0 0 0 - - 0 0 0 - - 0 0 0 - - 0 0 0 - - 0 0 0 - - 0 0 0 - - 0 0 0 - - 0 0 | Burlington Avenue Eastbound Eastbound Eastbound | Burlington Avenue Eastbound U-Turn Left Right Peds App. Total U-Turn | U-Turn | Burlington Avenue Eastbound U-Turn Left Right Peds App. Total U-Turn Left Thru | Burlington Avenue Eastbound High Peds App. Total U-Turn Left Thru Peds App. Total Thru Peds App. Total U-Turn Left Thru Peds App. Total App | Heat | Burlington Avenue Eastbound U-Turn Left Right Peds App. Total U-Turn Left Thru Peds App. Total U-Turn Left Thru Peds App. Total U-Turn | Burlington Avenue Eastbound U-Turn Left Thru Peds App. Total U-Turn Thru O O O O O O O O O | Burlington Avenue Eastbound U-Trum Left Right Peds App. Total U-Trum Left Right Peds App. Total U-Trum Left Thru Peds App. Total U-Trum Right Right Peds App. Total U-Trum Thru Right Right Right Peds App. Total U-Trum Thru Right Right Peds Desc Desc | U-Turn | Burlington Avenue Eastbound Left Southbound Sou |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Village Hall Access Drive Site Code: Start Date: 02/13/2019 Page No: 1

| Chart Times | | | | n Avenue bound | | | | V | illage Hall <i>F</i> Westb | | ve . | | | | | s Drive bound | | | | | | ton Street bound | | | |
|-------------------------|--------|------|-------|-------------------|------|---------------|--------|-------|-------------------------------|-------|------|---------------|--------|------|-------|------------------|------|---------------|--------|------|------|---------------------|------|---------------|------------|
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 7:00 AM | 0 | 28 | . 1 | 2 | 0 | 31 | 0 | 0 | 1 | 10 | 0 | 11 | 0 | 2 | 0 | 0 | 5 | 2 | 0 | 3 | 3 | 21 | 19 | 27 | 71 |
| 7:15 AM | 0 | 47 | 3 | 1 | 0 | 51 | 0 | 0 | 3 | 17 | 0 | 20 | 0 | 1 | 0 | 0 | 5 | 1 | 0 | 4 | 2 | 27 | 22 | 33 | 105 |
| 7:30 AM | 0 | 46 | 0 | 0 | 0 | 46 | 0 | 0 | 2 | 27 | 1 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 1 | 35 | 19 | 40 | 115 |
| 7:45 AM | 0 | 39 | 2 | 0 | 0 | 41 | 0 | 0 | 4 | 33 | 0 | 37 | 0 | 3 | 0 | 1 | 2 | 4 | 0 | 10 | 3 | 46 | 52 | 59 | 141 |
| Hourly Total | 0 | 160 | 6 | 3 | 0 | 169 | 0 | 0 | 10 | 87 | 1 | 97 | 0 | 6 | 0 | 1 | 12 | 7 | 0 | 21 | 9 | 129 | 112 | 159 | 432 |
| 8:00 AM | 0 | 60 | 0 | 1 | 0 | 61 | 0 | 0 | 0 | 7 | 1 | 7 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 10 | 0 | 20 | 0 | 30 | 99 |
| 8:15 AM | 0 | 48 | 2 | 2 | 0 | 52 | 0 | 2 | 2 | 8 | 2 | 12 | 0 | 2 | 0 | 0 | 1 | 2 | 0 | 6 | 3 | 34 | 19 | 43 | 109 |
| 8:30 AM | 0 | 41 | 0 | 1 | 0 | 42 | 0 | 0 | 3 | 11 | 0 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 6 | 0 | 37 | 1 | 44 | 100 |
| 8:45 AM | 0 | 34 | 1 | 1 | 0 | 36 | 0 | 1 | 1 | 5 | 0 | 7 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 4 | 1 | 25 | 1 | 30 | 74 |
| Hourly Total | 0 | 183 | 3 | 5 | 0 | 191 | 0 | 3 | 6 | 31 | 3 | 40 | 0 | 2 | . 1 | 1 | 2 | 4 | 1 | 26 | 4 | 116 | 21 | 147 | 382 |
| *** BREAK *** | - | - | _ | _ | - | - | - | - | | - | - | _ | - | - | | | - | - | - | - | _ | _ | - | - | - |
| 4:30 PM | 0 | 29 | 0 | 0 | 0 | 29 | 0 | 1 | 1 | 6 | 1 | 8 | 0 | 2 | 0 | 0 | 2 | 2 | 0 | 16 | 1 | 58 | 0 | 75 | 114 |
| 4:45 PM | 0 | 24 | 0 | 1 | 0 | 25 | 0 | 0 | 5 | 10 | 1 | 15 | 0 | 2 | 0 | 0 | 12 | 2 | 0 | 14 | 2 | 56 | 2 | 72 | 114 |
| Hourly Total | 0 | 53 | 0 | 1 | 0 | 54 | 0 | 1 | 6 | 16 | 2 | 23 | 0 | 4 | 0 | 0 | 14 | 4 | 0 | 30 | 3 | 114 | 2 | 147 | 228 |
| 5:00 PM | 0 | 28 | 0 | 0 | 1 | 28 | 0 | 0 | 1 | 8 | 0 | 9 | 0 | 3 | 1 | 0 | 0 | 4 | 0 | 16 | 2 | 58 | 0 | 76 | 117 |
| 5:15 PM | 0 | 40 | 0 | 1 | 0 | 41 | 0 | 0 | 2 | 9 | 8 | 11 | 0 | 1 | 0 | 1 | 18 | 2 | 0 | 17 | 1 | 57 | 4 | 75 | 129 |
| 5:30 PM | 0 | 25 | 11 | 0 | 0 | 26 | 0 | 0 | 11 | 5 | 9 | 6 | 0 | 0 | 11 | 0 | 23 | 1 | 0 | 13 | 0 | 54 | 0 | 67 | 100 |
| 5:45 PM | 0 | 30 | 4 | 0 | 0 | 34 | 0 | 0 | 1 | 12 | 0 | 13 | 0 | 1 | 1 | 1 | 0 | 3 | 0 | 19 | 2 | 68 | 0 | 89 | 139 |
| Hourly Total | 0 | 123 | 5 | 1 | 1 | 129 | 0 | 0 | 5 | 34 | 17 | 39 | 0 | 5 | 3 | 2 | 41 | 10 | 0 | 65 | 5 | 237 | 4 | 307 | 485 |
| 6:00 PM | 0 | 27 | 0 | 1 | 1 | 28 | 0 | 0 | 1 | 11 | 26 | 12 | 0 | 2 | 0 | 0 | 14 | 2 | 0 | 16 | 1 | 44 | 0 | 61 | 103 |
| 6:15 PM | 0 | 38 | 0 | 0 | 0 | 38 | 0 | 0 | 4 | 15 | 0 | 19 | 0 | 1 | 1 | 1 | 1 | 3 | 0 | 16 | 3 | 86 | 0 | 105 | 165 |
| Grand Total | 0 | 584 | 14 | 11 | 2 | 609 | 0 | 4 | 32 | 194 | 49 | 230 | 0 | 20 | 5 | 5 | 84 | 30 | 1 | 174 | 25 | 726 | 139 | 926 | 1795 |
| Approach % | 0.0 | 95.9 | 2.3 | 1.8 | - | _ | 0.0 | 1.7 | 13.9 | 84.3 | - | _ | 0.0 | 66.7 | 16.7 | 16.7 | - | - | 0.1 | 18.8 | 2.7 | 78.4 | - | _ | - |
| Total % | 0.0 | 32.5 | 0.8 | 0.6 | - | 33.9 | 0.0 | 0.2 | 1.8 | 10.8 | - | 12.8 | 0.0 | 1.1 | 0.3 | 0.3 | - | 1.7 | 0.1 | 9.7 | 1.4 | 40.4 | - | 51.6 | - |
| Lights | 0 | 578 | 14 | 11 | - | 603 | 0 | 4 | 32 | 192 | - | 228 | 0 | 19 | . 5 | . 5 | - | 29 | 1 | 169 | 24 | 722 | - | 916 | 1776 |
| % Lights | - | 99.0 | 100.0 | 100.0 | - | 99.0 | - | 100.0 | 100.0 | 99.0 | - | 99.1 | - | 95.0 | 100.0 | 100.0 | - | 96.7 | 100.0 | 97.1 | 96.0 | 99.4 | - | 98.9 | 98.9 |
| Buses | 0 | 2 | 0 | 0 | - | 2 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 3 | 0 | 1 | - | 4 | 6 |
| % Buses | - | 0.3 | 0.0 | 0.0 | - | 0.3 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 | 1.7 | 0.0 | 0.1 | - | 0.4 | 0.3 |
| Single-Unit Trucks | 0 | 4 | 0 | 0 | - | 4 | 0 | 0 | 0 | 2 | - | 2 | 0 | 1 | 0 | 0 | - | 1 | 0 | 1 | 1 | 3 | - | 5 | 12 |
| % Single-Unit Trucks | - | 0.7 | 0.0 | 0.0 | - | 0.7 | - | 0.0 | 0.0 | 1.0 | - | 0.9 | - | 5.0 | 0.0 | 0.0 | - | 3.3 | 0.0 | 0.6 | 4.0 | 0.4 | - | 0.5 | 0.7 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 1 | 0 | 0 | - | 1 | 1 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 | 0.6 | 0.0 | 0.0 | - | 0.1 | 0.1 |
| Pedestrians | - | - | - | | 2 | _ | - | _ | - | - | 49 | _ | - | - | - | _ | 84 | | - | - | - | | 139 | | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Village Hall Access Drive Site Code: Start Date: 02/13/2019 Page No: 3

| | 1 | | | | | | 1 | | 9 | | .0 | Jan | 1001 | Data | (1.10 | ,, | | | ı | | | | | | 1 |
|-------------------------|--------|-------|-----------|----------|------|---------------|--------|-------|-------------|-------------|-------|---------------|--------|-------|-------|---------|-------|---------------|--------|-------|----------|-----------|-------|---------------|------------|
| | | | Burlingto | n Avenue | | | | V | illage Hall | Access Driv | ve | | | | Acces | s Drive | | | | | Washingt | on Street | | | |
| | | | Eastl | oound | | | | | West | bound | | | | | North | bound | | | | | South | bound | | | |
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 7:15 AM | 0 | 47 | 3 | 1 | 0 | 51 | 0 | 0 | 3 | 17 | 0 | 20 | 0 | 1 | 0 | 0 | 5 | 1 | 0 | 4 | 2 | 27 | 22 | 33 | 105 |
| 7:30 AM | 0 | 46 | 0 | 0 | 0 | 46 | 0 | 0 | 2 | 27 | 1 | 29 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 1 | 35 | 19 | 40 | 115 |
| 7:45 AM | 0 | 39 | 2 | 0 | 0 | 41 | 0 | 0 | 4 | 33 | 0 | 37 | 0 | 3 | 0 | 1 | 2 | 4 | 0 | 10 | 3 | 46 | 52 | 59 | 141 |
| 8:00 AM | 0 | 60 | 0 | 1 | 0 | 61 | 0 | 0 | 0 | 7 | 1 | 7 | 0 | 0 | 0 | 1 | 0 | 1 | 0 | 10 | 0 | 20 | 0 | 30 | 99 |
| Total | 0 | 192 | 5 | 2 | 0 | 199 | 0 | 0 | 9 | 84 | 2 | 93 | 0 | 4 | 0 | 2 | 7 | 6 | 0 | 28 | 6 | 128 | 93 | 162 | 460 |
| Approach % | 0.0 | 96.5 | 2.5 | 1.0 | - | - | 0.0 | 0.0 | 9.7 | 90.3 | - | - | 0.0 | 66.7 | 0.0 | 33.3 | - | - | 0.0 | 17.3 | 3.7 | 79.0 | - | - | - |
| Total % | 0.0 | 41.7 | 1.1 | 0.4 | - | 43.3 | 0.0 | 0.0 | 2.0 | 18.3 | - | 20.2 | 0.0 | 0.9 | 0.0 | 0.4 | - | 1.3 | 0.0 | 6.1 | 1.3 | 27.8 | - | 35.2 | - |
| PHF | 0.000 | 0.800 | 0.417 | 0.500 | - | 0.816 | 0.000 | 0.000 | 0.563 | 0.636 | - | 0.628 | 0.000 | 0.333 | 0.000 | 0.500 | - | 0.375 | 0.000 | 0.700 | 0.500 | 0.696 | - | 0.686 | 0.816 |
| Lights | 0 | 187 | 5 | 2 | - | 194 | 0 | 0 | 9 | 82 | - | 91 | 0 | 3 | 0 | 2 | - | 5 | 0 | 26 | 5 | 127 | - | 158 | 448 |
| % Lights | - | 97.4 | 100.0 | 100.0 | - | 97.5 | - | - | 100.0 | 97.6 | - | 97.8 | - | 75.0 | - | 100.0 | - | 83.3 | - | 92.9 | 83.3 | 99.2 | - | 97.5 | 97.4 |
| Buses | 0 | 2 | 0 | 0 | - | 2 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 2 | 0 | 0 | - | 2 | 4 |
| % Buses | - | 1.0 | 0.0 | 0.0 | - | 1.0 | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | 7.1 | 0.0 | 0.0 | - | 1.2 | 0.9 |
| Single-Unit Trucks | 0 | 3 | 0 | 0 | - | 3 | 0 | 0 | 0 | 2 | - | 2 | 0 | 1 | 0 | 0 | - | 1 | 0 | 0 | 1 | 1 | - | 2 | 8 |
| % Single-Unit Trucks | - | 1.6 | 0.0 | 0.0 | - | 1.5 | - | - | 0.0 | 2.4 | - | 2.2 | - | 25.0 | - | 0.0 | - | 16.7 | - | 0.0 | 16.7 | 0.8 | - | 1.2 | 1.7 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Pedestrians | - | - | - | - | 0 | - | - | - | - | - | 2 | - | - | - | - | - | 7 | - | - | - | - | - | 93 | - | - |
| % Pedestrians | - | - | - | - | - | - | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | - | | - | | - | 100.0 | - | - |
| | | | | | | | | | | | | | | | | | | | | | | | _ | _ | |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Village Hall Access Drive Site Code: Start Date: 02/13/2019 Page No: 4

| | i . | | | | | | i | | | | | | 1 | | (| , | | | i | | | | | | 1 |
|-------------------------|--------|-------|-----------|----------|-------|---------------|--------|-------|-------------|-------------|-------|---------------|--------|-------|-------|---------|-------|---------------|--------|-------|---------|------------|------|---------------|------------|
| | | | Burlingto | n Avenue | | | | V | illage Hall | Access Driv | /e | | | | Acces | s Drive | | | 1 | | Washing | ton Street | | | |
| | | | Eastl | oound | | | | | West | bound | | | | | North | bound | | | | | South | bound | | | |
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 5:30 PM | 0 | 25 | 1 | 0 | 0 | 26 | 0 | 0 | 1 | 5 | 9 | 6 | 0 | 0 | 1 | 0 | 23 | 1 | 0 | 13 | 0 | 54 | 0 | 67 | 100 |
| 5:45 PM | 0 | 30 | 4 | 0 | 0 | 34 | 0 | 0 | 1 | 12 | 0 | 13 | 0 | 1 | 1 | 1 | 0 | 3 | 0 | 19 | 2 | 68 | 0 | 89 | 139 |
| 6:00 PM | 0 | 27 | 0 | 1 | 1 | 28 | 0 | 0 | 1 | 11 | 26 | 12 | 0 | 2 | 0 | 0 | 14 | 2 | 0 | 16 | 1 | 44 | 0 | 61 | 103 |
| 6:15 PM | 0 | 38 | 0 | 0 | 0 | 38 | 0 | 0 | 4 | 15 | 0 | 19 | 0 | 1 | 1 | 1 | 1 | 3 | 0 | 16 | 3 | 86 | 0 | 105 | 165 |
| Total | 0 | 120 | 5 | 1 | 1 | 126 | 0 | 0 | 7 | 43 | 35 | 50 | 0 | 4 | 3 | 2 | 38 | 9 | 0 | 64 | 6 | 252 | 0 | 322 | 507 |
| Approach % | 0.0 | 95.2 | 4.0 | 0.8 | - | - | 0.0 | 0.0 | 14.0 | 86.0 | - | - | 0.0 | 44.4 | 33.3 | 22.2 | - | - | 0.0 | 19.9 | 1.9 | 78.3 | - | - | - |
| Total % | 0.0 | 23.7 | 1.0 | 0.2 | - | 24.9 | 0.0 | 0.0 | 1.4 | 8.5 | - | 9.9 | 0.0 | 0.8 | 0.6 | 0.4 | - | 1.8 | 0.0 | 12.6 | 1.2 | 49.7 | - | 63.5 | - |
| PHF | 0.000 | 0.789 | 0.313 | 0.250 | - | 0.829 | 0.000 | 0.000 | 0.438 | 0.717 | - | 0.658 | 0.000 | 0.500 | 0.750 | 0.500 | - | 0.750 | 0.000 | 0.842 | 0.500 | 0.733 | - | 0.767 | 0.768 |
| Lights | 0 | 120 | 5 | 1 | - | 126 | 0 | 0 | 7 | 43 | - | 50 | 0 | 4 | 3 | 2 | - | 9 | 0 | 64 | 6 | 251 | - | 321 | 506 |
| % Lights | - | 100.0 | 100.0 | 100.0 | - | 100.0 | - | - | 100.0 | 100.0 | - | 100.0 | - | 100.0 | 100.0 | 100.0 | - | 100.0 | - | 100.0 | 100.0 | 99.6 | - | 99.7 | 99.8 |
| Buses | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 1 | - | 1 | 1 |
| % Buses | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.4 | - | 0.3 | 0.2 |
| Single-Unit Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Single-Unit Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Pedestrians | - | - | - | - | 1 | - | - | - | - | - | 35 | - | - | - | - | - | 38 | - | - | - | - | - | 0 | - | T - |
| % Pedestrians | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | - | - | - | - | - | - | - | - |
| | | | - | | • | | - | | | | | | | | | | • | • | | - | | | | | |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Warren

Avenue
Site Code:
Start Date: 02/14/2019
Page No: 1

| | | | Warren Eastb | Avenue | | | | | Warren Westl | Avenue | 9 | | | | | ton Street | | | | | | ton Street bound | | | |
|-------------------------|--------|-------|-----------------|--------|------|---------------|--------|-------|-----------------|--------|------|---------------|--------|------|------|------------|------|---------------|--------|-------|------|---------------------|------|---------------|------------|
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 7:00 AM | 0 | 2 | 0 | 1 | 14 | 3 | 0 | 3 | 0 | 1 | 0 | 4 | 0 | 4 | 31 | 1 | 6 | 36 | 0 | 3 | 27 | 6 | 0 | 36 | 79 |
| 7:15 AM | 0 | 2 | 2 | 2 | 13 | 6 | 0 | 1 | 2 | 2 | 0 | 5 | 0 | 13 | 66 | 2 | 10 | 81 | 0 | 0 | 46 | 1 | 0 | 47 | 139 |
| 7:30 AM | 0 | 1 | 0 | 5 | 0 | 6 | 0 | 0 | 1 | 0 | 2 | 1 | 0 | 5 | 68 | 5 | 2 | 78 | 0 | 0 | 41 | 1 | 1 | 42 | 127 |
| 7:45 AM | 0 | 2 | 0 | 3 | 15 | 5 | 0 | 2 | 1 | 0 | 4 | 3 | 0 | 1 | 52 | 2 | 10 | 55 | 0 | 0 | 34 | 5 | 0 | 39 | 102 |
| Hourly Total | 0 | 7 | 2 | 11 | 42 | 20 | 0 | 6 | 4 | 3 | 6 | 13 | 0 | 23 | 217 | 10 | 28 | 250 | 0 | 3 | 148 | 13 | 1 | 164 | 447 |
| 8:00 AM | 0 | 2 | 1 | 4 | 0 | 7 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 3 | 63 | 1 | 0 | 67 | 0 | 1 | 38 | 2 | 0 | 41 | 116 |
| 8:15 AM | 0 | 2 | 1 | 6 | 5 | 9 | 0 | 0 | 1 | 1 | 0 | 2 | 0 | 1 | 47 | 0 | 2 | 48 | 0 | 1 | 27 | 2 | 0 | 30 | 89 |
| 8:30 AM | 0 | 1 | 0 | 5 | 1 | 6 | 0 | 1 | 2 | 0 | 0 | 3 | 0 | 4 | 55 | 0 | 1 | 59 | 0 | 0 | 36 | 1 | 2 | 37 | 105 |
| 8:45 AM | 0 | 1 | 1 | 7 | 1 | 9 | 0 | 1 | 1 | 0 | 0 | 2 | 0 | 4 | 24 | 0 | 0 | 28 | 0 | 0 | 35 | 0 | 0 | 35 | 74 |
| Hourly Total | 0 | 6 | 3 | 22 | 7 | 31 | 0 | 2 | 5 | 1 | 0 | 8 | 0 | 12 | 189 | . 1 | 3 | 202 | 0 | 2 | 136 | 5 | 2 | 143 | 384 |
| *** BREAK *** | - | - | _ | _ | - | - | - | - | - | - | - | | - | - | _ | | - | - | - | - | - | _ | - | - | - |
| 4:30 PM | 0 | 3 | 5 | 2 | 2 | 10 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 4 | 40 | 1 | 0 | 45 | 0 | 0 | 45 | 4 | 1 | 49 | 105 |
| 4:45 PM | 0 | 4 | 4 | 8 | 8 | 16 | 0 | 2 | 1 | 1 | 1 | 4 | 0 | 5 | 55 | 0 | 10 | 60 | 0 | 0 | 60 | 6 | 3 | 66 | 146 |
| Hourly Total | 0 | 7 | 9 | 10 | 10 | 26 | 0 | 2 | 2 | 1 | 2 | 5 | 0 | 9 | 95 | 1 | 10 | 105 | 0 | 0 | 105 | 10 | 4 | 115 | 251 |
| 5:00 PM | 0 | 5 | 0 | 17 | 10 | 22 | 0 | 1 | 4 | 2 | 2 | 7 | 0 | 9 | 43 | 0 | 20 | 52 | 0 | 1 | 85 | 5 | 0 | 91 | 172 |
| 5:15 PM | 0 | 2 | 0 | 16 | 0 | 18 | 0 | 2 | 7 | 1 | 1 | 10 | 0 | 4 | 33 | 0 | 2 | 37 | 0 | 0 | 51 | 8 | 0 | 59 | 124 |
| 5:30 PM | 0 | 4 | 4 | 23 | 9 | 31 | 0 | 5 | 2 | 3 | 10 | 10 | 0 | 7 | 50 | 1 | 34 | 58 | 0 | 0 | 73 | 6 | 1 | 79 | 178 |
| 5:45 PM | 0 | 2 | 0 | 15 | 1 | 17 | 0 | 4 | 0 | 2 | 34 | 6 | 0 | 9 | 29 | 1 | 83 | 39 | 0 | 0 | 39 | 4 | 6 | 43 | 105 |
| Hourly Total | 0 | 13 | 4 | 71 | 20 | 88 | 0 | 12 | 13 | 8 | 47 | 33 | 0 | 29 | 155 | 2 | 139 | 186 | 0 | 1 | 248 | 23 | 7 | 272 | 579 |
| 6:00 PM | 0 | 4 | 3 | 25 | 1 | 32 | 0 | 3 | 0 | 0 | 3 | 3 | 0 | 7 | 68 | 1 | 2 | 76 | 0 | 0 | 85 | 6 | 0 | 91 | 202 |
| 6:15 PM | 0 | 6 | 2 | 11 | 16 | 19 | 0 | 1 | 1 | 4 | 3 | 6 | 0 | 13 | 35 | 0 | 9 | 48 | 0 | 1 | 40 | 10 | 1 | 51 | 124 |
| Grand Total | 0 | 43 | 23 | 150 | 96 | 216 | 0 | 26 | 25 | 17 | 61 | 68 | 0 | 93 | 759 | 15 | 191 | 867 | 0 | 7 | 762 | 67 | 15 | 836 | 1987 |
| Approach % | 0.0 | 19.9 | 10.6 | 69.4 | - | - | 0.0 | 38.2 | 36.8 | 25.0 | - | _ | 0.0 | 10.7 | 87.5 | 1.7 | - | - | 0.0 | 8.0 | 91.1 | 8.0 | _ | - | - |
| Total % | 0.0 | 2.2 | 1.2 | 7.5 | - | 10.9 | 0.0 | 1.3 | 1.3 | 0.9 | - | 3.4 | 0.0 | 4.7 | 38.2 | 0.8 | - | 43.6 | 0.0 | 0.4 | 38.3 | 3.4 | - | 42.1 | - |
| Lights | 0 | 43 | 23 | 149 | | 215 | 0 | 26 | 25 | 16 | - | 67 | 0 | 92 | 750 | 15 | - | 857 | 0 | 7 | 757 | 67 | | 831 | 1970 |
| % Lights | - | 100.0 | 100.0 | 99.3 | - | 99.5 | - | 100.0 | 100.0 | 94.1 | - | 98.5 | - | 98.9 | 98.8 | 100.0 | - | 98.8 | - | 100.0 | 99.3 | 100.0 | - | 99.4 | 99.1 |
| Buses | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 4 | 0 | - | 4 | 0 | 0 | 1 | 0 | - | 1 | 5 |
| % Buses | - | 0.0 | 0.0 | 0.0 | | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.5 | 0.0 | - | 0.5 | - | 0.0 | 0.1 | 0.0 | | 0.1 | 0.3 |
| Single-Unit Trucks | 0 | 0 | 0 | 1 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 1 | 4 | 0 | - | 5 | 0 | 0 | 3 | 0 | _ | 3 | 9 |
| % Single-Unit Trucks | - | 0.0 | 0.0 | 0.7 | - | 0.5 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 1.1 | 0.5 | 0.0 | - | 0.6 | - | 0.0 | 0.4 | 0.0 | - | 0.4 | 0.5 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 1 | - | 1 | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 1 | 0 | - | 1 | 3 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 5.9 | - | 1.5 | - | 0.0 | 0.1 | 0.0 | - | 0.1 | - | 0.0 | 0.1 | 0.0 | - | 0.1 | 0.2 |
| Pedestrians | - | _ | _ | _ | 96 | _ | - | - | _ | - | 61 | _ | - | - | _ | | 191 | _ | - | - | - | _ | 15 | - | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Warren Avenue Site Code: Start Date: 02/14/2019 Page No: 3

| | 1 | | | | | | 1 | | 9 | | | | 1 | | | , | | | I | | | | | | 1 |
|-------------------------|--------|-------|--------|--------|-------|---------------|--------|-------|--------|--------|-------|---------------|--------|-------|---------|------------|-------|---------------|--------|-------|---------|-----------|-------|---------------|------------|
| | | | Warren | Avenue | | | | | Warren | Avenue | | | | | Washing | ton Street | | | 1 | | Washing | on Street | | | |
| | | | Easth | oound | | | | | West | bound | | | | | North | bound | | | | | South | bound | | | |
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 7:15 AM | 0 | 2 | 2 | 2 | 13 | 6 | 0 | 1 | 2 | 2 | 0 | 5 | 0 | 13 | 66 | 2 | 10 | 81 | 0 | 0 | 46 | 1 | 0 | 47 | 139 |
| 7:30 AM | 0 | 1 | 0 | 5 | 0 | 6 | 0 | 0 | 1 | 0 | 2 | 1 | 0 | 5 | 68 | 5 | 2 | 78 | 0 | 0 | 41 | 1 | 1 | 42 | 127 |
| 7:45 AM | 0 | 2 | 0 | 3 | 15 | 5 | 0 | 2 | 1 | 0 | 4 | 3 | 0 | 1 | 52 | 2 | 10 | 55 | 0 | 0 | 34 | 5 | 0 | 39 | 102 |
| 8:00 AM | 0 | 2 | 1 | 4 | 0 | 7 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 3 | 63 | 1 | 0 | 67 | 0 | 1 | 38 | 2 | 0 | 41 | 116 |
| Total | 0 | 7 | 3 | 14 | 28 | 24 | 0 | 3 | 5 | 2 | 6 | 10 | 0 | 22 | 249 | 10 | 22 | 281 | 0 | 1 | 159 | 9 | 1 | 169 | 484 |
| Approach % | 0.0 | 29.2 | 12.5 | 58.3 | - | - | 0.0 | 30.0 | 50.0 | 20.0 | - | - | 0.0 | 7.8 | 88.6 | 3.6 | - | - | 0.0 | 0.6 | 94.1 | 5.3 | - | - | - |
| Total % | 0.0 | 1.4 | 0.6 | 2.9 | - | 5.0 | 0.0 | 0.6 | 1.0 | 0.4 | - | 2.1 | 0.0 | 4.5 | 51.4 | 2.1 | - | 58.1 | 0.0 | 0.2 | 32.9 | 1.9 | - | 34.9 | - |
| PHF | 0.000 | 0.875 | 0.375 | 0.700 | - | 0.857 | 0.000 | 0.375 | 0.625 | 0.250 | - | 0.500 | 0.000 | 0.423 | 0.915 | 0.500 | - | 0.867 | 0.000 | 0.250 | 0.864 | 0.450 | - | 0.899 | 0.871 |
| Lights | 0 | 7 | 3 | 14 | - | 24 | 0 | 3 | 5 | 2 | - | 10 | 0 | 21 | 244 | 10 | - | 275 | 0 | 1 | 158 | 9 | - | 168 | 477 |
| % Lights | - | 100.0 | 100.0 | 100.0 | - | 100.0 | - | 100.0 | 100.0 | 100.0 | - | 100.0 | - | 95.5 | 98.0 | 100.0 | - | 97.9 | - | 100.0 | 99.4 | 100.0 | - | 99.4 | 98.6 |
| Buses | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 4 | 0 | - | 4 | 0 | 0 | 1 | 0 | - | 1 | 5 |
| % Buses | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 1.6 | 0.0 | - | 1.4 | - | 0.0 | 0.6 | 0.0 | - | 0.6 | 1.0 |
| Single-Unit Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 1 | 1 | 0 | - | 2 | 0 | 0 | 0 | 0 | - | 0 | 2 |
| % Single-Unit Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 4.5 | 0.4 | 0.0 | - | 0.7 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.4 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Pedestrians | - | - | - | - | 28 | - | - | - | - | - | 6 | - | - | - | - | - | 22 | - | - | - | - | - | 1 | - | - |
| % Pedestrians | _ | | | | 100.0 | | _ | | - | | 100.0 | • | _ | | • | • | 100.0 | | | | | | 100.0 | | T - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Warren

Avenue
Site Code:
Start Date: 02/14/2019
Page No: 4

| | | | | Avenue | | | | | | Avenue | | | | | - | ton Street | | | | | Washingt South | on Street | | | |
|-------------------------|--------|-------|-------|--------|-------|---------------|--------|-------|-------|--------|-------|---------------|--------|-------|-------|------------|-------|---------------|--------|-------|-------------------|-----------|-------|---------------|------------|
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 5:30 PM | 0 | 4 | 4 | 23 | 9 | 31 | 0 | 5 | 2 | 3 | 10 | 10 | 0 | 7 | 50 | 1 | 34 | 58 | 0 | 0 | 73 | 6 | 1 | 79 | 178 |
| 5:45 PM | 0 | 2 | 0 | 15 | 1 | 17 | 0 | 4 | 0 | 2 | 34 | 6 | 0 | 9 | 29 | 1 | 83 | 39 | 0 | 0 | 39 | 4 | 6 | 43 | 105 |
| 6:00 PM | 0 | 4 | 3 | 25 | 1 | 32 | 0 | 3 | 0 | 0 | 3 | 3 | 0 | 7 | 68 | 1 | 2 | 76 | 0 | 0 | 85 | 6 | 0 | 91 | 202 |
| 6:15 PM | 0 | 6 | 2 | 11 | 16 | 19 | 0 | 1 | 1 | 4 | 3 | 6 | 0 | 13 | 35 | 0 | 9 | 48 | 0 | 1 | 40 | 10 | 1 | 51 | 124 |
| Total | 0 | 16 | 9 | 74 | 27 | 99 | 0 | 13 | 3 | 9 | 50 | 25 | 0 | 36 | 182 | 3 | 128 | 221 | 0 | 1 | 237 | 26 | 8 | 264 | 609 |
| Approach % | 0.0 | 16.2 | 9.1 | 74.7 | - | - | 0.0 | 52.0 | 12.0 | 36.0 | - | - | 0.0 | 16.3 | 82.4 | 1.4 | - | - | 0.0 | 0.4 | 89.8 | 9.8 | - | _ | - |
| Total % | 0.0 | 2.6 | 1.5 | 12.2 | - | 16.3 | 0.0 | 2.1 | 0.5 | 1.5 | - | 4.1 | 0.0 | 5.9 | 29.9 | 0.5 | - | 36.3 | 0.0 | 0.2 | 38.9 | 4.3 | - | 43.3 | - |
| PHF | 0.000 | 0.667 | 0.563 | 0.740 | - | 0.773 | 0.000 | 0.650 | 0.375 | 0.563 | - | 0.625 | 0.000 | 0.692 | 0.669 | 0.750 | - | 0.727 | 0.000 | 0.250 | 0.697 | 0.650 | - | 0.725 | 0.754 |
| Lights | 0 | 16 | 9 | 74 | - | 99 | 0 | 13 | 3 | 8 | - | 24 | 0 | 36 | 181 | 3 | - | 220 | 0 | 1 | 235 | 26 | - | 262 | 605 |
| % Lights | - | 100.0 | 100.0 | 100.0 | _ | 100.0 | - | 100.0 | 100.0 | 88.9 | - | 96.0 | - | 100.0 | 99.5 | 100.0 | | 99.5 | - | 100.0 | 99.2 | 100.0 | - | 99.2 | 99.3 |
| Buses | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Buses | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Single-Unit Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 2 | 0 | - | 2 | 3 |
| % Single-Unit Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.5 | 0.0 | - | 0.5 | - | 0.0 | 0.8 | 0.0 | - | 0.8 | 0.5 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 1 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 1 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 11.1 | - | 4.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.2 |
| Pedestrians | - | - | - | - | 27 | - | - | - | - | - | 50 | - | - | - | - | - | 128 | - | - | - | - | - | 8 | - | - |
| % Pedestrians | - | - | - | _ | 100.0 | _ | - | - | _ | - | 100.0 | _ | - | _ | - | _ | 100.0 | _ | - | - | _ | - | 100.0 | - | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Rogers Street Site Code: Start Date: 02/14/2019 Page No: 1

| Ohart Tirra | | | - | s Street bound | | | | | | s Street bound | J | | | | | ton Street | | | | | | ton Street | | | |
|-------------------------|--------|-------|------|-------------------|------|---------------|--------|------|------|-------------------|------|---------------|--------|------|------|------------|------|---------------|--------|-------|------|------------|------|---------------|------------|
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 7:00 AM | 0 | 4 | . 7 | 3 | 6 | 14 | 0 | 5 | 10 | 4 | 1 | 19 | 0 | 3 | 25 | 4 | 1 | 32 | 0 | 2 | 24 | 2 | 0 | 28 | 93 |
| 7:15 AM | 0 | 1 | 15 | 5 | 6 | 21 | 0 | 7 | 15 | 7 | 1 | 29 | 0 | 10 | 51 | 10 | 2 | 71 | 0 | 6 | 35 | 3 | 3 | 44 | 165 |
| 7:30 AM | 0 | 5 | 9 | 5 | 1 | 19 | 0 | 12 | 23 | 12 | 1 | 47 | 0 | 8 | 56 | 4 | 0 | 68 | 0 | 2 | 26 | 6 | 1 | 34 | 168 |
| 7:45 AM | 0 | 0 | 15 | . 7 | 8 | 22 | 0 | 5 | 22 | 12 | 6 | 39 | 0 | 8 | 39 | 6 | 3 | 53 | 0 | 4 | 36 | 2 | 1 | 42 | 156 |
| Hourly Total | 0 | 10 | 46 | 20 | 21 | 76 | 0 | 29 | 70 | 35 | 9 | 134 | 0 | 29 | 171 | 24 | 6 | 224 | 0 | 14 | 121 | 13 | 5 | 148 | 582 |
| 8:00 AM | 0 | 1 | 8 | 4 | 0 | 13 | 0 | 10 | 18 | 1 | 0 | 29 | 0 | 11 | 43 | 10 | 0 | 64 | 0 | 2 | 21 | 3 | 0 | 26 | 132 |
| 8:15 AM | 0 | 1 | 8 | 2 | 1 | 11 | 0 | 5 | 16 | 12 | 1 | 33 | 0 | 5 | 36 | 8 | 1 | 49 | 0 | 5 | 29 | 2 | 0 | 36 | 129 |
| 8:30 AM | 0 | 1 | 17 | 2 | 1 | 20 | 0 | 9 | 16 | 9 | 0 | 34 | 0 | 6 | 42 | 9 | 0 | 57 | 0 | 1 | 19 | 2 | 0 | 22 | 133 |
| 8:45 AM | 0 | 0 | 7 | 3 | 0 | 10 | 0 | 10 | 17 | 2 | 0 | 29 | 0 | 4 | 20 | 1 | 2 | 25 | 0 | 1 | 28 | 2 | 0 | 31 | 95 |
| Hourly Total | 0 | 3 | 40 | 11 | 2 | 54 | 0 | 34 | 67 | 24 | 1 | 125 | 0 | 26 | 141 | 28 | 3 | 195 | 0 | 9 | 97 | 9 | 0 | 115 | 489 |
| *** BREAK *** | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - | - |
| 4:30 PM | 0 | 2 | 13 | 6 | 1 | 21 | 0 | 12 | 16 | 9 | 0 | 37 | 0 | 3 | 35 | 4 | 2 | 42 | 0 | 6 | 39 | 3 | 0 | 48 | 148 |
| 4:45 PM | 0 | 2 | 14 | 1 | 8 | 17 | 0 | 14 | 18 | 3 | 1 | 35 | 0 | 5 | 47 | 8 | 6 | 60 | 0 | 2 | 44 | 4 | 1 | 50 | 162 |
| Hourly Total | 0 | 4 | 27 | 7 | 9 | 38 | 0 | 26 | 34 | 12 | 1 | 72 | 0 | 8 | 82 | 12 | 8 | 102 | 0 | 8 | 83 | 7 | 1 | 98 | 310 |
| 5:00 PM | 0 | 3 | 14 | 9 | 6 | 26 | 0 | 18 | 16 | 12 | 1 | 46 | 0 | 4 | 43 | 3 | 11 | 50 | 0 | 2 | 58 | 3 | 17 | 63 | 185 |
| 5:15 PM | 0 | 3 | 13 | 9 | 1 | 25 | 0 | 13 | 13 | 8 | 1 | 34 | 0 | 4 | 27 | 5 | 0 | 36 | 0 | 7 | 43 | 1 | 6 | 51 | 146 |
| 5:30 PM | 0 | 3 | 10 | 9 | 6 | 22 | 0 | 13 | 20 | 9 | 3 | 42 | 0 | 2 | 53 | 3 | 3 | 58 | 0 | 5 | 53 | 6 | 6 | 64 | 186 |
| 5:45 PM | 0 | 1 | 12 | 5 | 6 | 18 | 0 | 7 | 13 | 2 | 1 | 22 | 0 | 2 | 28 | 5 | 3 | 35 | 0 | 2 | 41 | 4 | 3 | 47 | 122 |
| Hourly Total | 0 | 10 | 49 | 32 | 19 | 91 | 0 | 51 | 62 | 31 | 6 | 144 | 0 | 12 | 151 | 16 | 17 | 179 | 0 | 16 | 195 | 14 | 32 | 225 | 639 |
| 6:00 PM | 0 | 1 | 17 | 8 | . 1 | 26 | 0 | 10 | 18 | 6 | 0 | 34 | 0 | 10 | 51 | 11 | 0 | 72 | 0 | 3 | 61 | 2 | 1 | 66 | 198 |
| 6:15 PM | 0 | 8 | 19 | 8 | 4 | 35 | 0 | 13 | 14 | 7 | 4 | 34 | 0 | 2 | 36 | 7 | 2 | 45 | 0 | 3 | 34 | 5 | 2 | 42 | 156 |
| Grand Total | 0 | 36 | 198 | 86 | 56 | 320 | 0 | 163 | 265 | 115 | 21 | 543 | 0 | 87 | 632 | 98 | 36 | 817 | 0 | 53 | 591 | 50 | 41 | 694 | 2374 |
| Approach % | 0.0 | 11.3 | 61.9 | 26.9 | - | - | 0.0 | 30.0 | 48.8 | 21.2 | - | - | 0.0 | 10.6 | 77.4 | 12.0 | - | - | 0.0 | 7.6 | 85.2 | 7.2 | - | - | - |
| Total % | 0.0 | 1.5 | 8.3 | 3.6 | - | 13.5 | 0.0 | 6.9 | 11.2 | 4.8 | - | 22.9 | 0.0 | 3.7 | 26.6 | 4.1 | - | 34.4 | 0.0 | 2.2 | 24.9 | 2.1 | - | 29.2 | - |
| Lights | 0 | 36 | 192 | 85 | - | 313 | 0 | 160 | 263 | 111 | - | 534 | 0 | 85 | 626 | 96 | - | 807 | 0 | 53 | 588 | 50 | - | 691 | 2345 |
| % Lights | - | 100.0 | 97.0 | 98.8 | - | 97.8 | - | 98.2 | 99.2 | 96.5 | - | 98.3 | - | 97.7 | 99.1 | 98.0 | - | 98.8 | - | 100.0 | 99.5 | 100.0 | - | 99.6 | 98.8 |
| Buses | 0 | 0 | 1 | 1 | - | 2 | 0 | 0 | 0 | 2 | - | 2 | 0 | 1 | 2 | 1 | - | 4 | 0 | 0 | 1 | 0 | - | 1 | 9 |
| % Buses | - | 0.0 | 0.5 | 1.2 | | 0.6 | - | 0.0 | 0.0 | 1.7 | - | 0.4 | - | 1.1 | 0.3 | 1.0 | - | 0.5 | - | 0.0 | 0.2 | 0.0 | | 0.1 | 0.4 |
| Single-Unit Trucks | 0 | 0 | 3 | 0 | - | 3 | 0 | 2 | 2 | 0 | - | 4 | 0 | 1 | 1 | 1 | - | 3 | 0 | 0 | 0 | 0 | _ | 0 | 10 |
| % Single-Unit Trucks | - | 0.0 | 1.5 | 0.0 | - | 0.9 | - | 1.2 | 0.8 | 0.0 | - | 0.7 | - | 1.1 | 0.2 | 1.0 | - | 0.4 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.4 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 2 | - | 2 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 2 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 1.7 | - | 0.4 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.1 |
| Bicycles on Road | 0 | 0 | 2 | 0 | - | 2 | 0 | 1 | 0 | 0 | - | 1 | 0 | 0 | 3 | 0 | - | 3 | 0 | 0 | 2 | 0 | - | 2 | 8 |
| % Bicycles on Road | - | 0.0 | 1.0 | 0.0 | - | 0.6 | - | 0.6 | 0.0 | 0.0 | - | 0.2 | - | 0.0 | 0.5 | 0.0 | - | 0.4 | - | 0.0 | 0.3 | 0.0 | - | 0.3 | 0.3 |
| Pedestrians | - | - | | | 56 | _ | - | - | _ | | 21 | | - | - | - | | 36 | - | - | - | | | 41 | | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Rogers Street Site Code: Start Date: 02/14/2019 Page No: 3

| | | | Rogers Easth | Street oound | | | | | • | s Street bound | | | | | Washing | ton Street | | | | | Washingt South | | | | |
|-------------------------|--------|-------|-----------------|-----------------|-------|---------------|--------|-------|-------|-------------------|-------|---------------|--------|-------|---------|------------|-------|---------------|--------|-------|-------------------|-------|-------|---------------|------------|
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 7:15 AM | 0 | 1 | 15 | 5 | 6 | 21 | 0 | 7 | 15 | 7 | 1 | 29 | 0 | 10 | 51 | 10 | 2 | 71 | 0 | 6 | 35 | 3 | 3 | 44 | 165 |
| 7:30 AM | 0 | 5 | 9 | 5 | 1 | 19 | 0 | 12 | 23 | 12 | 1 | 47 | 0 | 8 | 56 | 4 | 0 | 68 | 0 | 2 | 26 | 6 | 1 | 34 | 168 |
| 7:45 AM | 0 | 0 | 15 | 7 | 8 | 22 | 0 | 5 | 22 | 12 | 6 | 39 | 0 | 8 | 39 | 6 | 3 | 53 | 0 | 4 | 36 | 2 | 1 | 42 | 156 |
| 8:00 AM | 0 | 1 | 8 | 4 | 0 | 13 | 0 | 10 | 18 | 1 | 0 | 29 | 0 | 11 | 43 | 10 | 0 | 64 | 0 | 2 | 21 | 3 | 0 | 26 | 132 |
| Total | 0 | 7 | 47 | 21 | 15 | 75 | 0 | 34 | 78 | 32 | 8 | 144 | 0 | 37 | 189 | 30 | 5 | 256 | 0 | 14 | 118 | 14 | 5 | 146 | 621 |
| Approach % | 0.0 | 9.3 | 62.7 | 28.0 | - | - | 0.0 | 23.6 | 54.2 | 22.2 | - | - | 0.0 | 14.5 | 73.8 | 11.7 | - | - | 0.0 | 9.6 | 80.8 | 9.6 | - | - | - |
| Total % | 0.0 | 1.1 | 7.6 | 3.4 | - | 12.1 | 0.0 | 5.5 | 12.6 | 5.2 | | 23.2 | 0.0 | 6.0 | 30.4 | 4.8 | _ | 41.2 | 0.0 | 2.3 | 19.0 | 2.3 | - | 23.5 | - |
| PHF | 0.000 | 0.350 | 0.783 | 0.750 | - | 0.852 | 0.000 | 0.708 | 0.848 | 0.667 | - | 0.766 | 0.000 | 0.841 | 0.844 | 0.750 | - | 0.901 | 0.000 | 0.583 | 0.819 | 0.583 | - | 0.830 | 0.924 |
| Lights | 0 | 7 | 45 | 21 | - | 73 | 0 | 34 | 76 | 29 | - | 139 | 0 | 36 | 187 | 28 | - | 251 | 0 | 14 | 116 | 14 | - | 144 | 607 |
| % Lights | - | 100.0 | 95.7 | 100.0 | - | 97.3 | - | 100.0 | 97.4 | 90.6 | - | 96.5 | - | 97.3 | 98.9 | 93.3 | - | 98.0 | - | 100.0 | 98.3 | 100.0 | - | 98.6 | 97.7 |
| Buses | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 0 | 1 | - | 1 | 0 | 1 | 2 | 1 | - | 4 | 0 | 0 | 1 | 0 | - | 1 | 7 |
| % Buses | - | 0.0 | 2.1 | 0.0 | - | 1.3 | - | 0.0 | 0.0 | 3.1 | - | 0.7 | - | 2.7 | 1.1 | 3.3 | - | 1.6 | - | 0.0 | 8.0 | 0.0 | - | 0.7 | 1.1 |
| Single-Unit Trucks | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 2 | 0 | | 2 | 0 | 0 | 0 | 1 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 4 |
| % Single-Unit Trucks | - | 0.0 | 2.1 | 0.0 | - | 1.3 | - | 0.0 | 2.6 | 0.0 | - | 1.4 | - | 0.0 | 0.0 | 3.3 | - | 0.4 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.6 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 2 | - | 2 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 2 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 6.3 | - | 1.4 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.3 |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 1 | 0 | - | 1 | 1 |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.8 | 0.0 | - | 0.7 | 0.2 |
| Pedestrians | - | - | - | - | 15 | _ | - | - | - | - | 8 | - | - | - | - | - | 5 | _ | - | - | - | - | 5 | - | - |
| % Pedestrians | - | - | - | _ | 100.0 | _ | - | - | - | | 100.0 | - | - | - | | - | 100.0 | _ | - | - | _ | - | 100.0 | _ | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Washington Street with Rogers Street Site Code: Start Date: 02/14/2019 Page No: 4

| | | | | | | | | | | | | Jan | | | (0.00 | , | | | | | | | | | i |
|-------------------------|--------|-------|--------|----------|-------|---------------|--------|-------|-------|----------|-------|---------------|--------|-------|---------|------------|-------|---------------|--------|-------|---------|-----------|-------|---------------|------------|
| | | | Rogers | s Street | | | | | Roger | s Street | | | | | Washing | ton Street | | | [| | Washing | on Street | | | |
| | | | Eastl | oound | | | | | West | bound | | | | | North | bound | | | | | South | bound | | | |
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 5:30 PM | 0 | 3 | 10 | 9 | 6 | 22 | 0 | 13 | 20 | 9 | 3 | 42 | 0 | 2 | 53 | 3 | 3 | 58 | 0 | 5 | 53 | 6 | 6 | 64 | 186 |
| 5:45 PM | 0 | 1 | 12 | 5 | 6 | 18 | 0 | 7 | 13 | 2 | 1 | 22 | 0 | 2 | 28 | 5 | 3 | 35 | 0 | 2 | 41 | 4 | 3 | 47 | 122 |
| 6:00 PM | 0 | 1 | 17 | 8 | 1 | 26 | 0 | 10 | 18 | 6 | 0 | 34 | 0 | 10 | 51 | 11 | 0 | 72 | 0 | 3 | 61 | 2 | 1 | 66 | 198 |
| 6:15 PM | 0 | 8 | 19 | 8 | 4 | 35 | 0 | 13 | 14 | 7 | 4 | 34 | 0 | 2 | 36 | 7 | 2 | 45 | 0 | 3 | 34 | 5 | 2 | 42 | 156 |
| Total | 0 | 13 | 58 | 30 | 17 | 101 | 0 | 43 | 65 | 24 | 8 | 132 | 0 | 16 | 168 | 26 | 8 | 210 | 0 | 13 | 189 | 17 | 12 | 219 | 662 |
| Approach % | 0.0 | 12.9 | 57.4 | 29.7 | - | - | 0.0 | 32.6 | 49.2 | 18.2 | - | - | 0.0 | 7.6 | 80.0 | 12.4 | - | - | 0.0 | 5.9 | 86.3 | 7.8 | - | - | - |
| Total % | 0.0 | 2.0 | 8.8 | 4.5 | - | 15.3 | 0.0 | 6.5 | 9.8 | 3.6 | - | 19.9 | 0.0 | 2.4 | 25.4 | 3.9 | - | 31.7 | 0.0 | 2.0 | 28.5 | 2.6 | - | 33.1 | - |
| PHF | 0.000 | 0.406 | 0.763 | 0.833 | - | 0.721 | 0.000 | 0.827 | 0.813 | 0.667 | - | 0.786 | 0.000 | 0.400 | 0.792 | 0.591 | - | 0.729 | 0.000 | 0.650 | 0.775 | 0.708 | - | 0.830 | 0.836 |
| Lights | 0 | 13 | 57 | 30 | - | 100 | 0 | 42 | 65 | 24 | - | 131 | 0 | 16 | 166 | 26 | - | 208 | 0 | 13 | 189 | 17 | - | 219 | 658 |
| % Lights | - | 100.0 | 98.3 | 100.0 | - | 99.0 | - | 97.7 | 100.0 | 100.0 | - | 99.2 | - | 100.0 | 98.8 | 100.0 | - | 99.0 | - | 100.0 | 100.0 | 100.0 | - | 100.0 | 99.4 |
| Buses | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | | 0 | 0 |
| % Buses | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | | 0.0 | 0.0 |
| Single-Unit Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 1 | 0 | 0 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 1 |
| % Single-Unit Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 2.3 | 0.0 | 0.0 | - | 0.8 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.2 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 2 | 0 | - | 2 | 0 | 0 | 0 | 0 | - | 0 | 3 |
| % Bicycles on Road | - | 0.0 | 1.7 | 0.0 | - | 1.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 1.2 | 0.0 | - | 1.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.5 |
| Pedestrians | - | - | - | - | 17 | - | - | - | - | - | 8 | - | - | - | - | - | 8 | - | - | - | - | - | 12 | - | - |
| % Pedestrians | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | - | - | - | - | - | 100.0 | - | - |
| | | | | | | | • | | | | | | | | | | | | | | | | | | |

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Study Name Curtiss Street with Belden Avenue

Start Date Wednesday, February 13, 2019 7:00 AM

End Date Wednesday, February 13, 2019 6:30 PM

Site Code

Report Summary

| | | | | | astbou | nd | | | | | W | 'estbou | nd | | | | | N | orthbo | und | | | | | S | outhbo | und | | | | | Sout | thwestb | ound | | | | | Crossv | walk |
|-------------------|---------------------|----|------|------|--------|------|------|------|-----|------|----------------|---------|-----|------|---------|-----|----------|------|--------|------|------|---------|-----|------|------|--------|------|------|---------|-----|------|------|---------|------|---------|---------|-------|-----|-----------|-------|
| Time Period | Class. | U | L | BL | T | R | ı | 0 | U | L | Т | R | HR | | 0 | U | L | Т | BR | R | I | 0 | U | HL | L | Т | R | ı | 0 | U | HL | BL | BR | HR | ı | 0 | Total | | destria ' | Total |
| Peak 1 | Lights | 0 | 31 | 6 | 11 | 4 | 52 | 14 | 0 | 0 | 7 | 55 | 0 | 62 | 22 | 0 | 3 | 22 | 0 | 1 | 26 | 10 | 0 | 3 | 10 | 5 | 4 | 22 | 108 | 0 | 0 | 1 | 0 | 0 | 1 | 9 | 163 | W | 0 | 0 |
| Specified Period | % | 0% | 100% | 100% | 69% | 80% | 90% | 93% | 0% | 0% | 88% | 96% | 0% | 95% | 79% | 0% | 100% | 96% | 0% | 50% | 93% | 77% | 0% | 100% | 100% | 71% | 100% | 92% | 97% | 0% | 0% | 100% | 0% | 0% | 100% | 100% | 93% | | 0% | |
| 7:15 AM - 8:15 AM | Buses | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 | Ε | 0 | 0 |
| One Hour Peak | % | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 15% | 0% | 0% | 0% | 29% | 0% | 8% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 1% | | 0% | |
| 7:15 AM - 8:15 AM | าgle-Unit Truc | 0 | 0 | 0 | 5 | 1 | 6 | 1 | 0 | 0 | 1 | 2 | 0 | 3 | 6 | 0 | 0 | 1 | 0 | 1 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 11 | S | 2 | 2 |
| | % | 0% | 0% | 0% | 31% | 20% | 10% | 7% | 0% | 0% | 13% | 4% | 0% | 5% | 21% | 0% | 0% | 4% | 0% | 50% | 7% | 8% | 0% | 0% | 0% | 0% | 0% | 0% | 3% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 6% | | 100% | |
| | ticulated Truc | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | N | 4 | 4 |
| | % | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | | 100% | |
| | icycles on Roa | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | NE | 0 | 0 |
| | % | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | | 0% | |
| | Total | 0 | 31 | 6 | 16 | 5 | 58 | 15 | 0 | 0 | 8 | 57 | 0 | 65 | 28 | 0 | 3 | 23 | 0 | 2 | 28 | 13 | 0 | 3 | 10 | 7 | 4 | 24 | 111 | 0 | 0 | 1 | 0 | 0 | 1 | 9 | 176 | | 6 | 6 |
| | PHF | 0 | 0.65 | 0.75 | 0.5 | 0.42 | 0.58 | | 0 | 0 | 0.67 | 0.65 | 0 | 0.68 | 0.54 | 0 | 0.38 | 0.52 | 0 | 0.5 | 0.54 | | 0 | 0.38 | 0.5 | 0.88 | 0.5 | 0.6 | 0.62 | 0 | 0 | 0.25 | 0 | 0 | 0.25 | 0.56 | 0.62 | | | |
| | Approach % | | | | | | 33% | 9% | | | | | | 37% | 16% | | | | | | 16% | 7% | | | | | | 14% | 63% | | | | | | 1% | 5% | | | | |
| | | _ | | _ | | | | | _ | _ | | | _ | | | | _ | | _ | | | | | | | | | | | | | | _ | _ | | _ | | | _ | |
| Peak 2 | Lights | 0 | 11 | 2 | 20 | 16 | 49 | 41 | 0 | 2 | 9 | 17 | 0 | 28 | 62 | 0 | 5 | 10 | 0 | 1 | 16 | 57 | 0 | 0 | 38 | 39 | 20 | 97 | 41 | 0 | 3 | 0 | 7 | 3 | 13 | 2 | 203 | W | 7 | 7 |
| Specified Period | % | 0% | 100% | 100% | 100% | 100% | 100% | | 0% | 100% | 100% | 100% | 0% | 100% | 100% | 0% | 100% | 100% | 0% | 100% | 100% | 100% | 0% | 0% | 100% | 100% | 95% | 99% | 100% | 0% | 100% | 0% | 100% | 100% | 100% | 100% | 100% | _ | 100% | |
| 5:30 PM - 6:30 PM | Buses | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | Ł | 0 | 0 |
| One Hour Peak | % | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | | 0% | - |
| 5:30 PM - 6:30 PM | ngle-Unit Truc | | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 5 | 5 |
| | % ticulated Truc | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% 0 | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% 0 | 0% 0 | 0% | NI. | 100% | 14 |
| | " | 0% | 00/ | 00/ | 00/ | 00/ | | 00/ | 00/ | 00/ | 00/ | 00/ | 00/ | 00/ | _ | | 00/ | 00/ | 00/ | 00/ | | 00/ | 00/ | 00/ | 00/ | 00/ | 00/ | | 00/ | 00/ | 00/ | 00/ | 00/ | 00/ | _ | - | 00/ | IN | 14 | 14 |
| | icycles on Roa | | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% | 0% 0 | 0% | 0% | 0% | 0% | 0% | 0% | 0% O | 0% | 0% | 0% | 0% | 0% | 0% | 0% O | 0% | 0% | 0% | 0% | 0% | 0% 0 | 0% 0 | 0% | NE | 100% | 10 |
| | % | 0% | 00/ | 0% | 0% | 0% | 0% | 2% | 0% | 0% | 00/ | 00/ | 0% | 0% | 0% | 0% | 00/ | 00/ | 00/ | 0% | 0% | 0% | 0% | 00/ | 00/ | 00/ | 5% | 1% | 0% | 0% | 00/ | 00/ | 0% | 0% | 0% | 0% | 0% | INL | 100% | 10 |
| | Total | 0% | 11 | 2 | 20 | 16 | 49 | 42 | 0 | 2 | ο ₀ | 17 | 0% | 28 | 62 | 0,0 | U/8 E | 10 | 0% | 1 | 16 | 57 | 0 | 0% | 38 | 39 | 21 | 98 | 41 | 0% | 2 | 0% | 7 | 3 | 13 | 2 | 204 | | 36 | 36 |
| | PHF | 0 | 0.69 | 0.5 | 0.71 | | 0.82 | | 0 | 0.5 | 0.75 | 0.71 | 0 | 0.88 | 0.78 | 0 | 0.42 | 0.62 | 0 | 0.25 | 0.5 | 0.65 | 0 | 0 | 0.68 | | | 0.91 | 0.93 | 0 | 0.75 | 0 | 0.35 | | 0.46 | 0.5 | 0.85 | | 30 | 30 |
| | Approach % | J | 0.03 | 0.5 | 0.71 | 0.5 | | 21% | | 0.5 | 0.75 | 0.71 | | | 30% | | 0.42 | 0.02 | U | 0.23 | 8% | 28% | | U | 0.00 | 0.75 | 0.00 | | 20% | | 0.75 | U | 0.55 | 0.50 | 6% | 1% | 0.03 | | | |
| | Арргоаст // | | | | | | 24/0 | 21/0 | | | | | | 14/0 | 30% | | | | | | 070 | 2070 | | | | | | 4070 | 2070 | | | | | | 070 | 170 | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |
| | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | | |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Curtiss Street with Mackie Place Site Code: Start Date: 02/13/2019 Page No: 1

| | | | | Street cound | | | | Pu | ıblic Works West | Access Di | rive | | | | | e Place bound | | | | | | s Drive | | | |
|-------------------------|--------|------|------|-----------------|------|---------------|--------|------|---------------------|-----------|------|---------------|--------|------|------|------------------|------|---------------|--------|------|------|---------|------|---------------|------------|
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total |
| 7:00 AM | 0 | 1 | 1 | 2 | 0 | 4 | 0 | 0 | 2 | 0 | 8 | 2 | 0 | 6 | 1 | 0 | 0 | 7 | 0 | 0 | 0 | 3 | 0 | 3 | 16 |
| 7:15 AM | 0 | 1 | 0 | 2 | 0 | 3 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 17 | 0 | 0 | 0 | 17 | 0 | 0 | 0 | 0 | 0 | 0 | 20 |
| 7:30 AM | 0 | 2 | 0 | 3 | 0 | 5 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 18 | 3 | 0 | 0 | 21 | 0 | 0 | 1 | 0 | 0 | 1 | 28 |
| 7:45 AM | 0 | 3 | 2 | 6 | 0 | 11 | 0 | 1 | 0 | 0 | 7 | 1 | 0 | 23 | 3 | 0 | 0 | 26 | 0 | 0 | 1 | 1 | 0 | 2 | 40 |
| Hourly Total | 0 | 7 | 3 | 13 | 0 | 23 | 0 | 1 | 3 | 0 | 22 | 4 | 0 | 64 | 7 | 0 | 0 | 71 | 0 | 0 | 2 | 4 | 0 | 6 | 104 |
| 8:00 AM | 0 | 2 | 0 | 4 | 0 | 6 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 2 | 8 | 1 | 0 | 11 | 0 | 0 | 0 | 2 | 0 | 2 | 20 |
| 8:15 AM | 0 | 2 | 0 | 4 | 0 | 6 | 0 | 1 | . 0 | 0 | 4 | 1 | 0 | 5 | 2 | 1 | 0 | 8 | 0 | 0 | 0 | 1 | 0 | 1 | 16 |
| 8:30 AM | 0 | 2 | 1 | 2 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 5 | 3 | 0 | 0 | 8 | 0 | 0 | 1 | 0 | 0 | 1 | 14 |
| 8:45 AM | 0 | 4 | 2 | 3 | 0 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 4 | 1 | 0 | 0 | 5 | 1 | 0 | 1 | 0 | 0 | 2 | 16 |
| Hourly Total | 0 | 10 | 3 | 13 | 0 | 26 | 0 | 1 | 1 | 0 | 4 | 2 | 0 | 16 | 14 | 2 | 0 | 32 | 1 | 0 | 2 | 3 | 0 | 6 | 66 |
| *** BREAK *** | - | - | - | _ | - | _ | - | - | _ | - | - | _ | - | - | _ | - | - | _ | - | - | _ | _ | - | - | - |
| 4:30 PM | 0 | 1 | 0 | 19 | 0 | 20 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 1 | 0 | 0 | 2 | 1 | 0 | 3 | 24 |
| 4:45 PM | 0 | 1 | 0 | 12 | 0 | 13 | 0 | 0 | . 0 | 0 | . 1 | 0 | 0 | 3 | 0 | 0 | 0 | 3 | 0 | 0 | 2 | 2 | 0 | 4 | 20 |
| Hourly Total | 0 | 2 | 0 | 31 | 0 | 33 | 0 | 0 | 0 | 0 | . 1 | 0 | 0 | 4 | 0 | 0 | 0 | 4 | 0 | 0 | 4 | 3 | 0 | 7 | 44 |
| 5:00 PM | 0 | 1 | 0 | 14 | 0 | 15 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 2 | 0 | 0 | 0 | 2 | 0 | 0 | 6 | 4 | 1 | 10 | 27 |
| 5:15 PM | 1 | 1 | 0 | 8 | 0 | 10 | 0 | 0 | 0 | 0 | 1 | 0 | 0 | 6 | 1 | 0 | 0 | . 7 | 0 | 0 | 3 | 0 | 1 | 3 | 20 |
| 5:30 PM | 0 | 1 | 0 | 10 | 0 | 11 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 8 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 11 | 0 | 19 |
| 5:45 PM | 0 | 5 | 0 | 10 | 0 | 15 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 6 | 1 | 0 | 0 | 7 | 0 | 0 | 3 | 0 | 4 | 3 | 25 |
| Hourly Total | 1 | 8 | 0 | 42 | 0 | 51 | 0 | 0 | 0 | 0 | 16 | 0 | 0 | 22 | 2 | 0 | 0 | 24 | 0 | 0 | 12 | 4 | 17 | 16 | 91 |
| 6:00 PM | 0 | 1 | 0 | 14 | 0 | 15 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 6 | 0 | 0 | 2 | 0 | 4 | 2 | 23 |
| 6:15 PM | 0 | 1 | 0 | 21 | 0 | 22 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 6 | 0 | 0 | 0 | 6 | 0 | 0 | 2 | 4 | 7 | 6 | 34 |
| Grand Total | 1 | 29 | 6 | 134 | 0 | 170 | 0 | 2 | 4 | 0 | 56 | 6 | 0 | 118 | 23 | 2 | 0 | 143 | 1 | 0 | 24 | 18 | 28 | 43 | 362 |
| Approach % | 0.6 | 17.1 | 3.5 | 78.8 | - | - | 0.0 | 33.3 | 66.7 | 0.0 | - | - | 0.0 | 82.5 | 16.1 | 1.4 | - | - | 2.3 | 0.0 | 55.8 | 41.9 | - | - | - |
| Total % | 0.3 | 8.0 | 1.7 | 37.0 | - | 47.0 | 0.0 | 0.6 | 1.1 | 0.0 | - | 1.7 | 0.0 | 32.6 | 6.4 | 0.6 | - | 39.5 | 0.3 | 0.0 | 6.6 | 5.0 | - | 11.9 | - |
| Lights | 1 | 25 | . 5 | 131 | - | 162 | 0 | 1 | 4 | 0 | - | . 5 | 0 | 115 | 22 | 2 | - | 139 | 1 | 0 | 21 | 18 | - | 40 | 346 |
| % Lights | 100.0 | 86.2 | 83.3 | 97.8 | - | 95.3 | - | 50.0 | 100.0 | - | - | 83.3 | - | 97.5 | 95.7 | 100.0 | - | 97.2 | 100.0 | - | 87.5 | 100.0 | - | 93.0 | 95.6 |
| Buses | 0 | 0 | 0 | 1 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 1 | 0 | - | 1 | 2 |
| % Buses | 0.0 | 0.0 | 0.0 | 0.7 | - | 0.6 | - | 0.0 | 0.0 | - | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 | - | 4.2 | 0.0 | - | 2.3 | 0.6 |
| Single-Unit Trucks | 0 | 4 | 1 | 1 | - | 6 | 0 | 1 | 0 | 0 | - | 1 | 0 | 2 | 0 | 0 | - | 2 | 0 | 0 | 2 | 0 | - | 2 | 11 |
| % Single-Unit Trucks | 0.0 | 13.8 | 16.7 | 0.7 | - | 3.5 | - | 50.0 | 0.0 | - | - | 16.7 | - | 1.7 | 0.0 | 0.0 | - | 1.4 | 0.0 | - | 8.3 | 0.0 | - | 4.7 | 3.0 |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 1 |
| % Articulated Trucks | 0.0 | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | - | 0.0 | - | 0.0 | 4.3 | 0.0 | - | 0.7 | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.3 |
| Bicycles on Road | 0 | 0 | 0 | 1 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 0 | 1 | 0 | 0 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 2 |
| % Bicycles on Road | 0.0 | 0.0 | 0.0 | 0.7 | - | 0.6 | - | 0.0 | 0.0 | - | - | 0.0 | - | 0.8 | 0.0 | 0.0 | - | 0.7 | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.6 |
| Pedestrians | - | - | | | 0 | _ | | - | _ | | 56 | | - | - | - | - | 0 | _ | | - | _ | - | 28 | - | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Curtiss Street with Mackie Place Site Code: Start Date: 02/13/2019 Page No: 3

| | | | Curtiss | Street | | | Public Works Access Drive | | | | | | | Mackie Place | | | | | | | Access Drive | | | | | | | |
|-------------------------|-----------|-------|---------|--------|------|---------------|---------------------------|-----------|-------|-------|-------|---------------|--------|--------------|-------|-------|------|---------------|--------|------------|--------------|-------|------|---------------|------------|--|--|--|
| | Eastbound | | | | | | | Westbound | | | | | | Northbound | | | | | | Southbound | | | | | | | | |
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total | | | |
| 7:15 AM | 0 | 1 | 0 | 2 | 0 | 3 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 17 | 0 | 0 | 0 | 17 | 0 | 0 | 0 | 0 | 0 | 0 | 20 | | | |
| 7:30 AM | 0 | 2 | 0 | 3 | 0 | 5 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 18 | 3 | 0 | 0 | 21 | 0 | 0 | 1 | 0 | 0 | 1 | 28 | | | |
| 7:45 AM | 0 | 3 | 2 | 6 | 0 | 11 | 0 | 1 | 0 | 0 | 7 | 1 | 0 | 23 | 3 | 0 | 0 | 26 | 0 | 0 | 1 | 1 | 0 | 2 | 40 | | | |
| 8:00 AM | 0 | 2 | 0 | 4 | 0 | 6 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 2 | 8 | 1 | 0 | 11 | 0 | 0 | 0 | 2 | 0 | 2 | 20 | | | |
| Total | 0 | 8 | 2 | 15 | 0 | 25 | 0 | 1 | 2 | 0 | 14 | 3 | 0 | 60 | 14 | 1 | 0 | 75 | 0 | 0 | 2 | 3 | 0 | 5 | 108 | | | |
| Approach % | 0.0 | 32.0 | 8.0 | 60.0 | - | - | 0.0 | 33.3 | 66.7 | 0.0 | - | - | 0.0 | 80.0 | 18.7 | 1.3 | - | - | 0.0 | 0.0 | 40.0 | 60.0 | - | - | - | | | |
| Total % | 0.0 | 7.4 | 1.9 | 13.9 | - | 23.1 | 0.0 | 0.9 | 1.9 | 0.0 | - | 2.8 | 0.0 | 55.6 | 13.0 | 0.9 | - | 69.4 | 0.0 | 0.0 | 1.9 | 2.8 | - | 4.6 | - | | | |
| PHF | 0.000 | 0.667 | 0.250 | 0.625 | - | 0.568 | 0.000 | 0.250 | 0.500 | 0.000 | - | 0.750 | 0.000 | 0.652 | 0.438 | 0.250 | - | 0.721 | 0.000 | 0.000 | 0.500 | 0.375 | - | 0.625 | 0.675 | | | |
| Lights | 0 | 6 | 1 | 14 | - | 21 | 0 | 0 | 2 | 0 | - | 2 | 0 | 58 | 13 | 1 | - | 72 | 0 | 0 | 0 | 3 | - | 3 | 98 | | | |
| % Lights | - | 75.0 | 50.0 | 93.3 | - | 84.0 | - | 0.0 | 100.0 | - | - | 66.7 | - | 96.7 | 92.9 | 100.0 | - | 96.0 | - | - | 0.0 | 100.0 | - | 60.0 | 90.7 | | | |
| Buses | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | _ | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | | | |
| % Buses | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | | | |
| Single-Unit Trucks | 0 | 2 | . 1 | 1 | _ | 4 | 0 | 1 | 0 | 0 | - | 1 | 0 | 2 | 0 | 0 | - | 2 | 0 | 0 | 2 | 0 | - | 2 | 9 | | | |
| % Single-Unit Trucks | - | 25.0 | 50.0 | 6.7 | - | 16.0 | - | 100.0 | 0.0 | - | - | 33.3 | - | 3.3 | 0.0 | 0.0 | - | 2.7 | - | - | 100.0 | 0.0 | - | 40.0 | 8.3 | | | |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 0 | 0 | - | 0 | 1 | | | |
| % Articulated Trucks | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | - | 0.0 | - | 0.0 | 7.1 | 0.0 | - | 1.3 | - | - | 0.0 | 0.0 | - | 0.0 | 0.9 | | | |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | | | |
| % Bicycles on Road | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | - | 0.0 | - | 0.0 | 0.0 | 0.0 | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | | | |
| Pedestrians | - | - | - | - | 0 | - | - | - | - | - | 14 | - | - | - | - | - | 0 | - | - | - | - | - | 0 | - | - | | | |
| % Pedestrians | - | - | - | _ | - | - | - | - | - | - | 100.0 | - | - | - | - | - | - | - | - | - | - | - | - | - | - | | | |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Curtiss Street with Mackie Place Site Code: Start Date: 02/13/2019 Page No: 4

| | | | Curtiss | Street | | | Public Works Access Drive | | | | | | | Mackie Place | | | | | | | Access Drive | | | | | | | |
|-------------------------|--------|-------|---------|--------|------|---------------|---------------------------|-------|-------|-------|-------|---------------|------------|--------------|-------|-------|------|---------------|------------|-------|--------------|-------|-------|---------------|------------|--|--|--|
| | | | East | oound | | | Westbound | | | | | | Northbound | | | | | | Southbound | | | | | | | | | |
| Start Time | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | U-Turn | Left | Thru | Right | Peds | App. Total | Int. Total | | | |
| 5:30 PM | 0 | 1 | 0 | 10 | 0 | 11 | 0 | 0 | 0 | 0 | 10 | 0 | 0 | 8 | 0 | 0 | 0 | 8 | 0 | 0 | 0 | 0 | 11 | 0 | 19 | | | |
| 5:45 PM | 0 | 5 | 0 | 10 | 0 | 15 | 0 | 0 | 0 | 0 | 4 | 0 | 0 | 6 | 1 | 0 | 0 | 7 | 0 | 0 | 3 | 0 | 4 | 3 | 25 | | | |
| 6:00 PM | 0 | 1 | 0 | 14 | 0 | 15 | 0 | 0 | 0 | 0 | 6 | 0 | 0 | 6 | 0 | 0 | 0 | 6 | 0 | 0 | 2 | 0 | 4 | 2 | 23 | | | |
| 6:15 PM | 0 | 1 | 0 | 21 | 0 | 22 | 0 | 0 | 0 | 0 | 7 | 0 | 0 | 6 | 0 | 0 | 0 | 6 | 0 | 0 | 2 | 4 | 7 | 6 | 34 | | | |
| Total | 0 | 8 | 0 | 55 | 0 | 63 | 0 | 0 | 0 | 0 | 27 | 0 | 0 | 26 | 1 | 0 | 0 | 27 | 0 | 0 | 7 | 4 | 26 | 11 | 101 | | | |
| Approach % | 0.0 | 12.7 | 0.0 | 87.3 | - | - | 0.0 | 0.0 | 0.0 | 0.0 | - | - | 0.0 | 96.3 | 3.7 | 0.0 | - | - | 0.0 | 0.0 | 63.6 | 36.4 | - | - | - | | | |
| Total % | 0.0 | 7.9 | 0.0 | 54.5 | - | 62.4 | 0.0 | 0.0 | 0.0 | 0.0 | - | 0.0 | 0.0 | 25.7 | 1.0 | 0.0 | - | 26.7 | 0.0 | 0.0 | 6.9 | 4.0 | - | 10.9 | - | | | |
| PHF | 0.000 | 0.400 | 0.000 | 0.655 | - | 0.716 | 0.000 | 0.000 | 0.000 | 0.000 | - | 0.000 | 0.000 | 0.813 | 0.250 | 0.000 | - | 0.844 | 0.000 | 0.000 | 0.583 | 0.250 | - | 0.458 | 0.743 | | | |
| Lights | 0 | 8 | 0 | 55 | - | 63 | 0 | 0 | 0 | 0 | - | 0 | 0 | 26 | 1 | 0 | - | 27 | 0 | 0 | 7 | 4 | - | 11 | 101 | | | |
| % Lights | - | 100.0 | _ | 100.0 | - | 100.0 | - | - | - | _ | - | - | - | 100.0 | 100.0 | - | - | 100.0 | - | - | 100.0 | 100.0 | - | 100.0 | 100.0 | | | |
| Buses | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | | | |
| % Buses | - | 0.0 | - | 0.0 | - | 0.0 | - | - | - | - | - | - | - | 0.0 | 0.0 | - | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | | | |
| Single-Unit Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | | | |
| % Single-Unit Trucks | - | 0.0 | - | 0.0 | - | 0.0 | - | - | - | - | - | - | - | 0.0 | 0.0 | - | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | | | |
| Articulated Trucks | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | | | |
| % Articulated Trucks | - | 0.0 | - | 0.0 | - | 0.0 | - | - | - | - | - | - | - | 0.0 | 0.0 | - | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | | | |
| Bicycles on Road | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | 0 | - | 0 | 0 | | | |
| % Bicycles on Road | - | 0.0 | - | 0.0 | - | 0.0 | - | - | - | - | - | - | - | 0.0 | 0.0 | - | - | 0.0 | - | - | 0.0 | 0.0 | - | 0.0 | 0.0 | | | |
| Pedestrians | - | - | _ | - | 0 | - | - | - | - | _ | 27 | - | - | - | - | - | 0 | - | - | - | _ | - | 26 | - | - | | | |
| % Pedestrians | - | - | - | - | - | - | - | - | - | - | 100.0 | - | - | - | - | - | - | - | - | - | - | - | 100.0 | - | - | | | |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Curtiss Street with Metra Access Site Code: Start Date: 02/13/2019 Page No: 1

| Obsert Times | | | Curtiss Street Eastbound | | | | 9 | Curtiss Street Westbound | | | | | | | | |
|----------------------|--------|-------|-----------------------------|-------|------------|--------|------|-----------------------------|-------|------------|--------|-------|-------|-------|------------|------------|
| Start Time | U-Turn | Left | Thru | Peds | App. Total | U-Turn | Thru | Right | Peds | App. Total | U-Turn | Left | Right | Peds | App. Total | Int. Total |
| 7:00 AM | 0 | 1 | 6 | 0 | 7 | 0 | 5 | 1 | 0 | 6 | 0 | 0 | 0 | 2 | 0 | 13 |
| 7:15 AM | 0 | 4 | 11 | 0 | 15 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 17 |
| 7:30 AM | 0 | 3 | 14 | 0 | 17 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 22 |
| 7:45 AM | 0 | 13 | 26 | 0 | 39 | 0 | 6 | 2 | 0 | 8 | 0 | 1 | 2 | 0 | 3 | 50 |
| Hourly Total | 0 | 21 | 57 | 0 | 78 | 0 | 18 | 3 | 0 | 21 | 0 | 1 | 2 | 2 | 3 | 102 |
| 8:00 AM | 0 | 2 | 9 | 1 | 11 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 12 |
| 8:15 AM | 1 | . 5 | 9 | 0 | 15 | 0 | 3 | 0 | 0 | 3 | 0 | 0 | 2 | 0 | 2 | 20 |
| 8:30 AM | 0 | 0 | 7 | 0 | 7 | 0 | 2 | 1 | 0 | 3 | 0 | 0 | 0 | 1 | 0 | 10 |
| 8:45 AM | 0 | 1 | 11 | 0 | 12 | 0 | 1 | 1 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 14 |
| Hourly Total | 1 | . 8 | 36 | 1 | 45 | 0 | 7 | 2 | 0 | 9 | 0 | 0 | 2 | 1 | 2 | 56 |
| *** BREAK *** | - | | | - | - | - | - | | - | | - | - | | - | - | - |
| 4:30 PM | 0 | 0 | 4 | 0 | 4 | 0 | 5 | 0 | 0 | 5 | 0 | 2 | 4 | 0 | 6 | 15 |
| 4:45 PM | 0 | 0 | 14 | 0 | 14 | 0 | 4 | 0 | 0 | 4 | 0 | 1 | 1 | 0 | 2 | 20 |
| Hourly Total | 0 | 0 | 18 | 0 | 18 | 0 | 9 | 0 | 0 | 9 | 0 | 3 | 5 | 0 | 8 | 35 |
| 5:00 PM | 0 | 1 | 10 | 0 | 11 | 0 | 15 | 0 | 0 | 15 | 0 | 0 | 1 | 1 | 1 | 27 |
| 5:15 PM | 0 | . 2 | 14 | 0 | 16 | 0 | 7 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 0 | 23 |
| 5:30 PM | 1 | 1 | . 7 | 0 | 9 | 0 | 6 | 2 | 1 | 8 | 0 | 3 | 6 | 3 | 9 | 26 |
| 5:45 PM | 0 | 2 | 16 | 1 | 18 | 0 | 9 | 0 | 0 | 9 | 0 | 4 | 6 | 0 | 10 | 37 |
| Hourly Total | 1 | 6 | 47 | 1 | 54 | 0 | 37 | 2 | 1 | 39 | 0 | 7 | 13 | 4 | 20 | 113 |
| 6:00 PM | 0 | 1 | 9 | 0 | 10 | 0 | 12 | 2 | 0 | 14 | 0 | 2 | 3 | 1 | 5 | 29 |
| 6:15 PM | 0 | 1 | 9 | 1 | 10 | 0 | 13 | 0 | 0 | 13 | 0 | 3 | 10 | 0 | 13 | 36 |
| Grand Total | 2 | 37 | 176 | 3 | 215 | 0 | 96 | 9 | 1 | 105 | 0 | 16 | 35 | 8 | 51 | 371 |
| Approach % | 0.9 | 17.2 | 81.9 | - | - | 0.0 | 91.4 | 8.6 | - | - | 0.0 | 31.4 | 68.6 | - | - | - |
| Total % | 0.5 | 10.0 | 47.4 | - | 58.0 | 0.0 | 25.9 | 2.4 | - | 28.3 | 0.0 | 4.3 | 9.4 | - | 13.7 | - |
| Lights | 2 | 37 | 170 | - | 209 | 0 | 92 | 8 | - | 100 | 0 | 16 | 34 | - | 50 | 359 |
| % Lights | 100.0 | 100.0 | 96.6 | - | 97.2 | - | 95.8 | 88.9 | - | 95.2 | - | 100.0 | 97.1 | - | 98.0 | 96.8 |
| Buses | 0 | 0 | 0 | - | 0 | 0 | 1 | 0 | - | 1 | 0 | 0 | 0 | - | 0 | 1 |
| % Buses | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 1.0 | 0.0 | - | 1.0 | - | 0.0 | 0.0 | - | 0.0 | 0.3 |
| Single-Unit Trucks | 0 | 0 | 6 | - | 6 | 0 | 3 | 1 | - | 4 | 0 | 0 | 1 | - | . 1 | 11 |
| % Single-Unit Trucks | 0.0 | 0.0 | 3.4 | - | 2.8 | - | 3.1 | 11.1 | - | 3.8 | - | 0.0 | 2.9 | - | 2.0 | 3.0 |
| Articulated Trucks | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Bicycles on Road | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Pedestrians | - | - | - | 3 | - | - | - | - | 1 | - | - | - | - | 8 | - | - |
| % Pedestrians | - | - | - | 100.0 | - | - | - | | 100.0 | - | - | - | | 100.0 | - | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Curtiss Street with Metra Access Site Code: Start Date: 02/13/2019 Page No: 2

Turning Movement Peak Hour Data (7:15 AM)

| | | | | | runni | i i i i i i i i i i i i i i i i i i i | HELLI L E | ak Houi | Dala (1 | \cdot 13 \triangle \cdot \cdot \cdot \cdot | | | | | | |
|----------------------|--------|-------|----------------|-------|------------|---------------------------------------|-----------|----------------|---------|--|------------|-------|--------------|------|------------|------------|
| | | | Curtiss Street | | | | | Curtiss Street | | | | | Metra Access | | | |
| O T: | | | Eastbound | | | | Westbound | | | | Southbound | | | | | |
| Start Time | U-Turn | Left | Thru | Peds | App. Total | U-Turn | Thru | Right | Peds | App. Total | U-Turn | Left | Right | Peds | App. Total | Int. Total |
| 7:15 AM | 0 | 4 | 11 | 0 | 15 | 0 | 2 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 17 |
| 7:30 AM | 0 | 3 | 14 | 0 | 17 | 0 | 5 | 0 | 0 | 5 | 0 | 0 | 0 | 0 | 0 | 22 |
| 7:45 AM | 0 | 13 | 26 | 0 | 39 | 0 | 6 | 2 | 0 | 8 | 0 | 1 | 2 | 0 | 3 | 50 |
| 8:00 AM | 0 | 2 | 9 | 1 | 11 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 12 |
| Total | 0 | 22 | 60 | 1 | 82 | 0 | 14 | 2 | 0 | 16 | 0 | 1 | 2 | 0 | 3 | 101 |
| Approach % | 0.0 | 26.8 | 73.2 | - | - | 0.0 | 87.5 | 12.5 | - | - | 0.0 | 33.3 | 66.7 | - | - | - |
| Total % | 0.0 | 21.8 | 59.4 | - | 81.2 | 0.0 | 13.9 | 2.0 | - | 15.8 | 0.0 | 1.0 | 2.0 | - | 3.0 | - |
| PHF | 0.000 | 0.423 | 0.577 | - | 0.526 | 0.000 | 0.583 | 0.250 | - | 0.500 | 0.000 | 0.250 | 0.250 | - | 0.250 | 0.505 |
| Lights | 0 | 22 | 55 | - | 77 | 0 | 13 | 2 | - | 15 | 0 | 1 | 1 | - | 2 | 94 |
| % Lights | - | 100.0 | 91.7 | | 93.9 | - | 92.9 | 100.0 | - | 93.8 | - | 100.0 | 50.0 | - | 66.7 | 93.1 |
| Buses | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Buses | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Single-Unit Trucks | 0 | 0 | 5 | | 5 | 0 | . 1 | 0 | - | . 1 | 0 | 0 | . 1 | - | 1 | 7 |
| % Single-Unit Trucks | - | 0.0 | 8.3 | - | 6.1 | - | 7.1 | 0.0 | - | 6.3 | - | 0.0 | 50.0 | - | 33.3 | 6.9 |
| Articulated Trucks | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | - | 0.0 | 0.0 | | 0.0 | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Bicycles on Road | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Pedestrians | - | - | _ = | 1 | - | - | _ | - | 0 | - | - | - | _ = | 0 | - | - |
| % Pedestrians | - | - | - | 100.0 | - | - | - | - | - | - | - | - | - | - | - | - |

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990

Count Name: Curtiss Street with Metra Access Site Code: Start Date: 02/13/2019 Page No: 3

Turning Movement Peak Hour Data (5:30 PM)

| | | | | | | 9 1110 1011 | 1011111 00 | ait i ioui i | Jaia (0. | 00 1 111) | | | | | | |
|----------------------|--------|-------|----------------|-------|------------|-------------|------------|----------------|----------|------------|--------|-------|--------------|-------|------------|------------|
| | | | Curtiss Street | | | | | Curtiss Street | | | | | Metra Access | | | |
| Ot and Time a | | | Eastbound | | | | | Westbound | | | | | Southbound | | | |
| Start Time | U-Turn | Left | Thru | Peds | App. Total | U-Turn | Thru | Right | Peds | App. Total | U-Turn | Left | Right | Peds | App. Total | Int. Total |
| 5:30 PM | 1 | 1 | 7 | 0 | 9 | 0 | 6 | 2 | 1 | 8 | 0 | 3 | 6 | 3 | 9 | 26 |
| 5:45 PM | 0 | 2 | 16 | 1 | 18 | 0 | 9 | 0 | 0 | 9 | 0 | 4 | 6 | 0 | 10 | 37 |
| 6:00 PM | 0 | 1 | 9 | 0 | 10 | 0 | 12 | 2 | 0 | 14 | 0 | 2 | 3 | 1 | 5 | 29 |
| 6:15 PM | 0 | 1 | 9 | 1 | 10 | 0 | 13 | 0 | 0 | 13 | 0 | 3 | 10 | 0 | 13 | 36 |
| Total | 1 | 5 | 41 | 2 | 47 | 0 | 40 | 4 | 1 | 44 | 0 | 12 | 25 | 4 | 37 | 128 |
| Approach % | 2.1 | 10.6 | 87.2 | - | - | 0.0 | 90.9 | 9.1 | - | - | 0.0 | 32.4 | 67.6 | - | - | - |
| Total % | 0.8 | 3.9 | 32.0 | - | 36.7 | 0.0 | 31.3 | 3.1 | - | 34.4 | 0.0 | 9.4 | 19.5 | - | 28.9 | - |
| PHF | 0.250 | 0.625 | 0.641 | - | 0.653 | 0.000 | 0.769 | 0.500 | - | 0.786 | 0.000 | 0.750 | 0.625 | - | 0.712 | 0.865 |
| Lights | 1 | 5 | 41 | - | 47 | 0 | 40 | 4 | - | 44 | 0 | 12 | 25 | - | 37 | 128 |
| % Lights | 100.0 | 100.0 | 100.0 | _ | 100.0 | - | 100.0 | 100.0 | - | 100.0 | - | 100.0 | 100.0 | - | 100.0 | 100.0 |
| Buses | 0 | 0 | 0 | | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Buses | 0.0 | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Single-Unit Trucks | 0 | 0 | 0 | _ | 0 | 0 | 0 | . 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Single-Unit Trucks | 0.0 | 0.0 | 0.0 | | 0.0 | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Articulated Trucks | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Articulated Trucks | 0.0 | 0.0 | 0.0 | _ | 0.0 | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Bicycles on Road | 0 | 0 | 0 | _ | 0 | 0 | 0 | 0 | - | 0 | 0 | 0 | 0 | - | 0 | 0 |
| % Bicycles on Road | 0.0 | 0.0 | 0.0 | | 0.0 | - | 0.0 | 0.0 | - | 0.0 | - | 0.0 | 0.0 | - | 0.0 | 0.0 |
| Pedestrians | - | - | | 2 | - | - | | | 1 | - | - | - | | 4 | - | - |
| % Pedestrians | - | - | - | 100.0 | - | - | | - | 100.0 | - | | - | _ | 100.0 | - | - |

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Site Plan

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Downers Grove Civic Center

825 Burlington Ave, Downers Grove, IL 60515

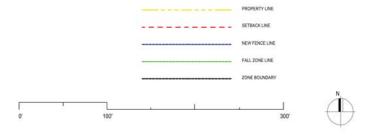


NEW VHPS BUILDING + SCHOOL DISTRICT 58 ADMINISTRATION
NEW CIVIC CENTER BUILDING = 80,000 SF +/- (2 STORY + BASEMENT)

SECURE POLICE STAFF PARKING = 80 SPACES (14 INTERIOR + 24 CARPORT + 42 EXTERIOR)

65 EXTERIOR SPACES 25 EXTERIOR SPACES VILLAGE HALL STAFF PARKING = SD58 STAFF PARKING = PUBLIC VISITOR PARKING = 82 EXTERIOR SPACES

TOTAL PARKING SPACES = 252 SPACES



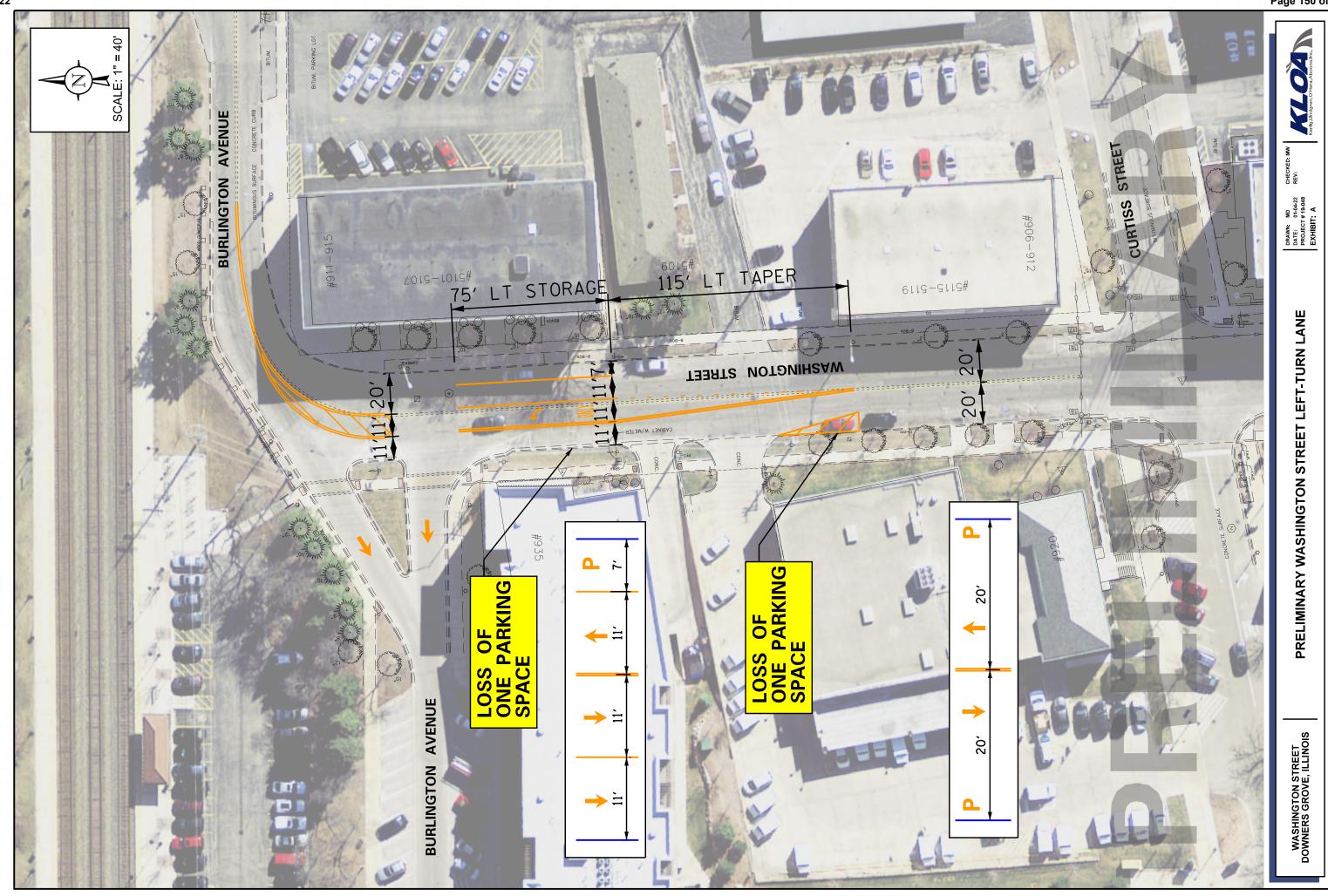






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| Washington Street Geometric Improvements | 2 |
| washington Street Geometrie Improvement | <u>3</u> |
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Level of Service Criteria

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LEVEL OF SERVICE CRITERIA

| ELVEL OF SE | ERVICE CRITERIA Signalized In | itersections | |
|---------------------|---|--|---|
| Level of Service | Interpretation | | Average Control Delay (seconds per vehicle) |
| A | Favorable progression. Most vel green indication and travel through stopping. | _ | ≤10 |
| В | Good progression, with more vel Level of Service A. | hicles stopping than for | >10 - 20 |
| С | Individual cycle failures (i.e., one are not able to depart as a result during the cycle) may begin to appreciately stopping is significant, although through the intersection without st | of insufficient capacity bear. Number of vehicles many vehicles still pass | >20 - 35 |
| D | The volume-to-capacity ratio is high is ineffective or the cycle length is stop and individual cycle failures a | too long. Many vehicles | >35 - 55 |
| Е | Progression is unfavorable. The volume high and the cycle length is long. are frequent. | <u> </u> | >55 - 80 |
| F | The volume-to-capacity ratio is very poor, and the cycle length is clear the queue. | | >80.0 |
| | Unsignalized 1 | Intersections | |
| | Level of Service | Average Total Del | lay (SEC/VEH) |
| | A | 0 - 1 | 0 |
| | В | > 10 - | 15 |
| | С | > 15 - | 25 |
| | D | > 25 - | 35 |
| | E | > 35 - | 50 |
| | F | > 50 |) |
| Source: Highwa | y Capacity Manual, 2010. | | |

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<u>Capacity Analysis Summary Reports</u> Weekday Morning Peak Hour – Existing Conditions

1: Washingotn Street/Washington Street & Maple Avenue

| Intersection Delay, s/veh | 21.1 | | | |
|---------------------------|------|--|--|--|
| Intersection LOS | С | | | |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | 7 | ĵ. | | | 4 | 7 | | 4 | | | ર્ન | 7 |
| Traffic Vol, veh/h | 263 | 386 | 8 | 13 | 132 | 49 | 8 | 223 | 33 | 38 | 41 | 65 |
| Future Vol, veh/h | 263 | 386 | 8 | 13 | 132 | 49 | 8 | 223 | 33 | 38 | 41 | 65 |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.92 |
| Heavy Vehicles, % | 2 | 1 | 0 | 0 | 1 | 2 | 0 | 2 | 0 | 0 | 2 | 0 |
| Mvmt Flow | 289 | 424 | 9 | 14 | 145 | 54 | 9 | 245 | 36 | 42 | 45 | 71 |
| Number of Lanes | 1 | 1 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 1 | 1 |
| Approach | EB | | | WB | | | NB | | | SB | | |
| Opposing Approach | WB | | | EB | | | SB | | | NB | | |
| Opposing Lanes | 2 | | | 2 | | | 2 | | | 1 | | |
| Conflicting Approach Left | SB | | | NB | | | EB | | | WB | | |
| Conflicting Lanes Left | 2 | | | 1 | | | 2 | | | 2 | | |
| Conflicting Approach Right | NB | | | SB | | | WB | | | EB | | |
| Conflicting Lanes Right | 1 | | | 2 | | | 2 | | | 2 | | |
| HCM Control Delay | 25.9 | | | 13 | | | 20.1 | | | 12 | | |
| HCM LOS | D | | | В | | | С | | | В | | |

| Lane | NBLn1 | EBLn1 | EBLn2 | WBLn1 | WBLn2 | SBLn1 | SBLn2 | |
|------------------------|-------|-------|-------|-------|-------|-------|-------|--|
| Vol Left, % | 3% | 100% | 0% | 9% | 0% | 48% | 0% | |
| Vol Thru, % | 84% | 0% | 98% | 91% | 0% | 52% | 0% | |
| Vol Right, % | 12% | 0% | 2% | 0% | 100% | 0% | 100% | |
| Sign Control | Stop | |
| Traffic Vol by Lane | 264 | 263 | 394 | 145 | 49 | 79 | 65 | |
| LT Vol | 8 | 263 | 0 | 13 | 0 | 38 | 0 | |
| Through Vol | 223 | 0 | 386 | 132 | 0 | 41 | 0 | |
| RT Vol | 33 | 0 | 8 | 0 | 49 | 0 | 65 | |
| Lane Flow Rate | 290 | 289 | 433 | 159 | 54 | 87 | 71 | |
| Geometry Grp | 6 | 7 | 7 | 7 | 7 | 7 | 7 | |
| Degree of Util (X) | 0.586 | 0.574 | 0.797 | 0.331 | 0.101 | 0.195 | 0.14 | |
| Departure Headway (Hd) | 7.268 | 7.155 | 6.63 | 7.478 | 6.73 | 8.083 | 7.151 | |
| Convergence, Y/N | Yes | |
| Cap | 499 | 506 | 550 | 481 | 532 | 444 | 501 | |
| Service Time | 5.28 | 4.893 | 4.35 | 5.225 | 4.476 | 5.83 | 4.898 | |
| HCM Lane V/C Ratio | 0.581 | 0.571 | 0.787 | 0.331 | 0.102 | 0.196 | 0.142 | |
| HCM Control Delay | 20.1 | 19.1 | 30.5 | 13.9 | 10.2 | 12.8 | 11.1 | |
| HCM Lane LOS | С | С | D | В | В | В | В | |
| HCM 95th-tile Q | 3.7 | 3.6 | 7.6 | 1.4 | 0.3 | 0.7 | 0.5 | |

2: Washington Street & Curtiss Street

| ntersection | |
|---------------------------|------|
| Intersection Delay, s/veh | 11.8 |
| Intersection LOS | В |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 4 | | | 4 | | | f) | | | ર્ન | |
| Traffic Vol, veh/h | 39 | 23 | 52 | 12 | 0 | 7 | 0 | 306 | 53 | 4 | 98 | 0 |
| Future Vol, veh/h | 39 | 23 | 52 | 12 | 0 | 7 | 0 | 306 | 53 | 4 | 98 | 0 |
| Peak Hour Factor | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 |
| Heavy Vehicles, % | 2 | 16 | 0 | 7 | 2 | 0 | 0 | 3 | 2 | 0 | 1 | 0 |
| Mvmt Flow | 51 | 30 | 68 | 16 | 0 | 9 | 0 | 397 | 69 | 5 | 127 | 0 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB | | | WB | | | | NB | | SB | | |
| Opposing Approach | WB | | | EB | | | | SB | | NB | | |
| Opposing Lanes | 1 | | | 1 | | | | 1 | | 1 | | |
| Conflicting Approach Left | SB | | | NB | | | | EB | | WB | | |
| Conflicting Lanes Left | 1 | | | 1 | | | | 1 | | 1 | | |
| Conflicting Approach Right | NB | | | SB | | | | WB | | EB | | |
| Conflicting Lanes Right | 1 | | | 1 | | | | 1 | | 1 | | |
| HCM Control Delay | 9.6 | | | 8.8 | | | | 13.5 | | 9 | | |
| HCM LOS | Α | | | Α | | | | В | | Α | | |

| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 | |
|------------------------|-------|-------|-------|-------|--|
| Vol Left, % | 0% | 34% | 63% | 4% | |
| Vol Thru, % | 85% | 20% | 0% | 96% | |
| Vol Right, % | 15% | 46% | 37% | 0% | |
| Sign Control | Stop | Stop | Stop | Stop | |
| Traffic Vol by Lane | 359 | 114 | 19 | 102 | |
| LT Vol | 0 | 39 | 12 | 4 | |
| Through Vol | 306 | 23 | 0 | 98 | |
| RT Vol | 53 | 52 | 7 | 0 | |
| Lane Flow Rate | 466 | 148 | 25 | 132 | |
| Geometry Grp | 1 | 1 | 1 | 1 | |
| Degree of Util (X) | 0.579 | 0.211 | 0.038 | 0.179 | |
| Departure Headway (Hd) | 4.469 | 5.12 | 5.518 | 4.876 | |
| Convergence, Y/N | Yes | Yes | Yes | Yes | |
| Cap | 807 | 696 | 643 | 731 | |
| Service Time | 2.515 | 3.189 | 3.605 | 2.94 | |
| HCM Lane V/C Ratio | 0.577 | 0.213 | 0.039 | 0.181 | |
| HCM Control Delay | 13.5 | 9.6 | 8.8 | 9 | |
| HCM Lane LOS | В | Α | Α | А | |
| HCM 95th-tile Q | 3.8 | 0.8 | 0.1 | 0.6 | |

3: Washington Street & Rogers Street

| 9.7 | | | | | |
|-----|---|---|---|---|---|
| А | | | | | |
| | ٨ | ٨ | Λ | Λ | ۸ |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 4 | | | 4 | | 7 | ĵ. | | | 4 | |
| Traffic Vol, veh/h | 7 | 47 | 21 | 34 | 78 | 32 | 37 | 191 | 30 | 14 | 117 | 14 |
| Future Vol, veh/h | 7 | 47 | 21 | 34 | 78 | 32 | 37 | 191 | 30 | 14 | 117 | 14 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, % | 0 | 4 | 0 | 0 | 3 | 9 | 3 | 1 | 7 | 0 | 2 | 0 |
| Mvmt Flow | 8 | 51 | 23 | 37 | 85 | 35 | 40 | 208 | 33 | 15 | 127 | 15 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB | | | WB | | | NB | | | SB | | |
| Opposing Approach | WB | | | EB | | | SB | | | NB | | |
| Opposing Lanes | 1 | | | 1 | | | 1 | | | 2 | | |
| Conflicting Approach Left | SB | | | NB | | | EB | | | WB | | |
| Conflicting Lanes Left | 1 | | | 2 | | | 1 | | | 1 | | |
| Conflicting Approach Right | NB | | | SB | | | WB | | | EB | | |
| Conflicting Lanes Right | 2 | | | 1 | | | 1 | | | 1 | | |
| HCM Control Delay | 8.8 | | | 9.5 | | | 10.4 | | | 9.3 | | |
| HCM LOS | А | | | Α | | | В | | | Α | | |

| Lane | NBLn1 | NBLn2 | EBLn1 | WBLn1 | SBLn1 | |
|------------------------|-------|-------|-------|-------|-------|--|
| Vol Left, % | 100% | 0% | 9% | 24% | 10% | |
| Vol Thru, % | 0% | 86% | 63% | 54% | 81% | |
| Vol Right, % | 0% | 14% | 28% | 22% | 10% | |
| Sign Control | Stop | Stop | Stop | Stop | Stop | |
| Traffic Vol by Lane | 37 | 221 | 75 | 144 | 145 | |
| LT Vol | 37 | 0 | 7 | 34 | 14 | |
| Through Vol | 0 | 191 | 47 | 78 | 117 | |
| RT Vol | 0 | 30 | 21 | 32 | 14 | |
| Lane Flow Rate | 40 | 240 | 82 | 157 | 158 | |
| Geometry Grp | 7 | 7 | 2 | 2 | 5 | |
| Degree of Util (X) | 0.065 | 0.344 | 0.114 | 0.217 | 0.215 | |
| Departure Headway (Hd) | 5.794 | 5.16 | 5.04 | 4.992 | 4.909 | |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | |
| Cap | 615 | 692 | 705 | 714 | 725 | |
| Service Time | 3.559 | 2.925 | 3.117 | 3.059 | 2.982 | |
| HCM Lane V/C Ratio | 0.065 | 0.347 | 0.116 | 0.22 | 0.218 | |
| HCM Control Delay | 9 | 10.6 | 8.8 | 9.5 | 9.3 | |
| HCM Lane LOS | А | В | А | А | А | |
| HCM 95th-tile Q | 0.2 | 1.5 | 0.4 | 8.0 | 0.8 | |

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HCM Unsignalized Intersection Capacity Analysis 4: Washington Street & Village Hall Access

| | • | - | ← | • | \ | 1 |
|--------------------------------|-------------|------|----------|-----------|------------|------------|
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | LDE | 4 | <u>₩</u> | ř | Y | ODIN |
| Traffic Volume (veh/h) | 197 | 7 | 9 | 84 | 45 | 134 |
| Future Volume (Veh/h) | 197 | 7 | 9 | 84 | 45 | 134 |
| Sign Control | .,, | Stop | Stop | | Free | |
| Grade | | 0% | 0% | | 0% | |
| Peak Hour Factor | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 |
| Hourly flow rate (vph) | 240 | 9 | 11 | 102 | 55 | 163 |
| Pedestrians | 2.0 | • | | .02 | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | | None | |
| Median storage veh) | | | | | | |
| Upstream signal (ft) | | | | | 76 | |
| pX, platoon unblocked | | | | | , 0 | |
| vC, conflicting volume | 299 | 192 | 273 | 0 | 0 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 299 | 192 | 273 | 0 | 0 | |
| tC, single (s) | 7.1 | 6.5 | 6.5 | 6.2 | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 4.0 | 4.0 | 3.3 | 2.2 | |
| p0 queue free % | 58 | 99 | 98 | 91 | 97 | |
| cM capacity (veh/h) | 569 | 683 | 616 | 1082 | 1617 | |
| | | | | | | |
| Direction, Lane # Volume Total | EB 1 249 | WB 1 | WB 2 | SB 1 | | |
| | 249 | 11 | 102 | 218 55 | | |
| Volume Left | | 0 | 0 102 | 163 | | |
| Volume Right | 0 | 0 | | | | |
| CSH | 572 | 616 | 1082 | 1617 | | |
| Volume to Capacity | 0.44 | 0.02 | 0.09 | 0.03 | | |
| Queue Length 95th (ft) | 55 | 11.0 | 8 | 3 | | |
| Control Delay (s) | 16.1 | 11.0 | 8.7 | 2.0 | | |
| Lane LOS | C 1/ 1 | В | А | A | | |
| Approach Delay (s) | 16.1 | 8.9 | | 2.0 | | |
| Approach LOS | С | Α | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 9.4 | | | |
| Intersection Capacity Utiliz | zation | | 35.4% | IC | CU Level c | of Service |
| Analysis Period (min) | | | 15 | | | |

HCM 6th TWSC 5: Belden Avenue & Curtiss Street

| Intersection | | | | | | |
|--------------------------------------|--------|--------|--------|------|--------|-------|
| Int Delay, s/veh | 2.1 | | | | | |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | 1> | LUIN | 1100 | 4 | Y | HOR |
| Traffic Vol, veh/h | 26 | 13 | 0 | 65 | 26 | 2 |
| Future Vol, veh/h | 26 | 13 | 0 | 65 | 26 | 2 |
| Conflicting Peds, #/hr | | 3 | 3 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - - | None |
| Storage Length | | - | | - | 0 | NOTIC |
| Veh in Median Storag | ie,# 0 | - | _ | 0 | 0 | - |
| | | | | | | |
| Grade, % | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 63 | 63 | 63 | 63 | 63 | 63 |
| Heavy Vehicles, % | 31 | 20 | 0 | 10 | 25 | 50 |
| Mvmt Flow | 41 | 21 | 0 | 103 | 41 | 3 |
| | | | | | | |
| Major/Minor | Major1 | N | Major2 | | Minor1 | |
| Conflicting Flow All | 0 | 0 | 65 | 0 | 158 | 55 |
| Stage 1 | - | - | - | - | 55 | - |
| Stage 2 | _ | _ | - | _ | 103 | _ |
| | - | - | 4.1 | | 6.65 | 6.7 |
| Critical Hdwy | | | | - | | |
| Critical Hdwy Stg 1 | - | - | - | - | 5.65 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.65 | - |
| Follow-up Hdwy | - | - | 2.2 | - | 3.725 | 3.75 |
| Pot Cap-1 Maneuver | - | - | 1550 | - | 782 | 892 |
| Stage 1 | - | - | - | - | 912 | - |
| Stage 2 | - | - | - | - | 867 | - |
| Platoon blocked, % | - | - | | - | | |
| Mov Cap-1 Maneuver | - | - | 1546 | - | 780 | 889 |
| Mov Cap-2 Maneuver | | - | - | - | 780 | - |
| Stage 1 | - | _ | - | - | 909 | - |
| Stage 2 | _ | _ | _ | _ | 867 | _ |
| Stage 2 | | | | | 007 | |
| | | | | | | |
| Approach | EB | | WB | | NB | |
| HCM Control Delay, s | 0 | | 0 | | 9.8 | |
| HCM LOS | | | | | Α | |
| | | | | | | |
| Minor Long/Major Ma | mt I | UDI ~1 | CDT | EDD | WDI | WDT |
| Minor Lane/Major Mvi | mı ı | VBLn1 | EBT | EBR | WBL | WBT |
| Capacity (veh/h) | | 787 | - | - | 1546 | - |
| HCM Lane V/C Ratio | | 0.056 | - | - | - | - |
| HCM Control Delay (s | s) | 9.8 | - | - | 0 | - |
| LICIAL | | Α | - | - | Α | - |
| HCM Lane LOS | | | | | | |
| HCM Lane LOS HCM 95th %tile Q(vel | h) | 0.2 | - | - | 0 | - |

6: Curtiss Street & Westerly Access Drive

| Intersection | | | | | | |
|------------------------|-------------|-------|--------|------|---------|--------|
| Int Delay, s/veh | 1.9 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | | 4 | 1> | | ₩ | JJII |
| Traffic Vol, veh/h | 22 | 60 | 14 | 2 | 1 | 2 |
| Future Vol, veh/h | 22 | 60 | 14 | 2 | 1 | 2 |
| Conflicting Peds, #/hr | | 0 | 0 | 0 | 0 | 1 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | | - | None | - | None |
| Storage Length | - | - | _ | - | 0 | - |
| Veh in Median Storag | ne.# - | 0 | 0 | _ | 0 | _ |
| Grade, % | JO, 11 - | 0 | 0 | _ | 0 | _ |
| Peak Hour Factor | 52 | 52 | 52 | 52 | 52 | 52 |
| Heavy Vehicles, % | 0 | 9 | 7 | 0 | 0 | 25 |
| Mymt Flow | 42 | 115 | 27 | 4 | 2 | 4 |
| IVIVIIII FIOW | 42 | 115 | 21 | 4 | Z | 4 |
| | | | | | | |
| Major/Minor | Major1 | N | Major2 | N | Minor2 | |
| Conflicting Flow All | 31 | 0 | - | 0 | 228 | 30 |
| Stage 1 | - | - | - | - | 29 | - |
| Stage 2 | - | - | - | - | 199 | - |
| Critical Hdwy | 4.1 | - | _ | _ | 6.4 | 6.45 |
| Critical Hdwy Stg 1 | - | _ | _ | _ | 5.4 | - |
| Critical Hdwy Stg 2 | _ | | _ | _ | 5.4 | _ |
| Follow-up Hdwy | 2.2 | - | - | - | | 3.525 |
| | | | | | 765 | |
| Pot Cap-1 Maneuver | | - | - | - | | 982 |
| Stage 1 | - | - | - | - | 999 | - |
| Stage 2 | - | - | - | - | 839 | - |
| Platoon blocked, % | | - | - | - | | |
| Mov Cap-1 Maneuver | | - | - | - | 744 | 981 |
| Mov Cap-2 Maneuver | r - | - | - | - | 744 | - |
| Stage 1 | - | - | - | - | 971 | - |
| Stage 2 | - | - | - | - | 839 | - |
| Ü | | | | | | |
| A | ED | | MD | | CD | |
| Approach | EB | | WB | | SB | |
| HCM Control Delay, s | s 2 | | 0 | | 9.1 | |
| HCM LOS | | | | | Α | |
| | | | | | | |
| Minor Lane/Major Mv | mt | EBL | EBT | WBT | WBR S | SRI n1 |
| Capacity (veh/h) | 1110 | | LUI | VVDI | VVDIX . | |
| | | 1595 | - | - | - | 887 |
| HCM Lane V/C Ratio | | 0.027 | - | - | | 0.007 |
| HCM Control Delay (s | 5) | 7.3 | 0 | - | - | 9.1 |
| HCM Lane LOS | | Α | Α | - | - | Α |
| HCM 95th %tile Q(ve | h) | 0.1 | - | - | - | 0 |
| | | | | | | |

7: Curtiss Street & VIIIage Hall Access

| Intersection | | | | | | |
|------------------------|---------|-------|----------|------|--------|--------|
| Int Delay, s/veh | 2.9 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | | 4 | f | | W | |
| Traffic Vol, veh/h | 31 | 18 | 11 | 80 | 20 | 4 |
| Future Vol, veh/h | 31 | 18 | 11 | 80 | 20 | 4 |
| Conflicting Peds, #/hr | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | | - | None |
| Storage Length | - | _ | - | - | 0 | - |
| Veh in Median Storage, | # - | 0 | 0 | _ | 0 | _ |
| Grade, % | - | 0 | 0 | _ | 0 | _ |
| Peak Hour Factor | 63 | 63 | 63 | 63 | 63 | 63 |
| Heavy Vehicles, % | 0 | 31 | 10 | 5 | 0 | 0 |
| Mvmt Flow | 49 | 29 | 17 | 127 | 32 | 6 |
| IVIVIIIL I IOVV | 47 | 21 | 17 | 127 | JZ | U |
| | | | | | | |
| | /lajor1 | N | Major2 | N | Minor2 | |
| Conflicting Flow All | 144 | 0 | - | 0 | 208 | 81 |
| Stage 1 | - | - | - | - | 81 | - |
| Stage 2 | - | - | - | - | 127 | - |
| Critical Hdwy | 4.1 | - | - | - | 6.4 | 6.2 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.4 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.4 | - |
| Follow-up Hdwy | 2.2 | - | - | - | 3.5 | 3.3 |
| Pot Cap-1 Maneuver | 1451 | - | - | - | 785 | 985 |
| Stage 1 | - | - | - | - | 947 | - |
| Stage 2 | - | _ | - | _ | 904 | _ |
| Platoon blocked, % | | _ | _ | _ | 701 | |
| Mov Cap-1 Maneuver | 1451 | _ | _ | _ | 758 | 985 |
| Mov Cap-1 Maneuver | - | _ | _ | _ | 758 | 703 |
| Stage 1 | - | | - | | 915 | |
| | | - | - | - | | |
| Stage 2 | - | - | - | - | 904 | - |
| | | | | | | |
| Approach | EB | | WB | | SB | |
| HCM Control Delay, s | 4.8 | | 0 | | 9.8 | |
| HCM LOS | | | | | Α | |
| | | | | | | |
| Minor Lang/Major Mumi | | EBL | EBT | WBT | WBR : | CDI n1 |
| Minor Lane/Major Mymi | l | | | WDI | | |
| Capacity (veh/h) | | 1451 | - | - | - | |
| HCM Lane V/C Ratio | | 0.034 | - | - | | 0.048 |
| HCM Control Delay (s) | | 7.6 | 0 | - | - | 7.0 |
| HCM Lane LOS | | Α | Α | - | - | Α |
| HCM 95th %tile Q(veh) | | 0.1 | - | - | - | 0.2 |
| | | | | | | |

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8: Curtiss Street & Police Department Access

| Int Delay, s/veh Movement Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/r Sign Control RT Channelized Storage Length Veh in Median Stora Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | 9 9 or 0 Free - | EBT 29 29 0 Free None 0 63 31 46 | WBT 91 91 0 Free - 0 0 63 10 144 Major2 | 0 | SBL 1 1 0 Stop 0 0 63 0 2 Winor2 218 144 | SBR 0 0 0 Stop None 63 0 0 |
|---|--|--|---|--|--|---|
| Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/r Sign Control RT Channelized Storage Length Veh in Median Stora Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuve Stage 1 | 9 9 9 rr 0 Free - - ge, # - - 63 0 14 Major1 144 - | 29 29 0 Free None - 0 0 63 31 46 | 91 91 0 Free - 0 0 63 10 144 Major2 | 0 0 0 Free None - - - 63 0 0 | 1 1 0 Stop - 0 0 0 63 0 2 Viinor2 | 0 0 0 Stop None - - - 63 0 |
| Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/r Sign Control RT Channelized Storage Length Veh in Median Stora Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuve Stage 1 | 9 9 9 rr 0 Free - - ge, # - - 63 0 14 Major1 144 - | 29 29 0 Free None - 0 0 63 31 46 | 91 91 0 Free - 0 0 63 10 144 Major2 | 0 0 0 Free None - - - 63 0 0 | 1 1 0 Stop - 0 0 0 63 0 2 Viinor2 | 0 0 0 Stop None - - - 63 0 |
| Traffic Vol, veh/h Future Vol, veh/h Future Vol, veh/h Conflicting Peds, #/h Sign Control RT Channelized Storage Length Veh in Median Stora Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | 9 9 9 9 9 1 | 29 29 0 Free None - 0 0 63 31 46 | 91 91 0 Free - 0 0 63 10 144 Major2 | 0 0 Free None - - 63 0 0 | 1 1 0 Stop 0 0 0 63 0 2 Minor2 | 0 0 Stop None - - - 63 0 |
| Future Vol, veh/h Conflicting Peds, #/h Sign Control RT Channelized Storage Length Veh in Median Stora Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | 9 nr 0 Free 63 0 14 Major1 | 29 0 Free None - 0 0 63 31 46 | 91 0 Free - 0 0 63 10 144 Major2 | 0 0 Free None - - 63 0 0 | 1 0 Stop 0 0 0 63 0 2 Minor2 | 0 0 Stop None - - - 63 0 |
| Conflicting Peds, #/r Sign Control RT Channelized Storage Length Veh in Median Stora Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | err 0 Free - ge, # - 63 0 14 Major1 144 - | 0 Free None - 0 0 63 31 46 | 0 Free - 0 0 63 10 144 Major2 | 0 Free None - - - 63 0 0 | 0 Stop 0 0 0 63 0 2 Minor2 | O Stop None - - - 63 0 |
| Sign Control RT Channelized Storage Length Veh in Median Stora Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuve Stage 1 | Free ge, # 63 0 14 Major1 144 | Free None - 0 0 0 63 31 46 M 0 | Free 0 0 63 10 144 Major2 | Free None 63 0 0 0 No | Stop | Stop None - - - 63 0 |
| RT Channelized Storage Length Veh in Median Stora Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuve Stage 1 | - ge, # - 63 0 14 Major1 144 - | None - 0 0 63 31 46 | - 0 0 63 10 144 Major2 | None 63 0 0 | 0 0 0 63 0 2 Minor2 | None - - - 63 0 |
| Storage Length Veh in Median Stora Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuve Stage 1 | ge, # - 63 0 14 Major1 144 - | 0 0 63 31 46 | 0 0 63 10 144 Major2 | - - 63 0 0 | 0 0 0 63 0 2 Minor2 218 | 63 0 |
| Veh in Median Stora Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuve Stage 1 | ge, # - 63 0 14 Major1 144 | 0 0 63 31 46 N 0 | 0 0 63 10 144 Major2 | - 63 0 0 | 0 0 63 0 2 Minor2 | 63 0 0 |
| Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | 63 0 14 Major1 144 | 0 63 31 46 0 - | 0 63 10 144 Major2 | 63 0 0 | 0 63 0 2 Minor2 218 | 63 0 0 |
| Grade, % Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | 63 0 14 Major1 144 | 63 31 46 N 0 | 63 10 144 Major2 | 63 0 0 | 63 0 2 Minor2 218 | 63 0 0 |
| Peak Hour Factor Heavy Vehicles, % Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | 0 14 Major1 144 | 31 46 N 0 | 10 144 Major2 - | 0 0 N 0 | 0 2 Minor2 218 | 0 |
| Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | 0 14 Major1 144 | 31 46 N 0 | 10 144 Major2 - | 0 0 N 0 | 0 2 Minor2 218 | 0 |
| Mvmt Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuve Stage 1 | 14 Major1 144 - | 46 0 - | 144 <u>Major2</u> - - | 0 N 0 | 2 <u>Minor2</u> 218 | 0 |
| Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuve Stage 1 | Major1 144 - | 0 - - | Major2 - - | 0 - | <u>Minor2</u> 218 | |
| Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | 144 - - | 0 - - | - | 0 | 218 | 144 |
| Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | 144 - - | 0 - - | - | 0 | 218 | 144 |
| Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | 144 - - | - | - | - | | 144 |
| Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | - | - | | | | |
| Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | | - | - | | | - |
| Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | 4.1 | _ | | - | 74 | - |
| Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuve Stage 1 | 1.1 | | _ | _ | 6.4 | 6.2 |
| Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | _ | | _ | _ | 5.4 | - 0.2 |
| Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 | | - | | | 5.4 | |
| Pot Cap-1 Maneuver Stage 1 | - | - | - | - | | - |
| Stage 1 | 2.2 | - | - | - | 3.5 | 3.3 |
| | r 1451 | - | - | - | 775 | 909 |
| Ctogo 2 | - | - | - | - | 888 | - |
| Stage 2 | - | - | - | - | 954 | - |
| Platoon blocked, % | | - | - | - | | |
| Mov Cap-1 Maneuve | er 1451 | - | - | - | 767 | 909 |
| Mov Cap-2 Maneuve | | _ | _ | _ | 767 | _ |
| Stage 1 | _ | _ | _ | _ | 879 | _ |
| Stage 2 | _ | _ | _ | _ | 954 | _ |
| Stage 2 | | - | | - | 734 | - |
| | | | | | | |
| Approach | EB | | WB | | SB | |
| HCM Control Delay, | s 1.8 | | 0 | | 9.7 | |
| HCM LOS | | | Ū | | Α | |
| TIOW EOO | | | | | , · · | |
| | | | | | | |
| Minor Lane/Major M | vmt | EBL | EBT | WBT | WBR | SBLn1 |
| Capacity (veh/h) | | 1451 | - | - | - | 767 |
| HCM Lane V/C Ratio |) | 0.01 | - | - | - | 0.002 |
| HCM Control Delay | | 7.5 | 0 | - | - | |
| HCM Lane LOS | V-1 | A | A | _ | _ | A |
| HCM 95th %tile Q(ve | | 0 | - | _ | _ | 0 |
| 113W 73W 70W Q(W | -h) | - 0 | | | | |

9: Apartment Drive & Washinton Street/Washington Street

| Intersection | | | | | | |
|---|--------|-------|--------|-----------|---------------|--------|
| Int Delay, s/veh | 0.3 | | | | | |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | ₽ | LDIN | VVDL | <u>₩Ы</u> | ₩. | ווטוז |
| Traffic Vol, veh/h | 202 | 2 | 6 | 137 | 4 | 2 |
| Future Vol, veh/h | 202 | 2 | 6 | 137 | 4 | 2 |
| Conflicting Peds, #/hr | 0 | 7 | 7 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | | - Jiop | None |
| Storage Length | _ | - | _ | - | 0 | - |
| Veh in Median Storage | | - | _ | 0 | 0 | |
| Grade, % | , # 0 | - | - | 0 | 0 | - |
| | 82 | | 82 | 82 | 82 | 82 |
| Peak Hour Factor | | 82 | | | | |
| Heavy Vehicles, % | 3 | 0 | 0 | 2 | 15 | 0 |
| Mvmt Flow | 246 | 2 | 7 | 167 | 5 | 2 |
| | | | | | | |
| Major/Minor N | Major1 | N | Major2 | N | Minor1 | |
| Conflicting Flow All | 0 | 0 | 255 | 0 | 435 | 254 |
| Stage 1 | - | _ | - | _ | 254 | _ |
| Stage 2 | _ | _ | _ | - | 181 | _ |
| Critical Hdwy | _ | _ | 4.1 | _ | 6.55 | 6.2 |
| Critical Hdwy Stg 1 | _ | _ | | _ | 5.55 | - |
| Critical Hdwy Stg 2 | _ | _ | _ | - | 5.55 | _ |
| Follow-up Hdwy | _ | - | 2.2 | _ | 3.635 | 3.3 |
| Pot Cap-1 Maneuver | | | 1322 | - | 555 | 790 |
| | - | - | | | 759 | |
| Stage 1 | - | - | - | - | | - |
| Stage 2 | - | - | - | - | 820 | - |
| Platoon blocked, % | - | - | 1010 | - | = | |
| Mov Cap-1 Maneuver | - | - | 1313 | - | 548 | 785 |
| Mov Cap-2 Maneuver | - | - | - | - | 548 | - |
| Stage 1 | - | - | - | - | 754 | - |
| Stage 2 | - | - | - | - | 815 | - |
| | | | | | | |
| Approach | EB | | WB | | NB | |
| | | | 0.3 | | 11 | |
| HCM Control Delay, s | 0 | | 0.3 | | | |
| HCM LOS | | | | | В | |
| | | | | | | |
| Minor Lane/Major Mvm | t I | NBLn1 | EBT | EBR | WBL | WBT |
| Capacity (veh/h) | | 609 | - | - | 1313 | - |
| | | 0.012 | _ | | 0.006 | _ |
| | | | | | | |
| HCM Lane V/C Ratio | | | _ | - | 7.8 | - () |
| HCM Lane V/C Ratio HCM Control Delay (s) | | 11 | | | 7.8 A | 0 A |
| HCM Lane V/C Ratio | | | - | - | 7.8 A 0 | A - |

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HCM Unsignalized Intersection Capacity Analysis 11: Washington Street & Burlington Street

| Tr. Washington St | | <u></u> | | | | | |
|-------------------------------|----------|---------------|----------|----------|------------|------------|---|
| | • | \rightarrow | ~ | † | ţ | 4 | |
| Movement | EBL | EBR | NBL | NBT | SBT | SBR | |
| Lane Configurations | | | | 4 | ^ | | |
| Traffic Volume (veh/h) | 0 | 0 | 140 | 204 | 96 | 0 | |
| Future Volume (Veh/h) | 0 | 0 | 140 | 204 | 96 | 0 | |
| Sign Control | Stop | | | Free | Free | | |
| Grade | 0% | | | 0% | 0% | | |
| Peak Hour Factor | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | |
| Hourly flow rate (vph) | 0.00 | 0 | 159 | 232 | 109 | 0 | |
| Pedestrians | 27 | 0 | 107 | 202 | 107 | U | |
| Lane Width (ft) | 0.0 | | | | | | |
| Walking Speed (ft/s) | 3.5 | | | | | | |
| Percent Blockage | 0 | | | | | | |
| | U | | | | | | |
| Right turn flare (veh) | | | | Mono | Mono | | |
| Median type | | | | None | None | | |
| Median storage veh) | | | | | | | |
| Upstream signal (ft) | | | | | | | |
| pX, platoon unblocked | | 407 | 407 | | | | |
| vC, conflicting volume | 686 | 136 | 136 | | | | |
| vC1, stage 1 conf vol | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | |
| vCu, unblocked vol | 686 | 136 | 136 | | | | |
| tC, single (s) | 6.4 | 6.2 | 4.1 | | | | |
| tC, 2 stage (s) | | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | | |
| p0 queue free % | 100 | 100 | 89 | | | | |
| cM capacity (veh/h) | 367 | 913 | 1436 | | | | |
| Direction, Lane # | NB 1 | SB 1 | | | | | |
| Volume Total | 391 | 109 | | | | | |
| Volume Left | 159 | 0 | | | | | |
| Volume Right | 0 | 0 | | | | | |
| cSH | 1436 | 1700 | | | | | |
| Volume to Capacity | 0.11 | 0.06 | | | | | |
| Queue Length 95th (ft) | 9 | 0.00 | | | | | |
| Control Delay (s) | 3.8 | 0.0 | | | | | |
| Lane LOS | 3.0 A | 0.0 | | | | | |
| Approach Delay (s) | 3.8 | 0.0 | | | | | |
| Approach LOS | 3.0 | 0.0 | | | | | |
| Intersection Summary | | | | | | | |
| | | | 2.0 | | | | |
| Average Delay | otion | | 3.0 | 10 | 2111 6 | of Comitee | Λ |
| Intersection Capacity Utiliza | auun | | 28.5% | IC | CU Level o | n Service | A |
| Analysis Period (min) | | | 15 | | | | |

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Intersection Capacity Utilization 12: 0/Washington Street & Warren Avenue

| | ۶ | → | • | • | • | 4 | 1 | † | / | / | ↓ | 4 |
|--------------------------------|------------|-----------|------------|------------|-----------|------------|-------------|----------|----------|----------|----------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | | 4 | | | 4 | | | 4 | |
| Volume (vph) | 7 | 3 | 14 | 3 | 5 | 2 | 22 | 249 | 10 | 1 | 162 | 9 |
| Pedestrians | 1 | | 22 | 22 | | 1 | 28 | | 6 | 6 | | 28 |
| Ped Button | | Yes | | | Yes | | | Yes | | | Yes | |
| Pedestrian Timing (s) | | 16.0 | | | 16.0 | | | 16.0 | | | 16.0 | |
| Free Right | | | No | | | No | | | No | | | No |
| Ideal Flow | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Green (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Refr Cycle Length (s) | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 |
| Volume Combined (vph) | 0 | 24 | 0 | 0 | 10 | 0 | 0 | 281 | 0 | 0 | 172 | 0 |
| Lane Utilization Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Factor (vph) | 0.95 | 0.90 | 0.85 | 0.95 | 0.96 | 0.85 | 0.95 | 0.99 | 0.85 | 0.95 | 0.99 | 0.85 |
| Saturated Flow (vph) | 0 | 1708 | 0 | 0 | 1815 | 0 | 0 | 1882 | 0 | 0 | 1885 | 0 |
| Ped Intf Time (s) | 0.0 | 1.5 | 2.5 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 | 8.0 | 0.0 | 0.2 | 3.1 |
| Pedestrian Frequency (%) | | 0.52 | | | 0.03 | | | 0.18 | | | 0.61 | |
| Protected Option Allowed | | No | | | No | | | No | | | No | |
| Reference Time (s) | | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 |
| Adj Reference Time (s) | | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 |
| Permitted Option | | | | | | | | | | | | |
| Adj Saturation A (vph) | 0 | 1304 | | 0 | 1356 | | 0 | 954 | | 0 | 1847 | |
| Reference Time A (s) | 0.0 | 3.7 | | 0.0 | 0.9 | | 0.0 | 35.4 | | 0.0 | 11.3 | |
| Adj Saturation B (vph | 0 | 0 | | 0 | 0 | | NA | NA | | NA | NA | |
| Reference Time B (s) | 8.5 | 11.2 | | 8.2 | 8.7 | | NA | NA | | NA | NA | |
| Reference Time (s) | | 3.7 | | | 0.9 | | | 35.4 | | | 11.3 | |
| Adj Reference Time (s) | | 14.2 | | | 8.4 | | | 39.4 | | | 18.2 | |
| Split Option | | | | | | | | | | | | |
| Ref Time Combined (s) | 0.0 | 3.2 | | 0.0 | 0.7 | | 0.0 | 17.9 | | 0.0 | 11.1 | |
| Ref Time Seperate (s) | 0.5 | 1.7 | | 0.2 | 0.4 | | 1.5 | 15.8 | | 0.1 | 10.5 | |
| Reference Time (s) | 3.2 | 3.2 | | 0.7 | 0.7 | | 17.9 | 17.9 | | 11.1 | 11.1 | |
| Adj Reference Time (s) | 14.2 | 14.2 | | 8.4 | 8.4 | | 21.9 | 21.9 | | 18.1 | 18.1 | |
| Summary | EB WB | | NB SB | Co | mbined | | | | | | | |
| Protected Option (s) | NA | | NA | | mbinea | | | | | | | |
| Permitted Option (s) | 14.2 | | 39.4 | | | | | | | | | |
| Split Option (s) | 22.6 | | 40.0 | | | | | | | | | |
| Minimum (s) | 14.2 | | 39.4 | | 53.6 | | | | | | | |
| | 1 1.2 | | 37.1 | | 00.0 | | | | | | | |
| Right Turns | | | | | | | | | | | | |
| Adj Reference Time (s) | | | | | | | | | | | | |
| Cross Thru Ref Time (s) | | | | | | | | | | | | |
| Oncoming Left Ref Time (s) | | | | | | | | | | | | |
| Combined (s) | | | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Intersection Capacity Utilizat | tion | | 44.7% | IC | CU Level | of Service | | | Α | | | |
| Reference Times and Phasis | ng Options | do not re | epresent a | an optimiz | ed timing | ı plan. | | | | | | |

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| | ۶ | - | \rightarrow | • | ← | * | 1 | † | ~ | - | ļ | 4 |
|-------------------------------|-------|-------|---------------|------|-------------|------------|------|----------|------|------|------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | | 4 | | | 4 | | | 4 | |
| Sign Control | | Yield | | | Stop | | | Yield | | | Stop | |
| Traffic Volume (vph) | 8 | 2 | 15 | 1 | 2 | 0 | 60 | 14 | 1 | 0 | 2 | 3 |
| Future Volume (vph) | 8 | 2 | 15 | 1 | 2 | 0 | 60 | 14 | 1 | 0 | 2 | 3 |
| Peak Hour Factor | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 |
| Hourly flow rate (vph) | 12 | 3 | 22 | 1 | 3 | 0 | 88 | 21 | 1 | 0 | 3 | 4 |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | SB 1 | | | | | | | | |
| Volume Total (vph) | 37 | 4 | 110 | 7 | | | | | | | | |
| Volume Left (vph) | 12 | 1 | 88 | 0 | | | | | | | | |
| Volume Right (vph) | 22 | 0 | 1 | 4 | | | | | | | | |
| Hadj (s) | -0.01 | 0.47 | 0.22 | 0.39 | | | | | | | | |
| Departure Headway (s) | 4.2 | 4.7 | 4.2 | 4.5 | | | | | | | | |
| Degree Utilization, x | 0.04 | 0.01 | 0.13 | 0.01 | | | | | | | | |
| Capacity (veh/h) | 839 | 746 | 835 | 787 | | | | | | | | |
| Control Delay (s) | 7.3 | 7.7 | 7.8 | 7.5 | | | | | | | | |
| Approach Delay (s) | 7.3 | 7.7 | 7.8 | 7.5 | | | | | | | | |
| Approach LOS | А | Α | Α | Α | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Delay | | | 7.7 | | | | | | | | | |
| Level of Service | | | Α | | | | | | | | | |
| Intersection Capacity Utiliza | ation | | 21.7% | IC | CU Level of | of Service | | | Α | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

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<u>Capacity Analysis Summary Reports</u> Weekday Evening Peak Hour – Existing Conditions

Number of Lanes

1: Washingotn Street/Washington Street & Maple Avenue

1

1

0

01/28/2022

1

| Intersection | | | | | | | | | | | | |
|---------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Intersection Delay, s/veh | 18.7 | | | | | | | | | | | |
| Intersection LOS | С | | | | | | | | | | | |
| | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | ሻ | ĥ | | | ર્ન | 7 | | 4 | | | 4 | 7 |
| Traffic Vol, veh/h | 120 | 222 | 10 | 12 | 321 | 49 | 7 | 81 | 16 | 82 | 146 | 275 |
| Future Vol, veh/h | 120 | 222 | 10 | 12 | 321 | 49 | 7 | 81 | 16 | 82 | 146 | 275 |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Heavy Vehicles, % | 5 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| Mvmt Flow | 126 | 234 | 11 | 13 | 338 | 52 | 7 | 85 | 17 | 86 | 154 | 289 |

0

| Approach | EB | WB | NB | SB |
|----------------------------|------|------|----|----|
| Opposing Approach | WB | EB | SB | NB |
| Opposing Lanes | 2 | 2 | 2 | 1 |
| Conflicting Approach Left | SB | NB | EB | WB |
| Conflicting Lanes Left | 2 | 1 | 2 | 2 |
| Conflicting Approach Right | NB | SB | WB | EB |
| Conflicting Lanes Right | 1 | 2 | 2 | 2 |
| HCM Control Delay | 16.2 | 24.4 | 14 | 17 |
| HCM LOS | С | С | В | С |

0

1

0

0

| Lane | NBLn1 | EBLn1 | EBLn2 | WBLn1 | WBLn2 | SBLn1 | SBLn2 | |
|------------------------|-------|-------|-------|-------|-------|-------|-------|--|
| Vol Left, % | 7% | 100% | 0% | 4% | 0% | 36% | 0% | |
| Vol Thru, % | 78% | 0% | 96% | 96% | 0% | 64% | 0% | |
| Vol Right, % | 15% | 0% | 4% | 0% | 100% | 0% | 100% | |
| Sign Control | Stop | |
| Traffic Vol by Lane | 104 | 120 | 232 | 333 | 49 | 228 | 275 | |
| LT Vol | 7 | 120 | 0 | 12 | 0 | 82 | 0 | |
| Through Vol | 81 | 0 | 222 | 321 | 0 | 146 | 0 | |
| RT Vol | 16 | 0 | 10 | 0 | 49 | 0 | 275 | |
| Lane Flow Rate | 109 | 126 | 244 | 351 | 52 | 240 | 289 | |
| Geometry Grp | 6 | 7 | 7 | 7 | 7 | 7 | 7 | |
| Degree of Util (X) | 0.249 | 0.281 | 0.501 | 0.715 | 0.095 | 0.499 | 0.531 | |
| Departure Headway (Hd) | 8.184 | 8.004 | 7.389 | 7.344 | 6.624 | 7.481 | 6.598 | |
| Convergence, Y/N | Yes | |
| Cap | 438 | 448 | 486 | 491 | 540 | 482 | 547 | |
| Service Time | 6.259 | 5.763 | 5.147 | 5.098 | 4.378 | 5.232 | 4.349 | |
| HCM Lane V/C Ratio | 0.249 | 0.281 | 0.502 | 0.715 | 0.096 | 0.498 | 0.528 | |
| HCM Control Delay | 14 | 13.9 | 17.4 | 26.5 | 10.1 | 17.5 | 16.6 | |
| HCM Lane LOS | В | В | С | D | В | С | С | |
| HCM 95th-tile Q | 1 | 1.1 | 2.8 | 5.7 | 0.3 | 2.7 | 3.1 | |

2: Washington Street & Curtiss Street

| Intersection | |
|--|------|
| Intersection Delay, s/veh Intersection LOS | 10.2 |
| Intersection LOS | В |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 4 | | | 4 | | | ĵ₃ | | | ર્ન | |
| Traffic Vol, veh/h | 86 | 20 | 95 | 65 | 0 | 9 | 0 | 151 | 19 | 10 | 183 | 0 |
| Future Vol, veh/h | 86 | 20 | 95 | 65 | 0 | 9 | 0 | 151 | 19 | 10 | 183 | 0 |
| Peak Hour Factor | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 |
| Heavy Vehicles, % | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 101 | 24 | 112 | 76 | 0 | 11 | 0 | 178 | 22 | 12 | 215 | 0 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB | | | WB | | | | NB | | SB | | |
| Opposing Approach | WB | | | EB | | | | SB | | NB | | |
| Opposing Lanes | 1 | | | 1 | | | | 1 | | 1 | | |
| Conflicting Approach Left | SB | | | NB | | | | EB | | WB | | |
| Conflicting Lanes Left | 1 | | | 1 | | | | 1 | | 1 | | |
| Conflicting Approach Right | NB | | | SB | | | | WB | | EB | | |
| Conflicting Lanes Right | 1 | | | 1 | | | | 1 | | 1 | | |
| HCM Control Delay | 10.3 | | | 9.3 | | | | 10.1 | | 10.4 | | |
| HCM LOS | В | | | Α | | | | В | | В | | |

| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 | |
|------------------------|-------|-------|-------|-------|--|
| Vol Left, % | 0% | 43% | 88% | 5% | |
| Vol Thru, % | 89% | 10% | 0% | 95% | |
| Vol Right, % | 11% | 47% | 12% | 0% | |
| Sign Control | Stop | Stop | Stop | Stop | |
| Traffic Vol by Lane | 170 | 201 | 74 | 193 | |
| LT Vol | 0 | 86 | 65 | 10 | |
| Through Vol | 151 | 20 | 0 | 183 | |
| RT Vol | 19 | 95 | 9 | 0 | |
| Lane Flow Rate | 200 | 236 | 87 | 227 | |
| Geometry Grp | 1 | 1 | 1 | 1 | |
| Degree of Util (X) | 0.279 | 0.319 | 0.133 | 0.314 | |
| Departure Headway (Hd) | 5.013 | 4.864 | 5.481 | 4.971 | |
| Convergence, Y/N | Yes | Yes | Yes | Yes | |
| Cap | 708 | 731 | 658 | 713 | |
| Service Time | 3.111 | 2.958 | 3.481 | 3.066 | |
| HCM Lane V/C Ratio | 0.282 | 0.323 | 0.132 | 0.318 | |
| HCM Control Delay | 10.1 | 10.3 | 9.3 | 10.4 | |
| HCM Lane LOS | В | В | Α | В | |
| HCM 95th-tile Q | 1.1 | 1.4 | 0.5 | 1.3 | |

3: Washington Street & Rogers Street

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 4 | | | 4 | | ň | f) | | | 4 | |
| Traffic Vol, veh/h | 13 | 57 | 30 | 43 | 65 | 24 | 16 | 166 | 26 | 13 | 189 | 17 |
| Future Vol, veh/h | 13 | 57 | 30 | 43 | 65 | 24 | 16 | 166 | 26 | 13 | 189 | 17 |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Heavy Vehicles, % | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 14 | 63 | 33 | 47 | 71 | 26 | 18 | 182 | 29 | 14 | 208 | 19 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB | | | WB | | | NB | | | SB | | |
| Opposing Approach | WB | | | EB | | | SB | | | NB | | |
| Opposing Lanes | 1 | | | 1 | | | 1 | | | 2 | | |
| Conflicting Approach Left | SB | | | NB | | | EB | | | WB | | |
| Conflicting Lanes Left | 1 | | | 2 | | | 1 | | | 1 | | |
| Conflicting Approach Right | NB | | | SB | | | WB | | | EB | | |
| Conflicting Lanes Right | 2 | | | 1 | | | 1 | | | 1 | | |
| HCM Control Delay | 9.2 | | | 9.7 | | | 10.4 | | | 10.4 | | |
| HCM LOS | А | | | Α | | | В | | | В | | |

| Lane | NBLn1 | NBLn2 | EBLn1 | WBLn1 | SBLn1 | |
|------------------------|-------|-------|-------|-------|-------|--|
| Vol Left, % | 100% | 0% | 13% | 33% | 6% | |
| Vol Thru, % | 0% | 86% | 57% | 49% | 86% | |
| Vol Right, % | 0% | 14% | 30% | 18% | 8% | |
| Sign Control | Stop | Stop | Stop | Stop | Stop | |
| Traffic Vol by Lane | 16 | 192 | 100 | 132 | 219 | |
| LT Vol | 16 | 0 | 13 | 43 | 13 | |
| Through Vol | 0 | 166 | 57 | 65 | 189 | |
| RT Vol | 0 | 26 | 30 | 24 | 17 | |
| Lane Flow Rate | 18 | 211 | 110 | 145 | 241 | |
| Geometry Grp | 7 | 7 | 2 | 2 | 5 | |
| Degree of Util (X) | 0.029 | 0.311 | 0.156 | 0.209 | 0.329 | |
| Departure Headway (Hd) | 5.886 | 5.303 | 5.114 | 5.199 | 4.918 | |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | |
| Cap | 603 | 671 | 692 | 683 | 724 | |
| Service Time | 3.669 | 3.085 | 3.21 | 3.29 | 2.999 | |
| HCM Lane V/C Ratio | 0.03 | 0.314 | 0.159 | 0.212 | 0.333 | |
| HCM Control Delay | 8.8 | 10.5 | 9.2 | 9.7 | 10.4 | |
| HCM Lane LOS | А | В | Α | Α | В | |
| HCM 95th-tile Q | 0.1 | 1.3 | 0.6 | 8.0 | 1.4 | |

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HCM Unsignalized Intersection Capacity Analysis 4: Washington Street & Village Hall Access

| | • | → | ← | 4 | > | 1 |
|------------------------------|-----------|----------|----------|----------|-------------|------------|
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | | 4 | † | 7 | W | |
| Traffic Volume (veh/h) | 151 | 2 | 7 | 63 | 64 | 258 |
| Future Volume (Veh/h) | 151 | 2 | 7 | 63 | 64 | 258 |
| Sign Control | | Stop | Stop | | Free | |
| Grade | | 0% | 0% | | 0% | |
| Peak Hour Factor | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 |
| Hourly flow rate (vph) | 196 | 3 | 9 | 82 | 83 | 335 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | | None | |
| Median storage veh) | | | | | | |
| Upstream signal (ft) | | | | | 76 | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 420 | 334 | 501 | 0 | 0 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 420 | 334 | 501 | 0 | 0 | |
| tC, single (s) | 7.1 | 6.5 | 6.5 | 6.2 | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 4.0 | 4.0 | 3.3 | 2.2 | |
| p0 queue free % | 59 | 99 | 98 | 92 | 95 | |
| cM capacity (veh/h) | 479 | 560 | 451 | 1091 | 1636 | |
| Direction, Lane # | EB 1 | WB 1 | WB 2 | SB 1 | | |
| Volume Total | 199 | 9 | 82 | 418 | | |
| Volume Left | 199 | 0 | 02 | 83 | | |
| Volume Right | 0 | 0 | 82 | 335 | | |
| cSH | 480 | 451 | 1091 | 1636 | | |
| Volume to Capacity | 0.41 | 0.02 | 0.08 | 0.05 | | |
| Queue Length 95th (ft) | 50 | 0.02 | 0.08 | 4 | | |
| | 17.7 | 13.1 | 8.6 | 1.8 | | |
| Control Delay (s) | | | | | | |
| Lane LOS | C | В | А | A 1.8 | | |
| Approach LOS | 17.7 C | 9.0 | | ۱.۵ | | |
| Approach LOS | C | А | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 7.2 | | | |
| Intersection Capacity Utiliz | zation | | 41.3% | IC | U Level o | of Service |
| Analysis Period (min) | | | 15 | | | |

HCM 6th TWSC 5: Belden Avenue & Curtiss Street

| Intersection | | | | | | |
|------------------------|--------|-------|--------|------|--------|------|
| Int Delay, s/veh | 1 | | | | | |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | - ↑ | | | 4 | W | |
| Traffic Vol, veh/h | 59 | 55 | 2 | 26 | 15 | 1 |
| Future Vol, veh/h | 59 | 55 | 2 | 26 | 15 | 1 |
| Conflicting Peds, #/hr | 0 | 5 | 5 | 0 | 7 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | _ | - | _ | - | 0 | - |
| Veh in Median Storage | e,# 0 | _ | _ | 0 | 0 | _ |
| Grade, % | 0 | _ | _ | 0 | 0 | _ |
| Peak Hour Factor | 85 | 85 | 85 | 85 | 85 | 85 |
| Heavy Vehicles, % | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 69 | 65 | 2 | 31 | 18 | 1 |
| IVIVIIIL FIOW | 09 | 00 | 2 | 31 | 10 | ı |
| | | | | | | |
| Major/Minor | Major1 | N | Major2 | [| Minor1 | |
| Conflicting Flow All | 0 | 0 | 139 | 0 | 149 | 107 |
| Stage 1 | - | - | - | - | 107 | - |
| Stage 2 | _ | _ | _ | - | 42 | _ |
| Critical Hdwy | - | _ | 4.1 | _ | 6.4 | 6.2 |
| Critical Hdwy Stg 1 | _ | _ | - | _ | 5.4 | - |
| Critical Hdwy Stg 2 | _ | _ | _ | _ | 5.4 | - |
| Follow-up Hdwy | _ | _ | 2.2 | _ | 3.5 | 3.3 |
| Pot Cap-1 Maneuver | _ | _ | 1457 | _ | 848 | 953 |
| Stage 1 | _ | _ | 1437 | _ | 922 | 755 |
| Stage 2 | _ | _ | | _ | 986 | |
| Platoon blocked, % | - | - | - | | 900 | - |
| | | - | 1450 | - | 027 | 040 |
| Mov Cap-1 Maneuver | | - | 1450 | - | 837 | 948 |
| Mov Cap-2 Maneuver | | - | - | - | 837 | - |
| Stage 1 | - | - | - | - | 917 | - |
| Stage 2 | - | - | - | - | 978 | - |
| | | | | | | |
| Approach | EB | | WB | | NB | |
| HCM Control Delay, s | | | 0.5 | | 9.4 | |
| HCM LOS | U | | 0.5 | | | |
| FICIVI LOS | | | | | А | |
| | | | | | | |
| Minor Lane/Major Mvn | nt l | NBLn1 | EBT | EBR | WBL | WBT |
| Capacity (veh/h) | | 843 | - | - | 1450 | - |
| HCM Lane V/C Ratio | | 0.022 | _ | | 0.002 | _ |
| HCM Control Delay (s |) | 9.4 | _ | _ | 7.5 | 0 |
| HCM Lane LOS | , | A | _ | _ | Α. | A |
| HCM 95th %tile Q(veh | 1) | 0.1 | - | - | 0 | - |
| | 7 | 3.1 | | | J | |

6: Curtiss Street & Westerly Access Drive

| Intersection | | | | | | |
|--------------------------------------|--------|--------|--------|--------|--------|-------------|
| Int Delay, s/veh | 2.9 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | 202 | 4 | 1≯ | TT DIC | ₩. | ODIN |
| Traffic Vol, veh/h | 6 | 41 | 40 | 4 | 12 | 25 |
| Future Vol, veh/h | 6 | 41 | 40 | 4 | 12 | 25 |
| Conflicting Peds, #/hr | | 0 | 0 | 0 | 12 | 23 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | | | | None | | |
| | - | None | - | None | - | None |
| Storage Length | - " | - | - | - | 0 | - |
| Veh in Median Storag | | 0 | 0 | - | 0 | - |
| Grade, % | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 87 | 87 | 87 | 87 | 87 | 87 |
| Heavy Vehicles, % | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 7 | 47 | 46 | 5 | 14 | 29 |
| | | | | | | |
| | | _ | | | | |
| Major/Minor | Major1 | | Major2 | | Minor2 | |
| Conflicting Flow All | 51 | 0 | - | 0 | 111 | 51 |
| Stage 1 | - | - | - | - | 49 | - |
| Stage 2 | - | - | - | - | 62 | - |
| Critical Hdwy | 4.1 | - | - | - | 6.4 | 6.2 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.4 | - |
| Critical Hdwy Stg 2 | - | _ | - | - | 5.4 | - |
| Follow-up Hdwy | 2.2 | _ | _ | _ | 3.5 | 3.3 |
| Pot Cap-1 Maneuver | 1568 | | _ | _ | 891 | 1023 |
| Stage 1 | 1300 | _ | _ | _ | 979 | 1023 |
| | _ | | _ | _ | 966 | _ |
| Stage 2 | - | - | - | | 900 | - |
| Platoon blocked, % | 45/0 | - | - | - | 007 | 1001 |
| Mov Cap-1 Maneuver | | - | - | - | 887 | 1021 |
| Mov Cap-2 Maneuver | r - | - | - | - | 887 | - |
| Stage 1 | - | - | - | - | 974 | - |
| Stage 2 | - | - | - | - | 966 | - |
| | | | | | | |
| | ED | |)A/D | | CD. | |
| Approach | EB | | WB | | SB | |
| HCM Control Delay, s | 0.9 | | 0 | | 8.9 | |
| HCM LOS | | | | | Α | |
| | | | | | | |
| Minor Lang/Major My | mt | EBL | EBT | WBT | WBR S | CDI n1 |
| Minor Lane/Major Mv | IIIL | | EDI | WDI | WBR. | |
| Capacity (veh/h) | | 1568 | - | - | - | 973 |
| HCM Lane V/C Ratio | | 0.004 | - | - | - | 0.044 |
| | s) | 7.3 | 0 | - | - | 8.9 |
| HCM Control Delay (s | | | | | | Α |
| HCM Control Delay (s HCM Lane LOS | | Α | Α | - | - | $^{\wedge}$ |
| | h) | A 0 | A - | - | - | 0.1 |

7: Curtiss Street & VIIIage Hall Access

| Movement EBL Lane Configurations Traffic Vol, veh/h 11 Future Vol, veh/h 11 Conflicting Peds, #/hr 0 Sign Control Free RT Channelized - Storage Length - Veh in Median Storage, # - Grade, % - Peak Hour Factor 85 Heavy Vehicles, % 0 Mvmt Flow 13 Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Minor Lane/Major Mvmt Capacity (veh/h) | Intersection | | | | | | |
|--|----------------------|------|-------|--------|------|-----------|--------|
| Lane Configurations Traffic Vol, veh/h 11 Future Vol, veh/h 11 Conflicting Peds, #/hr 0 Sign Control Free RT Channelized - Storage Length - Veh in Median Storage, # - Grade, % - Peak Hour Factor 85 Heavy Vehicles, % 0 Mvmt Flow 13 Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver 1555 Mov Cap-3 Mov C | Int Delay, s/veh | 5.1 | | | | | |
| Lane Configurations Traffic Vol, veh/h 11 Future Vol, veh/h 11 Conflicting Peds, #/hr 0 Sign Control Free RT Channelized - Storage Length - Veh in Median Storage, # - Grade, % - Peak Hour Factor 85 Heavy Vehicles, % 0 Mvmt Flow 13 Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver 1555 Mov Cap-3 Mov C | Movement | FBI | EBT | WBT | WBR | SBL | SBR |
| Traffic Vol, veh/h 11 Future Vol, veh/h 11 Conflicting Peds, #/hr 0 Sign Control Free RT Channelized - Storage Length - Veh in Median Storage, # - Grade, % - Peak Hour Factor 85 Heavy Vehicles, % 0 Mvmt Flow 13 Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy Stg 1 - Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Minor Lane/Major Mvmt Capacity (veh/h) HCM Los Minor Lane/Major Mvmt Capacity (veh/h) HCM Control Delay (s) HCM Control Delay (s) HCM Lane LOS | | LUL | 4 | 1€ | אטוע | Y | ODIN |
| Future Vol, veh/h Conflicting Peds, #/hr Sign Control Free RT Channelized Storage Length Veh in Median Storage, # Grade, % Peak Hour Factor Heavy Vehicles, % Mymt Flow Major/Minor Major1 Conflicting Flow All Stage 1 Stage 2 Critical Hdwy Critical Hdwy Stg 1 Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-2 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-2 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-2 Maneuver Stage 1 Stage 2 - Critical Hdwy Stg 2 Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver Stage 1 Stage 2 - Critical Hdwy Stg 3 - Stage 1 Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver Stage 1 Stage 2 - Critical Hdwy Stg 3 - Stage 3 - S | | 11 | 36 | 22 | 30 | 77 | 21 |
| Conflicting Peds, #/hr 0 Sign Control Free RT Channelized - Storage Length - Veh in Median Storage, # - Grade, % - Peak Hour Factor 85 Heavy Vehicles, % 0 Mvmt Flow 13 Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Minor Lane/Major Mvmt Capacity (veh/h) HCM Los Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | | 36 | 22 | 30 | 77 | 21 |
| Sign Control Free RT Channelized - Storage Length - Veh in Median Storage, # - Grade, % - Peak Hour Factor 85 Heavy Vehicles, % 0 Mvmt Flow 13 Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Minor Lane/Major Mvmt Capacity (veh/h) HCM Los Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | | 0 | 0 | 0 | 0 | 0 |
| RT Channelized - Storage Length - Veh in Median Storage, # - Grade, % - Peak Hour Factor 85 Heavy Vehicles, % 0 Mvmt Flow 13 Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver 1555 Mov Cap-2 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver 1555 Mov Cap-2 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver 1555 Mov Cap-1 | | | Free | Free | Free | Stop | Stop |
| Storage Length Veh in Median Storage, # - Grade, % - Peak Hour Factor 85 Heavy Vehicles, % 0 Mvmt Flow 13 Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Blatoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Minor Lane/Major Mvmt Capacity (veh/h) HCM Los Minor Lane/Major Mvmt Capacity (veh/h) HCM Control Delay (s) HCM Control Delay (s) HCM Control Delay (s) HCM Lane LOS | | | None | - | None | - | None |
| Veh in Median Storage, # - Grade, % - Peak Hour Factor 85 Heavy Vehicles, % 0 Mvmt Flow 13 Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy Stg 1 - Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Cane LOS | | _ | - | _ | - | 0 | - |
| Grade, % - Peak Hour Factor 85 Heavy Vehicles, % 0 Mvmt Flow 13 Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Control Delay (s) HCM Lane LOS | | .# - | 0 | 0 | _ | 0 | _ |
| Peak Hour Factor 85 Heavy Vehicles, % 0 Mvmt Flow 13 Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS | | _ | 0 | 0 | _ | 0 | _ |
| Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Black Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | 85 | 85 | 85 | 85 | 85 | 85 |
| Mover Flow Major/Minor Conflicting Flow All Stage 1 Stage 2 Critical Hdwy 4.1 Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy 2.2 Pot Cap-1 Maneuver Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-2 Maneuver Stage 1 Stage 2 FOMOV Cap-2 Maneuver Stage 3 FOMOV Cap-3 FOMOV Cap-3 FOMOV Cap-3 FOMOV Cap-3 FOMOV Cap-3 FOMOV Cap-3 FOMOV Cap-4 FO | | | 0 | 0 | 0 | 0 | 0 |
| Major/Minor Major1 Conflicting Flow All 61 Stage 1 - Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Control Delay (s) HCM Lane LOS | | | 42 | 26 | 35 | 91 | 25 |
| Conflicting Flow All Stage 1 Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | WWW. TOW | 10 | 72 | 20 | 33 | 71 | 20 |
| Conflicting Flow All Stage 1 Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | | | | | | |
| Stage 1 Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | | | Major2 | | Minor2 | |
| Stage 2 - Critical Hdwy 4.1 Critical Hdwy Stg 1 - Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | 61 | 0 | - | 0 | 112 | 44 |
| Critical Hdwy Stg 1 Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver Stage 1 Stage 2 - Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | • | - | - | - | - | 44 | - |
| Critical Hdwy Stg 1 Critical Hdwy Stg 2 Follow-up Hdwy Pot Cap-1 Maneuver Stage 1 Stage 2 Platoon blocked, % Mov Cap-1 Maneuver Stage 1 Stage 2 Follow-up Hdwy Stage 3 Follow-up Hdwy Follow- | | | - | - | - | 68 | - |
| Critical Hdwy Stg 2 - Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | Critical Hdwy | 4.1 | - | - | - | 6.4 | 6.2 |
| Follow-up Hdwy 2.2 Pot Cap-1 Maneuver 1555 Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | Critical Hdwy Stg 1 | - | - | - | - | 5.4 | - |
| Pot Cap-1 Maneuver Stage 1 Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver Stage 1 Stage 2 - Mov Cap-2 Maneuver Stage 2 - Stage 2 - Approach EB HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | Critical Hdwy Stg 2 | - | - | - | - | 5.4 | - |
| Stage 1 - Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | Follow-up Hdwy | 2.2 | - | - | - | 3.5 | 3.3 |
| Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | Pot Cap-1 Maneuver | 1555 | - | - | - | 890 | 1032 |
| Stage 2 - Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | Stage 1 | - | - | - | - | 984 | - |
| Platoon blocked, % Mov Cap-1 Maneuver 1555 Mov Cap-2 Maneuver - Stage 1 - Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | - | - | - | - | 960 | - |
| Mov Cap-1 Maneuver Mov Cap-2 Maneuver Stage 1 Stage 2 Approach HCM Control Delay, s Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | | - | - | - | | |
| Mov Cap-2 Maneuver Stage 1 Stage 2 Approach EB HCM Control Delay, s HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | 1555 | - | - | - | 882 | 1032 |
| Stage 1 - Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | | - | _ | - | 882 | - |
| Stage 2 - Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | | - | - | _ | 975 | _ |
| Approach EB HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | · · | | _ | _ | _ | 960 | _ |
| HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | Olago 2 | | | | | 700 | |
| HCM Control Delay, s 1.7 HCM LOS Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | | | | | | |
| Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | | | WB | | SB | |
| Minor Lane/Major Mvmt Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | 1.7 | | 0 | | 9.5 | |
| Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | HCM LOS | | | | | Α | |
| Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | | | | | | |
| Capacity (veh/h) HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | Minor Lane/Major Mym | t | EBL | EBT | WBT | WBR : | SRI n1 |
| HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS | | | 1555 | LDI | WDT | י אוטוע י | 910 |
| HCM Control Delay (s) HCM Lane LOS | | | 0.008 | - | - | - | 0.127 |
| HCM Lane LOS | | | | - | - | | |
| | | | 7.3 | 0 | - | - | 9.5 |
| HUM YOU WILL DIVEN | | | A | А | - | - | Α |
| | HUN YOUN %THE U(VEN) | | 0 | - | - | - | 0.4 |

8: Curtiss Street & Police Department Access

| Intersection | | | | | | |
|------------------------|--------|-----------------|--------|------|----------|-------|
| Int Delay, s/veh | 0.8 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | LUL | 4 | 130 | WDI(| Y | JUIN |
| Traffic Vol, veh/h | 2 | H 111 | 41 | 0 | T | 10 |
| Future Vol, veh/h | 2 | 111 | 41 | 0 | 3 | 10 |
| | 0 | 0 | 0 | 0 | 0 | 0 |
| Conflicting Peds, #/hr | | | | | | |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage | e,# - | 0 | 0 | - | 0 | - |
| Grade, % | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 85 | 85 | 85 | 85 | 85 | 85 |
| Heavy Vehicles, % | 0 | 0 | 0 | 0 | 0 | 5 |
| Mvmt Flow | 2 | 131 | 48 | 0 | 4 | 12 |
| | | | | | | |
| N A ' /N A' | | | 4 ' 0 | | A' 0 | |
| | Major1 | | Major2 | | Minor2 | |
| Conflicting Flow All | 48 | 0 | - | 0 | 183 | 48 |
| Stage 1 | - | - | - | - | 48 | - |
| Stage 2 | - | - | - | - | 135 | - |
| Critical Hdwy | 4.1 | - | - | - | 6.4 | 6.25 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.4 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.4 | - |
| Follow-up Hdwy | 2.2 | - | - | - | 3.5 | 3.345 |
| Pot Cap-1 Maneuver | 1572 | _ | - | _ | 811 | 1012 |
| Stage 1 | | _ | _ | _ | 980 | - |
| Stage 2 | _ | | _ | _ | 896 | _ |
| Platoon blocked, % | _ | _ | _ | _ | 070 | _ |
| | 1570 | - | - | | 010 | 1010 |
| Mov Cap-1 Maneuver | 1572 | - | - | - | 810 | 1012 |
| Mov Cap-2 Maneuver | - | - | - | - | 810 | - |
| Stage 1 | - | - | - | - | 979 | - |
| Stage 2 | - | - | - | - | 896 | - |
| | | | | | | |
| Approach | EB | | WB | | SB | |
| | | | | | | |
| HCM Control Delay, s | 0.1 | | 0 | | 8.8 | |
| HCM LOS | | | | | Α | |
| | | | | | | |
| Minor Lane/Major Mvn | nt | EBL | EBT | WBT | WBR : | SBLn1 |
| Capacity (veh/h) | | 1572 | | | - | 957 |
| HCM Lane V/C Ratio | | 0.001 | | | | 0.016 |
| | | | - 0 | - | | |
| HCM Long LOS | | 7.3 | 0 | - | - | 8.8 |
| HCM Lane LOS | , | A | Α | - | - | A |
| HCM 95th %tile Q(veh |) | 0 | - | - | - | 0 |
| | | | | | | |

9: Apartment Drive & Washinton Street/Washington Street

| Intersection | | | | | | |
|---------------------------------------|----------|----------|--------|------|--------|------|
| Int Delay, s/veh | 0.4 | | | | | |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | î, | | | 4 | ¥ | |
| Traffic Vol, veh/h | 150 | 1 | 6 | 259 | 4 | 5 |
| Future Vol, veh/h | 150 | 1 | 6 | 259 | 4 | 5 |
| Conflicting Peds, #/hr | 0 | 38 | 38 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | | - | - | - | 0 | - |
| Veh in Median Storage | , # 0 | - | - | 0 | 0 | - |
| Grade, % | 0 | _ | _ | 0 | 0 | _ |
| Peak Hour Factor | 77 | 77 | 77 | 77 | 77 | 77 |
| Heavy Vehicles, % | 3 | 0 | 0 | 2 | 0 | 0 |
| Mvmt Flow | 195 | 1 | 8 | 336 | 5 | 6 |
| IVIVIIIL I IOVV | 175 | ı | U | 330 | J | U |
| | | | | | | |
| | /lajor1 | | Major2 | | Vinor1 | |
| Conflicting Flow All | 0 | 0 | 234 | 0 | 586 | 234 |
| Stage 1 | - | - | - | - | 234 | - |
| Stage 2 | - | - | - | - | 352 | - |
| Critical Hdwy | - | - | 4.1 | - | 6.4 | 6.2 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.4 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.4 | - |
| Follow-up Hdwy | - | - | 2.2 | - | 3.5 | 3.3 |
| Pot Cap-1 Maneuver | - | - | 1345 | - | 476 | 810 |
| Stage 1 | - | - | - | - | 810 | - |
| Stage 2 | - | - | - | - | 716 | - |
| Platoon blocked, % | - | - | | - | | |
| Mov Cap-1 Maneuver | - | _ | 1296 | _ | 455 | 781 |
| Mov Cap-2 Maneuver | _ | _ | - | - | 455 | - |
| Stage 1 | _ | _ | _ | _ | 781 | _ |
| Stage 2 | _ | _ | _ | _ | 710 | _ |
| Stage 2 | | | | | 710 | |
| | | | | | | |
| Approach | EB | | WB | | NB | |
| HCM Control Delay, s | 0 | | 0.2 | | 11.2 | |
| HCM LOS | | | | | В | |
| | | | | | | |
| Minor Lane/Major Mvm | † 1 | NBLn1 | EBT | EBR | WBL | WBT |
| | <u> </u> | 592 | | | 1296 | |
| Capacity (veh/h) | | | - | | | - |
| HCM Cartral Dalay (a) | | 0.02 | - | | 0.006 | - |
| | | 11.2 | - | - | 7.8 | 0 |
| HCM Long LOS | | | | | | |
| HCM Lane LOS HCM 95th %tile Q(veh) | | B 0.1 | - | - | A 0 | A - |

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HCM Unsignalized Intersection Capacity Analysis 11: Washington Street & Burlington Street

| | ۶ | • | 4 | † | | 4 |
|------------------------------|--------|------|-------|------|--------------|------------|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | | | | ર્ન | † | |
| Traffic Volume (veh/h) | 0 | 0 | 89 | 150 | 203 | 0 |
| Future Volume (Veh/h) | 0 | 0 | 89 | 150 | 203 | 0 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 |
| Hourly flow rate (vph) | 0 | 0 | 122 | 205 | 278 | 0 |
| Pedestrians | 16 | | | | | |
| Lane Width (ft) | 0.0 | | | | | |
| Walking Speed (ft/s) | 3.5 | | | | | |
| Percent Blockage | 0 | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | None | None | |
| Median storage veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 743 | 294 | 294 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 743 | 294 | 294 | | | |
| tC, single (s) | 6.4 | 6.2 | 4.2 | | | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.3 | | | |
| p0 queue free % | 100 | 100 | 90 | | | |
| cM capacity (veh/h) | 345 | 745 | 1245 | | | |
| Direction, Lane # | NB 1 | SB 1 | | | | |
| Volume Total | 327 | 278 | | | | |
| Volume Left | 122 | 0 | | | | |
| Volume Right | 0 | 0 | | | | |
| cSH | 1245 | 1700 | | | | |
| Volume to Capacity | 0.10 | 0.16 | | | | |
| Queue Length 95th (ft) | 8 | 0 | | | | |
| Control Delay (s) | 3.6 | 0.0 | | | | |
| Lane LOS | А | | | | | |
| Approach Delay (s) | 3.6 | 0.0 | | | | |
| Approach LOS | | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 2.0 | | | |
| Intersection Capacity Utiliz | zation | | 30.2% | IC | CU Level o | of Service |
| Analysis Period (min) | | | 15 | | . 5 25 07 0 | |
| raidiyələ i chou (illii) | | | 10 | | | |

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Intersection Capacity Utilization 12: 0/Washington Street & Warren Avenue

| | ۶ | → | • | • | • | • | • | † | / | > | ļ | 4 |
|-------------------------------|-----------|------------|---------------|----------|------------|---|------|----------|----------|-------------|------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | | 4 | | | 4 | | | 4 | |
| Volume (vph) | 16 | 9 | 74 | 13 | 3 | 8 | 36 | 175 | 3 | 1 | 237 | 26 |
| Pedestrians | 4 | | 66 | 66 | | 4 | 27 | | 14 | 14 | | 27 |
| Ped Button | | Yes | | | Yes | | | Yes | | | Yes | |
| Pedestrian Timing (s) | | 16.0 | | | 16.0 | | | 16.0 | | | 16.0 | |
| Free Right | | | No | | | No | | | No | | | No |
| Ideal Flow | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Green (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Refr Cycle Length (s) | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 |
| Volume Combined (vph) | 0 | 99 | 0 | 0 | 24 | 0 | 0 | 214 | 0 | 0 | 264 | 0 |
| Lane Utilization Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Factor (vph) | 0.95 | 0.88 | 0.85 | 0.95 | 0.92 | 0.85 | 0.95 | 0.99 | 0.85 | 0.95 | 0.99 | 0.85 |
| Saturated Flow (vph) | 0 | 1673 | 0 | 0 | 1756 | 0 | 0 | 1880 | 0 | 0 | 1872 | 0 |
| Ped Intf Time (s) | 0.0 | 4.3 | 5.7 | 0.0 | 0.2 | 0.5 | 0.0 | 0.0 | 1.7 | 0.0 | 0.3 | 3.0 |
| Pedestrian Frequency (%) | | 0.89 | | | 0.12 | | | 0.37 | | | 0.59 | |
| Protected Option Allowed | | No | | | No | | | No | | | No | |
| Reference Time (s) | | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 |
| Adj Reference Time (s) | | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 |
| Permitted Option | | | | | | | | | | | | |
| Adj Saturation A (vph) | 0 | 1654 | | 0 | 673 | | 0 | 570 | | 0 | 1861 | |
| Reference Time A (s) | 0.0 | 11.4 | | 0.0 | 4.4 | | 0.0 | 45.1 | | 0.0 | 17.3 | |
| Adj Saturation B (vph | 0 | 0 | | 0 | 0 | | NA | NA | | NA | NA | |
| Reference Time B (s) | 9.1 | 19.4 | | 8.9 | 9.8 | | NA | NA | | NA | NA | |
| Reference Time (s) | | 11.4 | | | 4.4 | | | 45.1 | | | 17.3 | |
| Adj Reference Time (s) | | 19.5 | | | 9.9 | | | 49.1 | | | 21.3 | |
| Split Option | | | | | | | | | | | | |
| Ref Time Combined (s) | 0.0 | 11.4 | | 0.0 | 1.8 | | 0.0 | 13.7 | | 0.0 | 17.2 | |
| Ref Time Seperate (s) | 1.1 | 4.9 | | 0.9 | 0.4 | | 2.4 | 11.1 | | 0.1 | 15.5 | |
| Reference Time (s) | 11.4 | 11.4 | | 1.8 | 1.8 | | 13.7 | 13.7 | | 17.2 | 17.2 | |
| Adj Reference Time (s) | 19.5 | 19.5 | | 9.5 | 9.5 | | 18.5 | 18.5 | | 21.2 | 21.2 | |
| Summary | EB WB | | NB SB | | mbined | | | | | | | |
| Protected Option (s) | NA | | NA NA | CO | IIIbiiicu | | | | | | | |
| | 19.5 | | 49.1 | | | | | | | | | |
| Permitted Option (s) | 29.0 | | 39.8 | | | | | | | | | |
| Split Option (s) | 19.5 | | | | 59.3 | | | | | | | |
| Minimum (s) | 19.5 | | 39.8 | | 39.3 | | | | | | | |
| Right Turns | | | | | | | | | | | | |
| Adj Reference Time (s) | | | | | | | | | | | | |
| Cross Thru Ref Time (s) | | | | | | | | | | | | |
| Oncoming Left Ref Time (s) | | | | | | | | | | | | |
| Combined (s) | | | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Intersection Capacity Utiliza | tion | | 49.4% | IC | CU Level o | of Service | 1 | | A | | | |
| Reference Times and Phasi | | do not re | | | | | • | | А | | | |
| | g options | 30 1101 10 | , p. 000111 C | opuiiiiz | | ייייייייייייייייייייייייייייייייייייייי | | | | | | |

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|-------------------------------|-------|-------|---------------|-------|-----------|------------|------|----------|------|------|------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | | 4 | | | 4 | | | 4 | |
| Sign Control | | Yield | | | Stop | | | Yield | | | Stop | |
| Traffic Volume (vph) | 8 | 0 | 55 | 0 | 0 | 0 | 26 | 1 | 0 | 0 | 7 | 4 |
| Future Volume (vph) | 8 | 0 | 55 | 0 | 0 | 0 | 26 | 1 | 0 | 0 | 7 | 4 |
| Peak Hour Factor | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 |
| Hourly flow rate (vph) | 11 | 0 | 74 | 0 | 0 | 0 | 35 | 1 | 0 | 0 | 9 | 5 |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | SB 1 | | | | | | | | |
| Volume Total (vph) | 85 | 0 | 36 | 14 | | | | | | | | |
| Volume Left (vph) | 11 | 0 | 35 | 0 | | | | | | | | |
| Volume Right (vph) | 74 | 0 | 0 | 5 | | | | | | | | |
| Hadj (s) | -0.50 | 0.00 | 0.19 | -0.21 | | | | | | | | |
| Departure Headway (s) | 3.5 | 4.1 | 4.3 | 3.9 | | | | | | | | |
| Degree Utilization, x | 0.08 | 0.00 | 0.04 | 0.02 | | | | | | | | |
| Capacity (veh/h) | 1004 | 876 | 816 | 901 | | | | | | | | |
| Control Delay (s) | 6.8 | 7.1 | 7.5 | 6.9 | | | | | | | | |
| Approach Delay (s) | 6.8 | 0.0 | 7.5 | 6.9 | | | | | | | | |
| Approach LOS | Α | Α | Α | Α | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Delay | | | 7.0 | | | | | | | | | |
| Level of Service | | | Α | | | | | | | | | |
| Intersection Capacity Utiliza | ation | | 27.0% | IC | U Level o | of Service | | | Α | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

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<u>Capacity Analysis Summary Reports</u> Weekday Morning Peak Hour – Projected Conditions

1: Washingotn Street/Washington Street & Maple Avenue

| ersection Delay, s/veh 23.6 |
|--|
| ersection belay, siven 25.0 |
| ersection Delay, s/veh 23.6 ersection LOS C |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | ř | f. | | | ર્ન | 7 | | 4 | | | ની | 7 |
| Traffic Vol, veh/h | 274 | 402 | 8 | 13 | 136 | 50 | 8 | 232 | 36 | 39 | 42 | 67 |
| Future Vol, veh/h | 274 | 402 | 8 | 13 | 136 | 50 | 8 | 232 | 36 | 39 | 42 | 67 |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.92 |
| Heavy Vehicles, % | 2 | 1 | 0 | 0 | 1 | 2 | 0 | 2 | 0 | 0 | 2 | 0 |
| Mvmt Flow | 301 | 442 | 9 | 14 | 149 | 55 | 9 | 255 | 40 | 43 | 46 | 73 |
| Number of Lanes | 1 | 1 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 1 | 1 |
| Approach | EB | | | WB | | | NB | | | SB | | |
| Opposing Approach | WB | | | EB | | | SB | | | NB | | |
| Opposing Lanes | 2 | | | 2 | | | 2 | | | 1 | | |
| Conflicting Approach Left | SB | | | NB | | | EB | | | WB | | |
| Conflicting Lanes Left | 2 | | | 1 | | | 2 | | | 2 | | |
| Conflicting Approach Right | NB | | | SB | | | WB | | | EB | | |
| Conflicting Lanes Right | 1 | | | 2 | | | 2 | | | 2 | | |
| HCM Control Delay | 29.8 | | | 13.4 | | | 21.6 | | | 12.3 | | |
| HCM LOS | D | | | В | | | С | | | В | | |

| Lane | NBLn1 | EBLn1 | EBLn2 | WBLn1 | WBLn2 | SBLn1 | SBLn2 | |
|------------------------|-------|-------|-------|-------|-------|-------|-------|--|
| Vol Left, % | 3% | 100% | 0% | 9% | 0% | 48% | 0% | |
| Vol Thru, % | 84% | 0% | 98% | 91% | 0% | 52% | 0% | |
| Vol Right, % | 13% | 0% | 2% | 0% | 100% | 0% | 100% | |
| Sign Control | Stop | |
| Traffic Vol by Lane | 276 | 274 | 410 | 149 | 50 | 81 | 67 | |
| LT Vol | 8 | 274 | 0 | 13 | 0 | 39 | 0 | |
| Through Vol | 232 | 0 | 402 | 136 | 0 | 42 | 0 | |
| RT Vol | 36 | 0 | 8 | 0 | 50 | 0 | 67 | |
| Lane Flow Rate | 303 | 301 | 451 | 164 | 55 | 89 | 73 | |
| Geometry Grp | 6 | 7 | 7 | 7 | 7 | 7 | 7 | |
| Degree of Util (X) | 0.619 | 0.608 | 0.842 | 0.347 | 0.105 | 0.204 | 0.148 | |
| Departure Headway (Hd) | 7.343 | 7.272 | 6.73 | 7.633 | 6.884 | 8.232 | 7.299 | |
| Convergence, Y/N | Yes | |
| Cap | 491 | 497 | 541 | 472 | 520 | 436 | 492 | |
| Service Time | 5.379 | 5.011 | 4.469 | 5.38 | 4.631 | 5.977 | 5.043 | |
| HCM Lane V/C Ratio | 0.617 | 0.606 | 0.834 | 0.347 | 0.106 | 0.204 | 0.148 | |
| HCM Control Delay | 21.6 | 20.7 | 35.8 | 14.4 | 10.4 | 13.1 | 11.3 | |
| HCM Lane LOS | С | С | Е | В | В | В | В | |
| HCM 95th-tile Q | 4.1 | 4 | 8.7 | 1.5 | 0.3 | 0.8 | 0.5 | |

2: Washington Street & Curtiss Street

| Intersection | |
|---------------------------|------|
| Intersection Delay, s/veh | 14.3 |
| Intersection LOS | В |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 4 | | | 4 | | | f. | | | ર્ન | |
| Traffic Vol, veh/h | 38 | 29 | 54 | 12 | 0 | 90 | 0 | 320 | 55 | 52 | 101 | 0 |
| Future Vol, veh/h | 38 | 29 | 54 | 12 | 0 | 90 | 0 | 320 | 55 | 52 | 101 | 0 |
| Peak Hour Factor | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 |
| Heavy Vehicles, % | 2 | 16 | 0 | 7 | 2 | 0 | 0 | 3 | 2 | 0 | 1 | 0 |
| Mvmt Flow | 49 | 38 | 70 | 16 | 0 | 117 | 0 | 416 | 71 | 68 | 131 | 0 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB | | | WB | | | | NB | | SB | | |
| Opposing Approach | WB | | | EB | | | | SB | | NB | | |
| Opposing Lanes | 1 | | | 1 | | | | 1 | | 1 | | |
| Conflicting Approach Left | SB | | | NB | | | | EB | | WB | | |
| Conflicting Lanes Left | 1 | | | 1 | | | | 1 | | 1 | | |
| Conflicting Approach Right | NB | | | SB | | | | WB | | EB | | |
| Conflicting Lanes Right | 1 | | | 1 | | | | 1 | | 1 | | |
| HCM Control Delay | 10.7 | | | 10.1 | | | | 18 | | 11 | | |
| HCM LOS | В | | | В | | | | С | | В | | |

| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 | |
|------------------------|-------|-------|-------|-------|--|
| Vol Left, % | 0% | 31% | 12% | 34% | |
| Vol Thru, % | 85% | 24% | 0% | 66% | |
| Vol Right, % | 15% | 45% | 88% | 0% | |
| Sign Control | Stop | Stop | Stop | Stop | |
| Traffic Vol by Lane | 375 | 121 | 102 | 153 | |
| LT Vol | 0 | 38 | 12 | 52 | |
| Through Vol | 320 | 29 | 0 | 101 | |
| RT Vol | 55 | 54 | 90 | 0 | |
| Lane Flow Rate | 487 | 157 | 132 | 199 | |
| Geometry Grp | 1 | 1 | 1 | 1 | |
| Degree of Util (X) | 0.678 | 0.251 | 0.206 | 0.304 | |
| Departure Headway (Hd) | 5.015 | 5.755 | 5.599 | 5.514 | |
| Convergence, Y/N | Yes | Yes | Yes | Yes | |
| Cap | 721 | 622 | 638 | 650 | |
| Service Time | 3.051 | 3.809 | 3.654 | 3.562 | |
| HCM Lane V/C Ratio | 0.675 | 0.252 | 0.207 | 0.306 | |
| HCM Control Delay | 18 | 10.7 | 10.1 | 11 | |
| HCM Lane LOS | С | В | В | В | |
| HCM 95th-tile Q | 5.3 | 1 | 0.8 | 1.3 | |

3: Washington Street & Rogers Street

| Intersection | | | | | | | | | | | | |
|---------------------------|-----|-----|-----|-----|-----|-----|-----|-----|-------|-----|----------|------|
| Intersection Delay, s/veh | 9.9 | | | | | | | | | | | |
| Intersection LOS | А | | | | | | | | | | | |
| | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| | | | | | | | | | 11511 | 001 | <u> </u> | 00.1 |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 4 | | | 4 | | J. | f) | | | 4 | |
| Traffic Vol, veh/h | 7 | 48 | 22 | 35 | 80 | 33 | 38 | 197 | 31 | 14 | 124 | 14 |
| Future Vol, veh/h | 7 | 48 | 22 | 35 | 80 | 33 | 38 | 197 | 31 | 14 | 124 | 14 |
| Peak Hour Factor | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 | 0.92 |
| Heavy Vehicles, % | 0 | 4 | 0 | 0 | 3 | 9 | 3 | 1 | 7 | 0 | 2 | 0 |
| Mvmt Flow | 8 | 52 | 24 | 38 | 87 | 36 | 41 | 214 | 34 | 15 | 135 | 15 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB | | | WB | | | NB | | | SB | | |
| Opposing Approach | WB | | | EB | | | SB | | | NB | | |
| Opposing Lanes | 1 | | | 1 | | | 1 | | | 2 | | |
| Conflicting Approach Left | SB | | | NB | | | EB | | | WB | | |
| Conflicting Lanes Left | 1 | | | 2 | | | 1 | | | 1 | | |
| Conflicting Approach Right | NB | | | SB | | | WB | | | EB | | |
| Conflicting Lanes Right | 2 | | | 1 | | | 1 | | | 1 | | |
| HCM Control Delay | 8.9 | | | 9.6 | | | 10.5 | | | 9.5 | | |
| HCM LOS | А | | | Α | | | В | | | Α | | |

| Lane | NBLn1 | NBLn2 | EBLn1 | WBLn1 | SBLn1 | |
|------------------------|-------|-------|-------|-------|-------|--|
| Vol Left, % | 100% | 0% | 9% | 24% | 9% | |
| Vol Thru, % | 0% | 86% | 62% | 54% | 82% | |
| Vol Right, % | 0% | 14% | 29% | 22% | 9% | |
| Sign Control | Stop | Stop | Stop | Stop | Stop | |
| Traffic Vol by Lane | 38 | 228 | 77 | 148 | 152 | |
| LT Vol | 38 | 0 | 7 | 35 | 14 | |
| Through Vol | 0 | 197 | 48 | 80 | 124 | |
| RT Vol | 0 | 31 | 22 | 33 | 14 | |
| Lane Flow Rate | 41 | 248 | 84 | 161 | 165 | |
| Geometry Grp | 7 | 7 | 2 | 2 | 5 | |
| Degree of Util (X) | 0.067 | 0.357 | 0.118 | 0.225 | 0.227 | |
| Departure Headway (Hd) | 5.824 | 5.191 | 5.087 | 5.038 | 4.945 | |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | |
| Cap | 611 | 689 | 697 | 706 | 719 | |
| Service Time | 3.594 | 2.959 | 3.171 | 3.11 | 3.022 | |
| HCM Lane V/C Ratio | 0.067 | 0.36 | 0.121 | 0.228 | 0.229 | |
| HCM Control Delay | 9 | 10.8 | 8.9 | 9.6 | 9.5 | |
| HCM Lane LOS | А | В | А | Α | Α | |
| HCM 95th-tile Q | 0.2 | 1.6 | 0.4 | 0.9 | 0.9 | |

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HCM Unsignalized Intersection Capacity Analysis 4: Mackie Place/Fleet Maintenance Access Drive & Curtiss Street/Commuter Lot Access Drive 2

| | • | - | • | • | ← | • | • | † | ~ | - | ļ | 4 |
|-------------------------------|-------|-------|-------|------|-----------|------------|------|----------|------|------|------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | | 4 | | | 4 | | | 4 | |
| Sign Control | | Yield | | | Stop | | | Yield | | | Stop | |
| Traffic Volume (vph) | 8 | 2 | 15 | 1 | 2 | 0 | 64 | 14 | 1 | 0 | 2 | 3 |
| Future Volume (vph) | 8 | 2 | 15 | 1 | 2 | 0 | 64 | 14 | 1 | 0 | 2 | 3 |
| Peak Hour Factor | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 | 0.68 |
| Hourly flow rate (vph) | 12 | 3 | 22 | 1 | 3 | 0 | 94 | 21 | 1 | 0 | 3 | 4 |
| Direction, Lane # | EB 1 | WB 1 | NB 1 | SB 1 | | | | | | | | |
| Volume Total (vph) | 37 | 4 | 116 | 7 | | | | | | | | |
| Volume Left (vph) | 12 | 1 | 94 | 0 | | | | | | | | |
| Volume Right (vph) | 22 | 0 | 1 | 4 | | | | | | | | |
| Hadj (s) | -0.01 | 0.47 | 0.22 | 0.39 | | | | | | | | |
| Departure Headway (s) | 4.2 | 4.7 | 4.2 | 4.5 | | | | | | | | |
| Degree Utilization, x | 0.04 | 0.01 | 0.14 | 0.01 | | | | | | | | |
| Capacity (veh/h) | 835 | 743 | 835 | 786 | | | | | | | | |
| Control Delay (s) | 7.4 | 7.7 | 7.9 | 7.5 | | | | | | | | |
| Approach Delay (s) | 7.4 | 7.7 | 7.9 | 7.5 | | | | | | | | |
| Approach LOS | Α | Α | Α | Α | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Delay | | | 7.7 | | | | | | | | | |
| Level of Service | | | Α | | | | | | | | | |
| Intersection Capacity Utiliza | tion | | 21.9% | IC | U Level o | of Service | | | Α | | | |
| Analysis Period (min) | | | 15 | | | | | | | | | |

5: Belden Avenue/Access Drive & Curtiss Street

| Intersection | | | | | | | | | | | | |
|------------------------|---------|-------|-------|--------|------|------|--------|-------|-------|---------|------|------|
| Int Delay, s/veh | 5 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | | 4 | | | 4 | | | 4 | |
| Traffic Vol, veh/h | 63 | 36 | 13 | 0 | 51 | 15 | 27 | 7 | 2 | 5 | 0 | 22 |
| Future Vol, veh/h | 63 | 36 | 13 | 0 | 51 | 15 | 27 | 7 | 2 | 5 | 0 | 22 |
| Conflicting Peds, #/hr | 3 | 0 | 3 | 3 | 0 | 3 | 0 | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | - | - | - | - | - | - | - | - | - | - | - | - |
| Veh in Median Storage, | # - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Grade, % | - | 0 | - | - | 0 | - | - | 0 | - | - | 0 | - |
| Peak Hour Factor | 63 | 63 | 63 | 63 | 63 | 63 | 63 | 63 | 63 | 63 | 63 | 63 |
| Heavy Vehicles, % | 0 | 31 | 20 | 0 | 10 | 0 | 25 | 4 | 50 | 0 | 0 | 0 |
| Mvmt Flow | 100 | 57 | 21 | 0 | 81 | 24 | 43 | 11 | 3 | 8 | 0 | 35 |
| | | | | | | | | | | | | |
| Major/Minor N | /lajor1 | | 1 | Major2 | | 1 | Minor1 | | N | /linor2 | | |
| Conflicting Flow All | 108 | 0 | 0 | 81 | 0 | 0 | 382 | 379 | 71 | 371 | 377 | 96 |
| Stage 1 | - | - | - | - | - | - | 271 | 271 | - | 96 | 96 | - |
| Stage 2 | - | - | _ | - | - | - | 111 | 108 | - | 275 | 281 | - |
| Critical Hdwy | 4.1 | - | - | 4.1 | - | - | 7.35 | 6.54 | 6.7 | 7.1 | 6.5 | 6.2 |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.35 | 5.54 | - | 6.1 | 5.5 | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | 6.35 | 5.54 | - | 6.1 | 5.5 | - |
| Follow-up Hdwy | 2.2 | - | - | 2.2 | - | - | 3.725 | 4.036 | 3.75 | 3.5 | 4 | 3.3 |
| Pot Cap-1 Maneuver | 1495 | - | - | 1529 | - | - | 536 | 550 | 873 | 589 | 558 | 966 |
| Stage 1 | - | - | - | - | - | - | 687 | 682 | - | 916 | 819 | - |
| Stage 2 | - | - | - | - | - | - | 841 | 802 | - | 736 | 682 | - |
| Platoon blocked, % | | - | - | | - | - | | | | | | |
| Mov Cap-1 Maneuver | 1491 | - | - | 1525 | - | - | 488 | 508 | 871 | 545 | 516 | 963 |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 488 | 508 | - | 545 | 516 | - |
| Stage 1 | - | - | - | - | - | - | 637 | 632 | - | 849 | 817 | - |
| Stage 2 | - | - | - | - | - | - | 811 | 800 | - | 670 | 632 | - |
| | | | | | | | | | | | | |
| Approach | EB | | | WB | | | NB | | | SB | | |
| HCM Control Delay, s | 4.3 | | | 0 | | | 13.1 | | | 9.5 | | |
| HCM LOS | | | | | | | В | | | Α | | |
| | | | | | | | | | | | | |
| Minor Lane/Major Mvm | t ſ | VBLn1 | EBL | EBT | EBR | WBL | WBT | WBR S | SBLn1 | | | |
| Capacity (veh/h) | | 504 | | - | | 1525 | - | - | 843 | | | |
| HCM Lane V/C Ratio | | | 0.067 | - | - | - | _ | _ | 0.051 | | | |
| HCM Control Delay (s) | | 13.1 | 7.6 | 0 | - | 0 | - | - | 9.5 | | | |
| HCM Lane LOS | | В | А | A | - | A | - | - | A | | | |
| HCM 95th %tile Q(veh) | | 0.4 | 0.2 | - | - | 0 | - | - | 0.2 | | | |
| , | | | | | | | | | | | | |

6: Curtiss Street & Westerly Access Drive

| Intersection | | | | | | |
|------------------------|--------|----------|----------|------|--------|-------------------|
| Int Delay, s/veh | 0.8 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| | LDL | | | MDK | | JDK |
| Lane Configurations | 22 | 4 115 | } | 2 | Y | 2 |
| Traffic Vol, veh/h | 22 | 115 | 97 | 2 | 1 | 2 |
| Future Vol, veh/h | 22 | 115 | 97 | 2 | 1 | 2 |
| Conflicting Peds, #/hr | | 0 | 0 | 0 | 0 | 1 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storag | e,# - | 0 | 0 | - | 0 | - |
| Grade, % | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 52 | 52 | 52 | 52 | 52 | 52 |
| Heavy Vehicles, % | 0 | 9 | 7 | 0 | 0 | 25 |
| Mvmt Flow | 42 | 221 | 187 | 4 | 2 | 4 |
| IVIVIIILI IOVV | 42 | ZZ I | 107 | 7 | | 7 |
| | | | | | | |
| Major/Minor | Major1 | ľ | Major2 | N | Minor2 | |
| Conflicting Flow All | 191 | 0 | - | 0 | 494 | 190 |
| Stage 1 | - | - | - | - | 189 | - |
| Stage 2 | - | _ | - | - | 305 | - |
| Critical Hdwy | 4.1 | _ | - | _ | 6.4 | 6.45 |
| Critical Hdwy Stg 1 | 7.1 | | _ | _ | 5.4 | - 0.73 |
| | | _ | - | | 5.4 | - |
| Critical Hdwy Stg 2 | - | - | - | - | | |
| Follow-up Hdwy | 2.2 | - | - | - | 3.5 | 3.525 |
| Pot Cap-1 Maneuver | 1395 | - | - | - | 538 | 796 |
| Stage 1 | - | - | - | - | 848 | - |
| Stage 2 | - | - | - | - | 752 | - |
| Platoon blocked, % | | - | - | - | | |
| Mov Cap-1 Maneuver | 1395 | - | - | - | 520 | 795 |
| Mov Cap-2 Maneuver | | _ | _ | _ | 520 | _ |
| Stage 1 | _ | _ | _ | _ | 819 | _ |
| Stage 2 | _ | | _ | | 752 | _ |
| Staye 2 | | | - | - | 732 | - |
| | | | | | | |
| Approach | EB | | WB | | SB | |
| HCM Control Delay, s | 1.2 | | 0 | | 10.4 | |
| HCM LOS | 1.2 | | U | | В | |
| TICIVI EOS | | | | | U | |
| | | | | | | |
| Minor Lane/Major Mvi | mt_ | EBL | EBT | WBT | WBR | SBL _{n1} |
| Capacity (veh/h) | | 1395 | - | - | - | 676 |
| HCM Lane V/C Ratio | | 0.03 | _ | _ | | 0.009 |
| HCM Control Delay (s | :) | 7.7 | 0 | _ | | 10.4 |
| HCM Lane LOS | 9) | Α. | A | - | - | 10.4 B |
| | ۵) | | | - | | |
| HCM 95th %tile Q(vel | 1) | 0.1 | - | - | - | 0 |
| | | | | | | |

7: Curtiss Street & Middle Access Drive

| Intersection | | | | | | |
|------------------------|------------|-------|--------|--------|----------|--------|
| Int Delay, s/veh | 1.1 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | | 4 | 1≯ | TT DIC | Y | OBIN |
| Traffic Vol, veh/h | 15 | 24 | 67 | 2 | 1 | 0 |
| Future Vol, veh/h | 15 | 24 | 67 | 2 | 1 | 0 |
| Conflicting Peds, #/hr | | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | _ | - | - | - | 0 | - |
| Veh in Median Storag | e.# - | 0 | 0 | _ | 0 | _ |
| Grade, % | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| Heavy Vehicles, % | 0 | 7 | 3 | 0 | 0 | 0 |
| Mvmt Flow | 16 | 26 | 73 | 2 | 1 | 0 |
| WWW. Tiow | 10 | 20 | 70 | _ | • | U |
| | | | | | | |
| | Major1 | | Major2 | | Minor2 | |
| Conflicting Flow All | 75 | 0 | - | 0 | 132 | 74 |
| Stage 1 | - | - | - | - | 74 | - |
| Stage 2 | - | - | - | - | 58 | - |
| Critical Hdwy | 4.1 | - | - | - | 6.4 | 6.2 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.4 | - |
| Critical Hdwy Stg 2 | - | - | - | - | 5.4 | - |
| Follow-up Hdwy | 2.2 | - | - | - | 3.5 | 3.3 |
| Pot Cap-1 Maneuver | 1537 | - | - | - | 867 | 993 |
| Stage 1 | - | - | - | - | 954 | - |
| Stage 2 | - | - | - | - | 970 | - |
| Platoon blocked, % | | - | - | - | | |
| Mov Cap-1 Maneuver | 1537 | - | - | - | 857 | 993 |
| Mov Cap-2 Maneuver | | _ | - | _ | 857 | _ |
| Stage 1 | _ | _ | _ | _ | 944 | _ |
| Stage 2 | _ | _ | _ | _ | 970 | _ |
| Stage 2 | | | | | 770 | |
| | | | | | | |
| Approach | EB | | WB | | SB | |
| HCM Control Delay, s | 2.8 | | 0 | | 9.2 | |
| HCM LOS | | | | | Α | |
| | | | | | | |
| Minor Lane/Major Mvi | mt | EBL | EBT | WBT | WBR S | SRI n1 |
| Capacity (veh/h) | TIL . | 1537 | LDI | WDI | VVDIX . | 857 |
| HCM Lane V/C Ratio | | | - | - | - | |
| | .) | 0.011 | - | - | | 0.001 |
| HCM Long LOS |) | 7.4 | 0 | - | - | 9.2 |
| HCM Lane LOS | 5) | A | Α | - | - | A |
| HCM 95th %tile Q(vel | n) | 0 | - | - | - | 0 |
| | | | | | | |

8: Apartment Drive & Washinton Street

| Intersection | | | | | | |
|------------------------|----------|-------|--------|------|--------|-------|
| Int Delay, s/veh | 0.3 | | | | | |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | <u>₽</u> | | | 4 | ¥ | 7,2,1 |
| Traffic Vol, veh/h | 286 | 2 | 6 | 180 | 4 | 2 |
| Future Vol, veh/h | 286 | 2 | 6 | 180 | 4 | 2 |
| Conflicting Peds, #/hr | | 7 | 7 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | _ | - | _ | - | 0 | - |
| Veh in Median Storag | e,# 0 | - | _ | 0 | 0 | _ |
| Grade, % | 0 | _ | _ | 0 | 0 | _ |
| Peak Hour Factor | 82 | 82 | 82 | 82 | 82 | 82 |
| | 3 | 02 | | 2 | 15 | |
| Heavy Vehicles, % | | | 0 | | | 0 |
| Mvmt Flow | 349 | 2 | 7 | 220 | 5 | 2 |
| | | | | | | |
| Major/Minor | Major1 | ľ | Major2 | ı | Minor1 | |
| Conflicting Flow All | 0 | 0 | 358 | 0 | 591 | 357 |
| Stage 1 | - | - | - | - | 357 | _ |
| Stage 2 | - | _ | _ | _ | 234 | _ |
| Critical Hdwy | - | _ | 4.1 | - | 6.55 | 6.2 |
| Critical Hdwy Stg 1 | _ | _ | | _ | 5.55 | - 0.2 |
| | | _ | | | 5.55 | |
| Critical Hdwy Stg 2 | - | - | - | - | | - |
| Follow-up Hdwy | - | - | 2.2 | | 0.000 | 3.3 |
| Pot Cap-1 Maneuver | - | - | 1212 | - | 449 | 692 |
| Stage 1 | - | - | - | - | 680 | - |
| Stage 2 | - | - | - | - | 775 | - |
| Platoon blocked, % | - | - | | - | | |
| Mov Cap-1 Maneuver | · - | - | 1204 | - | 443 | 687 |
| Mov Cap-2 Maneuver | - | - | - | - | 443 | - |
| Stage 1 | - | - | - | - | 675 | - |
| Stage 2 | - | - | - | - | 770 | - |
| J | | | | | | |
| | | | | | | |
| Approach | EB | | WB | | NB | |
| HCM Control Delay, s | 0 | | 0.3 | | 12.3 | |
| HCM LOS | | | | | В | |
| | | | | | | |
| Minor Lane/Major Mvi | mt N | NBLn1 | EBT | EBR | WBL | WBT |
| | nt I | | | LDK | | VVDI |
| Capacity (veh/h) | | 502 | - | - | 1204 | - |
| HCM Lane V/C Ratio | | 0.015 | - | - | 0.006 | - |
| HCM Control Delay (s | 5) | 12.3 | - | - | 8 | 0 |
| HCM Lane LOS | | В | - | - | Α | Α |
| HCM 95th %tile Q(vel | n) | 0 | - | - | 0 | - |
| | | | | | | |

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HCM Unsignalized Intersection Capacity Analysis 9: Washington Street & Burlington Street

| | ٠ | • | 4 | † | ļ | ✓ | |
|------------------------------|-------|------|-------|----------|------------|-----------|--|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR | |
| Lane Configurations | | | ሻ | † | † | | |
| Traffic Volume (veh/h) | 0 | 0 | 153 | 287 | 147 | 0 | |
| Future Volume (Veh/h) | 0 | 0 | 153 | 287 | 147 | 0 | |
| Sign Control | Stop | | | Free | Free | | |
| Grade | 0% | | | 0% | 0% | | |
| Peak Hour Factor | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | 0.88 | |
| Hourly flow rate (vph) | 0 | 0 | 174 | 326 | 167 | 0 | |
| Pedestrians | 27 | | | | | | |
| Lane Width (ft) | 0.0 | | | | | | |
| Walking Speed (ft/s) | 3.5 | | | | | | |
| Percent Blockage | 0 | | | | | | |
| Right turn flare (veh) | | | | | | | |
| Median type | | | | None | None | | |
| Median storage veh) | | | | | | | |
| Upstream signal (ft) | | | | | | | |
| pX, platoon unblocked | | | | | | | |
| vC, conflicting volume | 868 | 194 | 194 | | | | |
| vC1, stage 1 conf vol | | | | | | | |
| vC2, stage 2 conf vol | | | | | | | |
| vCu, unblocked vol | 868 | 194 | 194 | | | | |
| tC, single (s) | 6.4 | 6.2 | 4.1 | | | | |
| tC, 2 stage (s) | | | | | | | |
| tF (s) | 3.5 | 3.3 | 2.2 | | | | |
| p0 queue free % | 100 | 100 | 87 | | | | |
| cM capacity (veh/h) | 282 | 847 | 1367 | | | | |
| Direction, Lane # | NB 1 | NB 2 | SB 1 | | | | |
| Volume Total | 174 | 326 | 167 | | | | |
| Volume Left | 174 | 0 | 0 | | | | |
| Volume Right | 0 | 0 | 0 | | | | |
| cSH | 1367 | 1700 | 1700 | | | | |
| Volume to Capacity | 0.13 | 0.19 | 0.10 | | | | |
| Queue Length 95th (ft) | 11 | 0 | 0 | | | | |
| Control Delay (s) | 8.0 | 0.0 | 0.0 | | | | |
| Lane LOS | А | | | | | | |
| Approach Delay (s) | 2.8 | | 0.0 | | | | |
| Approach LOS | | | | | | | |
| Intersection Summary | | | | | | | |
| Average Delay | | | 2.1 | | | | |
| Intersection Capacity Utiliz | ation | | 26.2% | IC | CU Level o | f Service | |
| Analysis Period (min) | | | 15 | | | | |

HCM Unsignalized Intersection Capacity Analysis 10: Washington Street

| | • | → | — | 4 | / | 1 |
|------------------------------|-------|----------|----------|------|-------------|------------|
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | * | | | | | 7 |
| Traffic Volume (veh/h) | 288 | 0 | 0 | 0 | 0 | 186 |
| Future Volume (Veh/h) | 288 | 0 | 0 | 0 | 0 | 186 |
| Sign Control | | Stop | Stop | | Free | |
| Grade | | 0% | 0% | | 0% | |
| Peak Hour Factor | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 | 0.82 |
| Hourly flow rate (vph) | 351 | 0 | 0 | 0 | 0 | 227 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | | None | |
| Median storage veh) | | | | | | |
| Upstream signal (ft) | | | | | 76 | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 0 | 0 | 227 | 0 | 0 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 0 | 0 | 227 | 0 | 0 | |
| tC, single (s) | 7.1 | 6.5 | 6.5 | 6.2 | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 4.0 | 4.0 | 3.3 | 2.2 | |
| p0 queue free % | 66 | 100 | 100 | 100 | 100 | |
| cM capacity (veh/h) | 1021 | 896 | 672 | 1085 | 1623 | |
| Direction, Lane # | EB 1 | SB 1 | | | | |
| Volume Total | 351 | 227 | | | | |
| Volume Left | 351 | 0 | | | | |
| Volume Right | 0 | 227 | | | | |
| cSH | 1021 | 1700 | | | | |
| Volume to Capacity | 0.34 | 0.13 | | | | |
| Queue Length 95th (ft) | 39 | 0 | | | | |
| Control Delay (s) | 10.4 | 0.0 | | | | |
| Lane LOS | В | 3.0 | | | | |
| Approach Delay (s) | 10.4 | 0.0 | | | | |
| Approach LOS | В | 0.0 | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 6.3 | | | |
| Intersection Capacity Utiliz | ation | | 19.3% | IC | CU Level o | of Service |
| Analysis Period (min) | .uuon | | 15 | 10 | , C LOVOI C | / Oct vice |
| Analysis renou (IIIII) | | | 10 | | | |

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Intersection Capacity Utilization 11: 0/Washington Street & Warren Avenue

01/28/2022

| | ۶ | → | • | • | ← | • | • | † | / | > | ļ | 4 |
|-------------------------------|------------|-----------|------------|------------|-----------|------------|------|----------|------|-------------|------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | | 4 | | | 4 | | | 4 | |
| Volume (vph) | 7 | 3 | 14 | 3 | 5 | 2 | 23 | 256 | 10 | 1 | 170 | 9 |
| Pedestrians | 1 | | 22 | 22 | | 1 | 28 | | 6 | 6 | | 28 |
| Ped Button | | Yes | | | Yes | | | Yes | | | Yes | |
| Pedestrian Timing (s) | | 16.0 | | | 16.0 | | | 16.0 | | | 16.0 | |
| Free Right | | | No | | | No | | | No | | | No |
| Ideal Flow | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Green (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Refr Cycle Length (s) | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 |
| Volume Combined (vph) | 0 | 24 | 0 | 0 | 10 | 0 | 0 | 289 | 0 | 0 | 180 | 0 |
| Lane Utilization Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Factor (vph) | 0.95 | 0.90 | 0.85 | 0.95 | 0.96 | 0.85 | 0.95 | 0.99 | 0.85 | 0.95 | 0.99 | 0.85 |
| Saturated Flow (vph) | 0 | 1708 | 0 | 0 | 1815 | 0 | 0 | 1883 | 0 | 0 | 1885 | 0 |
| Ped Intf Time (s) | 0.0 | 1.5 | 2.5 | 0.0 | 0.0 | 0.1 | 0.0 | 0.0 | 0.8 | 0.0 | 0.2 | 3.1 |
| Pedestrian Frequency (%) | | 0.52 | | | 0.03 | | | 0.18 | | | 0.61 | |
| Protected Option Allowed | | No | | | No | | | No | | | No | |
| Reference Time (s) | | 110 | 0.0 | | 140 | 0.0 | | 110 | 0.0 | | 110 | 0.0 |
| Adj Reference Time (s) | | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 |
| Permitted Option | | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 |
| Adj Saturation A (vph) | 0 | 1304 | | 0 | 1356 | | 0 | 943 | | 0 | 1850 | |
| Reference Time A (s) | 0.0 | 3.7 | | 0.0 | 0.9 | | 0.0 | 36.8 | | 0.0 | 11.8 | |
| Adj Saturation B (vph | 0.0 | 0 | | 0.0 | 0.7 | | NA | NA | | NA | NA | |
| Reference Time B (s) | 8.5 | 11.2 | | 8.2 | 8.7 | | NA | NA | | NA | NA | |
| Reference Time (s) | 0.5 | 3.7 | | 0.2 | 0.7 | | IVA | 36.8 | | IVA | 11.8 | |
| Adj Reference Time (s) | | 14.2 | | | 8.4 | | | 40.8 | | | 18.4 | |
| Split Option | | 17.2 | | | υ, τ | | | 70.0 | | | 10.4 | |
| Ref Time Combined (s) | 0.0 | 3.2 | | 0.0 | 0.7 | | 0.0 | 18.4 | | 0.0 | 11.6 | |
| Ref Time Seperate (s) | 0.0 | 1.7 | | 0.0 | 0.7 | | 1.5 | 16.3 | | 0.0 | 11.0 | |
| Reference Time (s) | 3.2 | 3.2 | | 0.2 | 0.4 | | 18.4 | 18.4 | | 11.6 | 11.6 | |
| Adj Reference Time (s) | 14.2 | 14.2 | | 8.4 | 8.4 | | 22.4 | 22.4 | | 18.3 | 18.3 | |
| Auj Reference Time (S) | 14.2 | 14.2 | | 0.4 | 0.4 | | 22.4 | 22.4 | | 10.3 | 10.3 | |
| Summary | EB WB | | NB SB | Со | mbined | | | | | | | |
| Protected Option (s) | NA | | NA | | | | | | | | | |
| Permitted Option (s) | 14.2 | | 40.8 | | | | | | | | | |
| Split Option (s) | 22.6 | | 40.7 | | | | | | | | | |
| Minimum (s) | 14.2 | | 40.7 | | 55.0 | | | | | | | |
| Right Turns | | | | | | | | | | | | |
| | | | | | | | | | | | | |
| Adj Reference Time (s) | | | | | | | | | | | | |
| Cross Thru Ref Time (s) | | | | | | | | | | | | |
| Oncoming Left Ref Time (s) | | | | | | | | | | | | |
| Combined (s) | | | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Intersection Capacity Utiliza | tion | | 45.8% | IC | CU Level | of Service | ! | | Α | | | |
| Reference Times and Phasi | ng Options | do not re | epresent a | ın optimiz | ed timing | plan. | | | | | | |
| | | | | | | | | | | | | |

19-048 - Village Hall Redevelopment - Downers Grove Projected Weekday Morning Peak Hour

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<u>Capacity Analysis Summary Reports</u> Weekday Evening Peak Hour – Projected Conditions

1: Washingotn Street/Washington Street & Maple Avenue

| Intersection | | | | | | | | | | | | |
|---------------------------|------|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|-----|
| Intersection Delay, s/veh | 20.5 | | | | | | | | | | | |
| Intersection LOS | С | | | | | | | | | | | |
| | | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | , | f) | | | ર્ન | 7 | | 4 | | | ર્ન | 7 |
| Traffic Vol, veh/h | 124 | 229 | 10 | 14 | 335 | 50 | 7 | 83 | 16 | 84 | 152 | 286 |
| Future Vol. veh/h | 124 | 229 | 10 | 14 | 335 | 50 | 7 | 83 | 16 | 84 | 152 | 286 |

| Traffic Vol, veh/h | 124 | 229 | 10 | 14 | 335 | 50 | 7 | 83 | 16 | 84 | 152 | 286 |
|----------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Future Vol, veh/h | 124 | 229 | 10 | 14 | 335 | 50 | 7 | 83 | 16 | 84 | 152 | 286 |
| Peak Hour Factor | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 | 0.95 |
| Heavy Vehicles, % | 5 | 1 | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 1 | 1 |
| Mvmt Flow | 131 | 241 | 11 | 15 | 353 | 53 | 7 | 87 | 17 | 88 | 160 | 301 |
| Number of Lanes | 1 | 1 | 0 | 0 | 1 | 1 | 0 | 1 | 0 | 0 | 1 | 1 |
| Approach | EB | | | WB | | | NB | | | SB | | |
| Opposing Approach | WB | | | EB | | | SB | | | NB | | |
| Opposing Lanes | 2 | | | 2 | | | 2 | | | 1 | | |
| Conflicting Approach Left | SB | | | NB | | | EB | | | WB | | |
| Conflicting Lanes Left | 2 | | | 1 | | | 2 | | | 2 | | |
| Conflicting Approach Right | NB | | | SB | | | WB | | | EB | | |
| Conflicting Lanes Right | 1 | | | 2 | | | 2 | | | 2 | | |
| HCM Control Delay | 17.1 | | | 28.1 | | | 14.4 | | | 18.2 | | |
| HCM LOS | С | | | D | | | В | | | С | | |

| Lane | NBLn1 | EBLn1 | EBLn2 | WBLn1 | WBLn2 | SBLn1 | SBLn2 | |
|------------------------|-------|-------|-------|-------|-------|-------|-------|--|
| Vol Left, % | 7% | 100% | 0% | 4% | 0% | 36% | 0% | |
| Vol Thru, % | 78% | 0% | 96% | 96% | 0% | 64% | 0% | |
| Vol Right, % | 15% | 0% | 4% | 0% | 100% | 0% | 100% | |
| Sign Control | Stop | |
| Traffic Vol by Lane | 106 | 124 | 239 | 349 | 50 | 236 | 286 | |
| LT Vol | 7 | 124 | 0 | 14 | 0 | 84 | 0 | |
| Through Vol | 83 | 0 | 229 | 335 | 0 | 152 | 0 | |
| RT Vol | 16 | 0 | 10 | 0 | 50 | 0 | 286 | |
| Lane Flow Rate | 112 | 131 | 252 | 367 | 53 | 248 | 301 | |
| Geometry Grp | 6 | 7 | 7 | 7 | 7 | 7 | 7 | |
| Degree of Util (X) | 0.26 | 0.296 | 0.527 | 0.763 | 0.099 | 0.525 | 0.563 | |
| Departure Headway (Hd) | 8.396 | 8.154 | 7.539 | 7.475 | 6.752 | 7.614 | 6.732 | |
| Convergence, Y/N | Yes | |
| Cap | 426 | 440 | 478 | 485 | 529 | 472 | 536 | |
| Service Time | 6.48 | 5.918 | 5.302 | 5.234 | 4.511 | 5.373 | 4.49 | |
| HCM Lane V/C Ratio | 0.263 | 0.298 | 0.527 | 0.757 | 0.1 | 0.525 | 0.562 | |
| HCM Control Delay | 14.4 | 14.3 | 18.5 | 30.6 | 10.3 | 18.5 | 17.9 | |
| HCM Lane LOS | В | В | С | D | В | С | С | |
| HCM 95th-tile Q | 1 | 1.2 | 3 | 6.6 | 0.3 | 3 | 3.5 | |

2: Washington Street & Curtiss Street

| Intersection | |
|---------------------------|----|
| Intersection Delay, s/veh | 12 |
| Intersection LOS | В |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | ₩ | | | 4 | | | f) | | | ર્ન | |
| Traffic Vol, veh/h | 92 | 18 | 98 | 72 | 0 | 75 | 0 | 161 | 15 | 74 | 188 | 0 |
| Future Vol, veh/h | 92 | 18 | 98 | 72 | 0 | 75 | 0 | 161 | 15 | 74 | 188 | 0 |
| Peak Hour Factor | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 | 0.85 |
| Heavy Vehicles, % | 0 | 0 | 1 | 0 | 0 | 0 | 0 | 5 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 108 | 21 | 115 | 85 | 0 | 88 | 0 | 189 | 18 | 87 | 221 | 0 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB | | | WB | | | | NB | | SB | | |
| Opposing Approach | WB | | | EB | | | | SB | | NB | | |
| Opposing Lanes | 1 | | | 1 | | | | 1 | | 1 | | |
| Conflicting Approach Left | SB | | | NB | | | | EB | | WB | | |
| Conflicting Lanes Left | 1 | | | 1 | | | | 1 | | 1 | | |
| Conflicting Approach Right | NB | | | SB | | | | WB | | EB | | |
| Conflicting Lanes Right | 1 | | | 1 | | | | 1 | | 1 | | |
| HCM Control Delay | 11.7 | | | 10.7 | | | | 11.4 | | 13.3 | | |
| HCM LOS | В | | | В | | | | В | | В | | |

| Lane | NBLn1 | EBLn1 | WBLn1 | SBLn1 | |
|------------------------|-------|-------|-------|-------|--|
| Vol Left, % | 0% | 44% | 49% | 28% | |
| Vol Thru, % | 91% | 9% | 0% | 72% | |
| Vol Right, % | 9% | 47% | 51% | 0% | |
| Sign Control | Stop | Stop | Stop | Stop | |
| Traffic Vol by Lane | 176 | 208 | 147 | 262 | |
| LT Vol | 0 | 92 | 72 | 74 | |
| Through Vol | 161 | 18 | 0 | 188 | |
| RT Vol | 15 | 98 | 75 | 0 | |
| Lane Flow Rate | 207 | 245 | 173 | 308 | |
| Geometry Grp | 1 | 1 | 1 | 1 | |
| Degree of Util (X) | 0.323 | 0.37 | 0.267 | 0.469 | |
| Departure Headway (Hd) | 5.622 | 5.446 | 5.563 | 5.475 | |
| Convergence, Y/N | Yes | Yes | Yes | Yes | |
| Cap | 638 | 658 | 643 | 657 | |
| Service Time | 3.68 | 3.504 | 3.626 | 3.527 | |
| HCM Lane V/C Ratio | 0.324 | 0.372 | 0.269 | 0.469 | |
| HCM Control Delay | 11.4 | 11.7 | 10.7 | 13.3 | |
| HCM Lane LOS | В | В | В | В | |
| HCM 95th-tile Q | 1.4 | 1.7 | 1.1 | 2.5 | |

3: Washington Street & Rogers Street

| Intersection Delay, s/veh 10.3 Intersection LOS B |
|---|
| |
| Intersection LOS B |

| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
|----------------------------|------|------|------|------|------|------|------|------|------|------|------|------|
| Lane Configurations | | 4 | | | 4 | | ሻ | ₽ | | | 4 | |
| Traffic Vol, veh/h | 13 | 59 | 31 | 44 | 67 | 25 | 16 | 174 | 27 | 13 | 195 | 17 |
| Future Vol, veh/h | 13 | 59 | 31 | 44 | 67 | 25 | 16 | 174 | 27 | 13 | 195 | 17 |
| Peak Hour Factor | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 | 0.91 |
| Heavy Vehicles, % | 0 | 2 | 0 | 2 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 14 | 65 | 34 | 48 | 74 | 27 | 18 | 191 | 30 | 14 | 214 | 19 |
| Number of Lanes | 0 | 1 | 0 | 0 | 1 | 0 | 1 | 1 | 0 | 0 | 1 | 0 |
| Approach | EB | | | WB | | | NB | | | SB | | |
| Opposing Approach | WB | | | EB | | | SB | | | NB | | |
| Opposing Lanes | 1 | | | 1 | | | 1 | | | 2 | | |
| Conflicting Approach Left | SB | | | NB | | | EB | | | WB | | |
| Conflicting Lanes Left | 1 | | | 2 | | | 1 | | | 1 | | |
| Conflicting Approach Right | NB | | | SB | | | WB | | | EB | | |
| Conflicting Lanes Right | 2 | | | 1 | | | 1 | | | 1 | | |
| HCM Control Delay | 9.3 | | | 9.9 | | | 10.7 | | | 10.7 | | |
| HCM LOS | Α | | | Α | | | В | | | В | | |

| Lane | NBLn1 | NBLn2 | EBLn1 | WBLn1 | SBLn1 | |
|------------------------|-------|-------|-------|-------|-------|--|
| Vol Left, % | 100% | 0% | 13% | 32% | 6% | |
| Vol Thru, % | 0% | 87% | 57% | 49% | 87% | |
| Vol Right, % | 0% | 13% | 30% | 18% | 8% | |
| Sign Control | Stop | Stop | Stop | Stop | Stop | |
| Traffic Vol by Lane | 16 | 201 | 103 | 136 | 225 | |
| LT Vol | 16 | 0 | 13 | 44 | 13 | |
| Through Vol | 0 | 174 | 59 | 67 | 195 | |
| RT Vol | 0 | 27 | 31 | 25 | 17 | |
| Lane Flow Rate | 18 | 221 | 113 | 149 | 247 | |
| Geometry Grp | 7 | 7 | 2 | 2 | 5 | |
| Degree of Util (X) | 0.029 | 0.327 | 0.166 | 0.222 | 0.341 | |
| Departure Headway (Hd) | 6.019 | 5.444 | 5.271 | 5.351 | 5.064 | |
| Convergence, Y/N | Yes | Yes | Yes | Yes | Yes | |
| Cap | 598 | 665 | 684 | 676 | 715 | |
| Service Time | 3.727 | 3.144 | 3.276 | 3.351 | 3.064 | |
| HCM Lane V/C Ratio | 0.03 | 0.332 | 0.165 | 0.22 | 0.345 | |
| HCM Control Delay | 8.9 | 10.8 | 9.3 | 9.9 | 10.7 | |
| HCM Lane LOS | А | В | Α | Α | В | |
| HCM 95th-tile Q | 0.1 | 1.4 | 0.6 | 8.0 | 1.5 | |

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HCM Unsignalized Intersection Capacity Analysis 4: Mackie Place/Fleet Maintenance Access Drive & Curtiss Street/Commuter Lot Access Drive 2

| • | - | • | • | • | • | 1 | † | ~ | - | ţ | 4 |
|-------|--|----------|----------|-----------|---------------|---------------|---|---|---------------------|---|--|
| EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| | 4 | | | 4 | | | 4 | | | 4 | |
| | Yield | | | Stop | | | Yield | | | Stop | |
| 8 | 0 | 59 | 0 | 0 | 0 | 27 | 1 | 0 | 0 | 7 | 4 |
| 8 | 0 | 59 | 0 | 0 | 0 | | 1 | 0 | 0 | 7 | 4 |
| 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 | 0.74 |
| 11 | 0 | 80 | 0 | 0 | 0 | 36 | 1 | 0 | 0 | 9 | 5 |
| EB 1 | WB 1 | NB 1 | SB 1 | | | | | | | | |
| 91 | 0 | 37 | 14 | | | | | | | | |
| 11 | 0 | 36 | 0 | | | | | | | | |
| 80 | 0 | 0 | 5 | | | | | | | | |
| -0.50 | 0.00 | 0.19 | -0.21 | | | | | | | | |
| 3.5 | 4.1 | | 3.9 | | | | | | | | |
| 0.09 | 0.00 | 0.04 | 0.02 | | | | | | | | |
| 1005 | 874 | 813 | 897 | | | | | | | | |
| 6.9 | 7.1 | 7.5 | 7.0 | | | | | | | | |
| | 0.0 | 7.5 | | | | | | | | | |
| Α | Α | Α | Α | | | | | | | | |
| | | | | | | | | | | | |
| | | 7.0 | | | | | | | | | |
| | | А | | | | | | | | | |
| ion | | 27.0% | IC | U Level o | of Service | | | Α | | | |
| | | 15 | | | | | | | | | |
| | 8 8 0.74 11 EB 1 91 11 80 -0.50 3.5 0.09 1005 6.9 A | Yield 8 | Yield 8 | Yield 8 | Yield Stop 8 | Yield Stop 8 | Yield Stop 8 0 59 0 0 0 27 8 0 59 0 0 0 27 0.74 0.74 0.74 0.74 0.74 0.74 0.74 11 0 80 0 0 0 36 EB 1 WB 1 NB 1 SB | Yield Stop Yield 8 0 59 0 0 0 27 1 8 0 59 0 0 0 27 1 0.74 0.74 0.74 0.74 0.74 0.74 0.74 0.74 11 0 80 0 0 0 36 1 EB 1 WB 1 NB 1 SB 1 | Yield Stop Yield 8 | Yield Stop Yield 8 0 59 0 0 0 27 1 0 0 8 0 59 0 0 0 27 1 0 0 0.74 | Yield Stop Yield Stop 8 0 59 0 0 0 27 1 0 0 7 8 0 59 0 0 0 27 1 0 0 7 0.74 0. |

5: Belden Avenue/Access Drive & Curtiss Street

| Intersection | | | | | | | | | | | | |
|------------------------|---------|-------|-------|---------|------|-------|--------|-----------|----------|------------|------|----------|
| Int Delay, s/veh | 4.1 | | | | | | | | | | | |
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | | 4 | | | 4 | | | 4 | |
| Traffic Vol., veh/h | 7 | 46 | 57 | 2 | 39 | 5 | 15 | 0 | 1 | 18 | 7 | 63 |
| Future Vol, veh/h | 7 | 46 | 57 | 2 | 39 | 5 | 15 | 0 | 1 | 18 | 7 | 63 |
| Conflicting Peds, #/hr | 0 | 0 | 5 | 5 | 0 | 0 | 7 | 0 | 0 | 0 | 0 | 7 |
| Sign Control | Free | Free | Free | Free | Free | Free | Stop | Stop | Stop | Stop | Stop | Stop |
| RT Channelized | - | - | None | - | - | None | - | - | None | - | - | None |
| Storage Length | _ | | - | _ | _ | - | _ | _ | - | _ | _ | - |
| Veh in Median Storage, | # - | 0 | _ | _ | 0 | _ | _ | 0 | _ | _ | 0 | _ |
| Grade, % | | 0 | _ | _ | 0 | _ | _ | 0 | _ | _ | 0 | _ |
| Peak Hour Factor | 85 | 85 | 85 | 85 | 85 | 85 | 85 | 85 | 85 | 85 | 85 | 85 |
| Heavy Vehicles, % | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mymt Flow | 8 | 54 | 67 | 2 | 46 | 6 | 18 | 0 | 1 | 21 | 8 | 74 |
| | | 01 | | | - 10 | | 10 | | - | L 1 | | |
| Major/Minor M | lajor1 | | N | Major2 | | N | Minor1 | | N | /linor2 | | |
| Conflicting Flow All | 52 | 0 | 0 | 126 | 0 | 0 | 210 | 165 | 93 | 157 | 195 | 56 |
| | 52 | - | U | 120 | | - | 109 | 109 | 93 | 53 | 53 | |
| Stage 1 | | | - | | - | | | | | | | - |
| Stage 2 | - 11 | - | - | - 11 | - | - | 101 | 56 4 F | - 4 2 | 104 | 142 | - 4 2 |
| Critical Edwy | 4.1 | - | - | 4.1 | - | - | 7.1 | 6.5 | 6.2 | 7.1 | 6.5 | 6.2 |
| Critical Hdwy Stg 1 | - | - | - | - | - | - | 6.1 | 5.5 | - | 6.1 | 5.5 | - |
| Critical Hdwy Stg 2 | - | - | - | - | - | - | 6.1 | 5.5 | - | 6.1 | 5.5 | - 2.2 |
| Follow-up Hdwy | 2.2 | - | - | 2.2 | - | - | 3.5 | 4 | 3.3 | 3.5 | 4 | 3.3 |
| | 1567 | - | - | 1473 | - | - | 752 | 731 | 970 | 814 | 704 | 1016 |
| Stage 1 | - | - | - | - | - | - | 901 | 809 | - | 965 | 855 | - |
| Stage 2 | - | - | - | - | - | - | 910 | 852 | - | 907 | 783 | - |
| Platoon blocked, % | 45/3 | - | - | 4411 | - | - | , = 0 | 700 | 0.45 | 000 | | 4000 |
| | 1567 | - | - | 1466 | - | - | 679 | 722 | 965 | 808 | 696 | 1009 |
| Mov Cap-2 Maneuver | - | - | - | - | - | - | 679 | 722 | - | 808 | 696 | - |
| Stage 1 | - | - | - | - | - | - | 891 | 800 | - | 959 | 854 | - |
| Stage 2 | - | - | - | - | - | - | 829 | 851 | - | 900 | 774 | - |
| | | | | | | | | | | | | |
| Approach | EB | | | WB | | | NB | | | SB | | |
| HCM Control Delay, s | 0.5 | | | 0.3 | | | 10.3 | | | 9.4 | | |
| HCM LOS | | | | | | | В | | | Α | | |
| | | | | | | | | | | | | |
| Minor Lane/Major Mvmt | | NBLn1 | EBL | EBT | EBR | WBL | WBT | WBR: | SBLn1 | | | |
| Capacity (veh/h) | | 692 | 1567 | - | - | 1466 | - | - | 929 | | | |
| HCM Lane V/C Ratio | | 0.027 | 0.005 | - | - | 0.002 | - | - | 0.111 | | | |
| HCM Control Delay (s) | | 10.3 | 7.3 | 0 | - | 7.5 | 0 | - | 9.4 | | | |
| HCM Lane LOS | | В | А | Α | - | А | Α | - | Α | | | |
| HCM 95th %tile Q(veh) | | 0.1 | 0 | - | - | 0 | - | - | 0.4 | | | |
| | | | | | | | | | | | | |

6: Curtiss Street & Westerly Access Drive

| Intersection | | | | | | |
|------------------------|--------|---------------|----------|--------|--------|----------|
| Int Delay, s/veh | 1.5 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | 202 | 4 | 1 | TT DIC | ₩. | OBIN |
| Traffic Vol, veh/h | 6 | 98 | 112 | 4 | 12 | 25 |
| Future Vol, veh/h | 6 | 98 | 112 | 4 | 12 | 25 |
| Conflicting Peds, #/hr | | 0 | 0 | 0 | 12 | 23 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | | | | None | | |
| | - | None | - | | - | None |
| Storage Length | - " | - | - | - | 0 | - |
| Veh in Median Storag | | 0 | 0 | - | 0 | - |
| Grade, % | - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 87 | 87 | 87 | 87 | 87 | 87 |
| Heavy Vehicles, % | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 7 | 113 | 129 | 5 | 14 | 29 |
| | | | | | | |
| | | _ | | _ | | |
| Major/Minor | Major1 | | Major2 | | Vinor2 | |
| Conflicting Flow All | 134 | 0 | - | 0 | 260 | 134 |
| Stage 1 | - | - | - | - | 132 | - |
| Stage 2 | - | - | - | - | 128 | - |
| Critical Hdwy | 4.1 | - | - | - | 6.4 | 6.2 |
| Critical Hdwy Stg 1 | - | - | - | - | 5.4 | - |
| Critical Hdwy Stg 2 | _ | _ | _ | _ | 5.4 | _ |
| Follow-up Hdwy | 2.2 | _ | _ | _ | 3.5 | 3.3 |
| Pot Cap-1 Maneuver | 1463 | | | _ | 733 | 920 |
| | 1405 | - | - | - | 899 | 720 |
| Stage 1 | | - | - | | | |
| Stage 2 | - | - | - | - | 903 | - |
| Platoon blocked, % | | - | - | - | | |
| Mov Cap-1 Maneuver | | - | - | - | 729 | 918 |
| Mov Cap-2 Maneuver | | - | - | - | 729 | - |
| Stage 1 | - | - | - | - | 895 | - |
| Stage 2 | _ | _ | _ | _ | 903 | _ |
| J 10 J | | | | | | |
| | | | | | | |
| Approach | EB | | WB | | SB | |
| HCM Control Delay, s | 0.4 | | 0 | | 9.5 | |
| HCM LOS | | | | | Α | |
| | | | | | | |
| | | F 5. | | 14/5- | 14/55 | 001 1 |
| Minor Lane/Major Mv | mt | EBL | EBT | WBT | WBR S | SBLn1 |
| Capacity (veh/h) | | 1463 | - | - | - | 847 |
| | | 0.005 | - | - | - | 0.05 |
| HCM Lane V/C Ratio | | | ^ | | _ | 9.5 |
| | s) | 7.5 | 0 | | | |
| HCM Control Delay (s | s) | 7.5 A | | - | _ | А |
| | | 7.5 A 0 | A | - | | A 0.2 |

7: Curtiss Street & Middle Access Drive

| Intersection | | | | | | |
|------------------------|--------------|-------|--------|--------|-----------|--------|
| Int Delay, s/veh | 1.5 | | | | | |
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | | 4 | 4 | TI DIK | ¥ | OBIT |
| Traffic Vol, veh/h | 2 | 63 | 31 | 0 | 4 | 15 |
| Future Vol, veh/h | 2 | 63 | 31 | 0 | 4 | 15 |
| Conflicting Peds, #/hr | | 0 | 0 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | Jiop - | None |
| Storage Length | _ | - | _ | - | 0 | - |
| Veh in Median Storag | | 0 | 0 | _ | 0 | |
| Grade, % | yc, π - - | 0 | 0 | - | 0 | - |
| Peak Hour Factor | 92 | 92 | 92 | 92 | 92 | 92 |
| | | | | | | |
| Heavy Vehicles, % | 0 | 0 | 0 | 0 | 0 | 0 |
| Mvmt Flow | 2 | 68 | 34 | 0 | 4 | 16 |
| | | | | | | |
| Major/Minor | Major1 | Ŋ | Major2 | I | Minor2 | |
| Conflicting Flow All | 34 | 0 | - | 0 | 106 | 34 |
| Stage 1 | - | - | _ | - | 34 | - |
| Stage 2 | _ | _ | _ | _ | 72 | _ |
| Critical Hdwy | 4.1 | | _ | _ | 6.4 | 6.2 |
| Critical Hdwy Stg 1 | 4.1 | | _ | | 5.4 | 0.2 |
| | | | | | | |
| Critical Hdwy Stg 2 | - | - | - | - | 5.4 | - |
| Follow-up Hdwy | 2.2 | - | - | - | 3.5 | 3.3 |
| Pot Cap-1 Maneuver | 1591 | - | - | - | 897 | 1045 |
| Stage 1 | - | - | - | - | 994 | - |
| Stage 2 | - | - | - | - | 956 | - |
| Platoon blocked, % | | - | - | - | | |
| Mov Cap-1 Maneuver | r 1591 | - | - | - | 896 | 1045 |
| Mov Cap-2 Maneuver | r - | - | - | - | 896 | - |
| Stage 1 | - | - | - | - | 993 | - |
| Stage 2 | _ | _ | _ | _ | 956 | _ |
| 51595 | | | | | | |
| | | | | | | |
| Approach | EB | | WB | | SB | |
| HCM Control Delay, s | 5 0.2 | | 0 | | 8.6 | |
| HCM LOS | | | | | Α | |
| | | | | | | |
| Minor Lane/Major Mv | mt | EBL | EBT | WBT | WBR S | CDI n1 |
| | HIL | | LDI | WDI | WDK . | |
| Capacity (veh/h) | | 1591 | - | - | - | 1010 |
| HCM Lane V/C Ratio | | 0.001 | - | - | - | 0.02 |
| HCM Control Delay (s | s) | 7.3 | 0 | - | - | 8.6 |
| HCM Lane LOS | | Α | Α | - | - | Α |
| HCM 95th %tile Q(vel | n) | 0 | - | - | - | 0.1 |
| | | | | | | |

8: Apartment Drive & Washinton Street

| Intersection | | | | | | |
|------------------------|--------|--------|--------|------------|-------------|------|
| Int Delay, s/veh | 0.3 | | | | | |
| Movement | EBT | EBR | WBL | WBT | NBL | NBR |
| Lane Configurations | ₽ | LDIN | VVDL | ₩ <u>₩</u> | ₩. | אטוז |
| Traffic Vol, veh/h | 219 | 1 | 6 | 324 | 'T ' | 5 |
| Future Vol, veh/h | 219 | - | | 324 | | 5 |
| | | 1 | 6 | | 4 | |
| Conflicting Peds, #/hr | 0 | 38 | 38 | 0 | 0 | 0 |
| Sign Control | Free | Free | Free | Free | Stop | Stop |
| RT Channelized | - | None | - | None | - | None |
| Storage Length | - | - | - | - | 0 | - |
| Veh in Median Storage | | - | - | 0 | 0 | - |
| Grade, % | 0 | - | - | 0 | 0 | - |
| Peak Hour Factor | 77 | 77 | 77 | 77 | 77 | 77 |
| Heavy Vehicles, % | 3 | 0 | 0 | 2 | 0 | 0 |
| Mvmt Flow | 284 | 1 | 8 | 421 | 5 | 6 |
| | | | | | | |
| | | | | | | |
| | Major1 | N | Major2 | <u> </u> | Minor1 | |
| Conflicting Flow All | 0 | 0 | 323 | 0 | 760 | 323 |
| Stage 1 | - | - | - | - | 323 | - |
| Stage 2 | - | - | - | - | 437 | - |
| Critical Hdwy | - | - | 4.1 | - | 6.4 | 6.2 |
| Critical Hdwy Stg 1 | _ | _ | _ | - | 5.4 | - |
| Critical Hdwy Stg 2 | - | - | _ | _ | 5.4 | _ |
| Follow-up Hdwy | _ | _ | 2.2 | _ | 3.5 | 3.3 |
| Pot Cap-1 Maneuver | | | 1248 | | 377 | 723 |
| • | - | - | | - | | |
| Stage 1 | - | - | - | - | 738 | - |
| Stage 2 | - | - | - | - | 655 | - |
| Platoon blocked, % | - | - | | - | | |
| Mov Cap-1 Maneuver | - | - | 1203 | - | 360 | 697 |
| Mov Cap-2 Maneuver | - | - | - | - | 360 | - |
| Stage 1 | - | - | - | - | 711 | - |
| Stage 2 | - | - | - | - | 649 | - |
| J | | | | | | |
| | | | | | | |
| Approach | EB | | WB | | NB | |
| HCM Control Delay, s | 0 | | 0.1 | | 12.5 | |
| HCM LOS | | | | | В | |
| | | | | | | |
| Minor Long/Major Mum | at N | UDI p1 | ГОТ | EDD | WDI | WDT |
| Minor Lane/Major Mvm | it ľ | VBLn1 | EBT | EBR | WBL | WBT |
| Capacity (veh/h) | | 492 | - | | 1203 | - |
| HCM Lane V/C Ratio | | 0.024 | - | - | 0.006 | - |
| HCM Control Delay (s) | | 12.5 | - | - | 8 | 0 |
| HCM Lane LOS | | В | - | - | Α | Α |
| HCIVI Lane LOS | | | | | | |
| HCM 95th %tile Q(veh |) | 0.1 | - | - | 0 | - |

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HCM Unsignalized Intersection Capacity Analysis 9: Washington Street & Burlington Street

| | ٠ | • | 1 | † | | 1 |
|-------------------------------|-------|------|-------|----------|------------|------------|
| Movement | EBL | EBR | NBL | NBT | SBT | SBR |
| Lane Configurations | | | ሻ | | + | |
| Traffic Volume (veh/h) | 0 | 0 | 102 | 219 | 273 | 0 |
| Future Volume (Veh/h) | 0 | 0 | 102 | 219 | 273 | 0 |
| Sign Control | Stop | | | Free | Free | |
| Grade | 0% | | | 0% | 0% | |
| Peak Hour Factor | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 | 0.73 |
| Hourly flow rate (vph) | 0 | 0 | 140 | 300 | 374 | 0 |
| Pedestrians | 16 | | | | | |
| Lane Width (ft) | 0.0 | | | | | |
| Walking Speed (ft/s) | 3.5 | | | | | |
| Percent Blockage | 0 | | | | | |
| Right turn flare (veh) | - | | | | | |
| Median type | | | | None | None | |
| Median storage veh) | | | | | | |
| Upstream signal (ft) | | | | | | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 970 | 390 | 390 | | | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 970 | 390 | 390 | | | |
| tC, single (s) | 6.4 | 6.2 | 4.2 | | | |
| tC, 2 stage (s) | J., | 5.2 | | | | |
| tF (s) | 3.5 | 3.3 | 2.3 | | | |
| p0 queue free % | 100 | 100 | 88 | | | |
| cM capacity (veh/h) | 247 | 658 | 1147 | | | |
| | | | | | | |
| Direction, Lane # | NB 1 | NB 2 | SB 1 | | | |
| Volume Total | 140 | 300 | 374 | | | |
| Volume Left | 140 | 0 | 0 | | | |
| Volume Right | 0 | 0 | 0 | | | |
| cSH | 1147 | 1700 | 1700 | | | |
| Volume to Capacity | 0.12 | 0.18 | 0.22 | | | |
| Queue Length 95th (ft) | 10 | 0 | 0 | | | |
| Control Delay (s) | 8.6 | 0.0 | 0.0 | | | |
| Lane LOS | Α | | | | | |
| Approach Delay (s) | 2.7 | | 0.0 | | | |
| Approach LOS | | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 1.5 | | | |
| Intersection Capacity Utiliza | ation | | 26.7% | IC | CU Level o | of Service |
| Analysis Period (min) | | | 15 | | | |

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HCM Unsignalized Intersection Capacity Analysis 10: Washington Street

| | • | → | ← | 4 | / | 4 |
|-----------------------------|----------|----------|----------|------|------------|------------|
| Movement | EBL | EBT | WBT | WBR | SBL | SBR |
| Lane Configurations | ች | | | | | 7 |
| Traffic Volume (veh/h) | 222 | 0 | 0 | 0 | 0 | 331 |
| Future Volume (Veh/h) | 222 | 0 | 0 | 0 | 0 | 331 |
| Sign Control | | Stop | Stop | | Free | |
| Grade | | 0% | 0% | | 0% | |
| Peak Hour Factor | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 | 0.77 |
| Hourly flow rate (vph) | 288 | 0 | 0 | 0 | 0 | 430 |
| Pedestrians | | | | | | |
| Lane Width (ft) | | | | | | |
| Walking Speed (ft/s) | | | | | | |
| Percent Blockage | | | | | | |
| Right turn flare (veh) | | | | | | |
| Median type | | | | | None | |
| Median storage veh) | | | | | | |
| Upstream signal (ft) | | | | | 76 | |
| pX, platoon unblocked | | | | | | |
| vC, conflicting volume | 0 | 0 | 430 | 0 | 0 | |
| vC1, stage 1 conf vol | | | | | | |
| vC2, stage 2 conf vol | | | | | | |
| vCu, unblocked vol | 0 | 0 | 430 | 0 | 0 | |
| tC, single (s) | 7.1 | 6.5 | 6.5 | 6.2 | 4.1 | |
| tC, 2 stage (s) | | | | | | |
| tF (s) | 3.5 | 4.0 | 4.0 | 3.3 | 2.2 | |
| p0 queue free % | 72 | 100 | 100 | 100 | 100 | |
| cM capacity (veh/h) | 1021 | 896 | 518 | 1085 | 1623 | |
| Direction, Lane # | EB 1 | SB 1 | | | | |
| Volume Total | 288 | 430 | | | | |
| Volume Left | 288 | 0 | | | | |
| Volume Right | 0 | 430 | | | | |
| cSH | 1021 | 1700 | | | | |
| Volume to Capacity | 0.28 | 0.25 | | | | |
| Queue Length 95th (ft) | 29 | 0.23 | | | | |
| Control Delay (s) | 9.9 | 0.0 | | | | |
| Lane LOS | 7.7 A | 0.0 | | | | |
| Approach Delay (s) | 9.9 | 0.0 | | | | |
| Approach LOS | 7.9 A | 0.0 | | | | |
| | | | | | | |
| Intersection Summary | | | | | | |
| Average Delay | | | 4.0 | | | |
| Intersection Capacity Utili | ization | | 23.8% | IC | :U Level c | of Service |
| Analysis Period (min) | | | 15 | | | |

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Intersection Capacity Utilization 11: 0/Washington Street & Warren Avenue

| | ۶ | → | • | • | • | • | 4 | † | / | > | ↓ | 4 |
|-------------------------------|-------|-----------|-------|------|-----------|------------|----------|----------|------|-------------|----------|------|
| Movement | EBL | EBT | EBR | WBL | WBT | WBR | NBL | NBT | NBR | SBL | SBT | SBR |
| Lane Configurations | | 4 | | | 4 | | | 4 | | | 4 | |
| Volume (vph) | 16 | 9 | 76 | 13 | 3 | 8 | 37 | 183 | 3 | 1 | 242 | 27 |
| Pedestrians | 4 | | 66 | 66 | | 4 | 27 | | 14 | 14 | | 27 |
| Ped Button | | Yes | | | Yes | | | Yes | | | Yes | |
| Pedestrian Timing (s) | | 16.0 | | | 16.0 | | | 16.0 | | | 16.0 | |
| Free Right | | | No | | | No | | | No | | | No |
| Ideal Flow | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 | 1900 |
| Lost Time (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Minimum Green (s) | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 | 4.0 |
| Refr Cycle Length (s) | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 | 120 |
| Volume Combined (vph) | 0 | 101 | 0 | 0 | 24 | 0 | 0 | 223 | 0 | 0 | 270 | 0 |
| Lane Utilization Factor | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 | 1.00 |
| Turning Factor (vph) | 0.95 | 0.88 | 0.85 | 0.95 | 0.92 | 0.85 | 0.95 | 0.99 | 0.85 | 0.95 | 0.98 | 0.85 |
| Saturated Flow (vph) | 0 | 1672 | 0 | 0 | 1756 | 0 | 0 | 1880 | 0 | 0 | 1871 | 0 |
| Ped Intf Time (s) | 0.0 | 4.3 | 5.7 | 0.0 | 0.2 | 0.5 | 0.0 | 0.0 | 1.7 | 0.0 | 0.3 | 3.0 |
| Pedestrian Frequency (%) | | 0.89 | | | 0.12 | | | 0.37 | | | 0.59 | |
| Protected Option Allowed | | No | | | No | | | No | | | No | |
| Reference Time (s) | | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 |
| Adj Reference Time (s) | | | 0.0 | | | 0.0 | | | 0.0 | | | 0.0 |
| Permitted Option | | | | | | | | | | | | |
| Adj Saturation A (vph) | 0 | 1655 | | 0 | 665 | | 0 | 575 | | 0 | 1861 | |
| Reference Time A (s) | 0.0 | 11.6 | | 0.0 | 4.5 | | 0.0 | 46.5 | | 0.0 | 17.7 | |
| Adj Saturation B (vph | 0 | 0 | | 0 | 0 | | NA | NA | | NA | NA | |
| Reference Time B (s) | 9.1 | 19.5 | | 8.9 | 9.8 | | NA | NA | | NA | NA | |
| Reference Time (s) | | 11.6 | | | 4.5 | | | 46.5 | | | 17.7 | |
| Adj Reference Time (s) | | 19.5 | | | 9.9 | | | 50.5 | | | 21.7 | |
| Split Option | | | | | | | | | | | | |
| Ref Time Combined (s) | 0.0 | 11.5 | | 0.0 | 1.8 | | 0.0 | 14.3 | | 0.0 | 17.6 | |
| Ref Time Seperate (s) | 1.1 | 5.0 | | 0.9 | 0.4 | | 2.5 | 11.6 | | 0.1 | 15.8 | |
| Reference Time (s) | 11.5 | 11.5 | | 1.8 | 1.8 | | 14.3 | 14.3 | | 17.6 | 17.6 | |
| Adj Reference Time (s) | 19.5 | 19.5 | | 9.5 | 9.5 | | 18.9 | 18.9 | | 21.6 | 21.6 | |
| Summary | EB WB | | NB SB | | mbined | | | | | | | |
| Protected Option (s) | NA | | NA NA | CU | IIIbiiieu | | | | | | | |
| Permitted Option (s) | 19.5 | | 50.5 | | | | | | | | | |
| Split Option (s) | 29.0 | | 40.5 | | | | | | | | | |
| Minimum (s) | 19.5 | | 40.5 | | 60.0 | | | | | | | |
| | 17.0 | | 40.5 | | 00.0 | | | | | | | |
| Right Turns | | | | | | | | | | | | |
| Adj Reference Time (s) | | | | | | | | | | | | |
| Cross Thru Ref Time (s) | | | | | | | | | | | | |
| Oncoming Left Ref Time (s) | | | | | | | | | | | | |
| Combined (s) | | | | | | | | | | | | |
| Intersection Summary | | | | | | | | | | | | |
| Intersection Capacity Utiliza | tion | | 50.0% | IC | CU Level | of Service | <u> </u> | | А | | | |
| Reference Times and Phasi | | do not re | | | | | | | ,, | | | |
| | 5 1 | | | 1 | 9 | | | | | | | |

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VILLAGE OF DOWNERS GROVE PLAN COMMISSION MEETING

April 4, 2022, 7:00 P.M.

Chairman Rickard called the April 4th, 2022 meeting of the Downers Grove Plan Commission to order at 7:00 p.m. and led the Plan Commissioners and public in the recital of the Pledge of Allegiance.

ROLL CALL:

PRESENT: Chairman Rickard, Commissioners Rector, Dmytryszyn, Boyle, Maurer, Toth

ABSENT: Commissioners Roche, Johnson, and Patel

STAFF: Director of Community Development Stan Popovich, Planning Manager Jason

Zawila, Senior Planner Flora Leon

OTHERS

PRESENT: Ron Olson, Matt McDonald, Muhammad Starks, Nancy Gazelle, Liz Chalberg, Bill

Chalberg, Brian Meade, David Yandel, Michael Werthman, Andy Sikich, Police

Chief Shanon Gillette, and Fire Chief Scott Spinazola.

APPROVAL OF MINUTES

Chairman Rickard entertained a motion to approve the minutes.

MINUTES OF THE MARCH 7, 2022 PLAN COMMISSION MEETING WERE APPROVED ON MOTION BY COMMISSIONER RECTOR, SECOND BY COMMISSIONER DMYTRYSZYN, MOTION PASSED BY VOICE VOTE OF 6-0.

PUBLIC HEARING

Chairman Rickard explained the protocol for the public hearing process and swore in those individuals that would be speaking during the public hearing.

FILE 22-PLC-0007: A PETITION SEEKING APPROVAL FOR A PLANNED UNIT DEVELOPMENT, A REZONING FROM DT TO DT/PUD, RIGHT-OF-WAY VACATION, FINAL PLAT OF SUBDIVISION AND A SPECIAL USE TO CONSTRUCT A WIRELESS TELECOMMUNICATIONS FACILITY. THE PETITIONER IS PROPOSING TO CONSTRUCT THE DOWNERS GROVE CIVIC CENTER BUILDING THAT WILL HOUSE THE OFFICES OF VILLAGE HALL, POLICE STATION AND SCHOOL DISTRICT 58. IN **ADDITION** TO REMOVING THE **EXISTING TELECOMMUNICATION AND CONSTRUCTING** TOWER **NEW** TELECOMMUNICATION TOWER.

THE PROPERTY IS CURRENTLY ZONED DT, DOWNTOWN TRANSITION. THE PROPERTY IS LOCATED DIRECTLY EAST OF THE INTERSECTION OF

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WASHINGTON STREET AND BURLINGTON AVENUE, COMMONLY KNOWN AS 700 BURLINGTON AVENUE, 801 BURLINGTON AVENUE, 825 BURLINGTON AVENUE, AND 842 CURTISS STREET (PINS 09-08-131-018, 09-08-813-019, 09-08-211-001, 09-08-211-002, 09-08-211-003, 09-08-211-014, 09-08-211-015, 09-08-211-016, 09-08-211-001, 09-08-304-008). VILLAGE OF DOWNERS GROVE, OWNER AND PETITIONER.

Petitioner, Mike Baker Deputy Village Manager for the Village of Downers Grove introduced himself as the Project Manager for the new Civic Center Facility Design and Construction. He then introduced the team that would partake in the petitioner's presentation.

Mr. Baker then thanked the public and board for their consideration of the petition. He then explained that the request from the Plan Commission was for a positive recommendation on the items listed on the screen, including: (1) plat of vacation; (2) final plat of subdivision; (3) final planned unit development; (4) map amendment from Downtown Transition to Transition/Planned Unit Development and a (5) special use.

Mr. Baker explained that the approvals would permit the construction of the new Downers Grove Civic Center, a new facility that would provide space for the Police Department, Village Hall and District 58 administrative offices. This would allow combining facilities that are currently in three separate buildings, into one and providing substantially more modern and efficient features and spaces to serve public safety, administrative and educational needs of this community for decades to come. These approvals would also allow for the construction of a new telecommunications tower, along with the removal of the existing tower.

Mr. Baker then noted that the subject property was located at the southeast corner of Burlington Avenue and Washington Street, commonly known as 700 Burlington Avenue, 801 Burlington Avenue, 825 Burlington Avenue and 842 Curtiss Street. As an overview of this project, he shared a three-minute video produced by the Village's Communications Department. He added at the end of the video that the neighborhood meeting mentioned occurred on March 21, 2022 and included approximately 50 residents in attendance.

Mr. Baker explained that his presentation would focus on three elements, the petition's (1) compliance with the recently adopted Downtown Design Guidelines, (2) compliance with the Comprehensive Plan; and (3) compliance with the Zoning Ordinance and how it meets the review and approval criteria for the planned unit development, zoning map amendment, and special use.

Mr. Baker then provided an overview on how the petition met the requirements of each entitlement request. As part of the presentation, Mr. Baker provided further explanation for the special use request related to the proposed telecommunications tower. He explained that after an evaluation of potential tower sites was conducted, this revealed that only a small area on the Civic Center site could serve as the future space for the telecommunications tower. Mr. Baker also provided photo renderings of the proposed new tower site compared to the existing tower site.

Mr. Baker then spoke about the improvements related to the Washington Crossing. He noted that the project would eliminate the eastern leg of Burlington Avenue and Washington Street intersection. Mr. Baker added that this option was reviewed thoroughly and that the Village Council considered and supported the configuration. He listed several benefits with the proposed

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configuration and noted that the improvements at the crossing represented just one of the many community benefits offered by the project. He added that the project provided environmentally sustainable features, consolidates existing public facilities, provides spaces that support modern work environments and improved customer interactions, along with landscaped paths and public plazas. Mr. Baker concluded his presentation by thanking the Plan Commission for their time and consideration.

Chairman Rickard invited public comment.

Mr. Ron Olson, 704 Maple Avenue, explained that his home would be directly backing up to the proposed tower. He noted that when they purchased their home the antenna was not right behind their house. Mr. Olson expressed his concern over the potential decrease in property values due to this proposal. As such, he suggested that there might be consideration for a real estate credit for the homes that would now have to bear the cost of the proposed tower. Mr. Olson asked about the required fall radius and how his research led him to believe the fall zone had to equal 300% of the height. Additionally, he requested additional screening to be provided in the form of a 12-foot fence and more landscaping. Finally, he expressed concern over the increase in height of the tower and the increased proximity the new tower would have with adjacent residential properties. To conclude, Mr. Olson expressed that the proposal appeared to give future residents of Downers Grove preference over existing residents considering the TIF district would only work if the future multi-family development on Lot 2 was not close to the tower.

Mr. Matt McDonald, 710 Maple Avenue, agreed with Mr. Olson's previous comments. He added that their properties were within the required setback for a new telecommunication tower. Specifically he noted that the requested relief would allow the tower four times closer than what would typically be allowed. Mr. McDonald then noted that the plans did not include any proposed landscaping. He added that additional buffer would make sense between the Public Works property and their backyards. He concluded by stating that the proposal did not preserve their property values or the pleasantness of their backyards.

Mr. Muhammad Starks, 706 Maple Avenue, asked what the future of the third parcel would be. He noted that in the past, this area has been used for a number of different purposes and now it seemed that with the tower the parcel appeared more industrial. As such, Mr. Starks asked if the tower was placed back there what the future for that property would be. Mr. Starks, concluded by stating that he had previously attended a Village Council meeting when they were considering development options for that property. He then decided he would invest in his property and now he wanted to confirm what the future plans were for this parcel. Mr. Starks also noted that he agreed with Mr. Olson and Mr. McDonald's previous comments.

Ms. Nancy Gazelle, introduced herself and noted that she was not an adjacent neighbor to the proposed site. Instead, she explained that she lived on the other side of town but commuted through the area frequently. Specifically Ms. Gazelle explained that she used the designated bike path and believed that rerouting the Washington Street intersection did not adequately take into consideration southbound cyclist traffic. She concluded by stating that routing all of the traffic through the Burlington and Washington intersection was dangerous for cars, pedestrians, and cyclists.

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Chairman Rickard invited for any additional public comment. No additional public comment was received. Staff was invited to present.

Mr. Jason Zawila, Planning Manager, showed the location of the site on a map and noted that the property was zoned Downtown Transition (DT) was currently improved with Village Hall, the Village's Police Station, and a converted single family house that sits on the southwest corner of the subject property. He explained that the site also included the Village's fleet management facility and a telecommunication tower located immediately east of Village Hall. Mr. Zawila also explained that notice was properly provided to all property owners 250 feet from the property in addition to publishing a legal notice and posting the public hearing signs as shown on the screen. Additionally, he noted that as required by the Zoning Ordinance and highlighted with the first presentation, the petitioner held a neighborhood meeting, which was summarized in the packet and earlier in the evening.

Mr. Zawila shared that the Village of Downers Grove, as the petitioner, was requesting approval for a planned unit development, a rezoning from DT to DT/PUD, right-of-way vacation, final plat of subdivision and a special use to construct a wireless telecommunications facility. He then noted that the approvals would permit the construction of a combined Village Hall and Police Station facility and relocation of a telecommunication tower.

Mr. Zawila provided a slide showing the proposed subdivision. He explained that the proposal included vacating part of the Burlington Avenue right-of-way. Additionally, he shared that on Lot 1 the Village was proposing the construction of the Downers Grove Civic Center, which would meet the Downtown Design Guidelines. Lot 2 would remain vacant and any future private development proposal would be required to go through the complete public process. Finally, Mr. Zawila noted that Lot 3 would be designated for the newly proposed monopole communications tower.

Mr. Zawila moved on to describe the two sets of requested relief associated with the petition. The first included the parking lot. To maximize the parking on site, the setback would be reduced along the northern property line. The other relief requested was associated with the relocation of the telecommunications tower. He highlighted that the existing tower was a critical part of the emergency services response network used by DU-COMM, the Village, and adjacent communities. Staff found that the petition met the goals of the Zoning Ordinance, Downtown Design Guidelines, and Comprehensive Plan. As such, he recommended that the commission formulate a motion to approve the petition based on the draft motion provided on page eleven of the staff report.

Chairman Rickard asked if staff could talk more about the telecommunications tower. Perhaps noting how far it is moving and what physically would be different from what is there now. Additionally he asked about any other immediate construction plans for Lot 3. Mr. Zawila stated the petitioner would respond to the operation functions of the tower when they came back up to respond. Commissioner Rector asked for clarification on the required 300% setback. Mr. Zawila explained that it is an additional buffer. The first requirement of 200 feet is to clear the fall zone. He added that in this case the petition was requesting relief from the setback because there was not one physical location that would meet that setback on the site without that relief.

Chairman Rickard asked if there was any current relief given for the tower. Mr. Zawila said that he did not believe so.

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Commissioner Boyle asked if there was a surplus of commuter parking. Additionally, he asked if this became an issue later on if there was a plan to address this. Mr. Zawila noted that it appeared that there were other parking options in the current network that would make up for the loss of commuter parking. Commissioner Boyle asked if that ended up not being the case does the Comprehensive Plan have other options to explore if parking becomes an issue in the future. Mr. Zawila shared that there were a multitude of options available as it related to downtown parking. The Village has worked on reviewing this in the past and will continue to work on this as situations arise.

Commissioner Boyle asked for clarification on the proposed Washington intersection. Mr. Zawila indicated that it would be more appropriate for the petitioner to address this question in more detail when they came back up to the podium.

Commissioner Maurer asked why if there were only 164 spaces required a total 252 spaces proposed. He asked if a nice green space could have been proposed instead. Mr. Zawila explained that there were portions of the parking designated as secured and the provided spaces are required for Village Hall and the School District operations.

Commissioner Maurer asked about the available locations for the tower. He asked what made that area the available location. Mr. Zawila noted that the petitioner would speak in more detail on that.

Commissioner Maurer asked if Lot 2 were proposed as private development, if this would push vehicular Village traffic between Lot 1 and Lot 3 onto Curtis Street. Mr. Zawila confirmed that was correct.

Commissioner Maurer asked if Lot 2 was supposed to take advantage of transit-oriented development features why the building was oriented in the opposite direction. Additionally with the primary entrance, facing east and no sidewalk connection along the train tracks there is no access to easily get to the train station. Mr. Zawila stated that there was no final plan for Lot 2. The image for review is only a diagram. However, there was a potential entrance shown on the plans that could also lead to the existing sidewalk network.

Commissioner Maurer asked if the 4.4 feet would not be intended to include a sidewalk. Mr. Zawila said no a sidewalk along the northern lot line would not be included due to the location of the proposed secured parking area.

Commissioner Toth asked what the proposed square footage building as compared to the existing facilities. Mr. Zawila noted that the petitioner would speak to those square footage difference.

Chairman Rickard asked if the location of Lot 2 came about based on the phasing for the construction of this project. Mr. Zawila confirmed that was correct.

Chairman Rickard invited the petitioner to speak to some of the questions that came up.

Mr. Baker acknowledged that the comments that were expressed by the residents were very understandable, given the proximity of their homes to the construction of the new tower. He noted that there were some suggestions that were made and the Village would be open to enhancing buffering between the Village property and rear yard of residential properties. With the proposed

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use of Lot 3, Village staff and customers would be using the space to park during the construction. Mr. Baker noted that the comments received presented an opportunity to explore how the space could be used in the future.

Chairman Rickard asked if in general the property moving forward would function as it has in the past. Mr. Baker confirmed that was correct. He did add that the existing garage, currently used for salt storage would be demolished to accommodate space for the proposed telecommunications tower.

Commissioner Maurer asked what made the designated area as the only available location for the telecommunications tower. Mr. Baker explained that the area designated was deemed available space based on the 200-foot setback required from any residential structure or public safety facility.

Commissioner Maurer asked if in that case the proposal considered the future private development on Lot 2. Mr. Baker confirmed that was correct.

Commissioner Maurer asked if there were any ideas of what the private development would be. He added if it were multi-family, residential could there not be an easement that stated the telecommunications tower must go on their roof. Mr. Baker stated that Lot 2 was anticipated to be an apartment building. As such, that would not accommodate the needs of the telecommunications tower.

Brian Meade, FGM Architects, explained that the phasing would be challenging because the tower would need to be in operations sooner than the private development is constructed. Commissioner Maurer noted that the existing tower is not on Lot 2. So he asked if the phasing could not proceed simply by keeping the exiting tower in operation. Mr. Meade explained that the existing tower was on Lot 2. He added that with demolition the issue of adjacency would come up.

Commissioner Maurer asked if Mr. Baker could address the traffic moving onto Curtiss Street and why the proposal included 30% more than what the required parking was. Mr. Baker explained that the main factors driving the additional parking included: public parking for police services, Village Hall, and District 58 services. Additionally, he added that with the council chambers being designed as more of an accessible and multifunctional space this would allow for more programming of this space during the day.

Commissioner Maurer asked if there was a continuous fence along the eastern lot line. Mr. Baker noted that as the plans were further refined and there was a better understanding for could be constructed on Lot 2, there could be an opportunity for sort of pedestrian access from the building, through and around the Village Hall site. Commissioner Maurer further stated that the suggested design would be an improvement. However, he noted the future development of Lot 2 would have a building giving its back to the civic center, the downtown, and the train station instead of embracing the civic center. Mr. Baker agreed and noted that staff could look at the eastern portion of the lot to encourage and promote more of a connection between the private development lot and civic center property.

Commissioner Maurer added that if more could be done for Lot 2 to embrace the civic center lot this could make lot 2 a more desirable development site for the developer, tenants, and Village. Mr. Baker offered to address a comment regarding cyclist safety. He noted that the width of the

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pathway on the west side of the building was such that it could be very multimodal in nature, and accommodate not just pedestrian traffic, but also bicycles as well. Mr. Baker added that as the design was further developed; they would make sure that the signage properly indicated and maintained safe bicycle access onto the site. He stated that this was just one opportunity to make sure that cyclists had the opportunity to navigate the site in ways that did not force them along the curve on Washington Street.

Commissioner Dmytryszyn thanked the petitioner for the detailed Telecommunication Tower FAQ that was provided. He asked if more detail could be provided on the proposed increased height of the new tower. Mr. Baker referred back to the proposed tower elevation. He explained that the proposed tower was a monopole design, thus making it more narrow vertically. Mr. Baker shared that the existing tower was a lattice design. He stated that the monopole tower required more height to accommodate the equipment necessary to support the use of the tower. Mr. Baker added that the priority was to accommodate the public safety communications required equipment to allow police fire and EMS to continue to interact and communicate with dispatch and Downers Grove and neighboring communities. Additionally, Mr. Baker explained that the Village wanted to accommodate existing private telecommunications providers. He said that the thought process behind doing this was that if they were not afforded opportunities here, it is likely that they would pursue other tower construction in the immediate vicinity to make sure that their coverage could be provided.

Commissioner Dmytryszyn asked if there were any requirements to upgrade the current tower. Mr. Baker sated that he did not believe any upgrades were required. Commissioner Dmytryszyn asked if the existing tower facilitated the current needs. Mr. Baker explained that the tower does meet the current needs but he noted that it was also forty years old. Commissioner Dmytryszyn asked if the Village was going to continue to lease the space and make revenue off that. Mr. Baker confirmed that was correct.

Commissioner Toth asked if the tower had to remain on this site. Mr. Baker confirmed that yes the tower had to remain on this site. David Yandel, FGM Architects, confirmed that the tower had to remain in close proximity to the police station. If the tower located further away additional fiber lines would have to be installed.

Chairman Rickard invited the petitioner to provide a closing statement.

Mr. Baker thanked the residents and the Plan Commission for the comments and questions and noted that this would help improve the project moving forward.

Chairman Rickard requested that the Plan Commission move on to deliberation. After receiving a request from the public, he instead allowed for additional public comment.

Mr. Ron Olson, 704 Maple Avenue, asked for clarification on three items. The first was regarding the setback requirement of 300%. He asked why an exception was being made to reduce from 600 feet to 145 feet. Next, he asked about the placement of the tower. He noted that if the only barrier to relocate the tower was running fiber, he was certain that with today's technology running fiber would be possible. Lastly, Mr. Olson asked about the height of the tower and if the maximum permitted is 140 feet, why this tower was allowed to exceed that.

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Mr. Muhammad Starks, 706 Maple Avenue, noted that if the city was willing to work with residents if there was another process to continue this dialogue. Chairman Rickard explained that the Plan Commission was a recommending body and that there would be additional meetings at the Village Council level where a final decision would be made. He noted that there are an addition two opportunities to participate and ask question. Mr. Starks asked if there would be an additional opportunity for the public to review design changes and offer their opinions. Chairman Rickard noted that the Village staff is available via email. He added that if residents wanted to reach out to share their concerns or participate at the Village Council hearing those were options to ask for consideration.

Chairman Rickard invited the petitioner to provide any closing statements.

Mr. Baker expressed his willingness to continue a dialogue with residents to understand and see what could be done to accommodate some of their concerns. He added that from the Village standpoint, the tower location as proposed was viewed as critical, because it supports public safety communications. Mr. Baker further noted that with the proposed development the existing tower would need to come down. As such, he explained that having the existing services on the tower accommodated somewhere else needed to be considered. For this reason, the location proposed, after evaluation, was determined to be the most desirable site for the proposed tower to accommodate the requirements of public safety, telecommunications, and for the private telecommunications providers as well. Mr. Baker concluded his statement by thanking the board for their time and consideration.

Commissioner Toth asked if a study had been prepared considering a relocation of the tower with added fiber optic cables to connect it back to the police station. Mr. Baker stated that a study did not exist looking at those types of alternatives. He added that the basis of the proposed location had to do with a number of factors that are not just limited to the public safety requirements. He added that the tower location was also based on the colocation capabilities of the tower.

Chairman Rickard moved to Plan Commission deliberation.

Commissioner Rector shared that she liked the proposal and the idea of having a building for multiple uses. She added that the design appears have been well thought out as it met the goals of the Village. Additionally, she noted that she was not thrilled with the tower option; however, she understood why the tower had to be replaced in the proposed location. Commissioner Rector noted that she was in favor of the proposal and felt that the standards were met.

Commissioner Dmytryszyn stated that it was a great proposal and believed a lot of care was put into the plans. He added that as he read through the standards of approval he could not argue that they had not been met. Commissioner Dmytryszyn noted that he would like to see more detail around the tower and why it needed to be placed where it is proposed. He stated that the petitioner should be prepared to answer more questions about additional options such as running fiber as Commissioner Toth proposed. Commissioner Dmytryszyn concluded by agreeing that the standards had been met.

Commissioner Boyle shared that he would like more detail on how to create buffering between the tower area and residential properties. Additionally, he added that there should be more details focused on how Lot 2 can be designed to be more in line with transit-oriented development. He stated that the plan met all of the criteria and supported the police, school, and Village staff.

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Commissioner Maurer agreed with the previous Commissioners. He noted that the new proposal would create better working conditions and increase efficiency. Commissioner Maurer added that the proposal did not contribute to the nature of the downtown. While he agreed with the intersection improvements, he did not believe the proposed arrival point of two blank facades was appropriate. Commissioner Maurer stated that even flipping the plan 180 degrees in either direction would be an improvement and would make the proposed park more accessible to the downtown.

Commissioner Toth stated that it was a great plan and an improvement to the current facilities. He noted that he believed the tower setback was still an issue. Commissioner Toth concluded by stating that a better explanation for the tower location should be prepared for the Council.

Chairman Rickard stated that he believed the standards had been met. He added that he agreed with Commissioner Maurer in the orientation of the building so that the park is more accessible. With regards to the tower he added that it appeared that the most appropriate location would be to place it near the police and Village facilities. Chairman Rickard stated that the reality was that there has been a tower there for forty years. Residents who live around that area purchased their homes understanding that there was a tower there. Chairman Rickard noted that the tower was getting slightly taller but he added that it is also getting more slender.

Chairman Rickard addressed a comment from the audience and stated that the public comment period had concluded and that the Plan Commission was now deliberating. He then suggested if the public had any additional comments they would have an opportunity at the Village Council meeting. Chairman Rickard concluded his statement by noting that he supported the project, that the standards had been met, and that this would be a huge improvement.

Chairman Rickard entertained a motion

WITH RESPECT TO FILE 22-PLC-0007 AND BASED ON THE PETITIONER'S SUBMITTAL, THE STAFF REPORT, AND THE TESTIMONY PRESENTED, I FIND THAT THE PETITIONER HAS MET THE STANDARDS OF APPROVAL FOR A PLANNED UNIT DEVELOPMENT, ACCOMPANYING REZONING, AND SPECIAL USE, PLAT OF VACATION AND PLAT OF SUBDIVISION AS REQUIRED BY THE VILLAGE OF DOWNERS GROVE ZONING ORDINANCE AND IS IN THE PUBLIC INTEREST COMMISIONER RECTOR MADE A MOTION THAT THE PLAN COMMISSION RECOMMEND TO THE VILLAGE COUNCIL APPROVAL OF 22-PLC-0007, SUBJECT TO THE FOLLOWING CONDITIONS:

- 1. THE SPECIAL USE, PLANNED UNIT DEVELOPMENT AND MAP AMENDMENT SHALL SUBSTANTIALLY CONFORM TO THE STAFF REPORT, RENDERINGS, ARCHITECTURE PLANS, LANDSCAPING PLANS AND ENGINEERING PLANS PREPARED BY FGM ARCHITECTS, DATED THROUGH MARCH 25, 2022, AND TRAFFIC PLANS PREPARED BY KLOA DATED MARCH 25, 2022 EXCEPT AS SUCH PLANS MAY BE MODIFIED TO CONFORM TO THE VILLAGE CODES AND ORDINANCES.
- 2. THE PETITIONER SHALL SUBDIVIDE THE LOT INTO THE THREE LOTS OF RECORD PURSUANT TO SECTION 20.5 OF THE SUBDIVISION ORDINANCE

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PRIOR TO THE ISSUANCE OF ANY SITE DEVELOPMENT OR BUILDING PERMITS.

- 3. A DEVELOPMENT APPLICATION FOR LOT 2 SHALL BE SUBMITTED TO THE PLAN COMMISSION AS A PUD AMENDMENT, AND ALONG WITH ANY OTHER REQUIRED ENTITLEMENTS, WITH FINAL APPROVAL BY THE DOWNERS GROVE VILLAGE COUNCIL.
- 4. A PLAT OF EASEMENT IS PROVIDED FOR PUBLIC UTILITY PURPOSES AND RECORDED WITH DUPAGE COUNTY ONCE INFRASTRUCTURE IS CONSTRUCTED.
- 5. A PLAT OF EASEMENT IS PROVIDED FOR TELECOMMUNICATION PURPOSES AND RECORDED WITH DUPAGE COUNTY ONCE TELECOMMUNICATIONS EQUIPMENT IS CONSTRUCTED.
- 6. A PLAT OF EASEMENT IS PROVIDED FOR STORMWATER PURPOSES AND MUST BE RECORDED WITH DUPAGE COUNTY ONCE INFRASTRUCTURE IS CONSTRUCTED.

SECOND BY COMMISSIONER RECTOR. ROLL CALL:

AYE: COMMISSIONERS RECTOR, DMYTRYSZYN, BOYLE, MAURER, TOTH, CHAIRMAN RICKARD

NAY: NONE

MOTION PASSED. VOTE: 6-0

THE MEETING WAS ADJOURNED AT 8:28 P.M. UPON MOTION BY COMMISSIONER PATEL. SECOND BY COMMISSIONER TOTH. A VOICE VOTE FOLLOWED AND THE MOTION PASSED UNANIMOUSLY.

/s/ Village Staff
Recording Secretary
(As transcribed by MP-3 audio)