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VILLAGE OF DOWNERS GROVE Report for the Village

Report for the Village 1/14/2025

SUBJECT:	SUBMITTED BY:
4919 Forest Avenue – Planned Unit Development, Zoning Map	Stan Popovich, AICP
Amendment and Special Use	Director of Community Development

SYNOPSIS

The petitioner is requesting approval of a Zoning Map Amendment, Planned Unit Development and a Special Use in the Downtown Business (DB) zoning district to permit the construction of a 62 unit apartment building at 4919 Forest Avenue.

STRATEGIC PLAN ALIGNMENT

The goals for the 2023-2025 Strategic Plan include acting as a *Strong and Diverse Local Economy* and acting as a *Steward of Financial, Environmental, and Neighborhood Sustainability*.

FISCAL IMPACT

N/A

RECOMMENDATION

UPDATE & RECOMMENDATION

This item was discussed by the Village Council at the January 7, 2025 meeting. Based upon that discussion, the petitioner has provided the following updates to the building:

- The north stair cornice will now match the same design as the cornice on the front southwest bay.
- The rear/east 2-story base has been redesigned with the grayish brick surrounding the pedestrian and overhead doors to the north, creating bays. These bays repeat along the façade to the south.

The petitioner has prepared additional elevations that appear immediately following this report on page 4 of this PDF.

Staff recommends approval on the January 14, 2025 Active Agenda.

BACKGROUND

Property Information and Zoning Request

The subject site, located approximately 175 feet south of the intersection of Franklin Street and Forest Avenue, consists of three lots that are proposed to be combined.

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The development will house a total of sixty-two units: thirteen (13) one bedroom units, forty (40) two bedroom units, and nine (9) three bedroom units. The first floor will feature a residential lobby, a package room, a shared amenity space, building mechanicals, a trash room, a secure bike room, and parking accessed from Forest Avenue. The second floor will house the remainder of the parking garage that will be accessed from the alley to the east of the property. Floors three through seven will house the dwelling units, the majority of which feature terraces or balconies.

The proposed building will have a strong masonry base, in addition to brick on all four sides of the building, with cast stone and metal panels used as accent material throughout the building and cornice lines. The lower levels of the building feature extensive use of a storefront along Forest Avenue to create an open and inviting pedestrian experience. Visual interest is emphasized with building recesses and inset balconies across the building facades. Window sizes differ between larger "square" windows and smaller "vertical" windows to create a rhythm that breaks up the façade further. The primary building entry faces directly onto Forest Avenue and is capped by an awning that is framed within the broader massing articulation, creating a distinct and inviting entry to the building. While the building top is designed to articulate the massing and complement the overall design of the building.

To assist drivers using the alley entrance, the petitioner is dedicating three (3) feet of private property along the east property line to provide additional width to the alley.

Compliance with the Comprehensive Plan

The proposed development meets the Comprehensive Plan's key concepts for this subarea as summarized in the PZC staff report, including such recommendations as development that is of an area of greater residential density to facilitate a vibrant and energetic downtown while providing economic sustainability to the core, a built form consistent with transit-oriented development, and a development that reinforces the walkable nature of downtown by orienting the building towards Forest Avenue.

The Comprehensive Plan also encourages transit oriented development to take advantage of transportation opportunities. The proposed development is consistent with transit oriented development approach as it provides higher density residential uses within a 10-minute walk of the Main Street Metra station.

The Residential Policy Recommendations in the Comprehensive Plan notes that future multifamily development should be located near significant activity centers. The proposed multifamily development is located in the downtown and maintains density in the downtown area. The proposed development is consistent with the intent of the Comprehensive Plan.

Compliance with the Zoning Ordinance

The property is zoned Downtown Business (DB). Per Section 28.5.010 of the Zoning Ordinance, apartments/condominiums are allowed as Special Uses in DB zoning district. The petitioner is requesting relief from the required lot area per dwelling unit. This level of density is appropriate given the proximity to the train station and similar projects in the downtown. The proposed development meets all other bulk requirements, including height, setbacks, density, and parking, of the Zoning Ordinance as demonstrated in Table 1 of the PZC staff report.

Compliance with the Downtown Design Guidelines

The Downtown Design Guidelines provide guidance for building and site design, which will assist in creating a vibrant downtown. The guidelines are divided into seven separate sections: site design, building design, building base, building middle, building top, utility considerations, and parking facilities. Each section describes elements, which support good design, and provides visual references, which identify both

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encouraged and discouraged elements. The proposed development meets the guidelines as demonstrated in Table 2 of the PZC staff report.

Public Comment

Prior to the public hearing, staff received seven letters of support for the project. During the PZC hearing, seven members of the public expressed concerns falling in the categories listed below. One member provided positive feedback on the public meetings held by the petitioner prior to the Planning and Zoning Commission hearing. Village staff offers the following comments:

Concern	Response	
Height	The proposed development meets the requirements of the DB zoning district.	
Insufficient setbacks	 The proposed development meets the requirements of the DB zoning district. The build-to zone regulation along Forest Avenue requires the building to be built close to the street, in conflict with providing additional landscaping in front of the building. During construction, areas of the building that are proposed to be built near adjacent property lines will meet all building code requirements, including shoring. 	
Traffic	 The proposed development provides 89 parking spaces when 87 are required. The Traffic Impact Study finds that the traffic generated by the development can be accommodated by the existing area roadway system. 	

ATTACHMENTS

Revised Renderings
Aerial Map
Ordinance
Staff Report with attachments dated December 2, 2024
Draft Minutes
Public Correspondence





VILLAGE OF DOWNERS GROVE COUNCIL ACTION SUMMARY

INITIATED:	Village Attorney	DATE:	January 14, 2025
	(Name)		
RECOMMEND		lanning & Zoning Commission (Board or Department)	FILE REF: 24-PCE-0029
NATURE OF A			TO IMPLEMENT ACTION:
X Ordinance	2	Motion to adopt "A	
Resolution	n	FOREST AVENUE	
Motion			OF A MULTIFAMILY
Other		RESIDENTIAL BU	ILDING", as presented.
			66
SUMMARY OF	ITEM:		•
_	ordinance shall auth	-	mit construction of a multifamily
Tosidonida bandin	ig at 1919 I forence	r venue.	
RECORD OF A	CTION TAKEN:		

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4919 Forest Special Use – 24-PCE-0029

ORDINA	NCE	NO.	

AN ORDINANCE AUTHORIZING A SPECIAL USE FOR 4919 FOREST AVENUE PLANNED UNIT DEVELOPMENT #70 TO PERMIT CONSTRUCTION OF A MULTIFAMILY RESIDENTIAL BUILDING

WHEREAS, the following described property, to wit:

LOTS 18, 19, 20, IN THE RESUBDIVISION OF BLOCK 5 OF E.H. PRINCE AND COMPANY'S ADDITION TO DOWNERS GROVE, IN SECTIONS 5, 6, 7 AND 8, TOWNSHIP 38 NORTH, RANGE 11 EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO THE PLAT OF SAID RESUBDIVISION RECORDED OCTOBER 24, 1891 AS DOCUMENT 46830,IN DUPAGE COUNTY, ILLINOIS.

Commonly known as: 4913, 4917, & 4921 Forest Avenue, Downers Grove, IL 60515

PIN: 09-08-116-004; -005; -006

(hereinafter referred to as the "Property") is presently zoned in the "DB-Downtown Business District" under the Comprehensive Zoning Ordinance of the Village of Downers Grove; and

WHEREAS, the owner of the Property has filed with the Planning and Zoning Commission, a written petition conforming to the requirements of the Zoning Ordinance, requesting that a Special Use per Section 28.12.050 of the Zoning Ordinance be granted to permit construction of a multifamily residential building; and

WHEREAS, such petition was referred to the Planning and Zoning Commission of the Village of Downers Grove, and said Planning and Zoning Commission has given the required public notice, has conducted a public hearing for the petition on December 2, 2024 and has made its findings and recommendations, all in accordance with the statutes of the State of Illinois and the ordinances of the Village of Downers Grove; and,

WHEREAS, the Planning and Zoning Commission recommended approval of the Special Use, subject to certain conditions; and,

WHEREAS, the Village Council finds that the evidence presented in support of said petition, is such as to establish the following:

- 1. That the proposed use is expressly authorized as a Special Use in the district in which it is to be located:
 - 2. That the proposed use at the proposed location is necessary or desirable to provide a service or a facility that is in the interest of public convenience and will contribute to the general welfare of the neighborhood or community.
 - 3. That the proposed use will not, in the particular case, be detrimental to the health, safety or general welfare of persons residing or working in the vicinity or be injurious to property values or improvements in the vicinity.

NOW, THEREFORE, BE IT ORDAINED by the Council of the Village of Downers Grove, in DuPage

County, Illinois, as follows:

<u>SECTION 1</u>. That Special Use of the Property is hereby granted to permit construction of a multifamily building.

SECTION 2. This approval is subject to the following conditions:

- 1. The Special Use shall substantially conform to the staff report dated December 2, 2024, renderings, architecture plans prepared by Kennedy Mann dated August 29, 2024 and last revised November 15, 2024 and engineering drawings prepared by Cage Civil Engineering dated October 16, 2024, and landscape plans prepared by Cage Civil Engineering dated August 30, 2024 with final revisions dated October 30, 2024, except as such plans may be modified to conform to the Village codes and ordinances.
- 2. The petitioner shall consolidate the three lots into a single lot of record pursuant to Section 20.507 of the Subdivision Ordinance prior to the issuance of any site development or building permits.
- 3. Prior to issuing any site development or building permits, the petitioner shall make park and school donations in the amount of \$501,400.19 (\$380,356.06 to the Park District, \$87,899.24 to Elementary School District 58, and \$33,154.89 to High School District 99).
- 4. A photometric plan will be required to be submitted with site development and building permit documents.
- 5. All vehicles exiting the building into the alley are limited to northbound only. Appropriate signage shall be provided.
- 6. The building materials shall be substantially consistent with the approved plans as verified by the Village and consistent with the Downtown Design Guidelines.

SECTION 3. The above conditions are hereby made part of the terms under which the Special Use is granted. Violation of any or all of such conditions shall be deemed a violation of the Village of Downers Grove Zoning Ordinance, the penalty for which may include, but is not limited to, a fine and/or revocation of the Special Use granted herein.

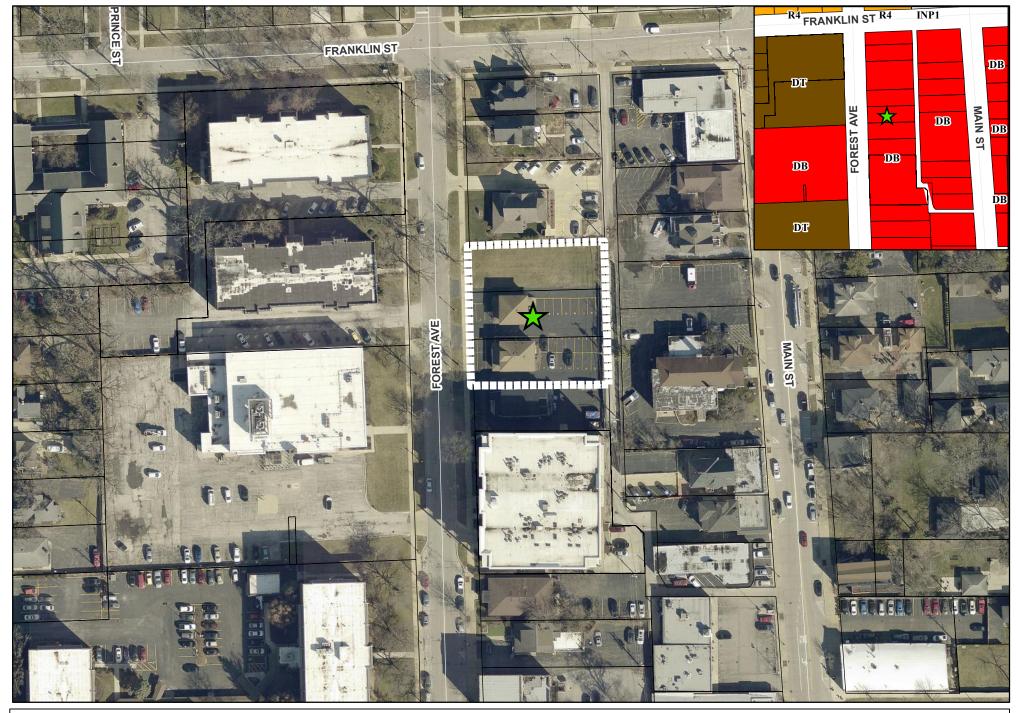
<u>SECTION 4</u>. It is the Petitioner's obligation to maintain compliance with all applicable Federal, State, County and Village laws, ordinances, regulations, and policies.

<u>SECTION 5</u>. That all ordinances or parts of ordinances in conflict with the provisions of this ordinance are hereby repealed.

	Mayor
Passed:	·
Published:	
Attest:	
Village Clerk	

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VILLAGE OF DOWNERS GROVE REPORT FOR THE PLANNING AND ZONING COMMISSION DECEMBER 2, 2024 AGENDA

SUBJECT:	TYPE:	SUBMITTED BY:
24-PCE-0029 4919 Forest Avenue	Zoning Map Amendment, Planned Unit Development and Special Use	

REQUEST

The petitioner is requesting approval of a Zoning Map Amendment, Planned Unit Development and a Special Use in the Downtown Business (DB) zoning district to permit the construction of a multifamily building.

NOTICE

The application has been filed in conformance with applicable procedural and public notice requirements.

GENERAL INFORMATION

OWNER: Duneland Mgmt One, LLC

2412 Marshall Ct Naperville, IL 60565

PETITIONER: 4 Corners, LLC

Liz Butler, Taft Law

111 East Wacker Drive Ste. 2600

Chicago, IL 60605

PROPERTY INFORMATION

EXISTING ZONING: DB, Downtown Business

EXISTING LAND USE: Commercial

PROPERTY SIZE: 0.49 acres (21,213.4 square feet)

Pins: 09-08-116-004, 09-08-116-005, 09-08-116-006

SURROUNDING ZONING AND LAND USES

ZONING FUTURE LAND USE

NORTH:DB, Downtown BusinessDowntownSOUTH:DB, Downtown BusinessDowntownEAST:DB, Downtown BusinessDowntownWEST:DB/DT, Downtown Business,Downtown

Downtown Transition

ANALYSIS

SUBMITTALS

This report is based on the following documents, which are on file with the Department of Community Development:

1. Application/Petition for Public Hearing

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24-PCE-0029, 4919 Forest Avenue December 2, 2024 Page 2

- 2. Location Map
- 3. Project Narrative
- 4. Plat of Survey
- 5. Color Elevations and Renderings
- 6. Architectural Drawings
- 7. Engineering Plans
- 8. Landscape Plans
- 9. Truck Turn Exhibit
- 10. Traffic Impact Study
- 11. Neighborhood Meeting Summary
- 12. Building Material Samples

PROJECT DESCRIPTION

The petitioner is seeking approval to establish a seven-story multifamily residential building in the Downtown Business zoning district, at 4919 Forest Avenue. The subject site is located approximately 175 feet south of the intersection of Franklin Street and Forest Avenue. The subject site consists of three lots that are proposed to be combined. The northernmost lot is vacant, while the southernmost lots are occupied by two commercial office buildings that have been substantially vacant for the last six years.

The new multi-family residential development will be located on a 0.46 acre lot. The applicant is seeking approval of the following requests:

- Final Planned Unit Development
- Zoning Map Amendment from D-B to D-B/PUD
- Special Use for apartments

The development will house a total of sixty-two units: thirteen (13) one bedroom units, forty (40) two bedroom units, and nine (9) three bedroom units. The first floor will feature a residential lobby, a package room, a shared amenity space, building mechanicals, a trash room, a secure bike room, and parking accessed from Forest Avenue. The second floor will house the remainder of the parking garage that will be accessed from the alley to the east of the property. Floors three through seven will house the dwelling units, the majority of which feature terraces or balconies.

The proposed building will have a strong masonry base, in addition to brick on all four sides of the building, with cast stone and metal panels used as accent material throughout the building and cornice lines. The lower levels of the building feature extensive use of storefront along Forest Avenue to create an open and inviting pedestrian experience. Visual interest is emphasized with building recesses and inset balconies across the building facades. Window sizes differ between larger "square" windows and smaller "vertical" windows to create a rhythm that breaks up the façade further. The primary building entry faces directly onto Forest Avenue and is capped by an awning that is framed within the broader massing articulation, creating a distinct and inviting entry to the building. While the building top is designed to articulate the massing and complement the overall design of the building.

To assist drivers using the alley entrance, the petitioner is dedicating three (3) feet of private property along the east property line to provide additional width to the alley. These items are further discussed under Traffic and Parking,

COMPLIANCE WITH THE COMPREHENSIVE PLAN

The Comprehensive Plan places this property within the Downtown Focus Area. The Downtown Focus Area key concepts include:

• Development that is pedestrian-oriented and walkable

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- Maintain a sense of enclosure
- Maintain a commitment to quality architecture
- Infill development and pedestrian-oriented redevelopment
- Future development that takes into account pedestrian-oriented design, architectural detailing and appropriate building heights

The Comprehensive Plan also places the subject site within the Downtown Functional Subarea - Downtown Edge. The Comprehensive Plan notes this area should be of greater residential density to facilitate a vibrant and energetic downtown while providing economical sustainability to the Core. The Comprehensive Plan identified the following key concepts for this subarea:

- Increased residential density
- Built form that is taller and creates a continuous street wall
- Denser development compared to the surrounding neighborhoods outside of the downtown

The Comprehensive Plan also encourages transit oriented development to take advantage of transportation opportunities. The proposed development is consistent with the transit oriented development approach as it provides higher density residential uses within a 10-minute walk of the Main Street Metra station.

Lastly, the Residential Policy Recommendations in the Comprehensive Plan notes that future multifamily development should be located near significant activity centers. The proposed multifamily development is located in the downtown and maintains density in the downtown area.

The proposed development is consistent with the intent of the Comprehensive Plan.

COMPLIANCE WITH THE ZONING ORDINANCE

The property is zoned DB, Downtown Business. Per Section 28.5.010 of the Zoning Ordinance, apartments/condos are allowed as Special Uses in the DB zoning district. Compliance with the applicable bulk and parking requirements of the Zoning Ordinance are highlighted in the table below:

Table 1: Zoning Requirements

4919 Forest Avenue	Downtown Business Bulk Requirements	Proposed
Lot Area per Dwelling Unit	800 sq. ft. (min)	342 sq. ft. *
Rear Setback – East property line	-	0 ft.
Side Setback – North property line	-	4 in.
Street Setback – West property line	0-10 ft. (min)	1 ft. 4 in.
Street Setback – South property line	-	3 ft.
Building Height	70 ft. (max)	70 ft.
Build-to Zone	119.76 ft. / 80%	138.5 ft. / 92.47%
Parking Spaces	1.4 spaces per unit (87)	89
Bicycle Parking	9	14

^{*} Indicates a deviation from the Zoning Ordinance Requirements

The petitioner is requesting relief from the required minimum lot area per dwelling unit, as reflected in the table. The level of density is appropriate given the proximity to the train station and similar projects in the downtown

Planned Unit Development Amendment Request

A Planned Unit Development is intended to accommodate development that may be difficult to carry out under applicable zoning standards and results in public benefits that are at least commensurate with the

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degree of flexibility provided. Examples of development types that are appropriate for PUD approval, per Section 28.4.030.A.1 of the Zoning Ordinance include:

- Developments that provide housing variety
- Developments that are consistent with the goals and policies of the Comprehensive Plan

The proposed development provides housing variety by providing a variety of apartments with different numbers of bedrooms. The proposed development is consistent with the goals and policies of the Comprehensive Plan.

A PUD Amendment will also achieve a variety of planning goals as outlined in Section 28.4.030.A.2 of the Zoning Ordinance:

- Implementation of and consistency with the comprehensive plan and other relevant plans and policies.
- Variety in housing types and sizes to accommodate households of all ages, sizes, incomes and lifestyle choices.
- Compact, mixed-use development patterns where residential, commercial, civic and open spaces are located in close proximity to one another.
- High-quality buildings and improvements that are compatible with surrounding areas, as determined by their arrangement, massing, form, character and landscaping.

The proposed development meets the provisions of a Planned Unit Development Amendment. The requested density deviation allow for increased numbers of households to locate near the downtown. The development provides a mix of bedroom counts that can accommodate households of different ages, sizes, incomes and lifestyles. The development is in close proximity to other institutional and civic spaces in the downtown.

The development provides a high-quality building and improvements that are compatible with the surrounding area. The massing of the proposed building respects similar multi-family developments in the immediate area. The building materials and modern design of the development continues the Village's commitment to quality architecture and

Parking

The Village Zoning ordinance requires 87 parking stalls for the 62 residential unit proposal. The petitioner is providing 89 parking stalls.

Signage

Signage is not proposed as part of this petition, and any signage proposed for the development shall comply with the Zoning Ordinance requirements through a separate sign permit application.

COMPLIANCE WITH DOWNTOWN DESIGN GUIDELINES

The Downtown Design Guidelines provide guidance for building and site design which will assist in creating a vibrant downtown. The guidelines are divided into seven separate sections: site design, building design, building base, building middle, building top, utility considerations, and parking facilities. Each section describes elements which support good design and provides visual references which identify both encouraged and discouraged elements. As recommended by the Downtown Design Guidelines, the proposed development incorporates the following features:

Table 2 – Downtown Design Guideline Compliance

Downtown Design	Summary of Compliance
Guideline Elements	
Site Design	• The building is located in the build-to zone, contributing to a continuous

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	street well slong Fenget Avenue
	street wall along Forest Avenue.
Building Design	• The apparent mass and bulk of the building is reduced by structural articulation, windows or other architectural and functional elements.
	• The façade is visually appealing through articulation, detailing, openings and materials of each elevation.
	• Consistent building materials and detailing on all sides of the structure that are open to public view has been provided.
	• Inset and protruding balconies and patios create visual appeal and interest, and follow rhythmically up the vertical plane of the building. They provide the desired solid and void.
Building Base	• Windows along the base create an open and inviting pedestrian experience along Forest Avenue.
	• The main entrance on Forest Avenue is capped by an awning that is framed within the broader massing articulation, creating a distinct entry.
	• The entrance to the parking garage is differentiated by different brick elaboration and color.
Building Middle	Windows create and a sense of rhythm and regularity that emphasizes the play of solid and void.
	• Visual interest is emphasized with inset and protruding balconies across the façade.
	• The third floor building setback and focus on brick patterning creates a strong
	base. This offers a strong horizontal expression separating the first two floors from the upper floors.
	• Window sizing varies in a formulaic method to further break up the façade and provide additional visual interest.
Building Top	 An articulated cornice is used to create a sense of finality and add to visual interest of the building where the building steps back at the seventh floor. This contrasts the southwestern corner where the brick façade is capped by a metal coping.
Utility Considerations	• The design of maintenance, utility and service areas were integrated into the overall design of the building. Trash is kept in the interior of the building and moved into the alley for scheduled pickup.
Parking Facilities	All proposed parking is interior.
	• The proposed development is decreasing the number of curb cuts on the site
	from two (2) to one (1) along Forest Avenue.
	• The proposed development also decreases the number of curb cuts on the site from two (2) to one (1) along the alley.

COMPLIANCE WITH THE SUBDIVISION AND DEVELOPMENT ORDINANCE

The Subdivision Ordinance requires that developments requesting Special Use approval for multi-family developments provide park and school donations to offset the impact of new residential units. The proposed development will include sixty-two (62) units (13 one bedroom, 40 two bedroom, and 9 three bedroom). Based upon the number of units and the number of bedrooms, the total donation is \$501,400.19 (\$380,356.06 to the Park District, \$87,899.24 to Elementary School District 58, and \$33,154.89 to High School District 99). Payment of these donations must be made to the Village prior to the issuance of any site development or building permits.

The existing 21,213 square foot site consists of three lots of record. Section 28.11.020 of the Zoning Ordinance requires the construction of a principal structure to occur on a single lot of record. Should the proposed development be approved, the petitioner will be required to administratively consolidate the three lots pursuant to Section 20.507 of the Subdivision Ordinance prior to building permit issuance.

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ENGINEERING/PUBLIC IMPROVEMENTS

Currently there are two (2) curb cuts on Forest Avenue providing access to the three lots that make up this development. The petitioner is proposing to reduce the number of curb cuts to one (1), the parking garage entrance. Access to the alley is also being reduced from two (2) access points to one (1). Further information on site circulation is discussed below in Traffic and Parking.

Based on the existing impervious area on the site and the proposed impervious area, the proposed development requires Post Construction Best Management Practices (PCBMPs). Storage volume is proposed in the form of an open bottom vault on the southeastern corner of the site underneath the building. This will collect runoff onsite to allow for regulated stormwater discharge into the municipal storm sewer. The vault will also provide for the additional storage capacity that is required for the portion of the site that is located in the 100 year floodplain. The proposed development will comply with the Village's Stormwater and Flood Plain Ordinance.

Lastly, a new water service and sanitary sewer service will be provided off of main lines located underneath and along Forest Avenue. The Downers Grove Sanitary District conceptually approved the request for sanitary sewer service to this development.

TRAFFIC AND PARKING

A traffic impact study was provided by the petitioner analyzing the proposed development, and found that the traffic generated by the development can be accommodated by the existing area roadway system.

Residents will access 42 of the total 89 parking spaces on the first floor through the Forest Avenue garage entrance. Access to the second floor parking deck, which includes 47 of the total 89 parking spaces, is provided from the alley. The two parking levels are not internally connected. As a condition of approval, any traffic exiting the building onto the alley will be required to turn north (left) onto the alley towards Franklin Street. This movement is restricted by signage provided inside the building.

In order to assist drivers when accessing the alley entrance, the petitioner is dedicating three (3) feet of private property along the east property line to provide additional width to the alley. A turning exhibit is provided in the attachments following the staff report and details the turning radius into and out of the building.

PUBLIC SAFETY REQUIREMENTS

The Fire Prevention Division reviewed the proposal. Access for the Fire Department will be along Forest Avenue. All floors will be equipped with fire alarms and will be sprinkled, as required by Village regulations.

NEIGHBORHOOD COMMENT

Notice was provided to all property owners 250 feet or less from the subject property in addition to posting the public hearing sign on the property and publishing a legal notice in the *Daily Herald*. Staff has received one public comment via a phone call, generally in opposition to increased development on Forest Avenue. Staff received the attached five letters in support of the project.

The Zoning Ordinance requires the petitioner hold a neighborhood meeting. The petitioner held two meetings. One in-person meeting was held September 23, 2024 and one virtual meeting was held on September 30, 2024 via Zoom. Feedback from participants included questions on parking and traffic, building height and design, construction considerations, stormwater management, and housing details. A summary of the meetings, which includes how feedback was incorporated into a final version of the proposal is provided as an attachment to this report.

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24-PCE-0029, 4919 Forest Avenue December 2, 2024 Page 7

STANDARDS OF APPROVAL

The petitioner is requesting a Planned Unit Development, Zoning Map Amendment to establish a Planned Unit Development, and Special Use approval for a multifamily development with a total of sixty-two (62) units. The petitioner has submitted a narrative that attempts to address all the standards of approval. The Planning and Zoning Commission should consider the petitioner's documentation, the staff report and the discussion at the Planning and Zoning Commission meeting in determining whether the standards for approval have been met:

Planned Unit Development

Section 28.12.040.C.5 Review and Approval Criteria

The decision to amend the zoning map to approve a PUD development plan and to establish a PUD overlay district are matters of legislative discretion that are not controlled by any single standard. In making recommendations and decisions regarding approval of planned unit developments, review and decision-making bodies must consider at least the following factors:

- a. The zoning map amendment review and approval criteria of Sec. 28.12.030.I.
- b. Whether the proposed PUD development plan and map amendment would be consistent with the comprehensive plan and any other adopted plans for the subject area.
- c. Whether PUD development plan complies with the PUD overlay district provisions of Sec. 28.4.030.
- d. Whether the proposed development will result in public benefits that are greater than or at least equal to those that would have resulted from development under conventional zoning regulations.
- e. Whether appropriate terms and conditions have been imposed on the approval to protect the interests of surrounding property owners and residents, existing and future residents of the PUD and the general public.

Zoning Map Amendment

Section 12.030.I. Zoning Map Amendment Review and Approval Criteria

The decision to amend the zoning map is a matter of legislative discretion that is not controlled by any single standard. In making recommendations and decisions about zoning map amendments, review and decision-making bodies must consider at least the following factors:

- 1. The existing use and zoning of nearby property.
- 2. The extent to which the particular zoning restrictions affect property values.
- 3. The extent to which any diminution in property value is offset by an increase in the public health, safety and welfare.
- 4. The suitability of the subject property for the zoned purposes.
- 5. The length of time that the subject property has been vacant as zoned, considering the context of land development in the vicinity.
- 6. The value to the community of the proposed use.
- 7. The comprehensive plan.

Special Use

Section 28.12.050.H Approval Criteria – Special Uses

No special use may be recommended for approval or approved unless the respective review or decision-making body determines that the proposed special use is constituent with and in substantial compliance with all Village Council policies and plans and that the petitioner has presented evidence to support each of the following conclusions:

- 1. That the proposed use is expressly authorized as a Special Use in the district in which it is to be located;
- 2. That the proposed use at the proposed location is necessary or desirable to provide a service or a facility that is in the interest of public convenience and will contribute to the general welfare of the neighborhood or community.

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3. That the proposed use will not, in the particular case, be detrimental to the health, safety or general welfare of persons residing or working in the vicinity or be injurious to property values or improvements in the vicinity.

DRAFT MOTION

Staff will provide a recommendation at the December 2, 2024 meeting. Should the Planning and Zoning Commission find that the request meets the standards of approval for a Zoning Map Amendment, Planned Unit Development and Special Use staff has prepared a draft motion that the Planning and Zoning Commission may make for the recommended approval of 24-PCE-0029:

Based on the petitioner's submittal, the staff report, and the testimony presented, I find that the petitioner has met the standards of approval for a Zoning Map Amendment, Planned Unit Development and Special Use as required by the Village of Downers Grove Zoning Ordinance and is in the public interest and therefore, I move that the Planning and Zoning Commission recommend to the Village Council approval of 24-PCE-0029, subject to the following conditions:

- 1. The Special Use shall substantially conform to the staff report, renderings, architecture plans prepared by Kennedy Mann dated August 29, 2024 and last revised November 15, 2024 and engineering drawings prepared by Cage Civil Engineering dated October 16, 2024, and landscape plans prepared by Cage Civil Engineering dated August 30, 2024 with final revisions dated October 30, 2024, except as such plans may be modified to conform to the Village codes and ordinances.
- 2. The petitioner shall consolidate the three lots into a single lot of record pursuant to Section 20.507 of the Subdivision Ordinance prior to the issuance of any site development or building permits.
- 3. Prior to issuing any site development or building permits, the petitioner shall make park and school donations in the amount of \$501,400.19 (\$380,356.06 to the Park District, \$87,899.24 to Elementary School District 58, and \$33,154.89 to High School District 99).
- 4. A photometric plan will be required to be submitted with site development and building permit documents.
- 5. All vehicles exiting the building into the alley are limited to northbound only. Appropriate signage shall be provided.
- 6. The building materials shall be substantially consistent with the approved plans as verified by the Village and consistent with the Downtown Design Guidelines.

Staff Report Approved By:

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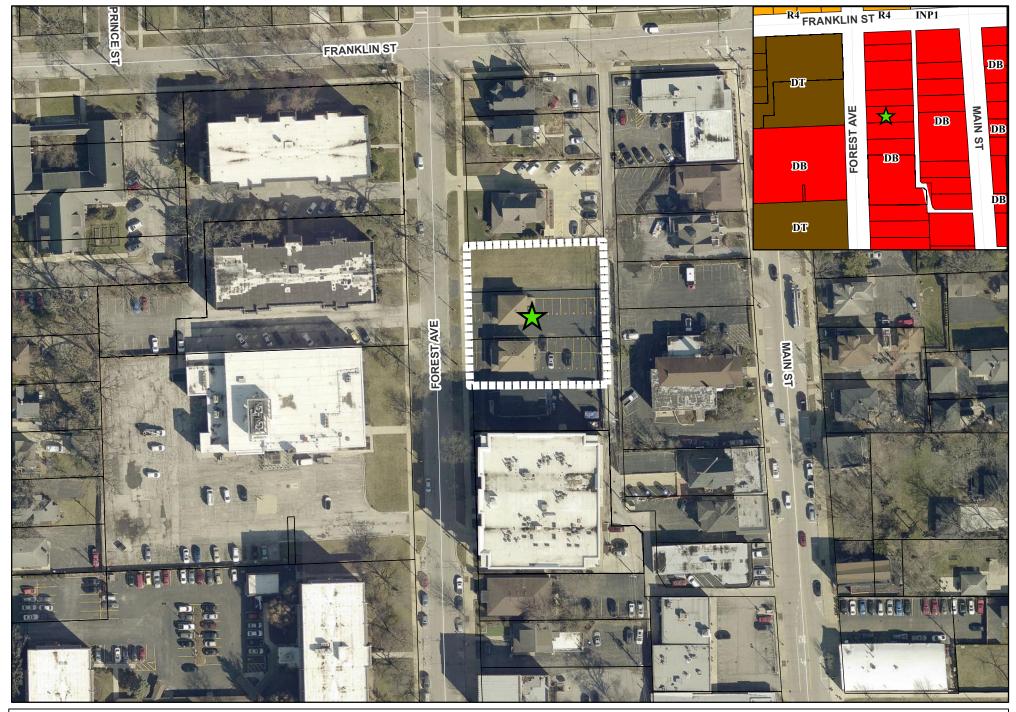
Stan Popovich, AICP

Director of Community Development

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SP; EH

 $P:\P\&CD\PROJECTS\PLAN\ COMMISSION\2024\ PC\ Petition\ Files\24-PCE-0029\ -\ 4919\ (4917-4921)\ Forest\ Avenue\ -\ PUD,\ Special\ Use,\ Lot\ Consolidation\24-PCE-0029\ -\ Staff\ Report.doc$





4919 Forest Avenue 24-PCE-0029



4919 Forest Avenue Project Narrative

4Corners, LLC ("**4Corners**" or the "**Applicant**") seeks approval of a zoning map amendment to establish a planned unit development, special use permit, planned unit development site plan approval, and lot consolidation for the redevelopment of the property located at 4919 Forest Avenue (the "**Property**"). The Applicant's development proposal involves the construction of a high-quality, seven-story multi-family residential building (the "**Proposed Development**"), designed to align with the Village's vision for a vibrant, pedestrian-friendly downtown area.

Overview of the Subject Property and Site Context

The Property consists of three existing lots of record situated on the east side of Forest Avenue within the block bordered by Franklin Street to the north, Main Street to the east, Warren Avenue to the south, and Forest Avenue to the west. Consisting of approximately 21,219 square feet of net site area, the Property is currently zoned as part of the Downtown Business District (the "**DB District**"). The surrounding land uses include medical office and personal services to the north; funeral services, office, retail and personal services to the east; and a mix of commercial and multi-unit residential uses to the south and west.

The Property is currently improved with two aging, increasingly obsolete two-story buildings and a surface parking lot. These buildings, which were previously utilized as offices, have been substantially vacant for six years despite efforts by the property owner to attract new users.

Description of the Proposed Development

The Proposed Development involves the demolition of the existing structures, consolidation of three lots into a single lot of record, and the construction of a new seven-story residential building. The development will feature 62 rental residential units and 89 vehicular parking spaces (63 regular spaces with 26 tandem spaces. No commercial space is proposed. The building design prioritizes use of high-quality building materials and appropriate bulk, height, massing and articulation, ensuring that the proposed building complements the existing character of the downtown area and adheres to the Village's Downtown Design Guidelines.

Ground Floor Features: The ground floor of the building includes a residential lobby accessible from Forest Avenue, a package room, a shared building amenity space, building mechanicals, a secure bike room with space for 14 bicycles, a fitness room, a trash room, and parking.

Residential Floors: Floors three through six will each contain thirteen residential units and level seven will contain ten units, resulting in a total of 62 units. Units will vary in size and layout to accommodate a diverse range of residents. The Proposed Development contemplates the following unit mix:

Unit Type	Quantity
Efficiency Units	0
1 Bedroom Units	13
2 Bedroom Units	40
3 Bedroom Units	9

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Vehicular Access, Parking and Loading: All parking will be contained within a two-level garage located inside the building. The lower garage level will be accessed via Forest Avenue. The upper garage level will occupy the second floor of the building and will be accessible via the 14-foot-wide public alley at the rear of the Property. This design allows curb cuts on Forest Avenue to be reduced/consolidated, thereby minimizing the impact on the streetscape, reducing opportunities for conflicts between pedestrians and vehicles, and supporting and enhancing the pedestrian orientation and walkable nature of the downtown area. The use of the public alley for services functions for the Proposed Development aligns with Key Concept Recommendations identified in the Village of Downers Grove 2017 Comprehensive Plan (the "Comprehensive Plan") (See Comprehensive Plan, 2017, p. 107).

A 9'-6" x 64'-4"on-street loading area is proposed on Forest Avenue to accommodate short term loading and deliveries.

Trash Collection: The trash collection operations for the site will be organized in a way that ensures efficiency and minimizes disruption. Trash will be collected and stored within a designated trash room inside the building, as shown on the site plan. On scheduled trash pick-up days, dumpsters will be moved to the alley for collection by the garage service. This arrangement allows for proper containment and storage of waste within the building, ensuring that trash is not stored outside, and it will only be moved to the alley for collection at designated times, thereby maintaining cleanliness and minimizing any potential impact on the surrounding area.

Outdoor Spaces and Amenities: The design includes setbacks on the 3rd to 6th floors with an additional setback at the 7th floor to create private outdoor terraces facing Forest Avenue. Private terraces or balconies will be provided for 58 of the 62 units. These features provide residents with usable outdoor space while maintaining the visual cohesion of the downtown streetscape.

Building Height and Materials: The building will have a maximum height of 70 feet and will be constructed using high-quality materials that are consistent with the architectural character of the surrounding neighborhood. The design of the façade, the articulation of the building's base, middle, and top, and the overall massing have been thoughtfully planned to reduce the perceived bulk and integrate the structure into the existing urban fabric.

Conformance with Downtown Design Guidelines

The Proposed Development is carefully designed to adhere to the Village of Downers Grove Downtown Design Guidelines, which serve as a framework for all new construction in the downtown area. The project's design aligns with the guidelines in several key areas:

Downtown Design Guideline Elements	Summary of Compliance
Site Design	The building will be positioned within the build-to-zone, close to the sidewalk and street property lines, contributing to a continuous street wall that enhances the pedestrian experience. The upper-level setbacks are utilized to create green spaces and avoid gaps in the street wall, further enhancing the walkable and inviting atmosphere of downtown.

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Building Design

The massing and height of the Proposed Development are proportionate to nearby buildings, with structural articulation and upper floor setbacks used to reduce the apparent mass of the building. These design choices create a sense of enclosure that is important for a downtown environment while ensuring that the building remains visually harmonious with its surroundings.

The façade design reflects the principles outlined in the Design Guidelines, with an emphasis on proportionate shapes, visually appealing articulation, and the use of high-quality materials. The building's base, middle, and top are clearly defined, with attention given to the detailing of windows, balconies, and rooflines to create a cohesive and aesthetically pleasing structure.

Building Base

The lower levels of the building feature extensive use of storefront along Forest Ave to create an open and inviting pedestrian experience.

The building setback at the third floor creates a distinct podium that is emphasized further with a strong masonry base and brick patterning that separates the base from the middle and top.

The primary building entry faces directly onto Forest Avenue and is capped by an awning that is framed within the broader massing articulation, creating a distinct and inviting entry to the building.

Building Middle

Windows are designed to create a sense of rhythm and regularity that emphasizes the play of solid and void.

Visual interest is emphasized with building recesses and inset balconies across the facade. Where windows occur, they are broken up into smaller units, creating further visual interest. Window sizes differ between larger "square" windows and smaller "vertical" windows to create a rhythm that breaks up the façade further.

Building Top

The building top is designed to articulate the massing and complement the overall design. Where the building steps back at the seventh floor, an articulated cornice is used to create a sense of finality and add to the visual interest of the building. This is in contrast to the top of the massing at the south portion of the building where the façade is capped with a formed metal coping that highlights the simple form of the massing while adding visual appeal.

Utility Considerations

The Proposed Development fully complies with the utility-related recommendations set forth in the Design Guidelines. The rear portions of the Property will be maintained in excellent condition, with trash receptacles and service areas carefully screened to ORD 2024-10610 Page 23 of 243

ensure they are not visible from nearby streets or sidewalks. The rear façade is designed to be attractive, incorporating maintenance, utility, and service areas seamlessly into the overall building design.

Parking Facilities

The parking solution provided by the Proposed Development follows the standards outlined in the Design Guidelines with respect to new parking. The inclusion of 89 parking spaces within a two-level garage inside the building ensures that no surface parking lots are exposed, thereby complying with the Village's screening requirements. The design minimizes curb cuts onto neighborhood streets by consolidating the existing curb cuts into one, reducing disruptions to pedestrian pathways and reducing conflicts between pedestrians and local traffic. The internal location of the parking facility also helps buffer off-street parking with fencing and landscaping, preventing light and sound trespass to adjacent residential areas and maintaining compliance with Zoning Ordinance requirements.

Compliance with the Comprehensive Plan

The Proposed Development is in alignment with the goals and policies outlined in Comprehensive Plan, particularly those relevant to the Downtown Key Focus Area. The Comprehensive Plan emphasizes the importance of creating a vibrant, pedestrian-oriented downtown that supports a mix of uses, including residential, commercial, and civic activities. The Proposed Development directly supports these goals in several key ways:

<u>Pedestrian-Oriented Development</u>: The Comprehensive Plan highlights the need for downtown developments to be pedestrian-oriented, fostering a walkable environment that encourages foot traffic and supports local businesses. The Proposed Development adheres to this principle by situating residential units within easy walking distance of downtown amenities, public transportation, and services, thereby promoting the walkability and pedestrian-friendly nature of the area.

<u>Infill Development and Redevelopment</u>: The Comprehensive Plan encourages infill development and the redevelopment of underutilized sites within the downtown area to maximize the potential of the district. The Proposed Development will transform a long-vacant and underutilized site into a high-quality, seven-story residential building, contributing to the revitalization of the downtown area and aligning with the Plan's focus on strategic infill development.

<u>Variety in Housing Options</u>: The Plan stresses the importance of providing a variety of housing types and sizes to accommodate households of all ages, sizes, incomes, and lifestyle choices. The Proposed Development will introduce 62 new residential units, offering a mix of unit sizes that cater to diverse housing needs, thereby enhancing the housing stock in Downers Grove and contributing to the community's long-term sustainability.

<u>Commitment to High-Quality Architecture</u>: The Comprehensive Plan calls for a commitment to quality architecture that complements the existing character of the downtown area. The Proposed Development emphasizes the use of high-quality building materials and thoughtful design,

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ensuring that the new building integrates seamlessly with the surrounding urban fabric and enhances the aesthetic appeal of the downtown.

Conclusion

The Proposed Development represents a thoughtful and high-quality addition to downtown Downers Grove. By adhering to the Downtown Design Guidelines and integrating well-planned site and building design elements, the project will contribute positively to the downtown's built environment and continued vitality.

The Proposed Development complies with all applicable standards of the Zoning Ordinance, including the approval criteria for zoning map amendments, special use permits, and Planned Unit Developments (PUDs). The project is designed to be an amenity to the community, providing significant public benefits, including the creation of 50-75 construction jobs. Additionally, the Proposed Development will result in donations/impact fees totaling approximately \$386,200, further contributing to the Village's resources.

The Proposed Development aligns with the community's vision for a vibrant and appealing downtown, attracting new residents, boosting walkability, and revitalizing a long-vacant site in the heart of the Village. By reactivating this underutilized property, the development introduces much-needed housing options and contributes to the overall vibrancy of Downers Grove.

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Project Contacts



Role	Name / Company	Contact Information
Developer	4Corners Real Estate J.P. Bartley	Mobile: (708) 935-9059 Email: jp.bartley@4cornersllc.com Mailing Address: 6405 Caton Farm Road Plainfield, IL 60586
General Contractor	4Corners Construction	Mobile: (630) 842-8843 Email: jim.roberts@4cornersllc.com
Land Use Attorney	Taft Law Liz Butler, AICP	Office: (312) 836-4121 Mobile: (786) 427-5499 Email: <u>LButler@taftlaw.com</u>
Project Architect	Kennedy Mann Benjamin Kennedy, AIA, NCARB Matt Mann AIA, NCARB, LEED AP BD+C	Ben Kennedy Office: (312) 384-0099 Mobile: (312) 752-7767 Email: ben@kennedymann.com Matt Mann Office: (312) 384-0099 Mobile: (773) 304-6933 Email: matt@kennedymann.com
Civil Engineer / Surveyor	CAGE Civil Tom Petermann, P.E. Claudia Welp Gaby Ptasinska, PLS	Tom Petermann Office: (630) 598-0007 Mobile: (773) 495-0242 Email: tpetermann@cagecivil.com Claudia Welp Mobile: (815) 757-0140 Office: (630) 598-0007 Email: cwelp@cagecivil.com Gaby Ptasinska Mobile: (773) 814-9880 Office: (630) 598-0007 Email: gptasinska@cagecivil.com
Traffic Engineer	KLOA Luay Aboona, PE, PTOE	Office: (847) 518-9990 Mobile: (847) 571-4331 Email: <u>laboona@kloainc.com</u>

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Neighborhood Meeting Summary

Project: 4919 Forest Avenue Redevelopment

Developer: 4Corners LLC

Location: 4913-21 Forest Avenue

Proposed Development: 7-story multi-family residential building with 62 rental residential units

and 89 vehicular parking spaces.

Notification Efforts

The 4Corners LLC team undertook several efforts to notify neighbors and stakeholders about the proposal:

- Mailed Notice: A flyer was mailed to property owners within 250 feet of the subject property via regular mail on or around September 13, 2023. The flyer invited neighbors to attend one of two community information meetings: an in-person meeting on Monday, September 23, 2024, and a virtual meeting on Monday, September 30, 2024.
- **Meeting Invitation and Notice List**: Attached to this report is a copy of the meeting invitation flyer and the list of individuals notified.
- **Downers Grove Economic Development Corporation**: Project details and neighborhood meeting invitations were sent to certain DGEDC board members.

Methods of Sharing Information

Flyer Distribution

The flyer provided detailed information about the redevelopment proposal and meeting details, including the in-person meeting location (Loyal Order of Moose Downers Grove Lodge 1535) and the Zoom link for the virtual meeting option.

Community Information Meetings

Two community information meetings were held:

<u>In-Person Meeting</u>: September 23, 2024, at 6:00 PM at Loyal Order of Moose Downers Grove Lodge 1535.

The in-person meeting was attended by several neighbors, representatives from 4Corners LLC, and project attorney Liz Butler. A sign-in sheet was collected, which is attached to this report.

Virtual Meeting: September 30, 2024, at 6:00 PM via Zoom.

Attendees included property owners who could not attend the in-person meeting, as well as project representatives.

Meeting Follow Ups

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Neighbors who attended the community information sessions were encouraged to reach out to project contacts with additional questions following the meeting. Neighbors who were unable to attend were also provided with project contact information and the team responded to answer questions and provide project information in response to neighbor inquiries.

Summary of Community Input

During the meetings, neighbors raised various questions and provided feedback regarding the proposed development. These discussion have been organized topically below:

Parking and Traffic

- Increased Traffic: Neighbors asked about the potential for increased traffic in the area, especially in relation to nearby train stations and intersections. Luay Aboona from KLOA addressed the traffic impact study, which indicated that the proposed development would result in minimal additional traffic.
- Parking Availability: There were questions about whether there would be adequate parking
 for visitors and concerns about the potential impact on surrounding streets. Suggestions
 were made to explore making the building's exit onto Forest Avenue one-way north to
 mitigate traffic flow. Notably this recommendation was not made or concurred with by
 KLOA or the Village.
- Alley Width: Several participants noted the narrowness of the alley at the rear of the building and inquired about how it might affect access for vehicles and service trucks. This concern has been addressed with revisions to the plans.

Building Design and Height

- Height and Neighborhood Character: Some neighbors raised comments about the building's height, asking how it might affect the character of the surrounding neighborhood. The development team clarified that the height of the proposed building is similar and in line with the adjacent condo building to the south, and is based on the average grade at the building along Forest Ave. The development team noted that the building steps back at the third and seventh floor to help reduce the sense of scale as one moves north along the site.
- Landscaping: There were inquiries about whether any landscaping or greenery would be included in front of the building to enhance its appearance and help it fit within the neighborhood.

Construction Considerations

Neighbors expressed curiosity about the construction timeline and the potential for damage to nearby property or disruption due to noise and dust. They were interested in learning more about the duration of the work and how it would be managed. The development team noted that construction would last approximately 18 months.

Stormwater Management

Several participants inquired about the stormwater management plan for the site and how runoff would be handled to prevent any negative effects on nearby properties. Claudia Welp from CAGE Civil Engineering described the project's stormwater management approach.

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Housing Details

Meeting participants inquired as to whether the units would be rental or for-sale product (with a preference for condo expressed on the basis that renters are not invested in the local community), whether affordable housing would be included, and questions regarding anticipated rental ranges.

Changes to the Proposal as a Result of Neighbor Input

Overall the project was well received by neighbors and many meeting participants complimented the aesthetic/building design. Several modifications have been made or are being explored as a result of neighborhood input:

Parking Considerations: Based on feedback from the meetings, the project team is reviewing the parking allocation, including designating certain parking within the building as visitor parking.

Alley Adjustments: To address concerns regarding the alley, the building was shifted three feet off of the alley and a 3-foot-wide portion of the site will be voluntarily dedicated to the Village in order to widen the alley. The developer will also repave the alley as part of the project.

This report provides a comprehensive summary of the neighborhood meetings and their outcomes. Please review the attached meeting flyer, notification list, and sign-in sheet for additional context.

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NOTICE OF COMMUNITY INFORMATION MEETING

We invite you to attend a community information session presented by 4Corners, LLC regarding its application for zoning approvals for the property located at 4913-21 Forest Avenue to redevelop the site with a 7-story multi-family residential building with 62 rental residential units and 89 vehicular parking spaces. This meeting will be an opportunity to inform and answer questions regarding the proposed development.





IN PERSON MEETING OPTION

Monday, September 23, 2024 at 6:00 pm

Meeting Location: Loyal Order of Moose, Downers Grove Lodge 1535

1030 Warren Avenue, Downers Grove, IL

VIRTUAL MEETING OPTION

Monday, September 30 at 6:00 pm

Join Zoom Meeting

https://taftlaw.zoom.us/j/95306541987

Meeting ID: 953 0654 1987

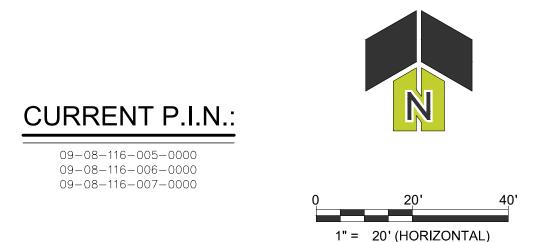
Join by Telephone

1 (312) 626 6799 US (Chicago) 1 (309) 205 3325 US

For more information about the proposal

Contact Liz Butler, the attorney for the project, at (312) 836-4121 or email at LButler@taftlaw.com

TOPOGRAPHIC AND BOUNDARY SURVEY



OWNER

SURVEYED AREA

4 CORNERS CONSTRUCTION, L 3945 OHIO AVENUE ST. CHARLES, IL 60174

21,219 SQUARE FEET (0.487 AC±)

COORDINATES AND BEARINGS ARE BASED UPON THE ILLINOIS STATE PLANE COORDINATE SYSTEM, EAST ZONE (NAD 83), ADJUSTED TO GROUND VALUES, AS ESTABLISHED BY REAL-TIME KINEMATIC (RTK) GLOBAL NAVIGATION SATELLITE SYSTEM (GNSS) UTILIZING GPS

LEGAL DESCRIPTION

LOTS 18, 19 AND 20 IN THE RESUBDIVISION OF BLOCK 5 OF E.H. PRINCE AND COMPANY'S ADDITION TO DOWNERS GROVE, IN SECTIONS 5, 6, 7 AND 8, TOWNSHIP 38 NORTH, RANGE 11 EAST OF THE THIRD PRINCIPAL MERIDIAN, ACCORDING TO THE PLAT OF SAID RESUBDIVISION RECORDED OCTOBER 24, 1891 AS DOCUMENT 46830, IN DUPAGE

BENCHMARKS

REFERENCE BENCHMARK: 2006 GEODETIC SURVEY MONUMENT DK3312 3.5" BRASS DISC SET IN CONCRETE ±0.2' ABOVE GRADE AT NORTHEAST CORNER OF WASHINGTON STREET AND WARREN AVENUE. STATION IS 57.4' SOUTHEAST OF A POWER POLE, 49.5' EAST OF A LIGHT POLE, AND 79.4' NORTHEAST OF A FIRE HYDRANT. ELEVATION: 718.78 DATUM: NAVD88-GEOID18

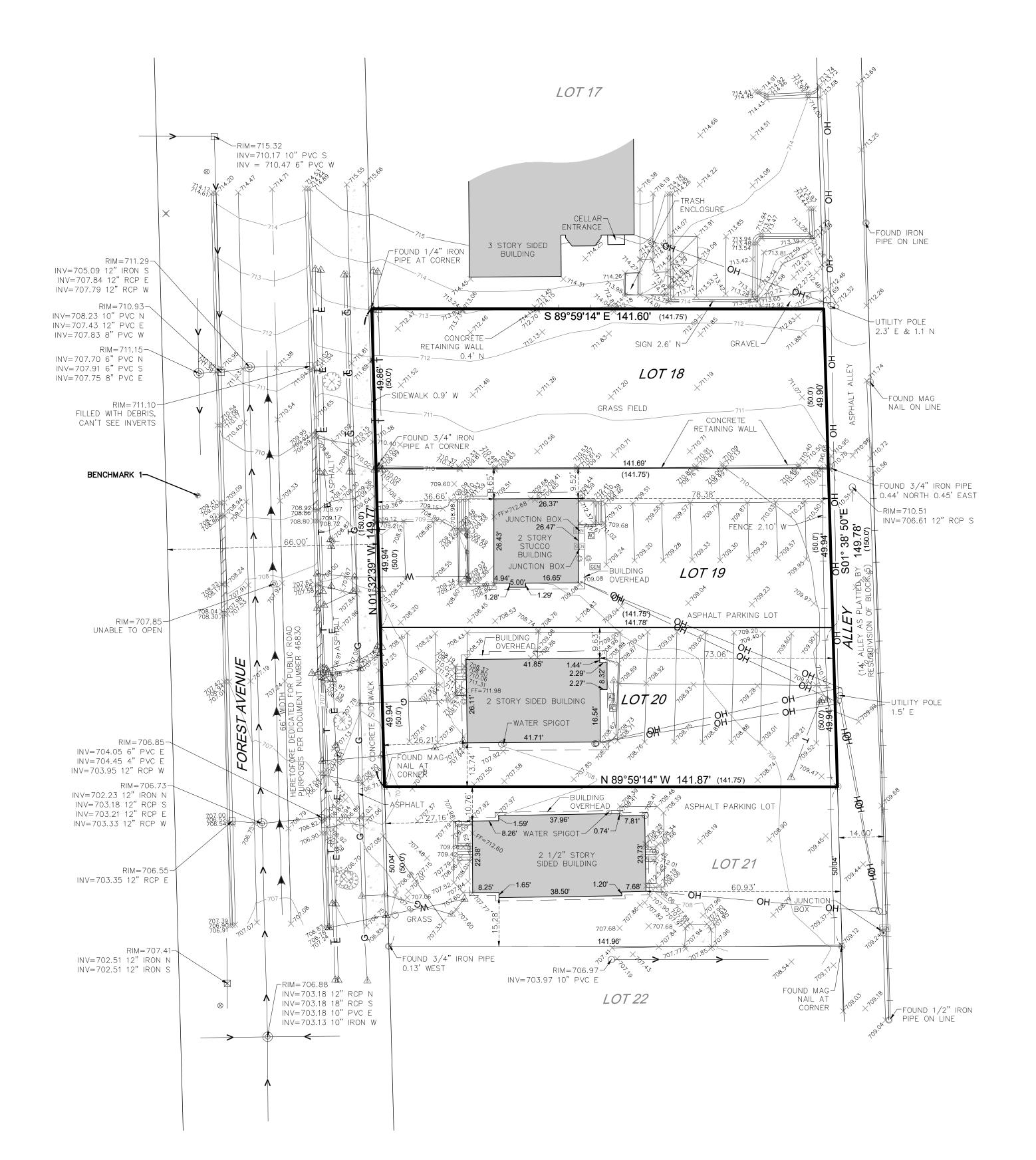
SITE BENCHMARK 1:

ELEVATION: 707.88

SOUTHWEST BOLT (TAGGED BOLT) FIRE HYDRANT AT 4910 FOREST ELEVATION: 711.23 DATUM: NAVD88-GEOID18

DATUM: NAVD88-GEOID18

SQUARE CONCRETE BASE OF FIRST LIGHT POLE SOUTH OF BENCHMARK 1 SQUARE CUT ON EAST SIDE OF BASE.



SURVEYOR'S NOTES

1. DISTANCES ARE MARKED IN FEET AND DECIMAL PLACES THEREOF. NO DIMENSION SHALL BE ASSUMED BY SCALE MEASUREMENT HEREON. DISTANCES AND/OR BEARINGS SHOWN IN PARENTHESIS (123.45') ARE RECORD OR DEED VALUES,

2.COMPARE THIS PLAT, BENCHMARKS AND ALL SURVEY MONUMENTS BEFORE BUILDING AND IMMEDIATELY REPORT ANY

3.THIS SURVEY IS SUBJECT TO MATTERS OF TITLE, WHICH MAY BE REVEALED BY A CURRENT TITLE REPORT, EASEMENTS, SETBACKS AND OTHER RESTRICTIONS WHICH MAY BE FOUND IN A CURRENT TITLE REPORT, LOCAL ORDINANCES, DEEDS OR OTHER INSTRUMENTS OF RECORD MAY NOT BE SHOWN.

4.UNLESS OTHERWISE NOTED, ONLY THE IMPROVEMENTS WHICH WERE VISIBLE FROM ABOVE GROUND AT THE TIME OF THE SURVEY AND THROUGH A NORMAL SEARCH AND WALK THROUGH OF THE SITE ARE SHOWN ON THE FACE OF THIS PLAT.

5.THIS SURVEY MAY NOT REFLECT ALL UTILITIES OR IMPROVEMENTS, IF SUCH ITEMS ARE HIDDEN BY LANDSCAPING OR ARE COVERED BY LEAVES OR OTHER OBSTRUCTIONS. THERE MAY BE ADDITIONAL UTILITIES OR IMPROVEMENTS THAT HAVE NOT

ELECTRIC CABLES OR CONDUITS, GAS MAINS AND ALL SERVICE LINES SHOWN HEREON ARE BASED ON THE ACTUAL OBSERVED LOCATION AT AN OPEN MANHOLE. THE EXACT LOCATION MAY DIFFER FROM THE LOCATION SHOWN HEREON.

7.OTHER THAN VISIBLE OBSERVATIONS NOTED HEREON, THIS SURVEY MAKES NO STATEMENT REGARDING THE ACTUAL PRESENCE OR ABSENCE OF ANY SERVICE OR UTILITY LINE. CONTROLLED UNDERGROUND EXPLORATORY EFFORTS TOGETHER WITH UTILITY MARKINGS (J.U.L.I.E., DIGGER, PRIVATE, ETC) IS RECOMMENDED TO DETERMINE THE FULL EXTENT OF UNDERGROUND SERVICE AND UTILITY LINES.

8. THIS SURVEY WAS PREPARED FOR DEJAMES BUILDERS, INC. (CLIENT), BASED ON A FIELD SURVEY COMPLETED ON MAY 28,

9.CAGE CIVIL ENGINEERING, LLC IS A PROFESSIONAL DESIGN FIRM, CURRENT LICENSE NO. 184007577, EXPIRES APRIL 30,

LEGEND

BOUNDARY LINE =		EX. CONTOUR =	740
			740
R.O.W. LINE =		FOUND IRON PIPE/ROD =	
EASEMENT LINE =		EX. STORM MANHOLE =	
PAVEMENT LINE =		EX. CATCH BASIN =	\circ
CURB & GUTTER =		EX. INLET =	
CONCRETE SIDEWALK =		EX. SANITARY MANHOLE =	\bigcirc
SANITARY LINE =		EX. DOWN DRAIN / CLEANOUT	=
STORM LINE =	$\longrightarrow \hspace{1cm}$	EX. WATER MAIN MARKER =	
EX. WATER MAIN =	W		
EX. UNDERGROUND ELECTRIC LINE =	———-E-——	EX. VALVE BOX =	⊗
EX. TELEPHONE LINE =	т	EX. VALVE VAULT =	
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		RECORD INFORMATION =	(XXX.XX)
EX. ELECTRIC LIGHT POLE =		MEASURED INFORMATION =	XXX.XX

SURVEYOR'S CERTIFICATE

STATE OF ILLINOIS)

COUNTY OF DUPAGE)

I, GABRIELA PTASINSKA, AN ILLINOIS PROFESSIONAL LAND SURVEYOR, HEREBY CERTIFY THAT THIS PLAT AND THE SURVEY UPON WHICH IT IS BASED HAS BEEN PREPARED FOR THE USES AND PURPOSES HEREIN SET FORTH.

ALL DIMENSIONS ARE GIVEN IN FEET AND DECIMALS THEREOF.

GIVEN UNDER MY HAND AND SEAL THIS 5TH DAY OF JUNE, A.D. 2024.

GABRIELA PTASINSKA U GPTASINSKA@CAGECIVIL.COM ILLINOIS PROFESSIONAL LAND SURVEYOR NO. 3892 LICENSE EXPIRES NOVEMBER 30, 2024

DESIGN FIRM PROFESSIONAL LICENSE NO. 184007577 LICENSE EXPIRES APRIL 30, 2025.

THIS PROFESSIONAL SERVICE CONFORMS TO THE CURRENT ILLINOIS MINIMUM STANDARDS FOR A BOUNDARY SURVEY.

DATE OF FIELD SURVEY: MAY 28, 2024



REVISIONS \triangle

HIGH RISE

ROJ NO: 230368

TE: 06/05/2024 CALE : 1" = 20' SHEET NUMBER







DISTINCTIVE FORMED METAL CAP TO CREATE A SENSE OF FINALITY AND VISUAL INTRIGUE













4CORNERS **MULTI-FAMILY** HIGH RISE

4 CORNERS CONSTRUCTION, LLC

ST CHARLES, IL 60174

4919 FOREST AVE DOWNERS GROVE, IL 60515

DESIGN FIRM REGISTRATION #:184.006200-0001

The drawings and building design are and shall remain property and copyrights of the Architect. No part thereof shall be copied or disclosed to others or used in the connection with any work or project other than the spec

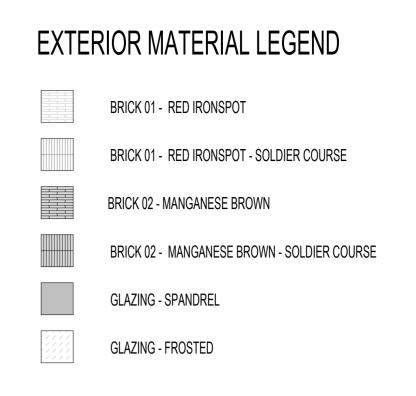
Contractor shall construct the work in conformance with all

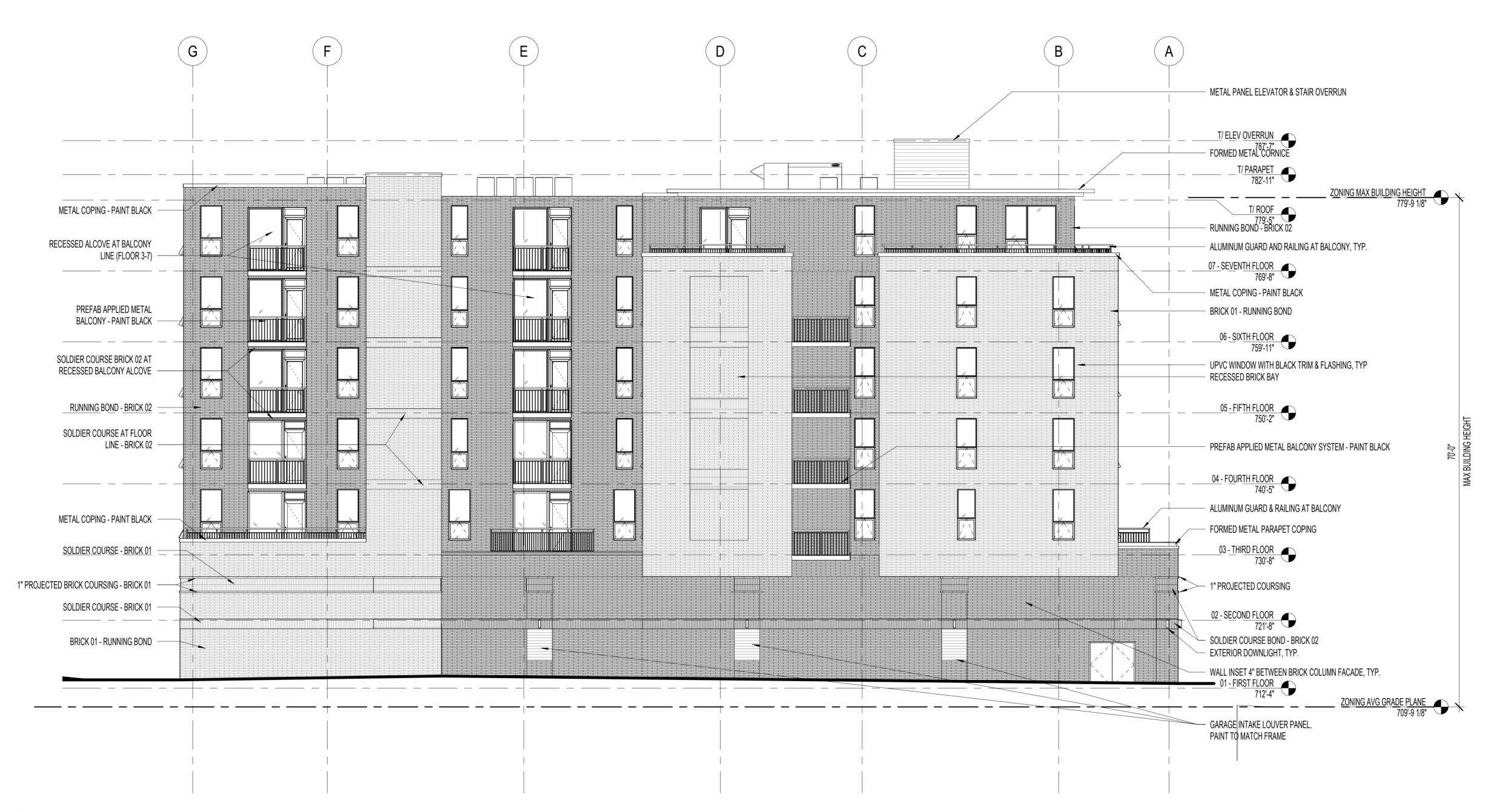
Written dimensions on these drawings shall precede over scaled dimensions on these drawings shall precede over scaled dimensions. Drawings shall not be scaled, notify Architect of dimensional information if not indicated. Contractor shall verify all existing conditions prior to proceeding with Construction. Architect shall be notified immediately of any discrepancies or conflicts.

Copyright 2023 Kennedy Mann Architecture, LLC

No.	Description	Date
	PLAN REVIEW SUBMISSION	08.29.2024
	RESPONSE TO PUD COMMENTS #1	10.18.2024
	RESPONSE TO PUD COMMENTS #2	10.31.2024
	RESPONSE TO PUD COMMENTS #3	11.15.2024
DRAWN BY JP		

BK As indicated 08.29.2024 2415





NORTH ELEVATION
SCALE: 1/8" = 1'-0"

KENEDY

2822 West Montrose Avenue Chicago, Illinois 60618 USA

> 4CORNERS MULTI-FAMILY HIGH RISE

4 CORNERS CONSTRUCTION, LLC

3945 OHIO AVE ST CHARLES, IL 60174

4919 FOREST AVE DOWNERS GROVE, IL 60515

DESIGN FIRM REGISTRATION #:184.006200-0001

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Written dimensions on these drawings shall precede over scaled dimensions. Drawings shall not be scaled, notify Architect of dimensional information if not indicated. Contractor shall verify all existing conditions prior to proceeding with Construction. Architect shall be notified immediately of any discrepancies or conflicts.

Contractor is responsible for design and installation of properly sized and loaded systems. Submit shop drawings to architect for approval on conformity to Architectural design intent.

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No.	Description	Date
	PLAN REVIEW SUBMISSION	08.29.2024
	RESPONSE TO PUD COMMENTS #1	10.18.2024
	RESPONSE TO PUD COMMENTS #2	10.31.2024
DRAW	N BY	Author

 DRAWN BY
 Author

 CHECKED BY
 Checker

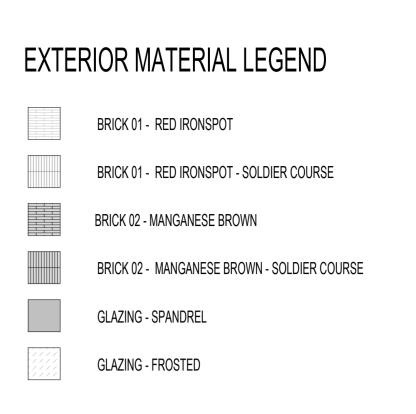
 SCALE
 1/8" = 1'-0"

 PROJECT START DATE
 08.29.2024

 PROJECT NUMBER
 2415

NORTH ELEVATION

SD2.02





EAST ELEVATION
SCALE: 1/8" = 1'-0"

2822 West Montrose Avenue Chicago, Illinois 60618 USA

4CORNERS **MULTI-FAMILY** HIGH RISE

4 CORNERS CONSTRUCTION, LLC

ST CHARLES, IL 60174

4919 FOREST AVE DOWNERS GROVE, IL 60515

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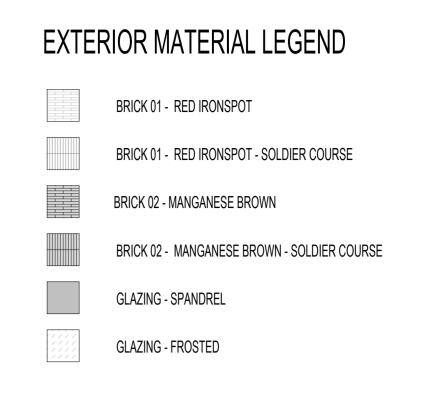
No.	Description	Date
	PLAN REVIEW SUBMISSION	08.29.2024
	RESPONSE TO PUD COMMENTS #1	10.18.2024
	RESPONSE TO PUD COMMENTS #2	10.31.2024
DRAW	N BY	Author

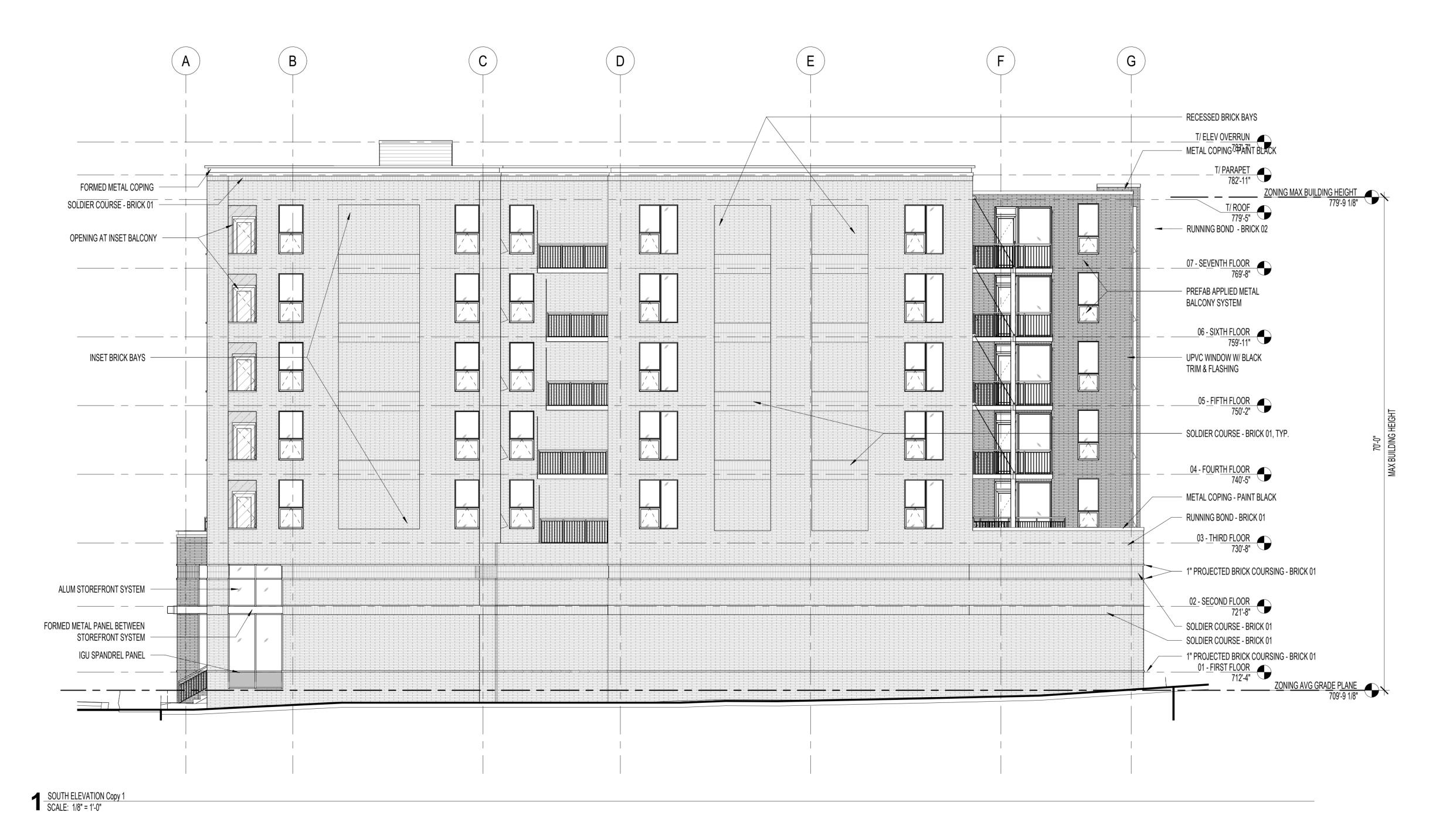
CHECKED BY Checker SCALE 1/8" = 1'-0" PROJECT START DATE 08.29.2024 PROJECT NUMBER

2415

EAST ELEVATION (ALLEY)

ORD 2024-10610 Page 40 of 243





2822 West Montrose Avenue Chicago, Illinois 60618 USA

4CORNERS **MULTI-FAMILY** HIGH RISE

4 CORNERS CONSTRUCTION, LLC

ST CHARLES, IL 60174

4919 FOREST AVE DOWNERS GROVE, IL 60515

DESIGN FIRM REGISTRATION #:184.006200-0001

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No.	Description	Date
	PLAN REVIEW SUBMISSION	08.29.202
	RESPONSE TO PUD	10.18.202
	COMMENTS #1	
	RESPONSE TO PUD	10.31.202
	COMMENTS #2	
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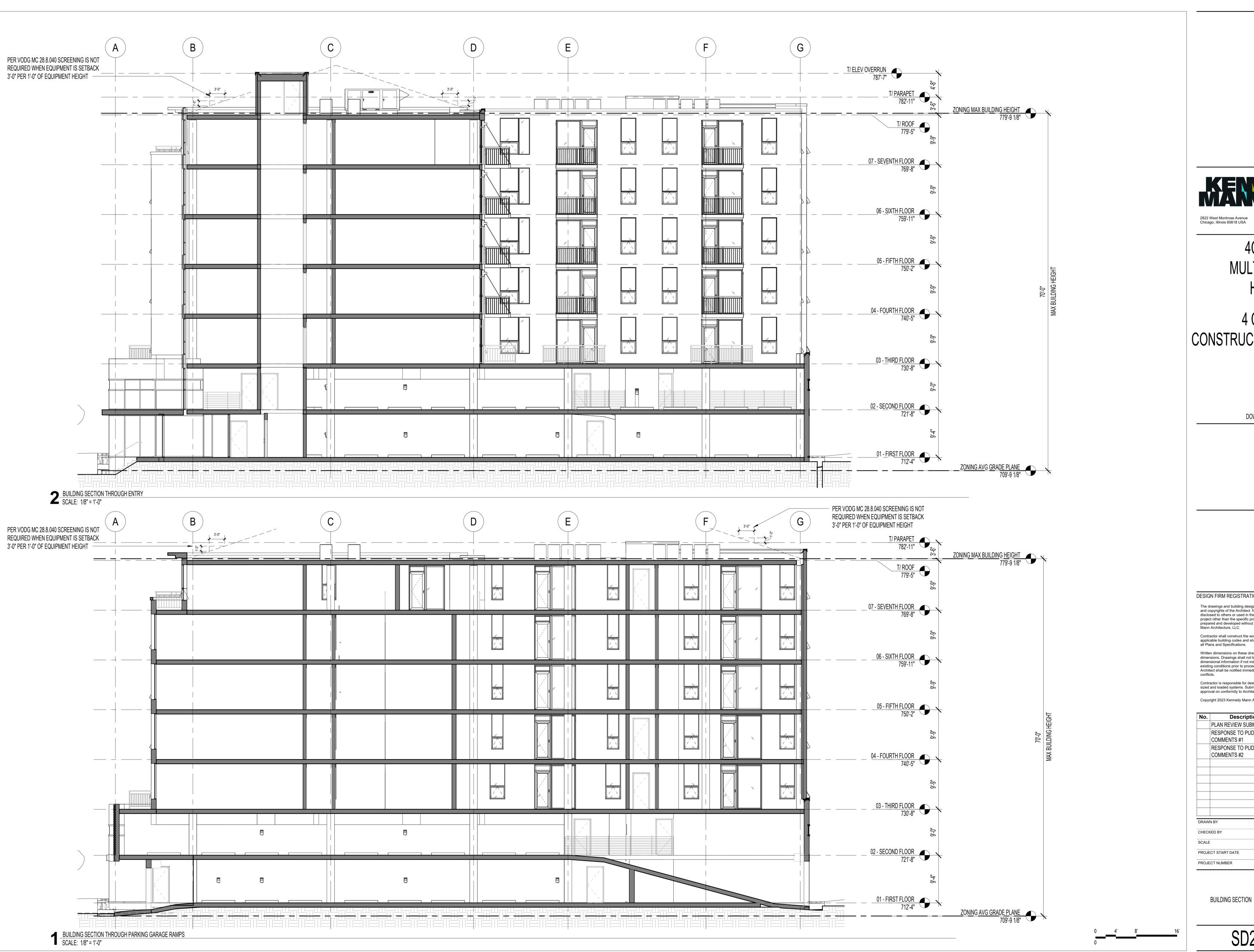
CHECKED BY Checker SCALE 1/8" = 1'-0" PROJECT START DATE 08.29.2024 PROJECT NUMBER

2415

SOUTH ELEVATION

SD2.04

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4CORNERS **MULTI-FAMILY** HIGH RISE

4 CORNERS CONSTRUCTION, LLC

3945 OHIO AVE ST CHARLES, IL 60174

4919 FOREST AVE DOWNERS GROVE, IL 60515

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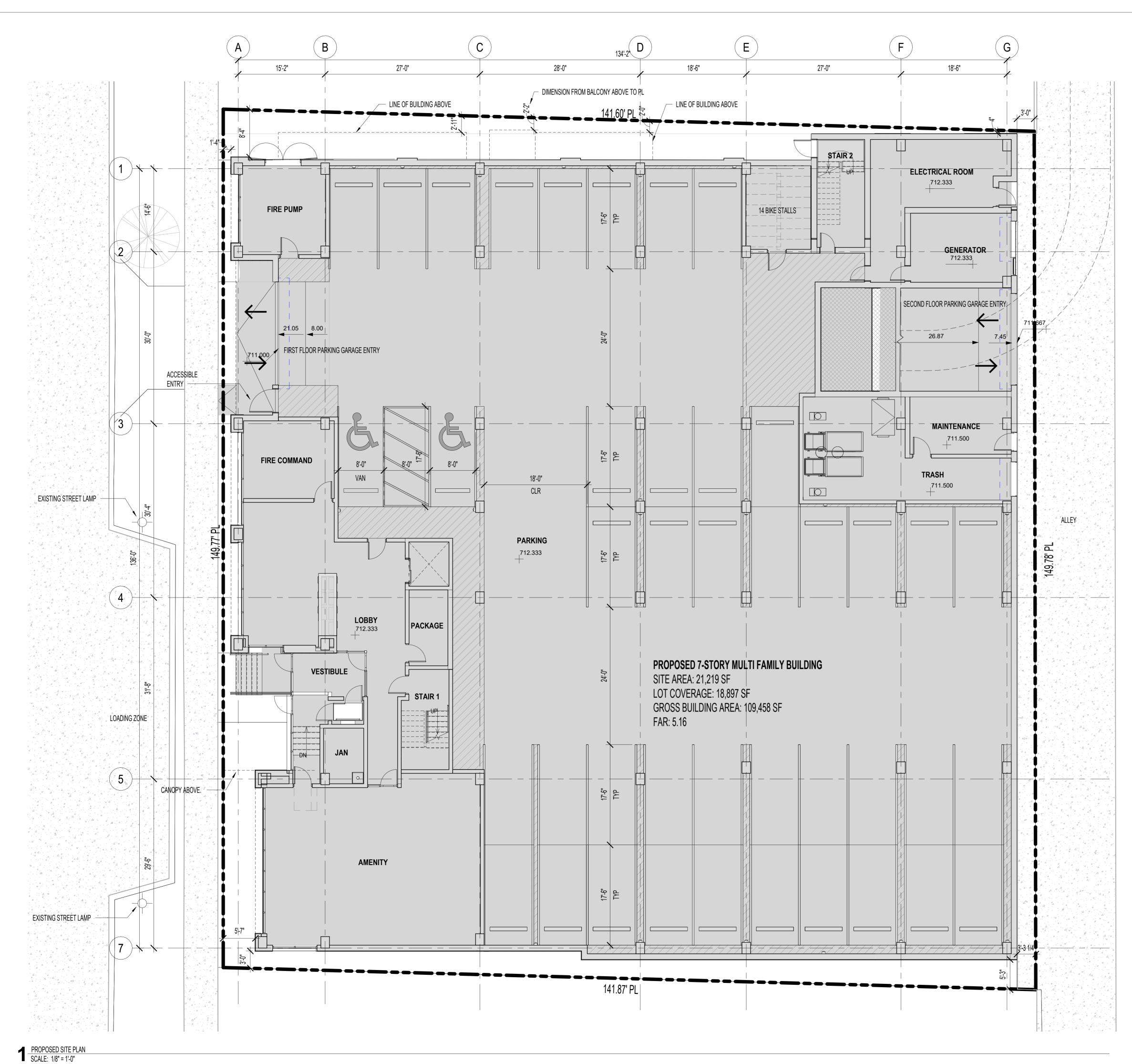
No.	Description	Date
	PLAN REVIEW SUBMISSION	08.29.2024
	RESPONSE TO PUD	10.18.2024
	COMMENTS #1	
	RESPONSE TO PUD	10.31.2024
	COMMENTS #2	
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SCALE		/8" = 1'-0"

08.29.2024

2415

SD2.05

ORD 2024-10610 Page 42 of 243



Count	Stall Type	Stall Size
01 - FIRST	FLOOR	
2	ACCESSIBLE STALL	8'-0" x 17'-6
18	STANDARD STALL	8'-6" x 17'-6
22	TANDEM STALL	8'-6" x 17'-6
42		
02 - SECOND FLOOR		
2	ACCESSIBLE STALL	8'-0" x 17'-6
17	STANDARD STALL	8'-6" x 17'-6
28	TANDEM STALL	8'-6" x 17'-6
47		'

GROSS BUILDING AREA	
Level	Area
01 - FIRST FLOOR	17,972 SF
02 - SECOND FLOOR	17,668 SF
03 - THIRD FLOOR	14,539 SF
04 - FOURTH FLOOR	14,539 SF
05 - FIFTH FLOOR	14,539 SF
06 - SIXTH FLOOR	14,539 SF
07 - SEVENTH FLOOR	13,912 SF
GROSS BUILDING AREA	107,707 SF



2822 West Montrose Avenue Chicago, Illinois 60618 USA

> 4CORNERS MULTI-FAMILY HIGH RISE

4 CORNERS CONSTRUCTION, LLC

3945 OHIO AVE ST CHARLES, IL 60174

4919 FOREST AVE DOWNERS GROVE, IL 60515

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	RESPONSE TO PUD COMMENTS #2	10.31.2024
	RESPONSE TO PUD COMMENTS #3	11.15.2024

 DRAWN BY
 JP

 CHECKED BY
 BK

 SCALE
 1/8" = 1'-0"

 PROJECT START DATE
 08.29.2024

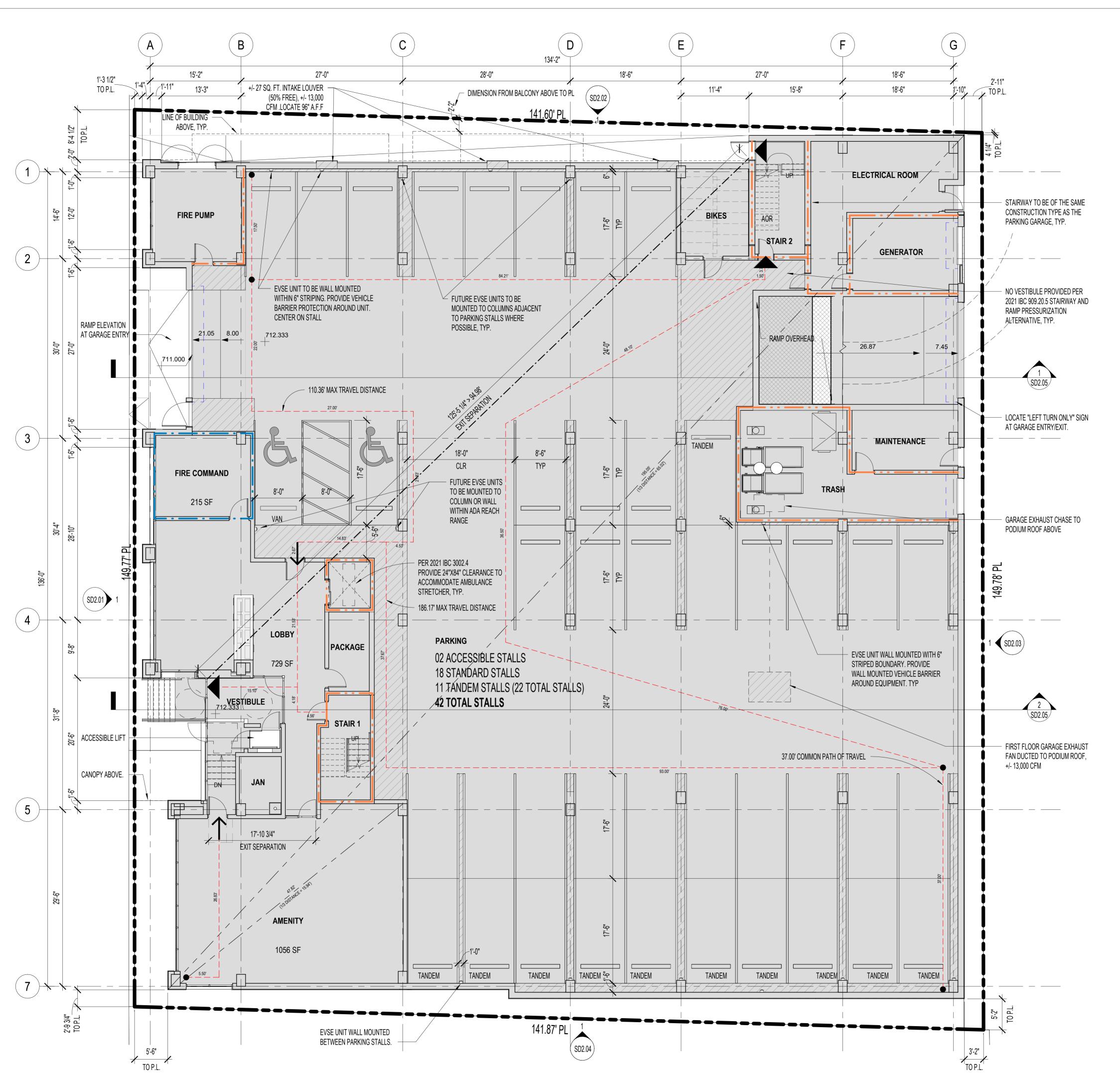
PROPOSED SITE PLAN

PROJECT NUMBER

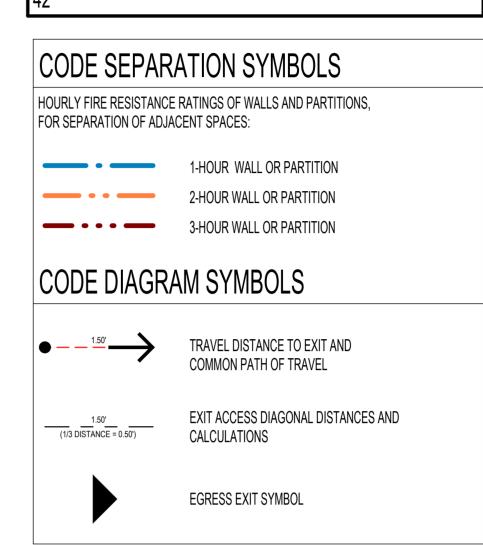
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2415

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PARKING SCHEDULE		LE
COUNT	STALL TYPE	STALL SIZE
01 - FIRST	FLOOR	
2	ACCESSIBLE STALL	8'-0" x 17'-6
18	STANDARD STALL	8'-6" x 17'-6
22	TANDEM STALL	8'-6" x 17'-6
42	•	,





2822 West Montrose Avenue Chicago, Illinois 60618 USA

> 4CORNERS MULTI-FAMILY HIGH RISE

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3945 OHIO AVE ST CHARLES, IL 60174

4919 FOREST AVE DOWNERS GROVE, IL 60515

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	RESPONSE TO PUD COMMENTS #2	10.31.2024
	RESPONSE TO PUD COMMENTS #3	11.15.2024

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BK

SCALE

As indicated

PROJECT START DATE

08.29.2024

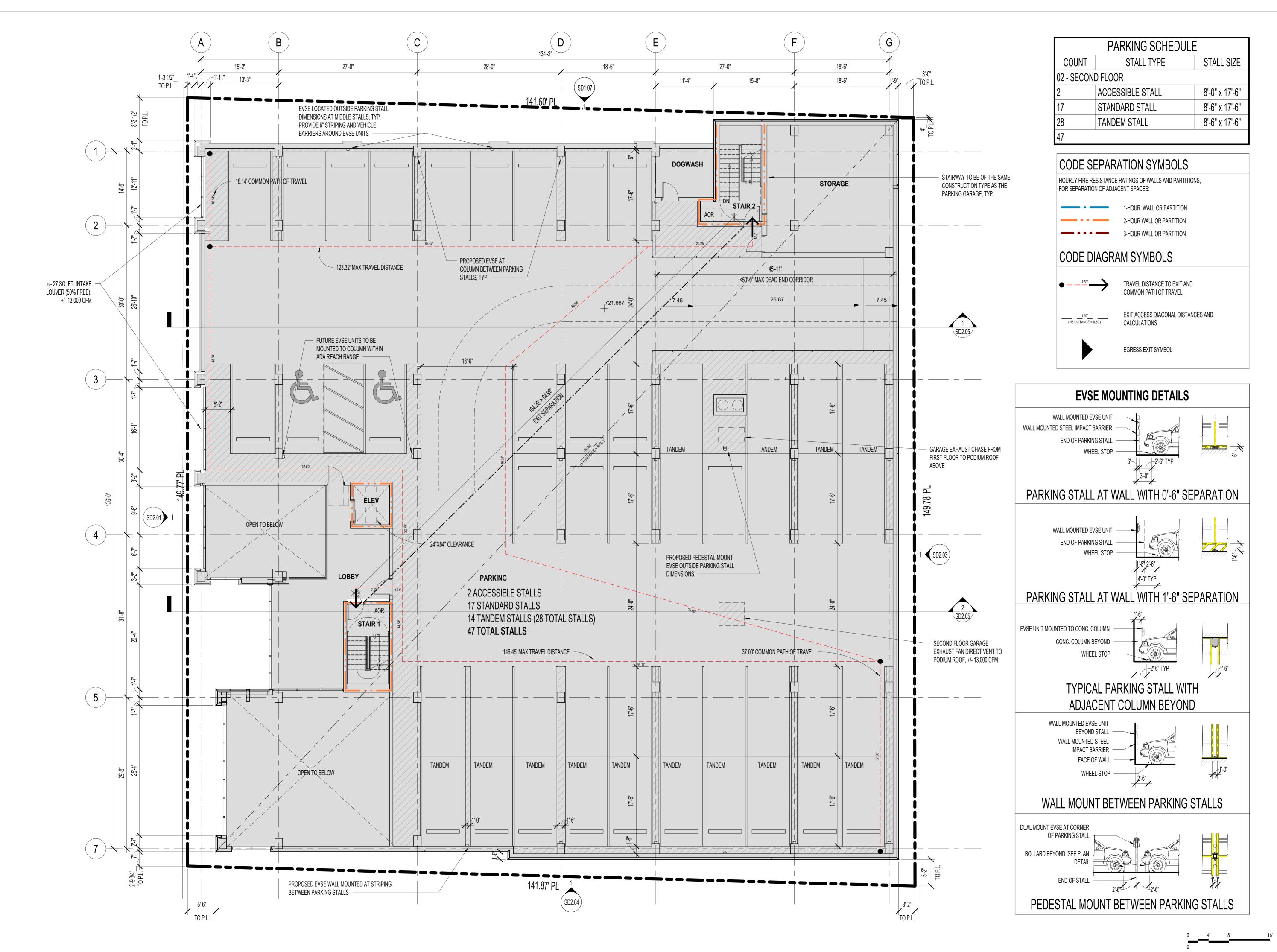
PROJECT NUMBER

2415

PROPOSED FIRST FLOOR PLAN

SD1.01

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4 CORNERS CONSTRUCTION, LLC

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Architect shall be notified immediately of any discrepancies or

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).	Description	Date
	PLAN REVIEW SUBMISSION	08.29.2024
	RESPONSE TO PUD	10.18.2024
	COMMENTS #1	
	RESPONSE TO PUD	10.31.2024
	COMMENTS #2	
	RESPONSE TO PUD	11.15.2024
	COMMENTS #3	
		ı

DRAWN BY

CHECKED BY

BK

SCALE

As indicated

PROJECT START DATE

08.29.2024

PROPOSED SECOND FLOOR PLAN

PROJECT NUMBER

SD1.02

2415

Page 45 0

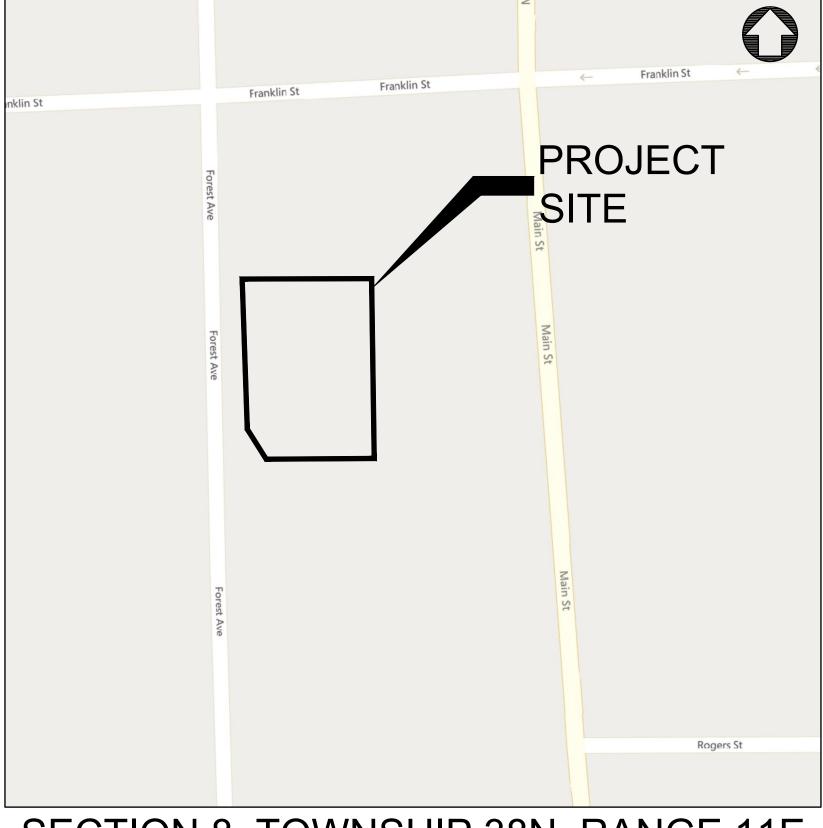
PRELIMINARY ENGINEERING FOR

4 CORNERS MULTI-FAMILY HIGH RISE

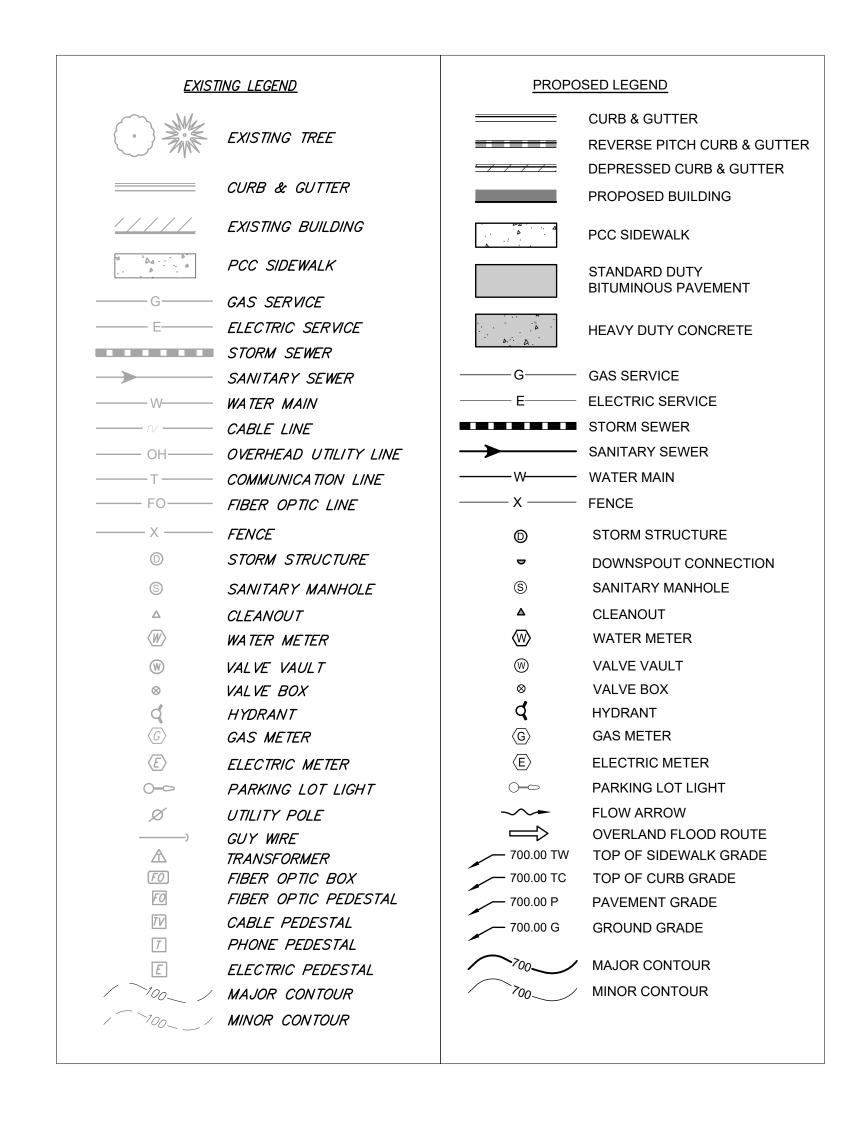
4919 FOREST AVE, DOWNERS GROVE, IL 60515

INDEX OF SHEETS		
Sheet Number	Sheet Title	
C0.0	SITE LOCATION MAP & CIVIL LEGEND	
C1.0	EXISTING CONDITIONS & DEMOLITION PLAN	
C2.0	SITE LAYOUT PLAN	
C3.0	SITE GRADING PLAN	
C3.1	SOIL EROSION & SEDIMENT CONTROL PLAN	
C3.2	SOIL EROSION & SEDIMENT CONTROL DETAILS	
C4.0	SITE UTILITY PLAN	
C5.0	CONSTRUCTION DETAILS	
C5.1	CONSTRUCTION DETAILS	
C5.2	CONSTRUCTION DETAILS	
C5.3	CONSTRUCTION DETAILS	
C5.4	CONSTRUCTION DETAILS	
C5.5	CONSTRUCTION DETAILS	

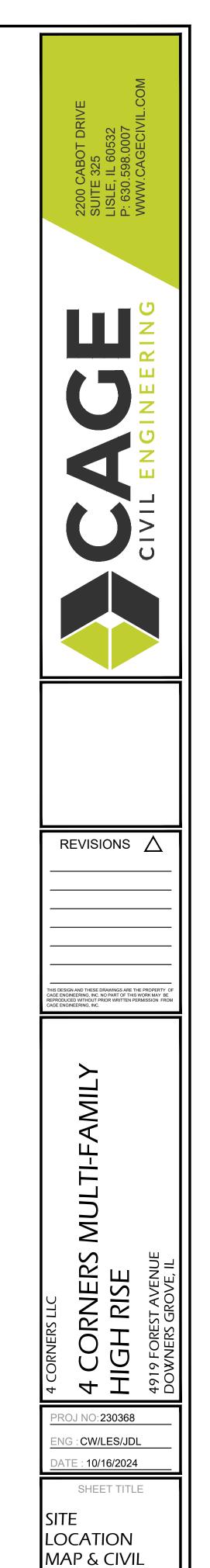
LOCATION MAP



SECTION 8, TOWNSHIP 38N, RANGE 11E



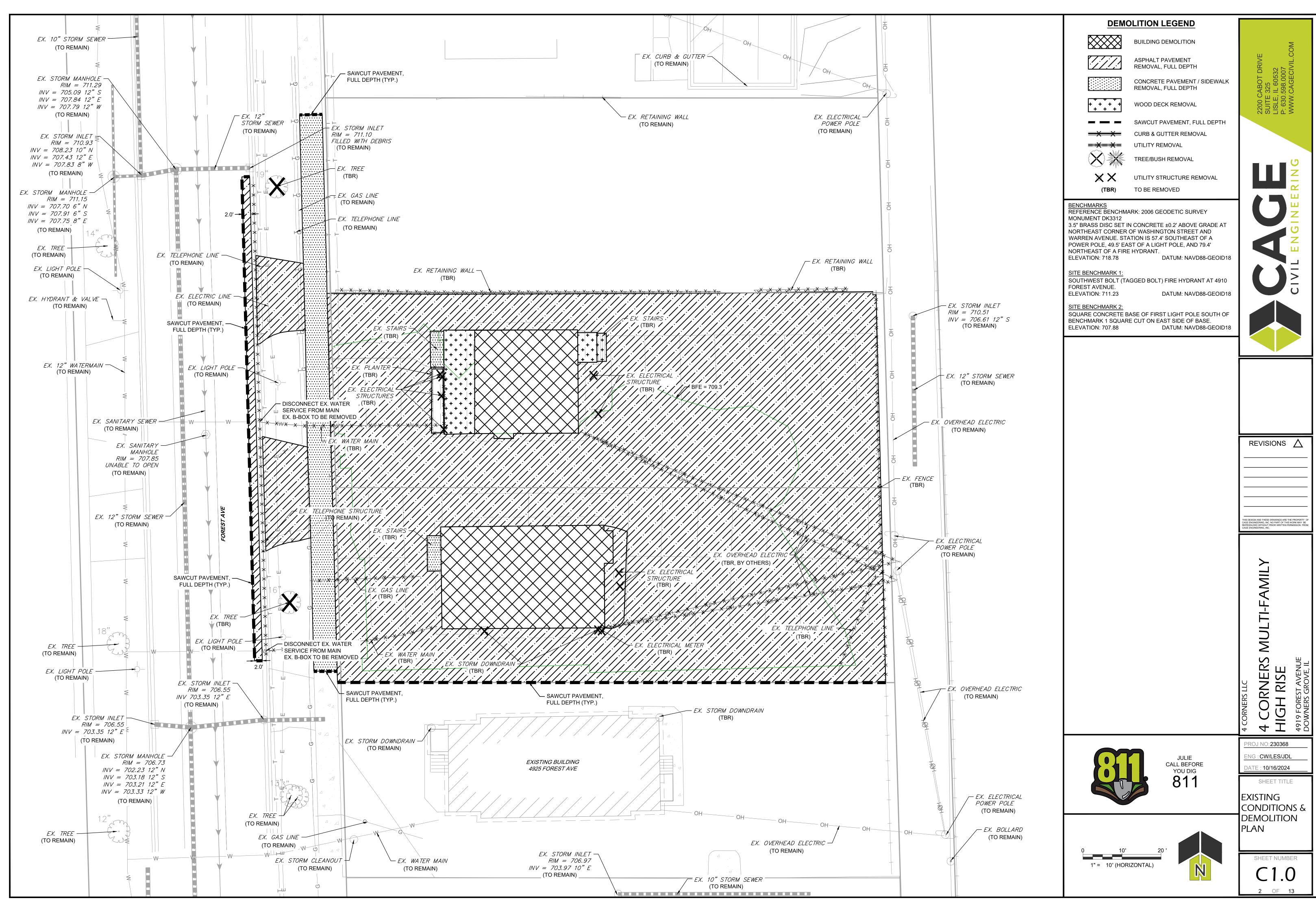


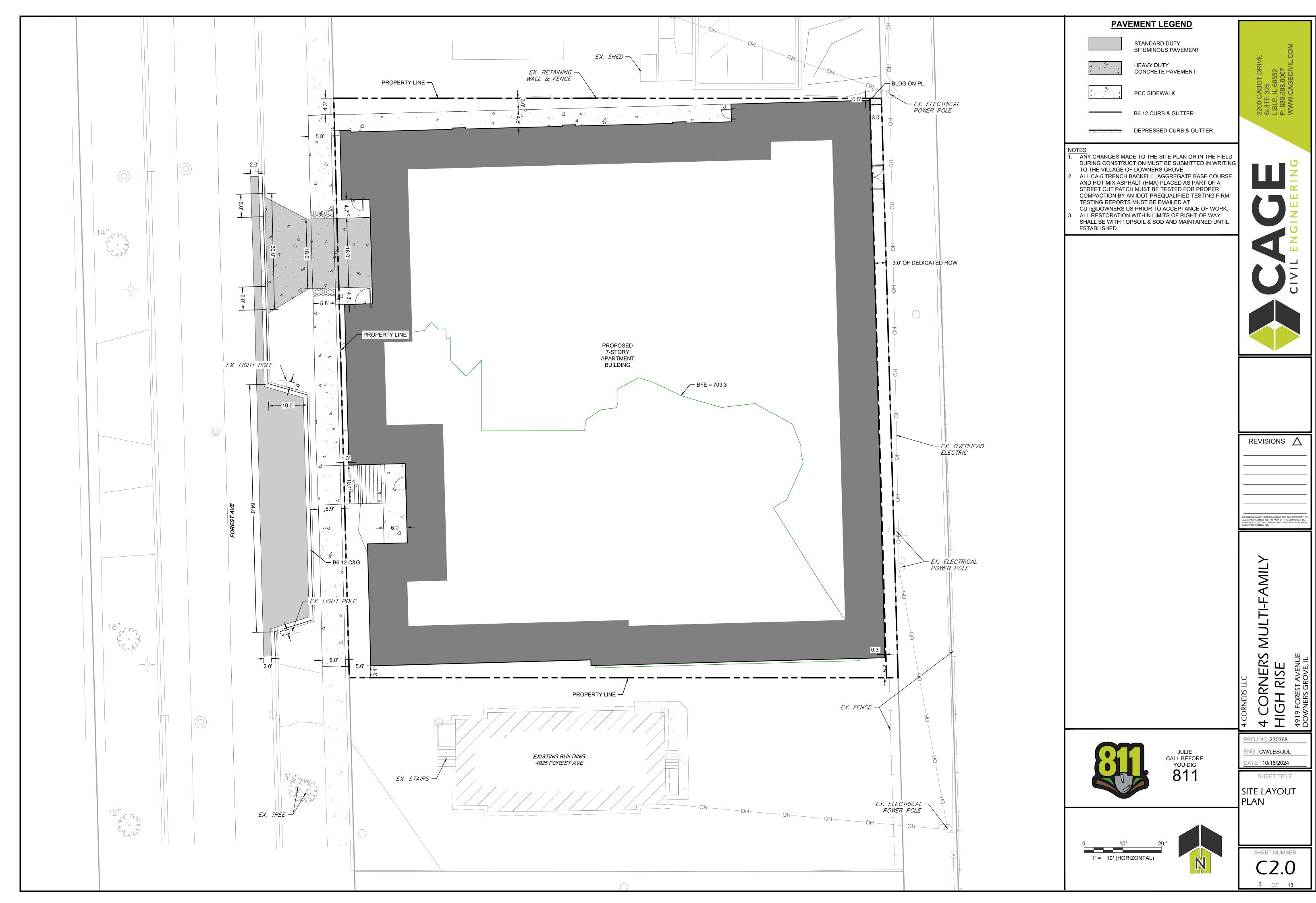


LEGEND

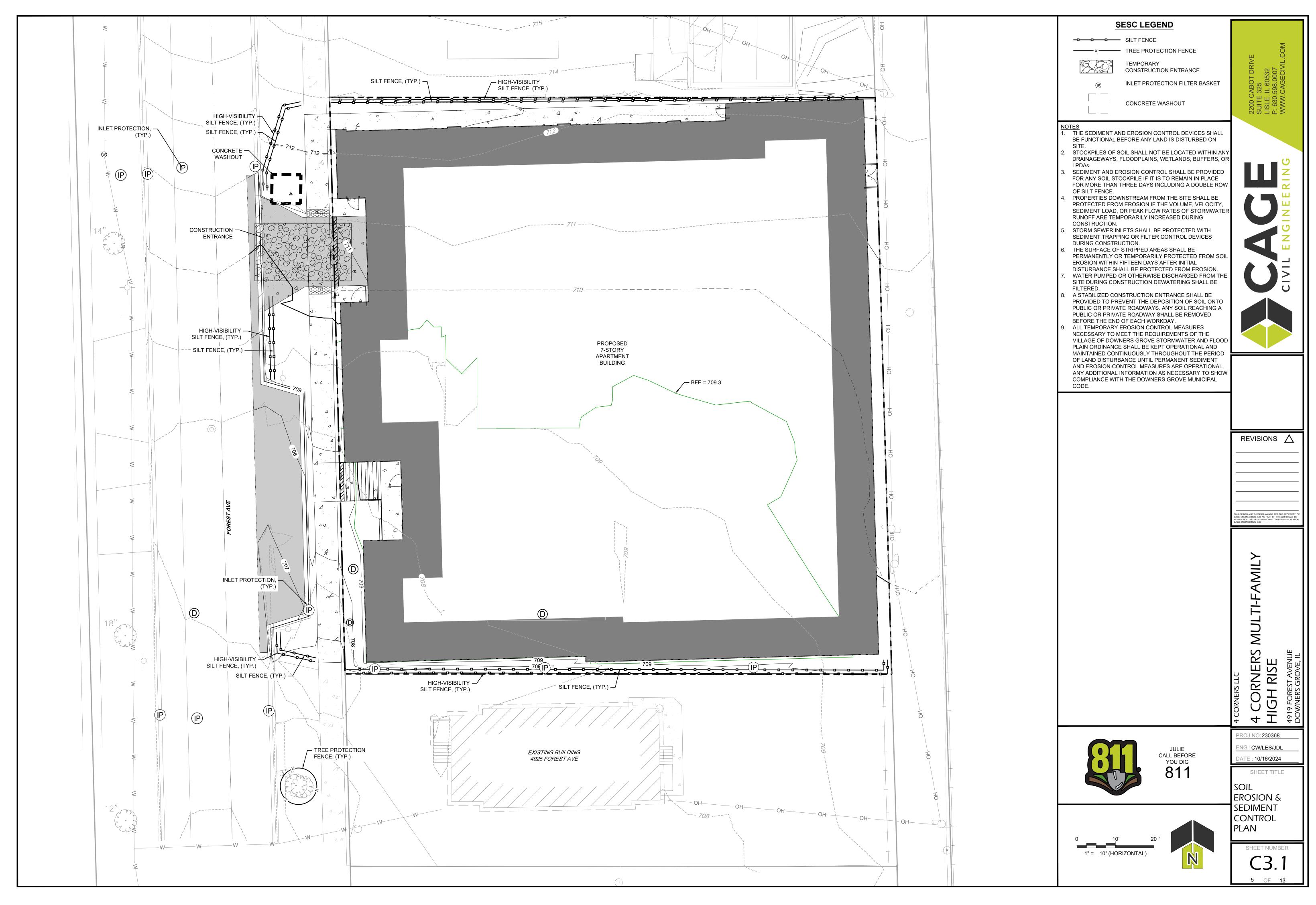
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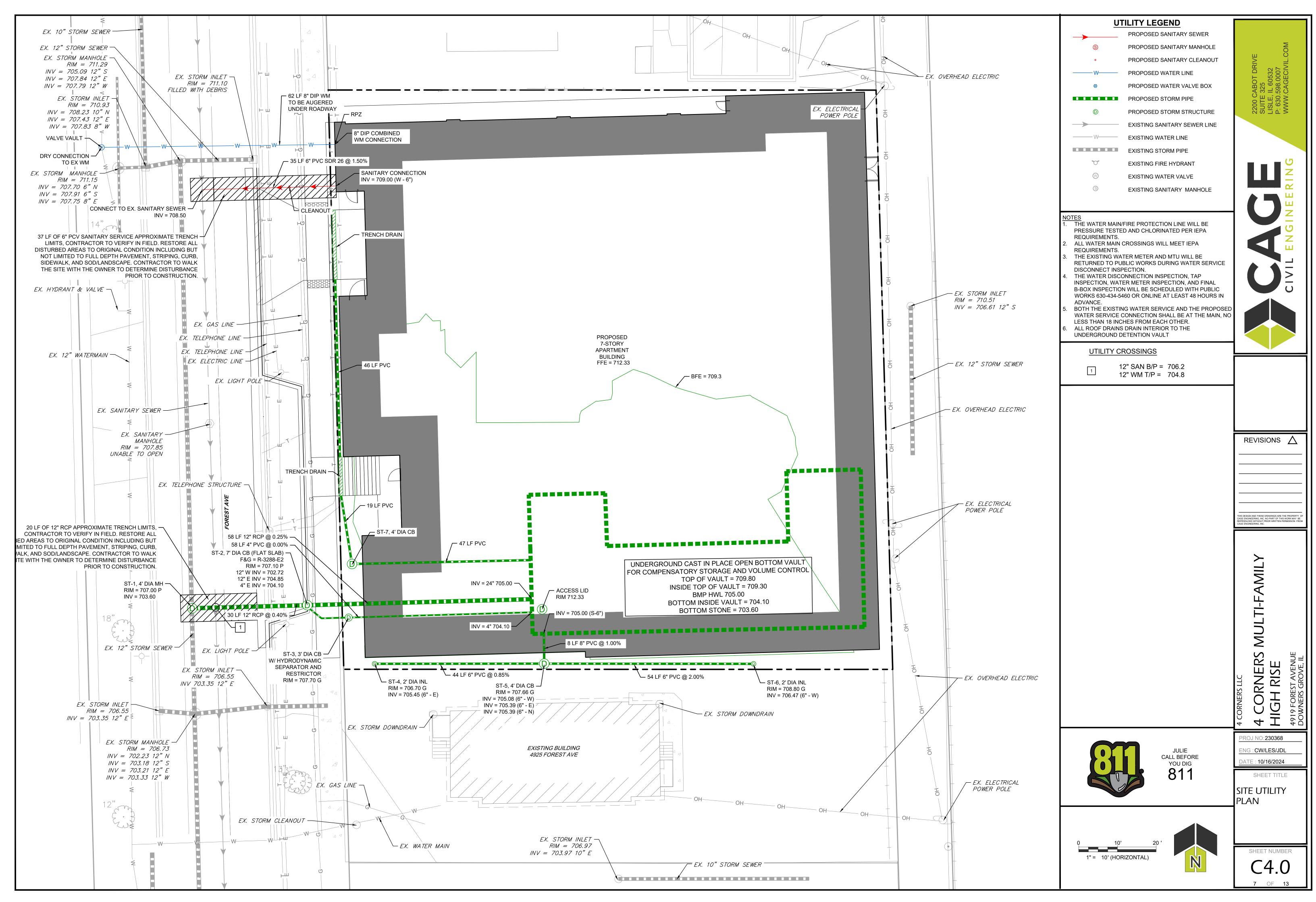




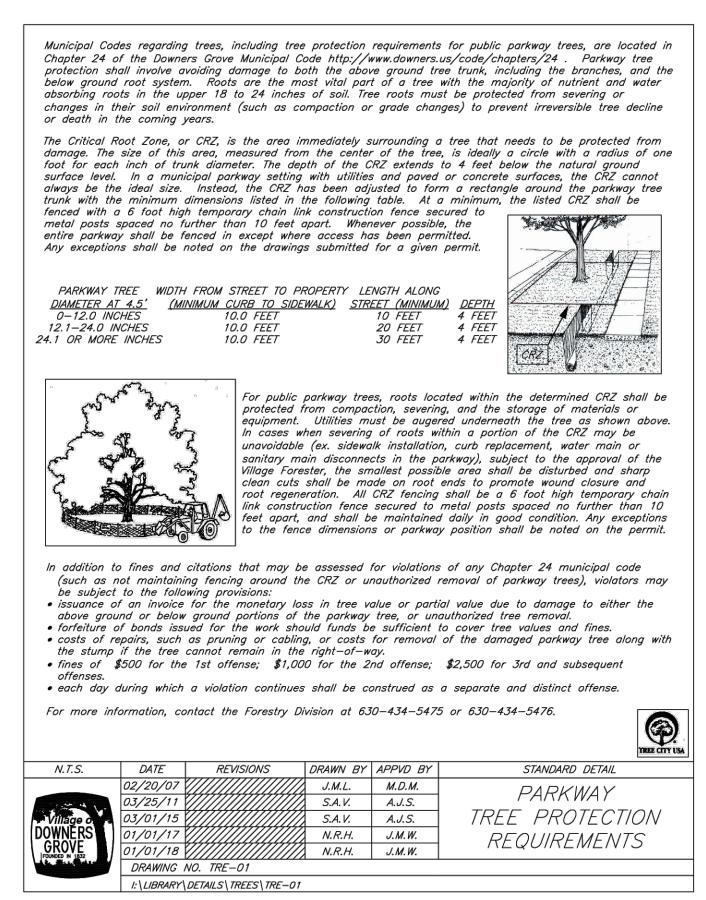




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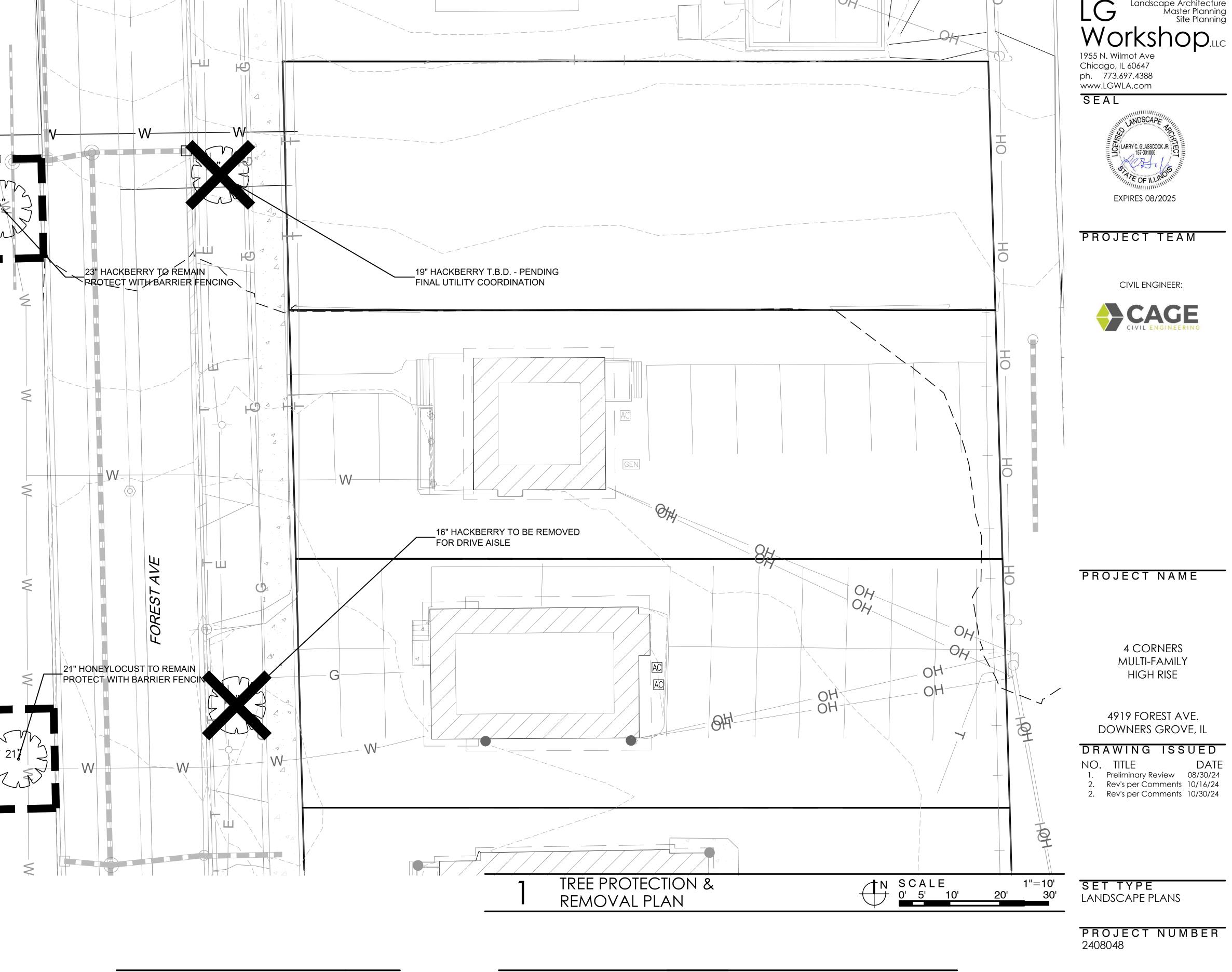


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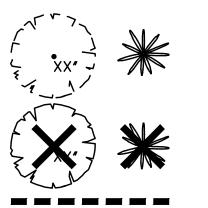


TREE PROTECTION & REMOVAL NOTES

- 1. A TREE REMOVAL PERMIT SHALL BE REQUIRED FOR THE REMOVAL OF ANY TREE ON VILLAGE OWNED OR MAINTAINED LAND.
- 2. CONTRACTOR SHALL OBTAIN ALL NECESSARY STATE AND LOCAL PERMITS AND PERMISSIONS TO PRUNE, REMOVE, AND/OR TRANSPLANT ANY TREES ON SITE.
- 3. DEAD AND DYING MATERIAL ON THE SITE SHALL BE REMOVED OR PRUNED. MATERIALS NOT LABELED ON THE PROTECTION PLAN SHALL BE BROUGHT TO THE ATTENTION OF THE LANDSCAPE ARCHITECT FOR REMEDIATION.
- 4. DURING CONSTRUCTION EXISTING TREES OVER FOUR INCHES IN CALIPER SHALL BE PROTECTED WITH BARRIER FENCING.
- 5. BARRIER SHALL BE CONSTRUCTED OF A MIN. 6' TALL TEMPORARY CHAINLINK OR SIMILAR AND SUPPORT POSTS MIN. 6' O.C. AND SHALL BE ERECTED ONE FOOT BEYOND THE DRIP LINE OF ALL EXISTING TREES ON SITE AND ADJACENT SITES TO REMAIN.
- 6. A TREE PROTECTION SIGN (AVAILABLE FROM THE VILLAGE AT TIME OF PERMIT PICK-UP) SHALL BE PLACED ON THE FENCE IDENTIFYING THE TREE PROTECTION AREA.
- 7. NO TRENCHING OR AUGURING MAY OCCUR PRIOR TO THE COMPLETION OF A WATER SERVICE PRE-TAP INSPECTION.
- 8. THE PUBLIC WORKS DEPARTMENT WILL INSPECT THE PARKWAY TREES AS PART OF THE WATER SERVICE PRE-TAP INSPECTION TO MAKE CERTAIN THAT THE INSTALLATION OF THE WATER SERVICE DOES NOT NEGATIVELY IMPACT THE TREES.
- 9. NO EXCESS SOIL OR ADDITIONAL FILL, BUILDING MATERIALS OR DEBRIS SHALL BE PLACED WITHIN THE PROTECTIVE BARRIER.
- 9. KEEP ALL EXCAVATIONS OUTSIDE THE TREE PROTECTION FENCE.
- 10. NO VEHICLES OR HEAVY MACHINERY SHALL BE ALLOWED TO WORK WITHIN THE BARRIER AREA.
- 11. NO ATTACHMENTS OR WIRES, OTHER THAN PROTECTIVE GUY WIRES, SHALL BE ATTACHED TO ANY OF THE TREES WHICH ARE WITHIN PROTECTIVE BARRIER.
- 12. STUMPS OR TREE REMAINS NOT TO BE FULLY EXCAVATED SHALL BE REMOVEED. A STUMP GRINDER SHALL BE USED TO REMOVE ALL REMAINING ROOTS AND WOODY MATERIAL. WITHIN A 24" RADIUS OF THE TREE TRUNK TO MIN. 6" BELOW GRADE. DISTURBED AREA SHALL BE BACKFILLED WITH COMPACTED TOPSOIL TO MEET SURROUNDING GRADES.
- 13. ALL RESTORATION WITHIN LIMITS OF RIGHT-OF-WAY SHALL BE WITH TOPSOIL & SOD AND MAINTAINED UNTIL ESTABLISHED



TREE PROTECTION & REMOVAL LEGEND



EXISTING TREE TO REMAIN

EXISTING TREE TO BE REMOVED

6' HT. TEMPORARY CHAIN LINK BARRIER FENCING

EXISTING VEGETATION DESCRIPTION

THE PROJECT SITE CONSISTS OF A THREE LOTS WITH TWO EXISTING STRUCTURES ON THEM. 2 PARKWAY TREES WILL BE REMOVED AS PART OF THIS CONTRACT. REPLACEMENT TREES FOR THE REMOVALS WILL HAVE TO BE CASH-IN-LIEU.



DATE

1. Preliminary Review 08/30/24 2. Rev's per Comments 10/16/24

PROJECT NUMBER

DATE 08-28-2024 DRAWN BY: APPROVED BY: SHEET TITLE TREE PROTECTION & REMOVAL

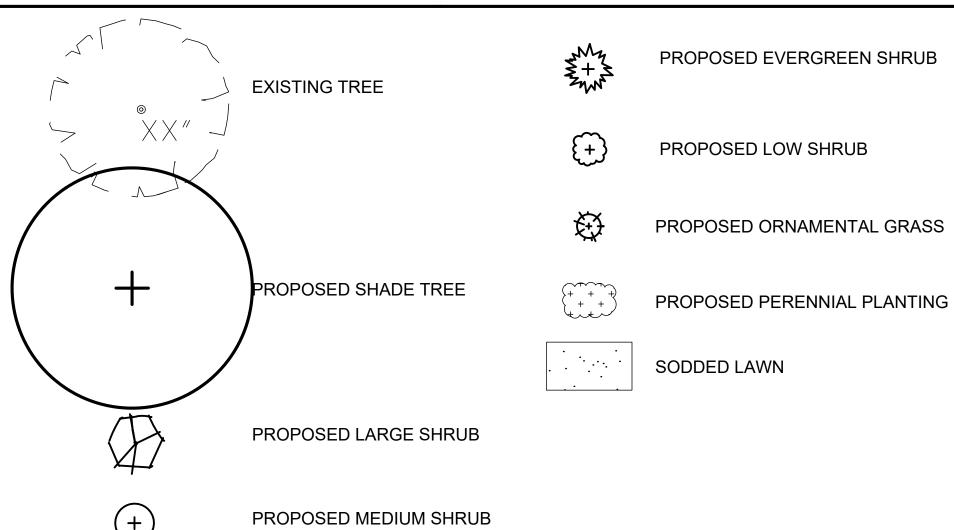
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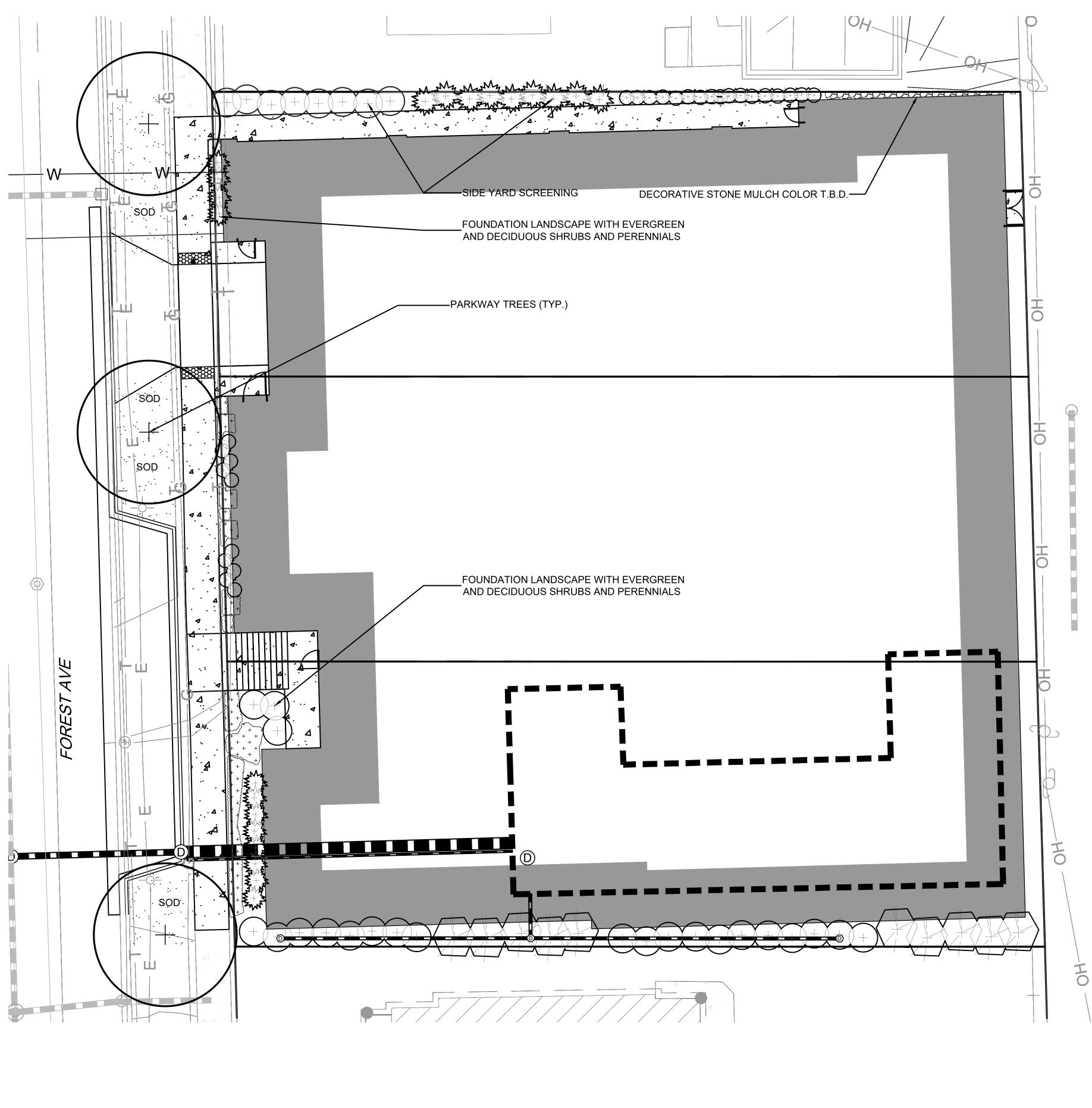
PLAN

PLANT LIST

SYM	SIZE	QTY	BOTANICAL NAME	COMMON NAME	COMMEN
	UOUS SH	ADE TE			_
CEO	2.5" cal.		Celtis occidentalis	Common Hackberry	B&B
GDE	2.5" cal.		Gymnocladus dioicus 'Espresso'	Espresso Kentucky Coffeetree	B&B
GTS	2.5" cal.		Gleditsia triacanthos 'Shademaster'	Shademaster Honeylocust	B&B
QUM	2.5" cal.		Quercus muehlenbergii	Chinkapin Oak	B&B
ULA	2.5" cal.		Ulmus davidiana var. japonica 'Morton'	Accolade Elm	B&B
DECID	UOUS SH	RUBS			
AAB	24" ht.		Aronia arbutifolia 'Brilliantissima'	Brilliant Red Chokeberry	B&B
AME	24" ht.		Aronia melanocarpa	Black Chokeberry	B&B
CEP	24" ht.		Cephalanthus occidentalis	Buttonbush	B&B
CSF	24" ht.		Cornus stolonifera 'Farrow'	Arctic Fire Redtwig Dogwood	B&B
CLA	36" ht.		Clethra alnifolia 'Ruby Spice'	Ruby Spice Clethra	B&B
HYA	24" ht.		Hydrangea arborescens 'Haas Halo'	Haas Halo Hydrangea	B&B
HYB	24" ht.		Hydrangea macrophylla 'PIIHM-II'	Bloomstruck Endless summer Hydrange	
HYP	24" ht.	†	Hydrangea paniculata 'Peegee Improved'		B&B
HYQ	24" ht.	1	Hydrangea quercifolia 'Alice'	Alice Oakleaf Hydrangea	B&B
SBT	18" w.		Spirea betulafolia 'tor'	Tor Birchleaf Spirea	B&B
STC	18" w.		Stephanandra incisa 'Crispa'	Cutleaf Stephanandra	B&B
SYM	24" ht.		Syringa patula 'Miss Kim'	Miss Kim Korean Lilac	B&B
VCC	24" ht.		Viburnum carlesii 'Cayuga'	Cayuga Koreanspice Viburnum	B&B
VBN	24" ht.	+	Viburnum nudum 'Bulk'	Brandywine Smooth Witherod	B&B
VDC	24" ht.	+	Viburnum dentatum 'Chicago Lustre'	Chicago Lustre Arrowwood Viburnum	B&B
WFR	24" ht.		Weigela florida 'Red Prince'	Red Prince Old Fashioned Weigela	B&B
	GREEN SH	HRUBS	Weigela horida Ned i filice	Tred I fillice Old I ashioned Weigela	Τυαυ
BUW	24" w	псово Т .	Buxus micro. x. B. sem. 'Wilson's Charm'	Wilson Northern Charm Boxwood	B&B
TMH	24" ht.		Taxus x media 'Hicksii'	Hick's Yew	B&B
TMT	24" w.		Taxus x media 'Taunton'	Taunton's Yew	B&B
ORNAI	MENTAL (SRASSI	<u>ES</u>		
CAK	#1 cont.				
PAV	#1 cont.		Panicum virgatum 'Shenandoah'	Shenandoah Red Switch Grass	
GROU	NDCOVE	R / PER	ENNIALS		
ALS	#1 cont.		Allium 'Summer Beauty'	Summer Beauty Onion	18" O.C
AMB	#1 cont.		Amsonia tabernaemontana 'Blue Ice'	Blue Ice Blue Star	18" O.C
AST	#1 cont.		Aster oblongifolius October Skies	October Skies Aromatic Aster	24" O.C
CVM	#1 cont.	1	Coreopsis verticillata 'Moonbeam'	Moonbeam Coreopsis	18" O.C
ECB	#1 cont.		Echinacea x. 'Balsomoblanc'	Sombrero Blanco Coneflower	18" O.C
GEM	#1 cont.		Geranium sanguineum 'Max frei'	Max Frei Bloody Cranesbill	18" O.C
HEC	#1 cont.	+	Hemerocallis x. 'Chicago Apache'	Chicago Apache Daylily	24" O.C
LAV	#1 cont.	†	Lavendula 'Munstead strain'	Munstead English Lavender	24" O.C
LEU	#1 cont.	+	Leucanthemum superbum 'becky'	Becky Shasta Daisy	24" O.C
NFW	#1 cont.	+	Nepeta fasseni 'Walker's low'	Walkers Low Catmint	24" O.C
	#1 cont.	+	Rudbeckia fulgida 'Little Goldstar'	Little Goldstar Black-Eyed Susan	18" O.C
RUD			TI TO A DO CONTA TATALINA LILITO UNICOLALI	LILLO ODIADIAI DIADIL EYDA DAJAH	1.5 0.0

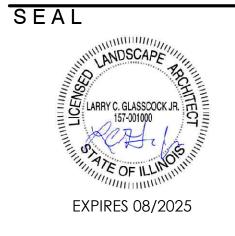
LANDSCAPE LEGEND





Landscape Architecture
Master Planning
Site Planning
Workshop,LLC

1955 N. Wilmot Ave Chicago, IL 60647 ph. 773.697.4388 www.LGWLA.com



PROJECT TEAM

CIVIL ENGINEER:



PROJECT NAME

4 CORNERS MULTI-FAMILY HIGH RISE

4919 FOREST AVE. DOWNERS GROVE, IL

DRAWING ISSUED

NO. TITLE DATE

1. Preliminary Review 08/30/24
2. Rev's per Comments 10/16/24
2. Rev's per Comments 10/30/24

SET TYPE LANDSCAPE PLANS

PROJECT NUMBER 2408048

DATE
08-28-2024
DRAWN BY: APPROVED BY:
LCG LCG
SHEET TITLE

LANDSCAPE PLAN

SHEET NUMBER

L.2

LANDSCAPE PLAN

I"=10'
0' 5' 10' 20' 30'

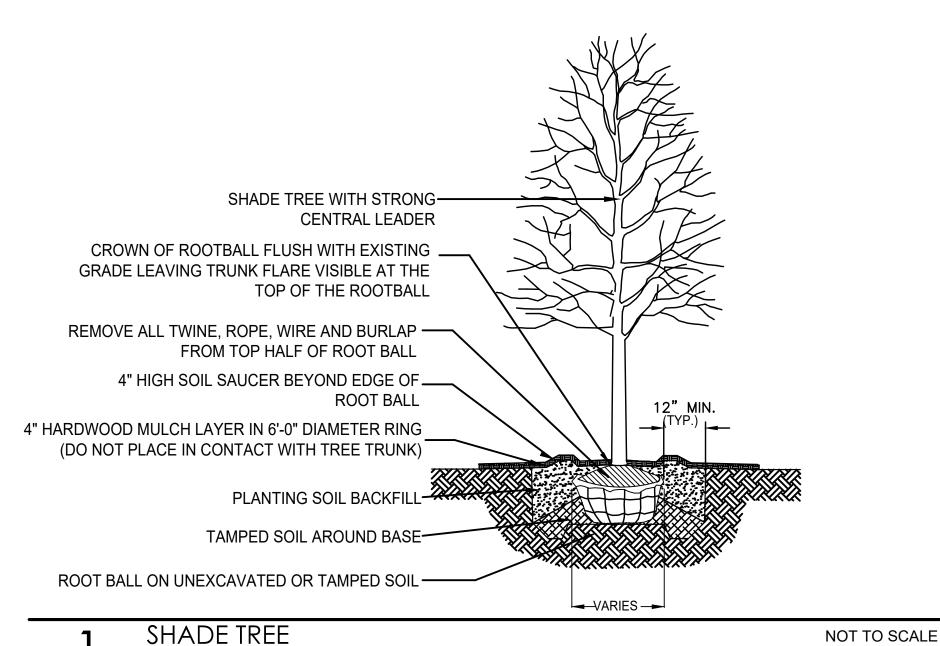
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LANDSCAPE NOTES

- CONTRACTOR SHALL OBTAIN ALL NECESSARY LOCAL PERMITS AND PERMISSIONS TO INSTALL THE PROPOSED IMPROVEMENTS
- 2. ALL LANDSCAPE MATERIALS SHALL BE INSTALLED IN ACCORDANCE WITH THE VILLAGE OF DOWNERS GROVE LANDSCAPING CODES AND ZONING ORDINANCES.
- 3. PRIOR TO COMMENCING ANY WORK, CONTRACTOR SHALL HAVE DIGGERS HOTLINE LOCATE AND MARK ALL UNDERGROUND UTILITY FACILITIES AND LINES.
- 4. ALL PLANT MATERIALS (EXCEPT FOR GROUNDCOVER, ANNUALS, AND PERENNIALS) SHALL BE BALLED AND BURLAPPED STOCK AND MEET CURRENT STANDARDS OF THE AMERICAN ASSOCIATION OF NURSERYMEN'S STANDARD FOR NURSERY STOCK (ANSI 260.1-1986) OR EQUAL. PLANT MATERIALS MUST BE SUPPLIED WITHIN A 150 MILE RADIUS OF PROJECT SITE. CONTRACTOR MAY SUBSTITUTE CONTAINER STOCK FOR SHRUBS IF SIZES ARE EQUAL TO SPECIFIED B&B STOCK, WITH THE APPROVAL OF THE LANDSCAPE ARCHITECT.
- 5. IF SPECIFIED PLANTS ARE NOT AVAILABLE AT THE TIME OF ORDERING, PLANTS WITH SIMILAR WHOLESALE VALUE AND LANDSCAPE CHARACTERISTICS MAY BE SUBSTITUTED UPON THE APPROVAL OF THE LANDSCAPE ARCHITECT AND VILLAGE STAFF.
- 6. SOIL IN GROUNDCOVER BEDS SHALL BE AMENDED USING 2 INCHES OF MUSHROOM COMPOST INCORPORATED INTO THE TOP 4 INCHES OF SOIL.
- DISTURBED AREAS TO RECEIVE SOD SHALL BE TILLED TO 6" DEPTH AND FINE GRADED TO PROVIDE SMOOTH BASE SURFACE. IF EXISTING SOIL IS A MAJORITY OF CLAY OR UNSUITABLE, 2" OF FINE GRADED TOPSOIL SHALL BE ADDED PRIOR TO TILLING. EXISTING SOD AREAS SHALL HAVE TURF REMOVED WITH AUTOMATED SODCUTTER OR HAND SPACE TO REMOVE ALL BLADES AND ROOTS. 1" OF FIND GRADED TOPSOIL SHALL BE TILLED AND GRADED.
- 8. TREE AND SHRUB BACKFILL MIXTURE SHALL BE 2 PARTS EXIST. NATIVE TOPSOIL AND 1 PART SPHAGNUM PEAT MOSS W/ DECOMPOSED MANURE.
- 9. ALL SHRUB BEDS AND INDIVIDUAL TREE PLANTINGS, UNLESS OTHERWISE NOTED, SHALL RECEIVE A 4 INCH LAYER OF SHREDDED HARDWOOD MULCH. ALL GROUNDCOVER, ANNUAL AND PERENNIAL BEDS SHALL RECEIVE A 2 INCH LAYER OF THE SAME MULCH MATERIAL. COSTS FOR MULCH SHALL BE CONSIDERED INCIDENTAL AND SHALL BE INCLUDED IN THE COST OF PLANTINGS.
- 10. NURSERY TAGS (SPECIES, SIZE) FOR ALL SHADE TREES SHALL REMAIN ATTACHED TO TREES UNTIL FINAL APPROVAL FROM MUNICIPALITY.
- 11. THE LANDSCAPE CONTRACTOR SHALL PROVIDE THE OWNER A BONDED WRITTEN ONE-YEAR WARRANTY AGREEMENT (BEGINNING ON THE OWNER'S POSSESSION DATE). THIS AGREEMENT SHALL COVER MAINTENANCE, REPAIR, AND REPLACEMENT OF ALL DEAD OR DAMAGED LANDSCAPING TO PRESERVE THE SAME QUANTITY AND QUALITY AS INITIALLY APPROVED.
- 12. CONTRACTOR SHALL PROVIDE A SEPARATE ESTIMATE FOR AN AUTOMATIC UNDERGROUND IRRIGATION SYSTEM FOR COMPLETE EFFECTIVE COVERAGE OF ALL LAWN AREAS AND SHRUB BEDS. CONTRACTOR SHALL SUBMIT SHOP DRAWINGS FOR APPROVAL AND APPLY FOR ALL NECESSARY PERMITS PRIOR TO COMMENCING WORK. IRRIGATION PLANS SHALL INCLUDE HUNTER PRO-C CONTROLLER W/WIRELESS SOLAR SYNC STATION AND HUNTER SPRAYHEADS AND NOZZLES. IRRIGATION WORK SHALL BE WARRANTY ALL LABOR AND MATERIALS FOR 1 FULL YEAR AFTER INSTALLATION AND TESTING.
- 13. SEEDED LAWN AREAS SHALL BE BID WITH A BID ALTERNATE FOR HYDROSEEDED LAWN. PRIOR TO SEEDING, 2" OF FINE TOPSOIL SHALL BE TILLED INTO EXIST SOIL MIXTURE. A MIX CONSISTING OF ROUGHLY 30 % BLUEGRASS / 30% FINE FESCUES / 40% RYE GRASSES (AND TACKIFIER FOR HYDROSEEDING) SHALL BE APPLIED AT MANUFACTURERS SPECIFIED RATES FOR NEW LAWNS BETWEEN 5 AND 10 LBS PER 1,0000 SF.
- 14. TREES AND SHRUBS SHALL NOT BE LOCATED CLOSER THAN TEN (10) FEET TO FIRE HYDRANTS, TRANSFORMERS OR OTHER ABOVE GROUND UTILITIES. ANY DISCREPANCY ON THE PLAN RELATED TO THESE PROXIMATE UTILITIES SHALL BE BROUGHT TO THE ATTENTION OF THE LANDSCAPE ARCHITECT FOR RESOLUTION.

VILLAGE LANDSCAPE REQUIREMENT CALCULATIONS

DESCRIPTION	REQUIREMENT	AREA / LENGTH	DETAILS
TOTAL SITE		21,217 SQ. FT.	
TOTAL BUILDING		18,997 SQ. FT.	
TOTAL PARKING		0 SQ. FT. AT GRADE.	
SEC 28.8.020 PARKING LOT PERIMETER LANDSCAPING		N/A	
SEC 28.8.030 PARKING LOT INTERIOR LANDSCAPING		N/A	
TREE REMOVALS		35 CAL. INCHES REMOVED FROM PARKWAY	CASH IN LIEU AS THERE IS NO OPEN SPACE FOR TREES ON-SITE.



NOTES: **NEVER CUT LEADERS TREE** SHALL BEAR SAME RELATION TO FINISHED GRADE AS IT BORE TO PREVIOUS GRADE. REMOVE BURLAP FROM TOP 1/3 OF BALL 4" HARDWOOD MULCH UNDISTURBED SUBGRADE **PLANTING SOIL** ROOT BALL ON UNEXCAVATED OR TAMPED SOIL

ORNAMENTAL TREE PLANTING DETAIL

EXISTING SUBGRADE

PLANTING DETAIL

NOT TO SCALE

CROWN OF ROOTBALL FLUSH ___ WITH EXISTING GRADE REMOVE BURLAP FROM TOP 1/2 OF BALL OR ENTIRE CONTAINER 4" HARDWOOD MULCH — PLANTING SOIL BACKFILL -

SHRUB PLANTING DETAIL

SET TYPE LANDSCAPE PLANS

PROJECT NUMBER 2408048

DATE 08-28-2024 DRAWN BY: APPROVED BY: SHEET TITLE

LANDSCAPE DETAILS & NOTES SHEET NUMBER

VARIES 2" HARDWOOD MULCH FINISHED GRADE -PLANTING BED RAISED FOR '-ADEQUATE DRAINAGE PLAN EXISTING SUBGRADE — * DIMENSION OF ON-CENTER PLANT SPACING IS INDICATED ON SECTION MASTER PLANT LIST

PERENNIAL / ANNUAL PLANTING DETAIL

NOT TO SCALE

NOT TO SCALE

Landscape Architecture

1955 N. Wilmot Ave Chicago, IL 60647 ph. 773.697.4388 www.LGWLA.com

LARRY C. GLASSCOCK JR. 157-001000

EXPIRES 08/2025

PROJECT TEAM

CIVIL ENGINEER:

SEAL

Master Planning Site Planning

PROJECT NAME

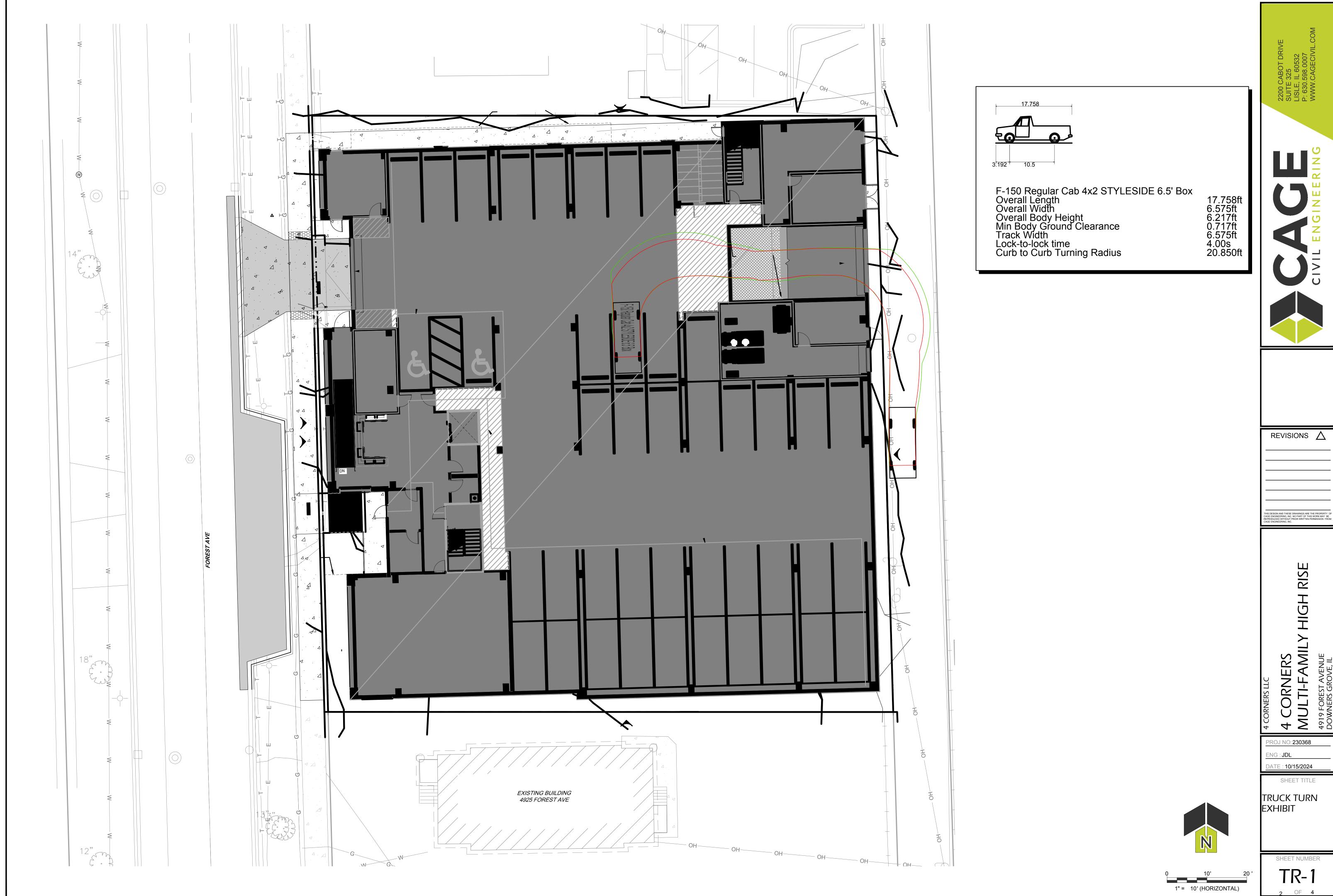
4 CORNERS MULTI-FAMILY HIGH RISE

4919 FOREST AVE. DOWNERS GROVE, IL

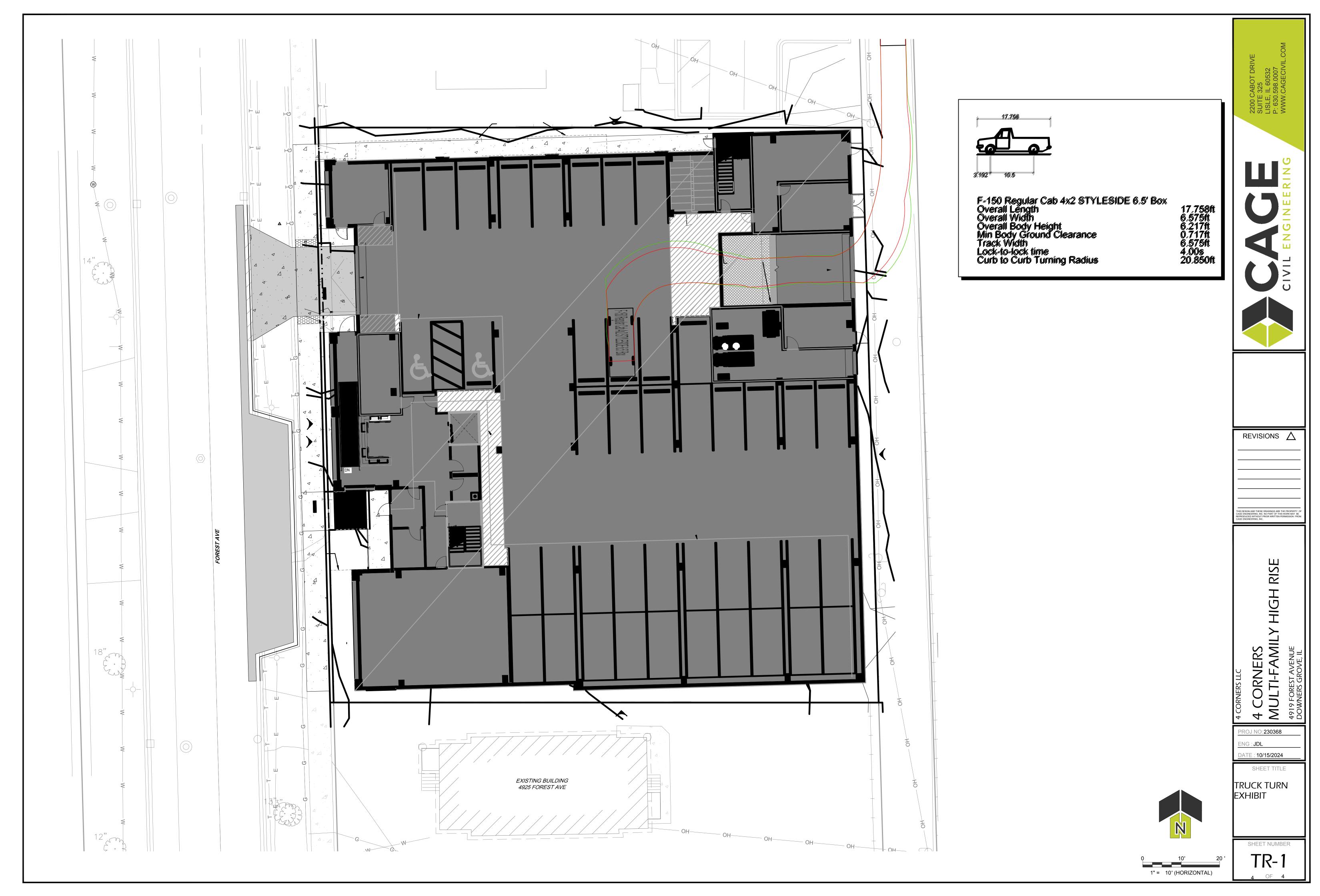
DRAWING ISSUED NO. TITLE DATE

Preliminary Review 08/30/24 2. Rev's per Comments 10/16/24 2. Rev's per Comments 10/30/24





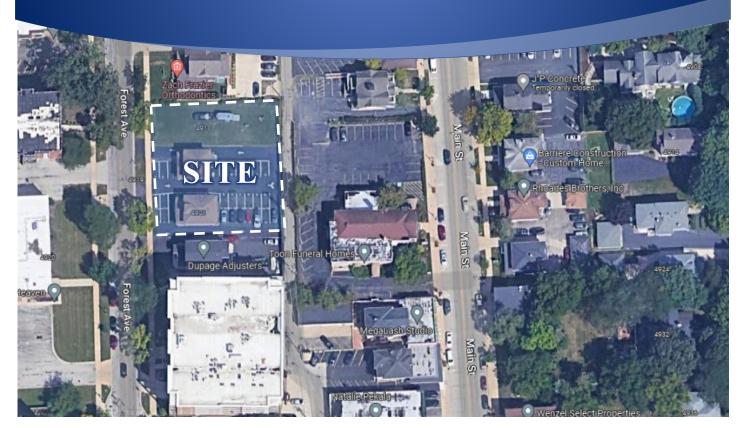




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Traffic Impact Study Multi-Family Residential Development

Downers Grove, Illinois



Prepared For:





October 17, 2024

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1. Introduction

This report summarizes the methodologies, results, and findings of a traffic impact study conducted by Kenig, Lindgren, O'Hara, Aboona, Inc. (KLOA, Inc.) for the proposed multi-family residential development to be located on the east side of Forest Avenue between Warren Avenue and Franklin Street in downtown Downers Grove, Illinois. As proposed, the site, which is currently occupied by two buildings utilized as office space and a vacant lot, will be redeveloped to provide a seven-story building with the upper floors containing residential units and the ground and second floors to be occupied by a parking garage. The plans call for 62 residential units and 89 parking spaces. Access to the garage will be provided off Forest Avenue and the alley.

The purpose of this study was to examine background traffic conditions, assess the impact that the proposed development will have on traffic conditions in the area, determine if any roadway or access improvements are necessary to accommodate traffic generated by the proposed development and to provide an assessment of the alley as a feasible entry/exit for the development.

Figure 1 shows the location of the site in relation to the area roadway system. **Figure 2** shows an aerial view of the site with other nearby area developments shown. The sections of this report present the following:

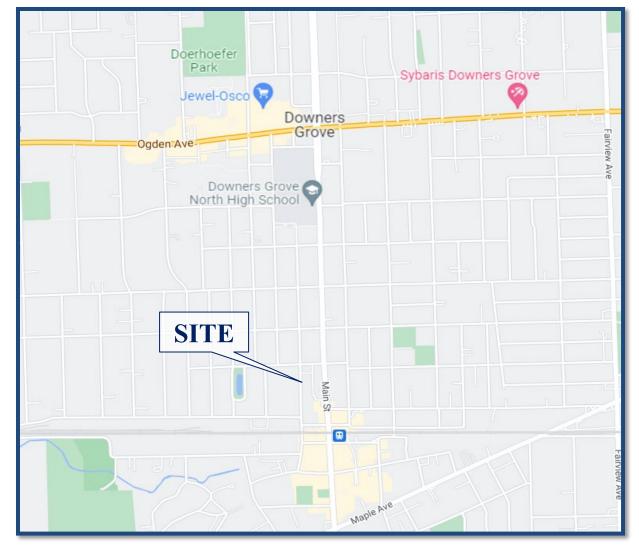
- Existing roadway conditions
- A description of the proposed development
- Directional distribution of the development traffic
- Vehicle trip generation for the development
- Future traffic conditions including access to the development
- Traffic analyses for the weekday morning and weekday evening peak hours
- Recommendations with respect to adequacy of the site access and adjacent roadway system
- Evaluation of the adequacy of the parking supply

Traffic capacity analyses were conducted for the weekday morning and weekday evening peak hours for the following conditions:

- 1. Existing Conditions Analyzes the capacity of the existing roadway system using existing peak hour traffic volumes.
- 2. Year 2030 No-Build Conditions Analyzes the capacity of the existing roadway system using the existing traffic volumes increased by an ambient growth factor (growth not attributable to any particular development) as well as any area developments.
- 3. Year 2030 Projected Conditions Analyzes the projected traffic volumes which includes the existing traffic volumes increased by an ambient area growth factor (growth not attributable to any particular development) as well as any area developments and the traffic estimated to be generated by the proposed subject development.

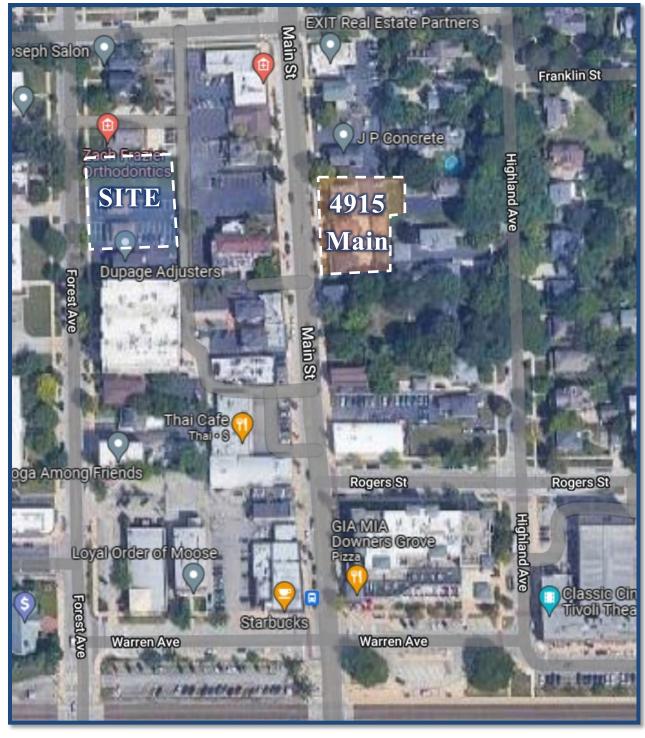


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Site Location Figure 1

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Aerial View of Site Figure 2



2. Existing Conditions

Existing transportation conditions in the vicinity of the site were documented based on field visits conducted by KLOA, Inc. in order to obtain a database for projecting future conditions. The following provides a description of the geographical location of the site, physical characteristics of the area roadway system including lane usage and traffic control devices and existing peak hour traffic volumes.

Site Location

The site is located in downtown Downers Grove on the east side of Forest Avenue between Warren Avenue and Franklin Street. The site is currently occupied by two houses utilized as office space and a vacant lot. The east side of Forest Avenue is a mix of office and residential uses. The west side of Forest Avenue is primarily residential uses. A north-south public alley borders the site to the east, providing access to businesses on Forest Avenue and Main Street. The Downers Grove Metra station is located approximately 1,100 feet southeast of the site.

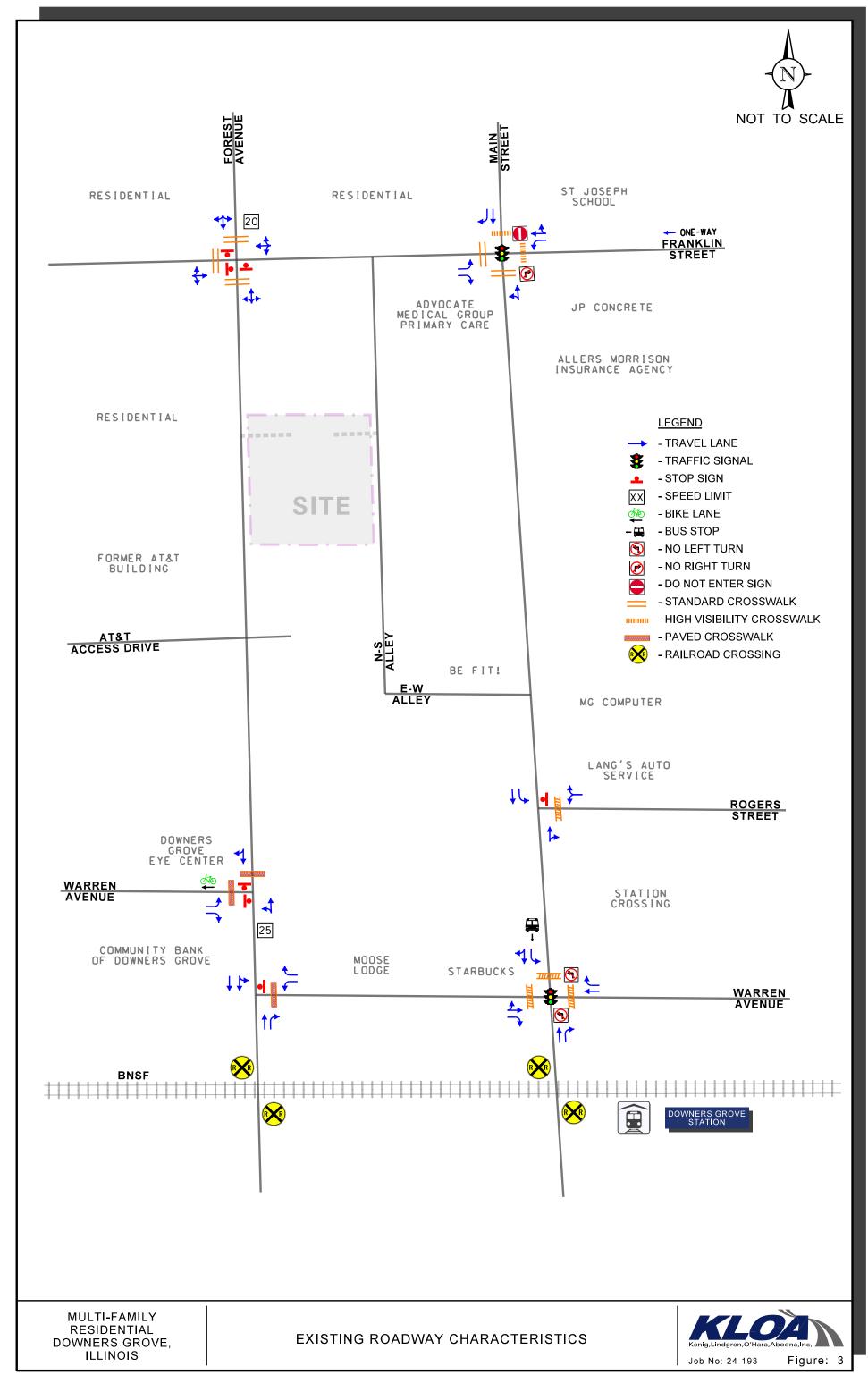
Existing Roadway System Characteristics

The characteristics of the existing roadways near the development are described below. **Figure 3** illustrates the existing roadway characteristics.

Forest Avenue is a north-south roadway that generally provides one lane in each direction. North of the west leg of Warren Avenue, Forest Avenue is classified as a local roadway, south of the west leg of Warren Avenue, it is classified as a collector roadway. At its unsignalized intersection with Franklin Street, Forest Avenue provides a combined left-turn/through/right-turn lane on the northbound and southbound approaches which are under stop sign control. Standard-style crosswalks are provided on the north and south legs of the intersection. At its unsignalized intersection with the AT&T access drive, Forest Avenue provides a combined left-turn through lane on the northbound approach and a combined through/right-turn lane on the southbound approach. At its unsignalized intersection with the west leg of Warren Avenue, Forest Avenue provides a combined left-turn/through lane on the northbound approach and a combined through/right-turn lane on the southbound approach, which is under stop sign control. A paved crosswalk is provided on the north leg of the intersection. At its unsignalized intersection with the east leg of Warren Avenue, Forest Avenue provides a combined left-turn/through lane and a through lane on the southbound approach. On the northbound approach, Forest Avenue provides a through lane and a right-turn lane. At the at-grade railroad crossing of the BNSF Railway rightof-way, Forest Avenue provides two lanes in each direction. Forest Avenue carries an annual average daily traffic (AADT) volume of 400 vehicles north of the west leg of Warren Avenue, 3,400 vehicles between the west and east legs of Warren Avenue, and 4,200 vehicles south of the east leg of Warren Avenue. Forest Avenue has a posted speed limit of 20 miles per hour.



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Main Street is a north-south minor arterial roadway that generally provides one lane in each direction. At its signalized intersection with Franklin Street, Main Street provides a combined left-turn/through lane on the northbound approach and a through lane and a right-turn lane on the southbound approach. A high-visibility crosswalk is provided on the north leg and a paved crosswalk is provided on the south leg. At its signalized intersection with Warren Avenue, Main Street provides a through lane and a right-turn lane on the northbound approach. On the southbound approach, Main Street provides a left-turn lane and a combined through/right-turn lane. A high-visibility crosswalk is provided on the north leg of the intersection. At its unsignalized intersection with Rogers Street, Main Street provides a combined through/right-turn lane on the northbound approach and a left-turn lane and a through lane on the southbound approach. At its unsignalized intersection with the public alley, Main Street provides a combined left-turn/through lane on the northbound approach and a combined through/right-turn lane on the southbound approach. Main Street carries an average annual daily traffic (AADT) volume of 6,800 vehicles (IDOT 2020). Main Street has a posted speed limit of 25 miles per hour.

Franklin Street is an east-west local roadway. East of Main Street, Franklin Street is a westbound one-way street providing two travel lanes. West of Main Street, Franklin Street provides one lane in each direction. At its signalized intersection with Main Street, Franklin Street provides a left-turn lane and a right-turn lane on the eastbound approach. On the westbound approach, Franklin Street provides a left-turn lane and a combined through/right-turn lane. A standard-style crosswalk is provided on the west leg of the intersection and a high-visibility crosswalk is provided on the east leg of the intersection. At its unsignalized intersection with Forest Avenue, Franklin Street provides combined left-turn/through/right-turn lanes on the eastbound and westbound approaches. The eastbound approach is under stop sign control. A standard-style crosswalk is provided on the west leg.

Warren Avenue is an east-west local roadway that provides one lane in each direction. At its signalized intersection with Main Street, Warren Avenue provides a combined left-turn/through lane and a right-turn lane on the eastbound approach. On the westbound approach, Warren Avenue provides a through lane and a right-turn lane. High-visibility crosswalks are provided on the east and west legs of the intersection. At its unsignalized, offset intersection with Forest Avenue, Warren Avenue provides a left-turn lane and a right-turn lane on the eastbound and westbound approaches that are under stop sign control. West of Forest Avenue, Warren Avenue provides an exclusive bike lane in each direction with both bike lanes terminating at Saratoga Avenue and the eastbound bike lane terminating at the eastbound Warren Avenue turn lanes at Forest Avenue. Paved crosswalks are provided on the east and west legs of the intersection.

North-South public alley is a north-south public alley that is approximately 14-feet-wide and extends from Franklin Street to the east-west public alley approximately 590 feet south of Frankling Street. This alley provides access to properties fronting Main Street and Forest Avenue. Two-way traffic is allowed on the alley and there is a posted speed limit of 10 miles per hour.



East-West public alley is an east-west public alley that is approximately 10 feet wide and extends from the north-south alley east to its intersection with Main Street between 4934 Main Street and 4946 Main Street. While angled parking spaces are provided on the north side of the alley which are oriented to be accessed by westbound traffic, there are no posted restrictions for one-way traffic.

Rogers Street is an east-west local roadway that provides one lane in each direction. At its unsignalized intersection with Main Street, Rogers Street provides a combined left-turn/right-turn lane that is under stop sign control.

Public Transportation

Metra commuter rail and Pace suburban bus provide public transportation options within downtown Downers Grove:

- The Metra BNSF line provides service between Union Station in downtown Chicago and downtown Aurora. Service is provided seven days a week, including holidays. Additional service may be provided for heavily attended events in Chicago. The Downers Grove station is located approximately 1,100 feet southeast of the site, in the northeast corner of Main Street with Burlington Avenue. The station provides an indoor waiting area as well as benches and covered outdoor waiting areas. A drop-off area for passenger vehicles is located in the southeast corner of the intersection of Main Street with Warren Avenue.
- Pace Bus Route 834 Joliet-Downers Grove provides weekday and Saturday service between downtown Joliet and Yorktown Center in Lombard, IL. Weekday service is provided from early morning to mid-evening. Saturday service is provided from midmorning to early evening. The nearest southbound stop to the site is located in the northwest corner of Main Street with Warren Avenue, approximately 920 feet to the southeast. This is a curbside stop that only provides a route sign. The nearest northbound stop to the site is located on the south side of the Metra station along Main Street. A bench and covered outdoor waiting area are provided nearby as part of the Metra station.

Pedestrian and Bicycle Facilities

Sidewalks are generally provided on both sides of roadways within downtown Downers Grove. The area provides marked crosswalks at most intersections within the downtown area. Pedestrian push buttons and countdown signals are provided at the signalized intersections included in the study area.

Per the Downers Grove Village Bikeway Plan, Main Street is designated as a bike route. Warren Avenue provides a bike lane in the eastbound and westbound direction west of Forest Avenue. Bike racks are provided throughout the downtown area.



Existing Traffic Volumes

In order to determine current traffic conditions in the vicinity of the site, KLOA, Inc. conducted peak period traffic counts using Miovision Scout Video Collection Units on Tuesday, August 20, 2024, during the weekday morning (7:00 A.M. to 9:00 A.M.) and weekday afternoon (2:00 P.M. to 6:00 P.M.) peak periods at the following intersections:

- Main Street with Franklin Street
- Main Street with Warren Avenue
- Forest Avenue with Franklin Street
- Forest Avenue with AT&T Access Drive/Apartment Access Drive
- Forest Avenue with Warren Avenue (West Leg)
- Forest Avenue with Warren Avenue (East Leg)
- Franklin Street with North-South Alley
- Main Street with Rogers Street

The results of the traffic counts showed that the weekday morning peak hour of traffic occurs from 7:30 A.M. to 8:30 A.M. and the weekday evening peak hour of traffic occurs from 5:00 P.M. to 6:00 P.M. It should be noted that during the early weekday afternoon peak hour that occurs from 3:00 P.M. to 4:00 P.M., which coincides with area school dismissal times, the overall traffic volumes are approximately 10 percent less than during the weekday evening peak hour. It should be noted that area schools were in session when the counts were conducted.

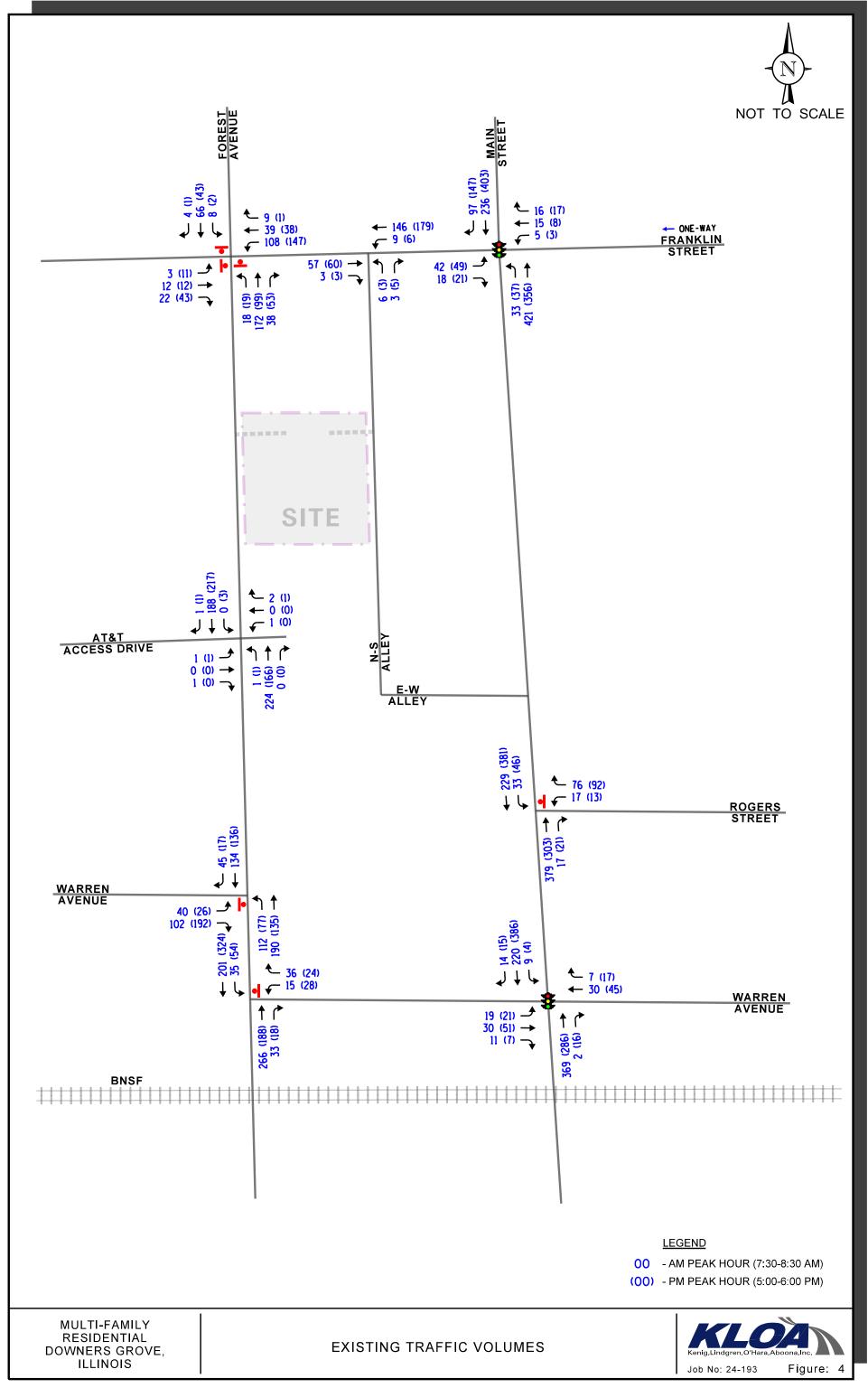
Furthermore, 24-hour two-way traffic counts were conducted for the public alley south of its intersection with Franklin Street to determine the existing daily utilization of the public alley. These counts were also conducted on Tuesday, August 20, 2024. The results of the 24-hour counts indicated that the public alley carried a two-way traffic volume of 281 vehicles of which 185 vehicles traveled northbound and 96 vehicles traveled southbound.

Copies of the traffic count summary sheets are included in the Appendix. **Figure 4** illustrates the existing traffic volumes. **Figure 5** illustrates the existing bicycle and pedestrian volumes.

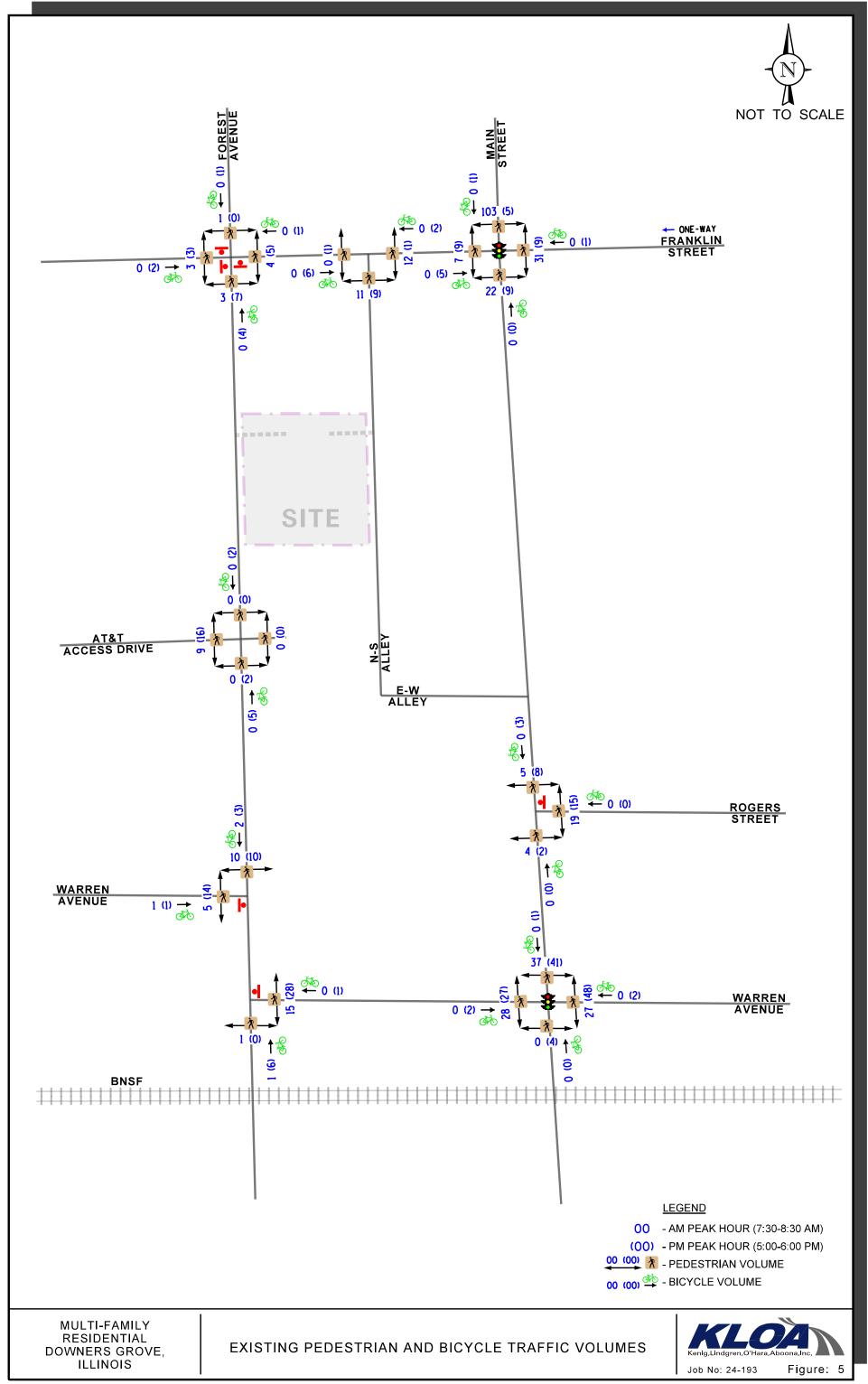
As discussed later in the report, due to the configuration of the north-south alley and east-west alleys south of the proposed site access drive, all traffic accessing the site will be directed to travel to/from the north only and therefore, traffic counts were not conducted at the intersection of Main Street with the east-west public alley. However, count sheets for counts previously conducted at Main Street with the east-west public alley are included in the Appendix for reference.



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BNSF Right-of-Way and At-Grade Crossings

The BNSF Railway has a three-track right-of-way that runs east-west through downtown Downers Grove. At-grade crossings are provided at Forest Avenue and Main Street approximately 100 feet south of Warren Avenue. The stop bars for the approaches at these intersections are approximately 18 feet from the edge of the railroad track. Based on the Illinois Commerce Commission's (ICC) inventory, an average of 132 trains traverse these crossings on a daily basis. Of these, approximately 33 trains are inbound Metra trains and approximately 37 are outbound Metra trains. There are an additional eight Amtrak trains that traverse these crossings daily. Every at-grade crossing provides signage, lights, gates, and signals.

The traffic signal at Main Street with Warren Avenue is interconnected with the railroad crossing signal. This results in longer green times for the northbound approach so that traffic clears the railroad crossing. It was observed that when trains stop, the gates are down for approximately two minutes. During this time, southbound queues are noted to extend to the intersection of Main Street with Rogers Street, and at times past the intersection. Once the train has passed and the crossing gates are up, traffic clears the crossing and the intersection of Main Street with Warren Avenue within approximately two cycle lengths.

At the intersection of Forest Avenue with Warren Avenue (both legs), the eastbound Warren Avenue, westbound Warren Avenue, and southbound Forest Avenue approach at its intersection with the west leg of Warren Avenue are under stop-sign control. This configuration allows for northbound traffic to clear the train tracks without stopping.



3. Traffic Characteristics of the Proposed Development

In order to properly evaluate future traffic conditions in the surrounding area, it was necessary to determine the traffic characteristics of the proposed development, including the directional distribution and volumes of traffic that it will generate.

Proposed Site and Development Plan

As proposed, the site will be developed with a seven-story multi-family residential building providing 62 units. Parking will be accommodated in the building in an indoor garage providing 89 spaces. Access to the ground floor of the garage with 42 parking spaces (18 standard, 2 ADA, and 11 tandem (22 spaces)) will be provided off Forest Avenue, while access to the second floor or the garage with 47 parking spaces (17 standard, 2 ADA, and 14 tandem (28 spaces)) will be provided off the north-south public alley.

Each access drive will provide one inbound lane and one outbound lane and outbound movements will be under stop sign control. The ground floor access drive will be located approximately 240 feet south of Franklin Street. To maintain safety for pedestrians on Forest Avenue, a visual warning device should be posted at the garage access. The second-floor access drive will be located approximately 240 feet south of Franklin Street. Due to the configuration of the alley system south of the proposed access drive serving the second floor, all traffic utilizing this access drive will be directed to travel to/from the north via signage.

It is important to note that the site currently has multiple curb cuts on Forest Avenue which will be consolidated into one access drive. This will reduce the potential for vehicular/pedestrian conflicts and improve traffic flow on Forest Avenue. A lay-by lane which will be designated as a loading zone will also be provided along the site frontage on Forest Avenue.

Furthermore, as part of the proposed development, the building will be offset three (3) feet from the property line to increase the effective width of the alley along the site frontage to 17 feet.

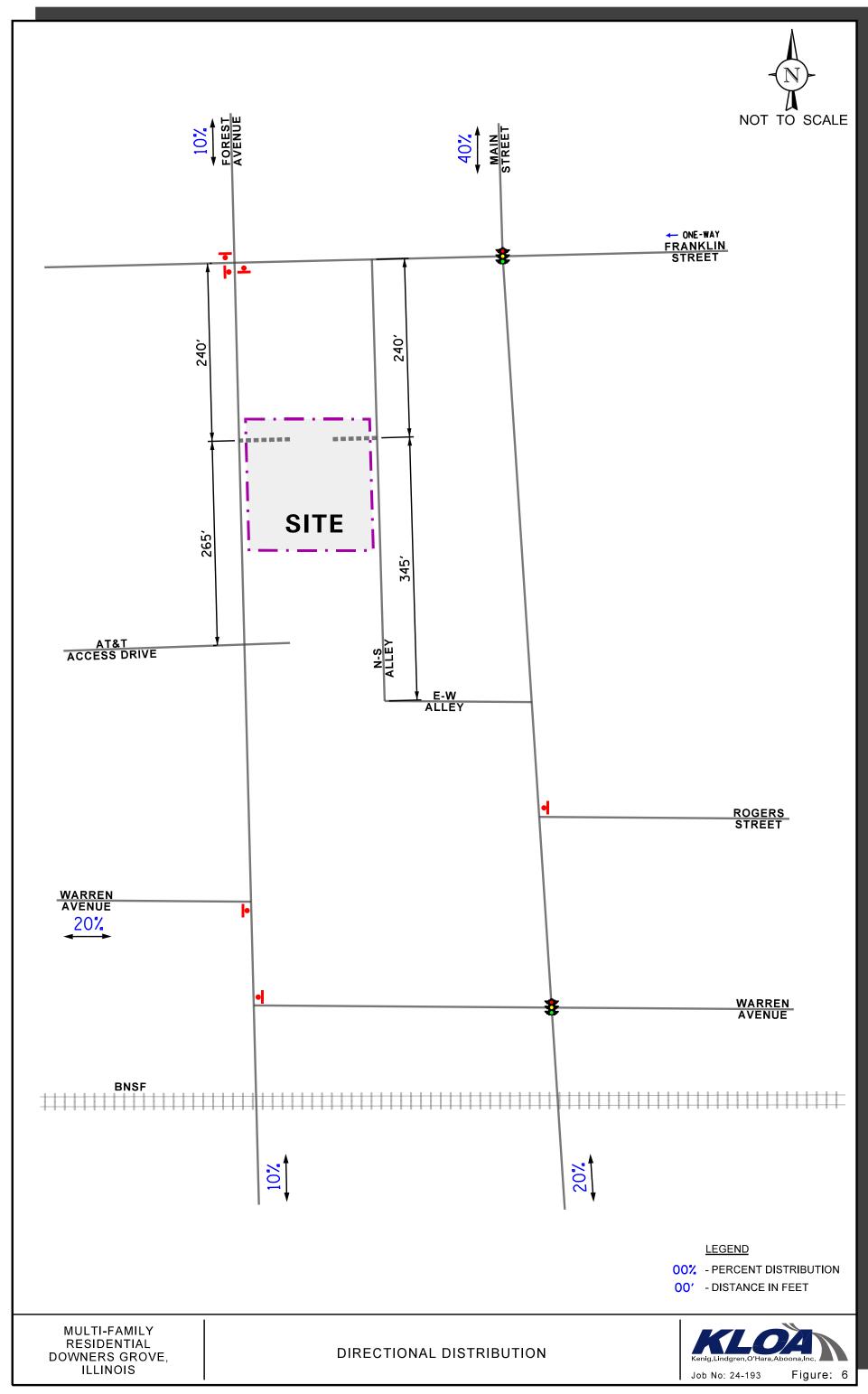
A copy of the proposed site plan is included in the Appendix.

Directional Distribution

The directions from which residents of the proposed development will approach and depart the site were estimated based on existing travel patterns, as determined from the traffic counts. **Figure** 6 illustrates the directional distribution of the development-generated traffic.



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Estimated Site Traffic Generation

The number of peak hour and daily trips estimated to be generated by the proposed multi-family residential development was based on vehicle trip generation rates contained in *Trip Generation Manual*, 11th Edition, published by the Institute of Transportation Engineers (ITE). The "Multifamily Housing (Mid-Rise)" (ITE Land-Use Code 221) rate was utilized.

While the Chicago Metropolitan Agency for Planning (CMAP) July 2024 Community Data Snapshot shows that approximately 9.0 percent of people in the village of Downers Grove commute to work via public transportation, 2.2 percent of people walk or bike to work, and approximately 20.9 percent of people work at home. To provide a conservative analysis, no reduction was taken in the number of trips estimated to be generated by the proposed multifamily residential development.

Table 1 summarizes the trips projected to be generated by the proposed development during the peak hours. A copy of the ITE trip generation sheets is included in the Appendix.

Table 1 ESTIMATED SITE-GENERATED TRAFFIC VOLUMES

ITE Land-	Type/Size	Weekday Morning Peak Hour		Weekday Evening Peak Hour			Daily Two-Way Trips			
Use Code		In	Out	Total	In	Out	Total	In	Out	Total
221	Multifamily Housing (Mid-Rise) – 62 Units	5	18	23	15	10	25	141	141	282



4. Projected Traffic Conditions

The total projected traffic volumes include the existing traffic volumes, increase in background traffic due to growth, and the traffic estimated to be generated by the proposed subject development.

Development Traffic Assignment

The estimated weekday morning and evening peak hour traffic volumes that will be generated by the proposed development were assigned to the roadway system in accordance with the previously described directional distribution (Figure 6). The total new traffic assignment for the residential development is illustrated in **Figure 7**. As previously indicated, all site generated traffic utilizing the second-floor parking garage will be directed to travel to/from the north on the public alley via signage.

Background (No-Build) Traffic Conditions

The existing traffic volumes (Figure 4) were increased by a regional growth factor to account for the increase in existing traffic related to regional growth in the area (i.e., not attributable to any particular planned development). Based on 2050 Annual Average Daily Traffic (AADT) projections provided by the Chicago Metropolitan Agency for Planning (CMAP) in a letter dated August 7, 2024, the existing traffic volume were increased by an annually compounded growth rate of 0.7 percent for six years (one-year buildout plus five years) totaling approximately five percent to represent Year 2030 no-build conditions.

Also included in the no-build traffic volumes is the traffic that will be generated by the 4915 Main Street Apartments.

A copy of the CMAP 2050 projections letter is included in the Appendix. **Figure 8** illustrates the Year 2030 no-build traffic volumes.

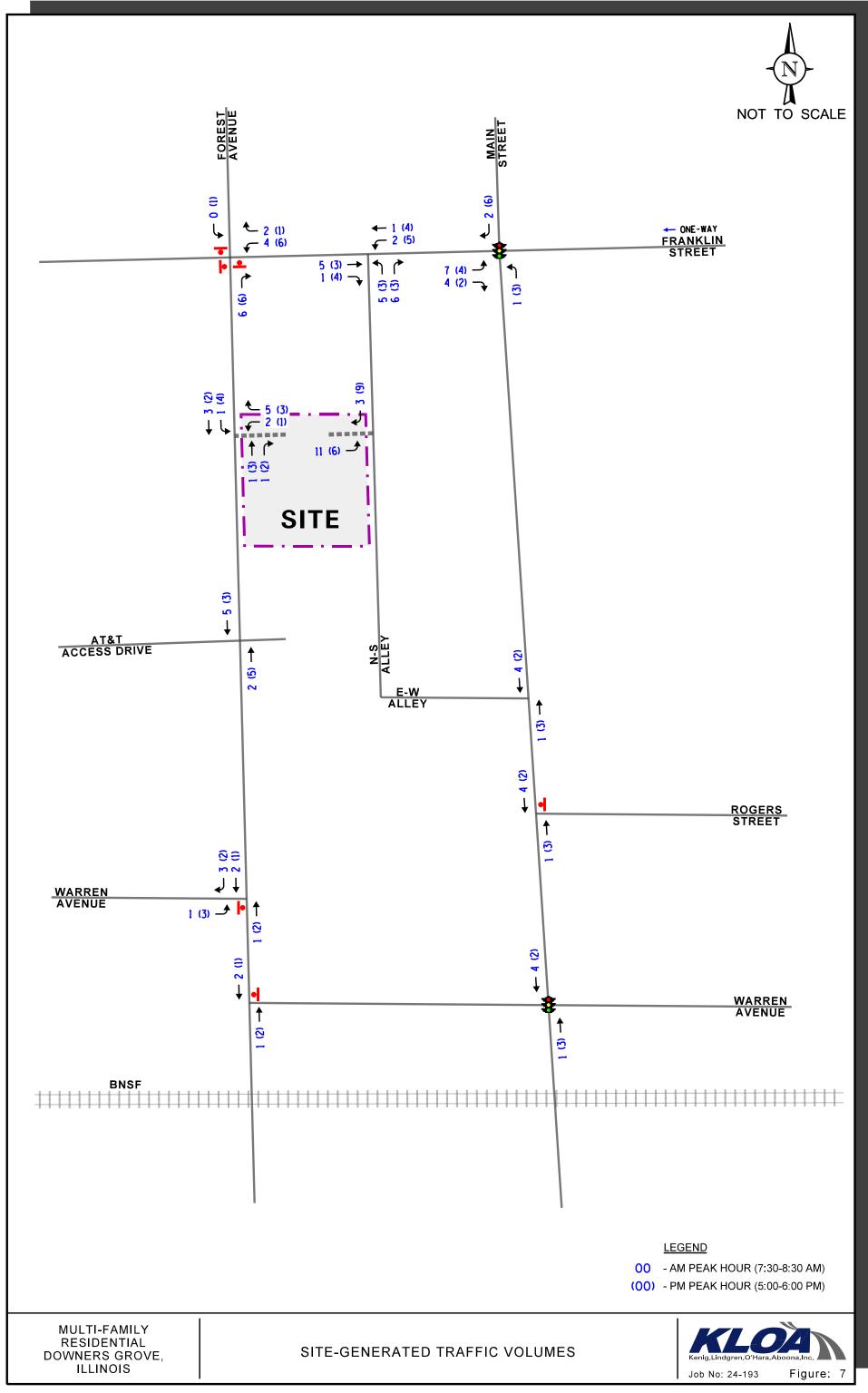
Total Projected Traffic Volumes

The development-generated traffic (Figure 7) was added to the existing traffic volumes increased by a regional growth factor (Figure 8) to determine the Year 2030 total projected traffic volumes, as illustrated in **Figure 9**.

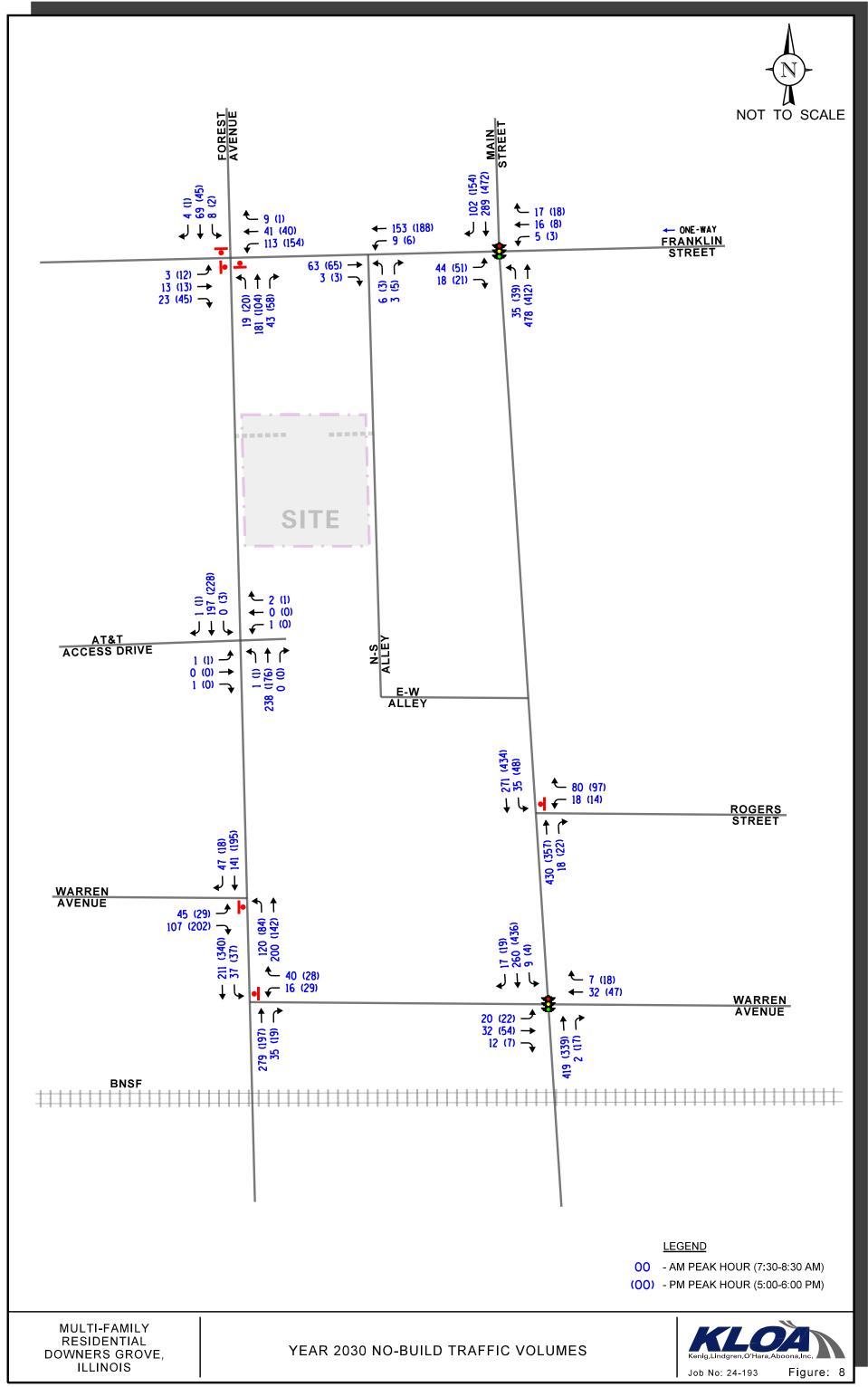


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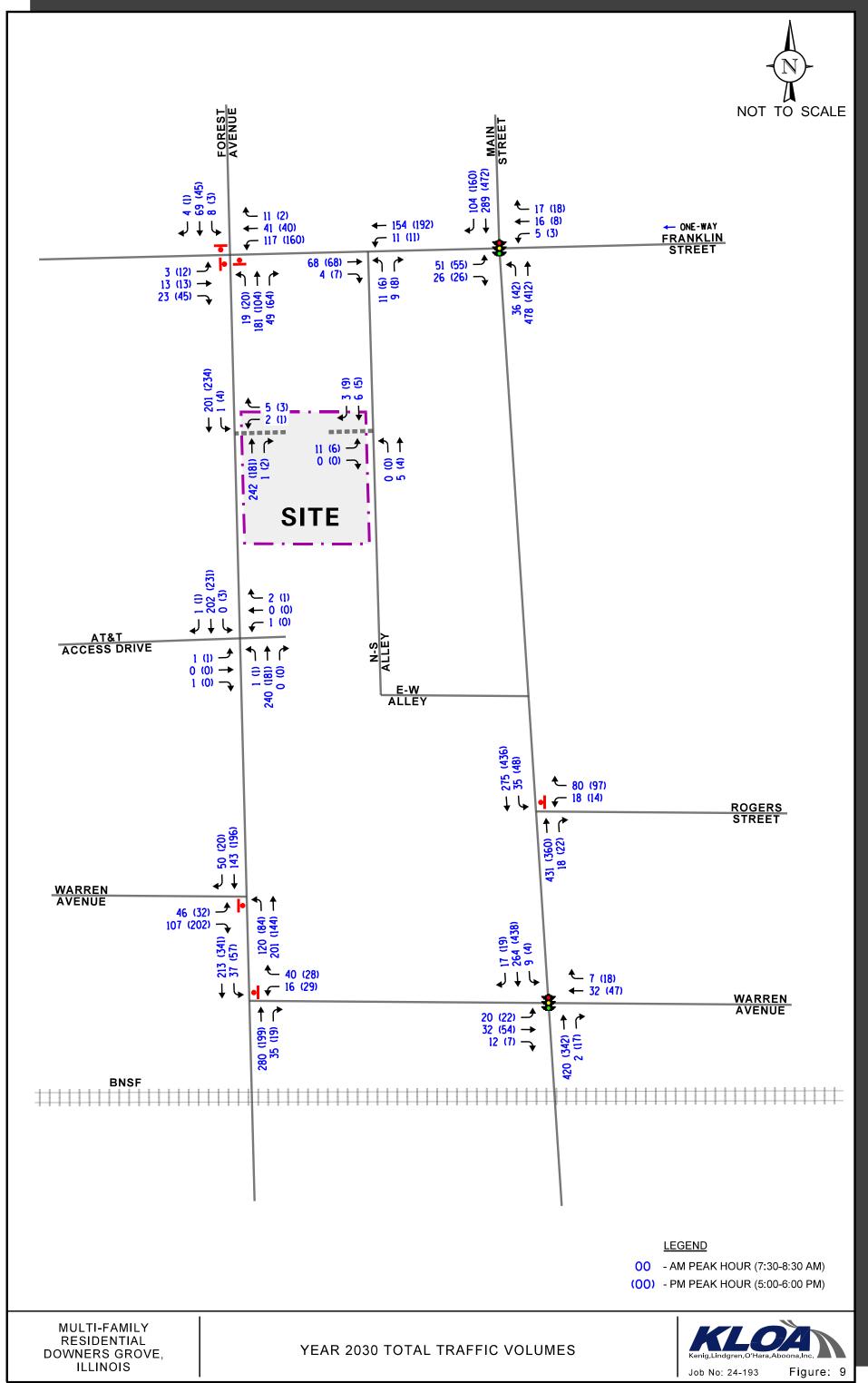
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5. Traffic Analysis and Recommendations

The following provides an evaluation conducted for the weekday morning and weekday evening peak hours. The analysis includes conducting capacity analyses to determine how well the roadway system and access drives are projected to operate and whether any roadway improvements or modification are required.

Traffic Analyses

Roadway and adjacent or nearby intersection analyses were performed for the weekday morning and weekday evening peak hours for the existing, Year 2030 no-build, and Year 2030 total projected traffic volumes.

The traffic analyses were performed using the methodologies outlined in the Transportation Research Board's *Highway Capacity Manual (HCM)*, 6th Edition and analyzed using Synchro/SimTraffic 11 computer software.

The analyses for the unsignalized intersections determine the average control delay to vehicles at an intersection. Control delay is the elapsed time from a vehicle joining the queue at a stop sign (includes the time required to decelerate to a stop) until its departure from the stop sign and resumption of free flow speed. The methodology analyzes each intersection approach controlled by a stop sign and considers traffic volumes on all approaches and lane characteristics.

The ability of an intersection to accommodate traffic flow is expressed in terms of level of service, which is assigned a letter from A to F based on the average control delay experienced by vehicles passing through the intersection. The *Highway Capacity Manual* definitions for levels of service and the corresponding control delay for signalized intersections and unsignalized intersections are included in the Appendix of this report.

Due to the unique traffic control configurations at the intersections of Forest Avenue with Franklin Street and Forest Avenue with the west leg of Warren Avenue, these intersections could not be analyzed using HCM procedures. As such, the intersections were analyzed using the Intersection Capacity Utilization (ICU) level of service. The ICU indicates how much reserve capacity is available or how much an intersection is over capacity. A description of these configurations, their purpose, and operations are included later in the report.

Summaries of the traffic analysis results showing the level of service and overall intersection delay (measured in seconds) for the base, no-build, and total projected conditions are presented in **Tables 2** through 6. A discussion of the intersections follows. Summary sheets for the capacity analyses are included in the Appendix.



Table 2 MAIN STREET WITH FRANKLIN STREET – SIGNALIZED

	Deal-Hann	Eastb	ound	W	estbound	Northbound	South	bound	0
	Peak Hour	L	R	L	T/R	L/T	Т	R	Overall
2	Weekday	D 47.9	B 16.4	C 33.8	C 24.3	A – 2.6	A 2.2	A 0.7	A
ting ition	Morning	D –	38.4		C - 25.5		A –	1.8	5.7
Existing Conditions	Weekday	D 45.1	B 16.5	C 34.3	C 21.2	A – 2.5	A 2.6	A 0.6	A
	Evening	D –	36.5		C - 22.6		A –	2.1	5.1
%	Weekday	D 48.2	B 15.9	C 33.6	C 23.9	A – 3.2	A 2.7	A 0.7	A
uild	Morning	D –	37.4		C - 25.1	11 3.2	A –	2.2	5.9
No-Build Conditions	Weekday	D 45.3	B 16.0	C 34.3	C 20.7	A – 2.8	A 2.8	A 0.6	A
	Evening	D –	35.9		C – 22.1	A – 2.0	A -	2.3	5.1
_ s	Weekday	D 49.0	B 14.7	C 32.8	C 23.1	A – 3.4	A 2.9	A 0.8	A
scted	Morning	D –	37.6		C - 24.3	11 00.	A -	2.3	6.3
Projected Conditions	Weekday	D 45.5	B 15.3	C 34.0	C 20.5	A – 2.9	A 2.9	A 0.7	A
	Evening	D –	35.8		C – 21.8	11 2.7	A –	2.3	5.3
	tes Level of Serv easured in second			Right Tu	rn				



Table 3 MAIN STREET WITH WARREN AVENUE – SIGNALIZED

	Dl- II	Eastbour	ıd	Westl	oound	North	bound	So	outhbound	0
	Peak Hour	L/T	R	Т	R	Т	R	L	T/R	Overall
70	Weekday	D 45.0	B 14.3	D 38.7	A 8.7	A 8.7	A 0.0	A 2.2	A 2.3	В
ting itions	Morning	D – 39.3		C -	L	A -	l		A – 2.3	10.3
Existing Conditions	Weekday	D 45.5	A 8.1	D 38.4	B 16.8	A 7.0	A 1.8	A 2.5	A 3.0	В
	Evening	D – 42.0)	C –	32.6	A –	6.7		A – 3.0	10.1
20	Weekday	D 45.2	B 14.7	D 38.6	A 8.6	A 9.8	A 0.0	A 2.2	A 2.4	В
Suild	Morning	D – 39.4		C –	33.3	A -	9.7		A – 2.4	10.6
No-Build Conditions	Weekday	D 45.6	A 8.0	D 38.1	B 16.2	A 7.9	A 1.9	A 2.5	A 3.3	В
	Evening	D – 42.3	3	C –	32.0	A –	7.7		A - 3.3	10.2
	Weekday	D 45.2	B 14.7	D 38.6	A 8.6	A 9.9	A 0.0	A 2.2	A 2.4	В
ected	Morning	D - 39.4		C –	33.3	A –	9.8		A - 2.4	10.6
Projected Conditions	Weekday	D 45.6	A 8.0	D 38.1	B 16.2	A 8.0	A 1.9	A 2.5	A 3.3	В
	Evening	D – 42.3	3	C –	32.0	A –	7.7		A - 3.3	10.2
	tes Level of Serv easured in second			Right Turn						



Table 4 UNSIGNALIZED – EXISTING CONDITIONS

Intersection	_	Morning Hour		y Evening K Hour
	LOS	Delay	LOS	Delay
Forest Avenue with Franklin Street ¹				
• ICU	A	37.3%	A	39.8%
Forest Avenue with Warren Avenue (We	est Leg)1			
• ICU	A	40.0%	A	36.5%
Forest Avenue with AT&T Access Drive	/Apartment	Access Drive ²		
Eastbound Approach	В	12.2	В	12.3
Westbound Approach	В	10.5	A	9.2
Northbound Left Turn	A	7.7	A	9.1
Southbound Left Turn	A	0.0	A	7.6
Forest Avenue with Warren Avenue (Ea	st Leg) ²			
Westbound Approach	В	10.7	A	9.7
Southbound Left Turn	A	8.0	A	7.8
Franklin Street with North-South Alley ²				
Northbound Approach	A	9.6	A	9.3
Westbound Left Turn	A	7.4	A	7.4
Main Street with Rogers Street ²				
Westbound Left Turn	В	11.7	В	10.9
Southbound Left Turn	A	8.4	A	8.1
LOS = Level of Service $1 - \text{Evaluated with}$ Delay is measured in seconds. $2 - \text{Two-way stop}$		n Capacity Utiliza	tion (ICU) me	ethod.



Table 5 UNSIGNALIZED – YEAR 2030 NO-BUILD CONDITIONS

Intersection		y Morning Hour		y Evening K Hour
	LOS	Delay	LOS	Delay
Forest Avenue with Franklin Street ¹				
• ICU	A	38.8%	A	41.0%
Forest Avenue with Warren Avenue (Wo	est Leg) ¹			
• ICU	A	41.4%	A	37.6%
Forest Avenue with AT&T Access Drive	/Apartment	Access Drive ²		
Eastbound Approach	В	12.5	В	12.6
Westbound Approach	В	10.6	A	9.3
Northbound Left Turn	A	7.7	A	9.2
Southbound Left Turn	A	0.0	A	7.6
Forest Avenue with Warren Avenue (Ea	st Leg) ²			
Westbound Approach	В	11.0	A	9.8
Southbound Left Turn	A	8.1	A	7.8
Franklin Street with North-South Alley ²				
Northbound Approach	A	9.7	A	9.3
Westbound Left Turn	A	7.4	A	7.4
Main Street with Rogers Street ²				
Westbound Left Turn	В	12.6	В	11.7
Southbound Left Turn	A	8.6	A	8.2
LOS = Level of Service 1 – Evaluated with Delay is measured in seconds. 2 – Two-way stop		n Capacity Utiliza	tion (ICU) me	ethod.



Table 6 UNSIGNALIZED – YEAR 2030 TOTAL CONDITIONS

Intersection		y Morning Hour		y Evening x Hour
	LOS	Delay	LOS	Delay
Forest Avenue with Franklin Street ¹				
• ICU	A	39.5%	A	40.6%
Forest Avenue with Warren Avenue (Wo	est Leg) ¹			
• ICU	A	41.7%	A	37.8%
Forest Avenue with AT&T Access Drive	/Apartment	Access Drive ²		
Eastbound Approach	В	12.6	В	12.7
Westbound Approach	В	10.7	A	9.3
Northbound Left Turn	A	7.7	A	9.2
Southbound Left Turn	A	0.0	A	7.6
Forest Avenue with Warren Avenue (Ea	st Leg) ²			
Westbound Approach	В	11.0	A	9.8
Southbound Left Turn	A	8.1	A	7.8
Franklin Street with North-South Alley ²				
Northbound Approach	A	9.7	A	9.5
Westbound Left Turn	A	7.4	A	7.4
Main Street with Rogers Street ²				
Westbound Left Turn	В	12.6	В	11.8
Southbound Left Turn	A	8.6	A	8.3
LOS = Level of Service 1 – Evaluated with Delay is measured in seconds. 2 – Two-way stop		n Capacity Utiliza	tion (ICU) me	ethod.



Table 6 – CONTINUED UNSIGNALIZED – YEAR 2030 TOTAL CONDITIONS

Intersection	-	Morning Hour		y Evening Hour
	LOS	Delay	LOS	Delay
Forest Avenue with Garage Access Drive	e^2			
Westbound Approach	В	10.2	A	9.8
Southbound Left Turn	A	7.7	A	7.6
North-South Alley with Garage Access I	Drive ²			
Eastbound Approach	A	8.6	A	8.6
Northbound Left Turn	A	0.0	A	0.0
		n Capacity Utiliza	ntion (ICU) me	ethod.



Discussion and Recommendations

The following summarizes how the intersections are projected to operate and identifies any roadway and traffic control improvements necessary to accommodate the development traffic.

Main Street with Franklin Street

The results of the capacity analysis indicate that the intersection currently operates overall at Level of Service (LOS) A during the weekday morning and weekday evening peak hours. The northbound and southbound approaches currently operate at LOS A during the peak hours. The eastbound and westbound approaches currently operate at an acceptable LOS D or better during both peak hours.

Under Year 2030 no-build conditions, the intersection is projected to continue operating at LOS A during the weekday morning and weekday evening peak hours with increases in delay of approximately less than one second over existing conditions. All approaches are projected to continue operating at their current LOS D or better during the peak hours.

Under Year 2030 total projected conditions, this intersection is projected to continue operating at LOS A during the weekday morning and weekday evening peak hours with increases in delay of approximately less than one second over Year 2030 no-build conditions. All approaches are projected to continue operating at their current LOS D or better during the peak hours. The proposed development is only projected to increase traffic through the intersection by less than one percent over no-build conditions. As such, the intersection has sufficient reserve to accommodate the traffic estimated to be generated by the proposed multi-family residential development and no additional roadway or traffic signal modifications are needed.

Main Street with Warren Avenue

The results of the capacity analysis indicate that the intersection currently operates overall at LOS B during the weekday morning and weekday evening peak hours. The northbound and southbound approaches currently operate at LOS A during the peak hours. The eastbound and westbound approaches currently operate at an acceptable LOS D or better during both peak hours.

Under Year 2030 no-build conditions, the intersection is projected to continue operating at LOS B during the weekday morning and weekday evening peak hours, with increases in delay of approximately less than one second over existing conditions. All approaches are projected to continue operating at LOS D or better during the peak hours with increases in delay of approximately less than one second over existing conditions.

Under Year 2030 total projected conditions, the intersection is projected to continue operating at LOS B during the weekday morning and weekday evening peak hours, with increases in delay of approximately less than one second over no-build conditions. All approaches are projected to continue operating at LOS D or better during the peak hours with increases in delay of approximately less than one second over no-build conditions.



The 95th percentile queues on the northbound and southbound approaches are projected to be three to four vehicles during the peak hours. The proposed development is projected to only increase the traffic traversing the intersection by less than one percent over no-build conditions.

As mentioned previously, this intersection is located approximately 100 feet north of the BNSF railroad crossing. As of August 2024, Metra BNSF trains utilize the crossing approximately six times during the weekday morning peak hour and approximately five times during the weekday evening peak hour. The traffic signal at Main Street with Warren Avenue is interconnected with the railroad crossing signal. This results in longer green times for the northbound approach to allow for traffic to clear the railroad crossing. During a train event, the southbound queues at the crossing can extend through the intersection with a 95th percentile queue of approximately 10 vehicles. Given that Rogers Street is approximately 255 feet north of Warren Avenue, queues can extend past its intersection with Main Street. It was observed that trains stop for approximately two minutes and once the crossing gates are up, queues typically clear within approximately two cycle lengths. Included in **Table 7** are the projected southbound maximum queues and 95th percentile queues compared to the link distance where the queues extend based on the simulation for the operation of the railroad crossing. The link distance is the distance in feet between the two intersections.

As such, this intersection has sufficient reserve capacity to accommodate the traffic projected to be generated by the proposed multi-family residential development and no roadway or traffic signal improvements will be required.

Table 7
MAXIMUM AND 95TH PERCENTILE QUEUES COMPARED TO LINK DISTANCE

	Weekday Morning Peak Hour	Weekday Evening Peak Hour
95 th Percentile Queue ¹	56	111
Maximum Queue ²	150	184
Link Distance	178	178
Queue and link distance are measured in feet.	1 – Based on Synchro 2 – Based on SimTraffic Si	mulation

Forest Avenue with Franklin Street

As previously indicated, because of the traffic control configuration of this intersection in which only three of the four legs are stop sign controlled, the intersection could not be analyzed using HCM procedures. Given this traffic control configuration and the limitations of the HCM procedures, the intersection was analyzed using the Intersection Capacity Utilization (ICU) level of service. The ICU indicates how much reserve capacity is available or how much an intersection is over capacity.



Based on the ICU analysis, the intersection currently utilizes 40 percent or less of the capacity of the intersection. Under Year 2030 no-build and total conditions, it is projected that the intersection will continue to utilize approximately 41 percent or less of the capacity of the intersection. As such, no roadway or traffic control improvements are required at this intersection in conjunction with the proposed multi-family residential development.

Forest Avenue with Warren Avenue, West Leg

As previously indicated, because of the traffic control configuration of this intersection in which only two of the three legs are stop sign controlled and they are adjacent legs to each other, the intersection could not be analyzed using HCM procedures. Given this traffic control configuration and the limitations of the HCM procedures, the intersection was analyzed using the Intersection Capacity Utilization (ICU) level of service. The ICU indicates how much reserve capacity is available or how much an intersection is over capacity.

Based on the ICU analysis, the intersection currently utilizes less than 40 percent of the capacity of the intersection. Under Year 2030 no-build and total conditions, it is projected that the intersection will continue to utilize less than 42 percent of the capacity of the intersection. As such, no roadway or traffic control improvements are required at this intersection in conjunction with the proposed multi-family residential development.

Forest Avenue with AT&T Access Drive/Apartment Access Drive

The results of the capacity analysis indicate that the eastbound and westbound approaches from the AT&T facility and the apartment building, respectively, currently operate at LOS B or better during the weekday morning and weekday evening peak hours. The northbound and southbound left-turn movements currently operate at LOS A during the peak hours.

Under Year 2030 no-build and total projected conditions, the critical approaches and movements are projected to continue operating at the current levels of service, with increases in delay of less than one second over existing conditions. As such, no roadway or traffic control improvements are required at this intersection in conjunction with the proposed multi-family residential development.

Forest Avenue with Warren Avenue, East Leg

The results of the capacity analysis indicate that the westbound approach currently operates at LOS B or better during the weekday morning and weekday evening peak hours. The southbound left turn currently operates at LOS A during the peak hours. Under Year 2030 no-build and total projected conditions, the westbound approach and southbound left turn are projected to continue operating at their current levels of service during the peak hours. As such, no roadway or traffic control improvements are required at this intersection in conjunction with the proposed multifamily residential development.



Franklin Street with North-South Alley

The results of the capacity analysis indicate that the northbound approach currently operates at LOS A during the weekday morning and weekday evening peak hours. The westbound left-turn movement into the alley currently operates at LOS A during the peak hours. Under Year 2030 nobuild and total projected conditions, the critical approaches and movements are projected to continue operating at the current levels of service. As such, no roadway or traffic control improvements are required at this intersection in conjunction with the proposed multi-family residential development.

Main Street with Rogers Street

The results of the capacity analysis indicate that the westbound approach currently operates at LOS B during the weekday morning and weekday evening peak hours. The southbound left turn currently operates at LOS A during the peak hours. Under Year 2030 no-build conditions, the westbound approach is projected to continue operating at LOS B during the weekday morning and weekday evening peak hours with increases in delay of approximately less than one second over existing conditions. The southbound left turn is projected to continue operating at LOS A during the peak hours.

Under Year 2030 total projected conditions, the westbound approach is projected to continue operating at LOS B during the weekday morning and weekday evening peak hours with increases in delay of approximately less than one second over Year 2030 no-build conditions. The southbound left turn is projected to continue operating at LOS A during the peak hours. As mentioned previously, during train events, the intersection is blocked by southbound queues extending from the intersection of Main Street with Warren Avenue. It should be noted that once the crossing gates are up, these queues clear within approximately two cycle lengths. As such, no roadway or traffic control improvements are required at this intersection in conjunction with the proposed multi-family residential development.

Forest Avenue with Garage Access Drive

Under Year 2030 total projected conditions, the garage will provide one inbound lane and one outbound lane with outbound movements under stop sign control. This garage access drive will serve parking spaces on the ground floor. The results of the capacity analysis indicate that the westbound approach is projected to operate at LOS B or better during the weekday morning and weekday evening peak hours.

The southbound left-turn movement into the garage is projected to operate at LOS A during the peak hours. As such, this access drive is projected to provide flexible and efficient access to the garage and no additional roadway or traffic control measures are required.



North-South Alley and Garage Access Evaluation

As previously indicated, the north-south alley that borders the site to the east is approximately 14-feet-wide. As part of the proposed development, the building will be offset three feet to the west, increasing the effective width of the alley along the site to 17-feet. Two-way traffic is allowed for the length of the alley and the posted speed limit is 10 miles per hour.

Based on the traffic counts conducted at the north end of the public alley, it was determined that the two-way traffic within the alley for a 24-hour period totaled approximately 281 vehicles. In this time period, 185 vehicles were traveling southbound and 96 vehicles were traveling northbound. The unbalanced traffic volumes are likely due to the fact that multiple commercial uses along Main Street and Forest Avenue have access drives which directly connect through their respective parking lots to the alley from those streets. Furthermore, the orientation of some parking fields result in inbound traffic from the roadway network and outbound traffic onto the public alley.

These relatively low volumes (compared to the area roadway network) combined with the additional pavement created by the commercial drives will continue to allow for two-way traffic to occur along the alley.

Access to the second-floor garage providing 47 parking spaces will be provided off the alley. While the north-south alley connects to the south to a 10-ft east-west alley that enters off Main Street, it is recommended that signs be posted at the garage exit directing traffic to travel to and from the north on the alley.

As the alley has relatively low traffic volumes throughout the day and low traffic volumes during the weekday morning and weekday evening peak hours, the minimal additional traffic generated by the proposed development will have a minimal impact on the operations of the alley. Furthermore, the parking garage will be utilized as parking for residents of the proposed building who will be familiar with the orientation, operation, and characteristics of the public alley when departing or arriving to the proposed building.

The results of the capacity analyses indicate that under Year 2030 total projected conditions, outbound movements from the access drive onto the public alley are projected to operate at LOS A during the weekday morning and weekday evening peak hours. As such, a single access drive serving the 47 proposed parking spaces will provide sufficient capacity to accommodate the traffic generated by these spaces during the peak hours.

Parking Evaluation

As previously indicated, the multi-family residential development will provide 62 residential units and a parking garage providing 89 parking spaces for the exclusive use of residents. Per the Village of Downers Grove Municipal Code, apartments/condominiums in the downtown zoning district are required to provide 1.4 parking spaces per dwelling unit. With 62 residential units in the proposed multi-family building, the parking garage should provide approximately 87 parking spaces. With 89 proposed parking spaces, the garage will meet the village requirements.



Per the Institute of Transportation Engineers, *Parking Generation Manual*, 6th Edition, the average rate of parking required is 1.23 spaces per dwelling unit. With 62 residential units in the proposed multi-family building, the parking garage should provide approximately 76 parking spaces, which is met by the proposed supply of 89 parking spaces.

Based on the above, the proposed parking supply will be adequate in meeting the parking needs of the proposed development.



6. Conclusion

Based on the preceding analyses and recommendations, the following conclusions have been made:

- The traffic that will be generated by the proposed multi-family residential development can be accommodated by the existing area roadway system.
- The development will provide indoor garage parking, with access off Forest Avenue and off the north-south alley bordering the site on the east.
- The proposed development-generated traffic will have a limited impact on the traffic operations of the adjacent intersections and as such it can be accommodated by the existing roadways and traffic control.
- The consolidation of multiple access drives on Forest Avenue into a single access drive will improve the traffic along Forest Avenue.
- The proposed access system will be adequate and efficient in serving the traffic estimated to be generated by the multi-family residential development.
- At the Forest Avenue access drive, a stop sign and a visual warning device should be posted at the garage access.
- At the public alley access drive, signage should be provided directing vehicles to travel to/from the north at the public alley's intersection with Franklin Street.
- The north-south alley currently carries low traffic volumes and under projected conditions, traffic within the alley will continue to operate well with minimal conflicts.



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Appendix

Traffic Count Summary Sheets
Site Plan
ITE Trip Generation Sheets
CMAP 2050 Projections Letter
Level of Service Criteria
Capacity Analysis Summary Sheets

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Traffic Count Summary Sheets

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Count Name: Franklin+Street+and+Main+Street TMC Site Code: Start Date: 08/20/2024 Page No: 1

			Int. Total	162	168	233	215	778	208	228	210	193	839		217	196	509	189	811	181	202	228	188	799	243	214	223	211	891	265	252	260	271	1048	5166			5027
			App. Total	53	29	73	84	569	80	26	77	99	320		106	93	101	92	392	82	100	133	96	411	132	120	112	127	491	129	141	130	151	551	2434		47.1	2381
			Peds	11	43	62	21	137	15	5	7	0	27		0	0	0	0	0	0	0	0	1	1	0	_	_	0	2	0	8	1	_	5	172			
	treet	puno	Right	12	16	20	28	92	24	25	20	19	88		29	23	23	15	06	23	23	35	21	102	30	30	20	32	112	49	36	27	36	148	616	25.3	11.9	605
	Main Street	Southbound	Thru	41	43	53	99	193	56	71	22	47	231		77	70	78	77	302	29	77	86	75	309	102	06	92	92	379	80	105	103	115	403	1817	74.7	35.2	1775
			Left	0	0	0	0	0	0	1	0	0	1		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0.0	0.0	1
			U-Tum	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
		-	App. Total	16	85	127	104	407	112	111	109	109	441		06	98	87	78	341	22	75	69	71	292	68	73	98	71	319	102	88	100	103	393	2193		42.5	2137
			Peds	0	2	10	7	19	3	2	4	2	11		3	3	2	6	17	2	0	1	4	7	0	7	_	8	11	4	3	1	1	6	74			-
	treet	punc	Right	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
	Main Street	Northbound	Thru	98	73	114	94	367	109	104	66	104	416		62	92	92	72	303	72	71	09	62	265	81	29	73	64	285	92	77	26	06	356	1992	8.06	38.6	1940
ata			Left	5	12	13	10	40	3	7	10	5	25		11	10	11	9	38	5	4	6	6	27	8	9	13	7	34	10	11	3	13	37	201	9.2	3.9	197
Turning Movement Data			U-Tum	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
ovem			App. Total	4	14	17	11	46	9	2	7	2	17		3	3	3	1	10	1	3	9	7	17	5	9	3	2	16	5	2	13	9	29	135		2.6	122
ng M)		Peds	1	6	16	8	34	4	3	3	0	10	-	0	1	0	1	2	1	2	0	1	4	0	10	30	_	41	_	7	1	0	6	100	-	,	-
Turni	Street	pun	Right	0	9	8	4	18	4	0	4	-	6		1	0	2	1	4	1	1	1	2	5	-	2	2	0	2	2	4	6	2	17	58	43.0	1.1	54
	Franklin Street	Westbound	Thru	4	4	9	2	19	2	2	1	-	9		2	3	0	0	5	0	2	3	5	10	2	4	0	2	80	3	0	2	3	8	56	41.5	1.1	52
			Left	0	4	3	2	6	0	0	2	0	2		0	0	1	0	1	0	0	2	0	2	2	0	_	0	3	0	_	2	1	4	21	15.6	0.4	16
			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
•			App. Total	14	10	16	16	99	10	18	17	16	61		18	14	18	18	89	21	24	20	14	62	17	15	22	11	65	29	18	17	11	75	404		7.8	387
			Peds	0	2	2	1	5	4	0	3	_	8	-	1	1	1	0	3	1	0	8	9	15	2	11	2	_	16	9	0	2	_	6	56			-
	Street	pun	Right	2	4	8	1	15	2	7	2	4	15		4	3	7	5	19	5	5	3	4	17	3	7	3	0	13	7	2	5	5	22	101	25.0	2.0	93
	Franklin Street	Eastbound	Thru	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	1	0	2	1	4	5	1.2	0.1	1
			Left	12	9	8	15	41	8	11	15	12	46		14	11	11	13	49	15	19	17	10	61	14	8	19	1	52	21	13	10	5	49	298	73.8	5.8	293
			U-Tum	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
			Start Time	7:00 AM	7:15 AM	7:30 AM	7:45 AM	Hourly Total	8:00 AM	8:15 AM	8:30 AM	8:45 AM	Hourly Total	*** BREAK ***	2:00 PM	2:15 PM	2:30 PM	2:45 PM	Hourly Total	3:00 PM	3:15 PM	3:30 PM	3:45 PM	Hourly Total	4:00 PM	4:15 PM	4:30 PM	4:45 PM	Hourly Total	5:00 PM	5:15 PM	5:30 PM	5:45 PM	Hourly Total	Grand Total	Approach %	Total %	Lights

% Lights		98.3	20.0	92.1		95.8		76.2	92.9	93.1		90.4	ı	98.0	97.4		1	97.4		100.0	7.76	98.2	-	8.76	97.3
Buses	0	0	0	2		2	0	4	2	4		10	0	1	26	0	-	27	0	0	14	5	-	19	58
% Buses		0.0	0.0	2.0		0.5		19.0	3.6	6.9		7.4	-	0.5	1.3		1	1.2		0.0	0.8	8.0	-	8.0	1.1
Single-Unit Trucks	0	2	0	4		9	0	0	0	0		0	0	-	21	0		22	0	0	23	2		28	26
% Single-Unit Trucks	,	0.7	0.0	4.0	,	1.5	,	0.0	0.0	0.0	,	0.0	,	0.5	1.1		,	1.0		0.0	1.3	8.0	,	1.2	1:1
Articulated Trucks	0	2	0	0		2	0	0	0	0		0	0	1	4	0		5	0	0	5	0	-	5	12
% Articulated Trucks	,	0.7	0.0	0.0	,	0.5		0.0	0.0	0.0	,	0.0	,	0.5	0.2		,	0.2		0.0	0.3	0.0	,	0.2	0.2
Bicycles on Road	0	-	4	2	,	7	0	1	2	0	,	3	0	1	1	0	-	2	0	0	0	1	-	1	13
% Bicycles on Road		0.3	80.0	2.0		1.7		4.8	3.6	0.0		2.2		0.5	0.1			0.1		0.0	0.0	0.2		0.0	0.3
Pedestrians	٠		•	•	56			•	•	•	100					•	74		•		,		172		
or ciutochec C /o					0 007						400						0007						4000		



Count Name: Franklin+Street+and+Main+Street TMC Site Code: Start Date: 08/20/2024 Page No: 3

Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

								Turr	M bul	loven	ent P	eak F	Turning Movement Peak Hour Data (7:30 AM))ata (,	7:30 4	(M)									
			Frankli	Franklin Street					Franklin Street	Street				•	Main Street	reet					Main Street	et,			
			East	Eastbound					Westbound	puno					Northbound	pun					Southbound	pui			
Start Time	U-Tum	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right P	Peds	App. Int	Int. Total
7:30 AM	0	8	0	8	2	16	0	3	9	8	16	17	0	13	114	0	10	127	0	0	53	20	62	73	233
7:45 AM	0	15	0	-	1	16	0	2	5	4	8	11	0	10	94	0	7	104	0	0	56	28	21	84	215
8:00 AM	0	8	0	2	4	10	0	0	2	4	4	9	0	3	109	0	3	112	0	0	56	24	15	80	208
8:15 AM	0	11	0	7	0	18	0	0	2	0	3	2	0	7	104	0	2	111	0	1	71	25	5	97	228
Total	0	42	0	18	7	60	0	5	15	16	31	36	0	33	421	0	22	454	0	1	236	26	103	334	884
Approach %	0.0	70.0	0.0	30.0			0.0	13.9	41.7	44.4		,	0.0	7.3	92.7	0.0	1		0.0	0.3	70.7	29.0	-		
Total %	0.0	4.8	0.0	2.0		6.8	0.0	9.0	1.7	1.8		4.1	0.0	3.7	47.6	0.0		51.4	0.0	0.1	26.7	11.0	-	37.8	
PHF	0.000	0.700	0.000	0.563		0.833	0.000	0.417	0.625	0.500		0.529	0.000	0.635	0.923	0.000	-	0.894	0.000	0.250	0.831 0	0.866	-	0.861 C	0.948
Lights	0	41	0	16		57	0	4	15	16		35	0	32	407	0		439	0	-	229	95	-	325	856
% Lights		97.6	٠	88.9		95.0	-	80.0	100.0	100.0		97.2		0.76	2.96			2.96		100.0	97.0	97.9		97.3	8.96
Buses	0	0	0	-		1	0	_	0	0		1	0	-	5	0		9	0	0	3	-		4	12
% Buses		0.0		5.6	,	1.7		20.0	0.0	0.0	,	2.8		3.0	1.2		,	1.3		0.0	1.3	1.0		1.2	1.4
Single-Unit Trucks	0	1	0	1		2	0	0	0	0		0	0	0	8	0		8	0	0	3	1		4	14
% Single-Unit Trucks		2.4		9.9		3.3		0.0	0:0	0.0		0.0		0.0	1.9			1.8		0.0	1.3	1.0		1.2	1.6
Articulated Trucks	0	0	0	0		0	0	0	0	0		0	0	0	1	0	-	1	0	0	1	0		1	2
% Articulated Trucks		0.0		0.0		0.0		0.0	0:0	0.0		0.0		0.0	0.2			0.2		0.0	0.4	0.0		0.3	0.2
Bicycles on Road	0	0	0	0		0	0	0	0	0	,	0	0	0	0	0	,	0	0	0	0	0		0	0
% Bicycles on Road		0.0		0.0		0.0		0.0	0.0	0.0		0.0		0.0	0.0			0.0		0.0	0.0	0.0		0.0	0.0
Pedestrians	,				7	-	-				31						22	-				-	103		
% Pedestrians					100.0						100.0						100.0					-	100.0		



Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Franklin+Street+and+Main+Street TMC Site Code: Start Date: 08/20/2024 Page No: 4

								Turn	Turning Movement Peak Hour Data (5:00 PM	ovem	ent P	eak h	our D)ata ({	5:00 F	(M									
			Franklin Street	Street					Franklin Stree	Street				•	Main Street	reet					Main Street	eet			
			Eastbound	puno					Westbound	pun					Northbound	pun					Southbound	pun			
Start Time	U-Tum	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	Int. Total
5:00 PM	0	21	-	7	9	29	0	0	3	2	1	5	0	10	92	0	4	102	0	0	80	49	0	129	265
5:15 PM	0	13	0	2	0	18	0	1	0	4	7	5	0	11	77	0	3	88	0	0	105	36	3	141	252
5:30 PM	0	10	2	2	2	17	0	2	2	6	1	13	0	3	26	0	1	100	0	0	103	27	1	130	260
5:45 PM	0	5	1	5	-	11	0	1	3	2	0	9	0	13	06	0	1	103	0	0	115	36	1	151	271
Total	0	49	4	22	6	75	0	4	8	17	6	29	0	37	356	0	6	393	0	0	403	148	5	551	1048
Approach %	0.0	65.3	5.3	29.3			0.0	13.8	27.6	58.6			0.0	9.4	9.06	0.0			0.0	0.0	73.1	26.9	1		
Total %	0.0	4.7	0.4	2.1		7.2	0.0	0.4	0.8	1.6		2.8	0.0	3.5	34.0	0.0		37.5	0.0	0.0	38.5	14.1	-	52.6	
PHF	0.000	0.583	0.500	0.786		0.647	0.000	0.500	0.667	0.472		0.558	0.000	0.712	0.918	0.000	-	0.954 (0.000	0.000	0.876	0.755	-	0.912	0.967
Lights	0	49	0	21		70	0	3	8	17		28	0	37	356	0		393	0	0	400	147	1	547	1038
% Lights		100.0	0.0	95.5		93.3		75.0	100.0	100.0		9.96		100.0	100.0			100.0			99.3	99.3	-	99.3	0.66
Buses	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	1	0	-	1	-
% Buses		0.0	0.0	0.0		0.0		0.0	0.0	0.0	,	0.0		0.0	0.0		,	0.0			0.2	0.0	,	0.2	0.1
Single-Unit Trucks	0	0	0	0		0	0	0	0	0		0	0	0	0	0	,	0	0	0	-	0		-	-
% Single-Unit Trucks		0.0	0.0	0.0		0.0		0.0	0.0	0.0		0.0		0.0	0:0			0.0			0.2	0.0		0.2	0.1
Articulated Trucks	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	1	0	-	1	-
% Articulated Trucks		0.0	0.0	0.0	,	0.0		0.0	0.0	0.0	,	0.0		0.0	0.0		,	0.0			0.2	0.0	,	0.2	0.1
Bicycles on Road	0	0	4	-	,	5	0	-	0	0		-	0	0	0	0	,	0	0	0	0	1		1	7
% Bicycles on Road	,	0.0	100.0	4.5	,	6.7		25.0	0.0	0.0		3.4		0.0	0.0		,	0.0			0.0	0.7		0.2	0.7
Pedestrians			,	'	6		·				6	•	۱.	•	١.	.	6						5		.
% Pedestrians					100.0						100.0	,	,				100.0						100.0	,	



Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Main+St+with+Warren+Ave TMC Site Code: Start Date: 08/20/2024 Page No: 1

			Int. Total	136	141	206	155	638	146	204	189	159	869		148	191	190	164	693	144	160	205	193	702	186	201	194	196	777	190	203	232	228	853	4361			4239
•			App. Total	37	45	70	22	207	39	19	99	46	230	-	63	89	91	71	314	99	71	112	94	343	98	100	84	06	369	62	06	124	113	406	1869		42.9	1814
			Peds	5	2	10	2	25	13	6	8	12	42		6	6	7	2	30	18	6	12	13	52	12	14	12	16	54	17	6	9	6	41	244		.	
	treet	puno	Right	1	2	3	7	13	2	2	5	3	12		1	4	1	1	7	0	0	5	5	10	4	2	2	3	11	2	4	1	2	15	89	3.6	1.6	65
	Main Street	Southbound	Thru	35	41	99	47	189	35	72	61	41	209		62	84	88	29	301	64	69	107	98	326	91	86	82	98	357	73	85	123	106	387	1769	94.6	40.6	1719
			Left	1	2	1	1	5	2	5	0	2	6		0	1	2	3	9	2	2	0	3	7	0	0	0	-	1	1	1	0	2	4	32	1.7	0.7	30
			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
	•		App. Total	79	72	110	78	339	84	66	26	92	375		61	75	78	73	287	22	69	63	9/	255	69	72	78	89	277	71	72	81	78	302	1835		42.1	1785
			Peds	0	0	0	0	0	0	0	0	0	0		3	0	2	3	8	0	4	1	2	7	0	1	_	9	8	0	0	3	1	4	27	,	-	
	treet	puno	Right	0	1	1	0	2	1	0	2	2	5		1	9	3	2	12	1	2	1	7	14	8	8	8	8	32	4	3	9	3	16	81	4.4	1.9	81
	Main Street	Northbound	Thru	62	20	109	78	336	83	66	94	93	369		09	69	75	71	275	26	54	62	69	241	51	64	20	29	244	29	69	75	75	286	1751	95.4	40.2	1702
ata			Left	0	1	0	0	1	0	0	1	0	1		0	0	0	0	0	0	0	0	0	0	0	0	0	_	1	0	0	0	0	0	3	0.2	0.1	2
ent D			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
Turning Movement Data			App. Total	12	6	8	6	38	11	6	12	2	34		13	8	10	7	38	7	13	6	8	37	17	8	14	13	52	18	20	6	17	64	263		0.9	256
ing M)		Peds	0	2	9	11	19	9	4	2	5	17		9	8	9	8	28	25	5	7	7	44	8	23	33	89	72	13	80	9	21	48	228		.	
Turn	Avenue	punc	Right	2	1	1	2	6	1	3	1	0	5		3	3	3	1	10	0	2	2	2	9	5	2	4	3	14	7	4	2	4	17	61	23.2	1.4	61
	Warren A	Westbound	Thru	9	8	7	7	28	10	5	11	2	28		10	2	7	9	28	9	11	9	9	29	12	9	6	10	37	11	13	9	12	42	192	73.0	4.4	185
			Left	1	0	0	0	1	0	1	0	0	1		0	0	0	0	0	1	0	1	0	2	0	0	1	0	1	0	3	1	1	5	10	3.8	0.2	10
			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
•	•	•	App. Total	8	15	18	13	54	12	17	14	16	59		11	19	11	13	54	14	17	21	15	29	15	21	18	25	79	22	21	18	20	81	394		9.0	384
			Peds	2	10	4	7	23	6	8	3	11	31		2	16	11	6	41	7	14	27	11	29	15	15	35	22	87	12	2	3	7	27	268		-	
	Avenue	punc	Right	0	2	2	4	11	1	1	4	4	10		3	9	0	1	10	4	3	9	9	19	3	3	4	3	13	2	0	2	3	7	20	17.8	1.6	29
	Warren Avenue	Eastbound	Thru	4	8	9	9	24	8	10	7	9	31		4	6	9	6	28	7	11	11	5	34	7	14	8	15	44	10	18	11	14	53	214	54.3	4.9	209
			Left	4	2	7	3	19	3	9	3	9	18		4	4	2	3	16	3	3	4	4	14	5	4	9	7	22	10	3	5	3	21	110	27.9	2.5	108
			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
		į	Start Time	7:00 AM	7:15 AM	7:30 AM	7:45 AM	Hourly Total	8:00 AM	8:15 AM	8:30 AM	8:45 AM	Hourly Total	*** BREAK ***	2:00 PM	2:15 PM	2:30 PM	2:45 PM	Hourly Total	3:00 PM	3:15 PM	3:30 PM	3:45 PM	Hourly Total	4:00 PM	4:15 PM	4:30 PM	4:45 PM	Hourly Total	5:00 PM	5:15 PM	5:30 PM	5:45 PM	Hourly Total	Grand Total	Approach %	Total %	Lights

% Lights	-	98.2	7.76	95.7		97.5	-	100.0	96.4	100.0		97.3	i	66.7	97.2	100.0	-	97.3		93.8	97.2	92.6		97.1	97.2
Buses	0	1	0	0		1	0	0	1	0		1	0	0	23	0	-	23	0	0	18	0	-	18	43
% Buses	-	6.0	0.0	0.0		0.3	-	0.0	0.5	0.0		0.4	-	0.0	1.3	0.0	-	1.3		0.0	1.0	0.0	-	1.0	1.0
Single-Unit Trucks	0	1	2	2		5	0	0	1	0		1	0	0	19	0	-	19	0	2	25	2		29	54
% Single-Unit Trucks		6.0	6.0	2.9	,	1.3		0.0	0.5	0.0	,	0.4	,	0.0	1.1	0.0	,	1.0	,	6.3	1.4	2.9		1.6	1.2
Articulated Trucks	0	0	0	0		0	0	0	-	0		1	0	0	9	0	-	9	0	0	5	0		5	12
% Articulated Trucks		0.0	0.0	0.0	,	0.0		0.0	0.5	0.0	,	0.4	,	0.0	0.3	0.0	,	0.3		0.0	0.3	0.0		0.3	0.3
Bicycles on Road	0	0	3	1		4	0	0	4	0		4	0	1	1	0	-	2	0	0	2	1	-	3	13
% Bicycles on Road		0.0	1.4	1.4		1.0		0.0	2.1	0:0		1.5		33.3	0.1	0.0		0.1		0.0	0.1	1.5		0.2	0.3
Pedestrians	-		•		268	,					228	,				,	27	-		•			244	-	
C / 0					000						000						000						000		



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Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

								Turn	ing M	ovem	ent P	eak F	our D	Turning Movement Peak Hour Data (7:30 AM)	7:30	(M									
			Warren	Warren Avenue					Warren Avenue	venue					Main Street	reet					Main Street	eet			
_			East	Eastbound					Westbound	punc		-			Northbound	pund					Southbound	pur.			
Start Time	U-Tum	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	Int. Total
7:30 AM	0	7	9	5	4	18	0	0	7	1	9	8	0	0	109	1	0	110	0	1	99	3	10	70	206
7:45 AM	0	3	9	4	7	13	0	0	7	2	11	6	0	0	78	0	0	78	0	1	47	7	5	22	155
8:00 AM	0	3	8	1	6	12	0	0	10	1	9	11	0	0	83	1	0	84	0	2	35	2	13	39	146
8:15 AM	0	9	10	1	8	17	0	1	5	3	4	6	0	0	66	0	0	66	0	5	72	2	6	19	204
Total	0	19	30	11	28	09	0	1	59	7	27	37	0	0	369	2	0	371	0	6	220	14	37	243	711
Approach %	0.0	31.7	50.0	18.3		•	0.0	2.7	78.4	18.9			0.0	0.0	99.5	0.5			0.0	3.7	90.5	5.8	1	-	
Total %	0.0	2.7	4.2	1.5		8.4	0.0	0.1	4.1	1.0		5.2	0.0	0.0	51.9	0.3		52.2	0.0	1.3	30.9	2.0	-	34.2	
PHF	0.000	0.679	0.750	0.550		0.833	0.000	0.250	0.725	0.583		0.841	0.000	0.000	0.846	0.500	-	0.843	0.000	0.450	0.764	0.500	-	0.769	0.863
Lights	0	19	30	11		09	0	1	29	7		37	0	0	360	2	-	362	0	8	211	14	1	233	692
% Lights		100.0	100.0	100.0		100.0		100.0	100.0	100.0		100.0			97.6	100.0		97.6		88.9	95.9	100.0		95.9	97.3
Buses	0	0	0	0		0	0	0	0	0		0	0	0	3	0		3	0	0	5	0		5	8
% Buses		0.0	0.0	0.0		0.0		0.0	0.0	0.0		0.0			8.0	0.0		0.8		0.0	2.3	0.0	-	2.1	1.1
Single-Unit Trucks	0	0	0	0		0	0	0	0	0		0	0	0	5	0		5	0	-	3	0		4	6
% Single-Unit Trucks	•	0.0	0.0	0.0		0.0		0.0	0:0	0.0	,	0:0			4.	0.0	,	1.3		11.1	4.	0.0	,	9.1	1.3
Articulated Trucks	0	0	0	0		0	0	0	0	0		0	0	0	-	0		1	0	0	1	0		1	2
% Articulated Trucks	,	0.0	0.0	0.0	,	0.0		0.0	0:0	0.0	,	0.0			0.3	0.0	,	0.3		0.0	0.5	0.0	,	4.0	0.3
Bicycles on Road	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles on Road	-	0.0	0.0	0.0		0.0	-	0.0	0.0	0.0		0.0			0.0	0.0	-	0.0		0.0	0.0	0.0	-	0.0	0.0
Pedestrians	,				28	'					27						0						37	,	
% Pedestrians					100.0						100.0												100.0		



Count Name: Main+St+with+Warren+Ave TMC Site Code: Start Date: 08/20/2024 Page No: 4

Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Warren Avenue Eastbound	Warren Avenue Eastbound	Warren Avenue Eastbound	Avenue			-	-	Turni	ng Move Warren Avenue	ovem venue	ent P	eak F	Turning Movement Peak Hour Data (5:00 PM)ata (5:00 PM Main Street Northbound	PM) street					Main Street Southbound	eet und		-	
ht Peds App. U-Turn Left T	Thru Right Peds App, U-Turn Left T	Right Peds App. U-Turn Left T	ht Peds App. U-Turn Left T	App. U-Turn Left T	U-Turn Left T	Left T	-	Thr		Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Ħ	Peds	App. Ir	Int. Total
0 10 10 2 12 22 0 0 1	10 2 12 22 0 0	2 12 22 0 0	12 22 0 0	22 0 0	0 0	0		[11	7	13	18	0	0	29	4	0	71	0	+	73	5	17	79	190
0 3 18 0 5 21 0 3	18 0 5 21 0	0 5 21 0	5 21 0	21 0	0		3		13	4	8	20	0	0	69	3	0	72	0	1	85	4	6	06	203
0 5 11 2 3 18 0 1	11 2 3 18	2 3 18	3 18	18		0 1	1		9	2	9	6	0	0	75	9	3	81	0	0	123	1	9	124	232
0 3 14 3 7 20 0 1	14 3 7 20	3 7 20	7 20	20		0 1	-	- 1	12	4	21	17	0	0	75	3	_	78	0	2	106	5	6	113	228
0 21 53 7 27 81 0 5	53 7 27 81 0	7 27 81 0	27 81 0	81 0	0		5		42	17	48	64	0	0	286	16	4	302	0	4	387	15	41	406	853
0.0 25.9 65.4 8.6 0.0 7.8	65.4 8.6 0.0	8.6 0.0	6 0.0				7.8		9.59	26.6	,		0.0	0.0	94.7	5.3	,		0.0	1.0	95.3	3.7			,
0.0 2.5 6.2 0.8 - 9.5 0.0 0.6	6.2 0.8 - 9.5 0.0	0.8 - 9.5 0.0	- 9.5 0.0	0.0	0.0		9.0		4.9	2.0	,	7.5	0.0	0.0	33.5	1.9	,	35.4	0.0	0.5	45.4	1.8		47.6	
0.000 0.525 0.736 0.583 - 0.920 0.000 0.417	0.736 0.583 - 0.920 0.000 0.417	0.583 - 0.920 0.000 0.417	- 0.920 0.000 0.417	0.000 0.417	0.000 0.417	0.417			0.808	0.607	,	0.800	0.000	0.000	0.953	0.667		0.932	0.000	0.500	0.787	0.750		0.819	0.919
0 21 51 7 - 79 0 5	51 7 - 79 0	7 - 79 0	0 62	0 62	0		5		40	17	,	62	0	0	284	16	,	300	0	4	383	15		402	843
- 100.0 96.2 100.0 - 97.5 - 100.0	96.2 100.0 - 97.5 -	- 97.5 - 97.5	- 97.5	'	'	- 100.0	100.0		95.2	100.0	,	6.96			99.3	100.0	,	99.3		100.0	0.66	100.0		0.66	98.8
0 0 0 - 0 0 0 0	0 0 - 0 0	0 0 - 0	0 0 -	0	0		0		0	0	1	0	0	0	-	0	,	-	0	0	-	0		-	2
- 0.0 - 0.0 - 0.0 - 0.0	- 0.0 - 0.0	0.0 - 0.0	- 0.0 -	,	,		0.0		0.0	0.0	1	0.0			0.3	0.0	,	0.3	,	0.0	0.3	0.0		0.2	0.2
0 0 0 - 0 0 0 0	0 0 - 0 0	0 0 - 0	0 0 -	0 0	0		0		0	0		0	0	0	0	0	'	0	0	0	-	0		-	-
- 0.0 0.0 0.0 - 0.0 - 0.0	0.0 - 0.0 -	- 0.0 - 0.0	- 0.0	-	-		0.0		0.0	0.0	,	0.0			0.0	0.0	,	0.0		0.0	0.3	0.0		0.2	0.1
0 0 0 - 0 0 0 0	0 0 - 0 0	0 0 - 0	0 0 -	0	0		0		0	0	1	0	0	0	~	0	,	-	0	0	-	0		-	2
- 0.0 0.0 0.0 - 0.0 - 0.0	0.0 - 0.0 -	- 0.0 - 0.0	- 0.0	0.0	-		0.0		0.0	0.0		0.0		-	0.3	0.0	-	0.3		0.0	0.3	0.0	-	0.2	0.2
0 0 2 0 - 2 0 0	2 0 - 2 0	0 - 2 0	- 2 0	2 0	0		0		2	0	,	2	0	0	0	0	,	0	0	0	_	0		-	2
- 0.0 3.8 0.0 - 2.5 - 0.0	3.8 0.0 - 2.5 -	0.0 - 2.5 -	- 2.5	2.5	,		0.0		4.8	0:0	,	3.1			0.0	0.0	,	0.0		0.0	0.3	0.0	,	0.2	9.0
	- 27	- 27	- 27	27				. 1	, ·	, ·	48			.			4	·					41	-	
- 10000	- 100.0	- 100.0	- 100.0 -	100.0							100.0	,					100.0					'	100.0		,



Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Franklin+St+with+Forest+Ave TMC Site Code: Start Date: 08/20/2024 Page No: 1

			Int. Total	72	96	125	126	419	140	108	83	69	400		92	89	92	83	356	98	111	116	97	410	119	103	86	94	414	138	130	93	116	477	2476			2411
·			App. Total	9	10	16	10	42	30	22	9	6	67		6	15	11	7	42	10	20	21	12	63	22	15	10	16	63	13	11	8	15	47	324	-	13.1	304
			Peds	1	0	1	0	2	0	0	0	0	0		0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	3			,
	venue	puno	Right	0	0	1	1	2	2	0	-	2	5		0	0	1	0	1	0	1	1	0	2	2	3	0	0	5	_	0	0	0	1	16	4.9	9.0	14
	Forest Avenue	Southbound	Thru	4	5	11	6	59	27	19	5	7	58		6	15	10	7	41	10	19	20	10	29	19	11	6	16	55	12	10	7	15	44	286	88.3	11.6	270
			Left	2	2	4	0	11	1	3	0	0	4		0	0	0	0	0	0	0	0	2	2	-	1	1	0	3	0	1	1	0	2	22	8.9	6.0	20
			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
	-		App. Total	43	20	26	59	211	71	39	33	27	170		36	59	32	35	132	38	46	30	35	149	48	34	42	28	152	53	49	40	33	175	686		39.9	896
			Peds	0	1	1	1	3	1	0	2	1	4		2	0	2	0	4	0	1	0	2	3	7	3	3	4	17	3	2	0	2	7	38	,	,	
	venue	puno	Right	14	16	17	8	22	9	7	11	6	33		13	12	11	11	47	19	17	11	6	26	14	6	15	12	20	21	13	14	7	22	296	29.9	12.0	289
	Forest Avenue	Northbound	Thru	27	32	41	46	146	26	29	21	15	121		22	14	17	17	20	17	28	17	21	83	32	22	21	15	06	27	31	22	21	101	611	61.8	24.7	599
ata			Left	2	2	1	5	10	6	3	_	3	16		1	3	4	7	15	2	1	2	5	10	2	3	9	_	12	2	5	4	5	19	82	8.3	3.3	80
ent D			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
Turning Movement Data	-		App. Total	17	25	42	48	132	28	38	35	24	125		40	38	33	26	137	30	35	55	36	156	40	45	38	44	167	59	48	32	48	187	904		36.5	887
ing M	ı		Peds	3	3	1	2	6	1	0	_	7	6		1	0	1	0	2	3	9	2	1	12	_	3	2	2	8	3	1	1	0	5	45			,
Turn	Street	punc	Right	0	0	2	2	7	0	2	_	1	4		0	1	2	0	3	1	0	2	2	2	0	3	0	_	4	0	1	0	0	1	24	2.7	1.0	23
	Franklin	Westbound	Thru	5	5	11	8	29	7	13	80	5	33		9	8	2	5	24	10	9	12	8	36	10	5	6	10	34	12	8	6	6	38	194	21.5	7.8	190
			Left	12	20	26	38	96	21	23	26	18	88		34	59	26	21	110	19	29	41	26	115	30	37	29	33	129	47	39	23	39	148	989	75.9	27.7	674
			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
•			App. Total	9	11	8	6	34	11	6	6	6	38		7	7	16	15	45	8	10	10	14	42	6	6	8	9	32	13	22	13	20	89	259		10.5	252
			Peds	1	0	0	0	1	3	0	0	0	3		1	0	0	0	1	2	0	0	0	2	0	1	0	0	1	2	0	1	0	3	11			,
	Street	pund	Right	3	5	5	5	18	9	9	6	5	26		4	9	10	8	28	2	5	9	10	26	4	7	3	2	16	8	14	7	14	43	157	9.09	6.3	154
	Franklin Street	Eastbound	Thru	3	4	2	3	12	4	3	0	4	11		3	1	4	5	13	2	2	4	2	10	2	0	3	2	7	4	5	2	3	14	29	25.9	2.7	63
			Left	0	2	1	1	4	1	0	0	0	1		0	0	2	2	4	1	3	0	2	9	3	2	2	2	6	1	3	4	3	11	35	13.5	1.4	35
			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
		F	Start Line	7:00 AM	7:15 AM	7:30 AM	7:45 AM	Hourly Total	8:00 AM	8:15 AM	8:30 AM	8:45 AM	Hourly Total	*** BREAK ***	2:00 PM	2:15 PM	2:30 PM	2:45 PM	Hourly Total	3:00 PM	3:15 PM	3:30 PM	3:45 PM	Hourly Total	4:00 PM	4:15 PM	4:30 PM	4:45 PM	Hourly Total	5:00 PM	5:15 PM	5:30 PM	5:45 PM	Hourly Total	Grand Total	Approach %	Total %	Lights

% Lights		100.0	94.0	98.1		97.3		98.3	97.9	92.8		98.1		9.76	98.0	97.6		97.9		6.06	94.4	87.5		93.8	97.4
Buses	0	0	1	0		1	0	7	1	0		8	0	0	7	0	-	7	0	1	6	0	-	10	26
% Buses	-	0.0	1.5	0.0		0.4	-	1.0	0.5	0.0		6.0		0.0	1.1	0.0	-	0.7		4.5	3.1	0.0	-	3.1	1.1
Single-Unit Trucks	0	0	0	1		1	0	4	0	1		2	0	1	1	4	-	9	0	0	0	2		2	14
% Single-Unit Trucks		0.0	0.0	9.0	,	0.4	,	9.0	0.0	4.2	,	9:0		1.2	0.2	1.4	,	9.0	,	0.0	0.0	12.5		9:0	9.0
Articulated Trucks	0	0	0	0		0	0	0	1	0		1	0	0	0	0	-	0	0	0	0	0	-	0	1
% Articulated Trucks		0.0	0.0	0.0	,	0.0	,	0.0	0.5	0.0	,	0.1		0.0	0.0	0.0	,	0.0	,	0.0	0.0	0.0		0.0	0.0
Bicycles on Road	0	0	3	2		5	0	-	2	0		3	0	1	4	3		8	0	1	7	0		8	24
% Bicycles on Road		0.0	4.5	1.3		1.9		0.1	1.0	0.0		0.3		1.2	0.7	1.0		8.0		4.5	2.4	0.0		2.5	1.0
Pedestrians					11				٠		45				1		38	-					3	-	
% Dodostrians	 -				100 0						1000						1000						1000		



Count Name: Franklin+St+with+Forest+Ave TMC Site Code: Start Date: 08/20/2024 Page No: 3

Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

								Turn	ing M	ovem	ent P	eak F	Turning Movement Peak Hour Data (7:30 AM))ata (7:30 /	(M/									
			Franklin Street	ו Street					Franklin Street	Street				•	Forest Avenue	venue		-			Forest Avenue	enne			
_			Eastbound	puno					Westbound	punc					Northbound	punc					Southbound	pund			
Start Time	U-Tum	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	Int. Total
7:30 AM	0	1	2	5	0	8	0	26	11	5	1	42	0	1	41	17	1	69	0	4	11	1	1	16	125
7:45 AM	0	1	3	5	0	6	0	38	8	2	2	48	0	5	46	8	1	26	0	0	6	1	0	10	126
8:00 AM	0	1	4	9	3	11	0	21	7	0	1	28	0	6	26	9	1	71	0	1	27	2	0	30	140
8:15 AM	0	0	3	9	0	6	0	23	13	2	0	38	0	3	59	7	0	39	0	3	19	0	0	22	108
Total	0	3	12	22	3	37	0	108	39	6	4	156	0	18	172	38	3	228	0	8	99	4	1	78	499
Approach %	0.0	8.1	32.4	59.5		•	0.0	69.2	25.0	5.8		,	0.0	7.9	75.4	16.7			0.0	10.3	84.6	5.1	-		
Total %	0.0	9.0	2.4	4.4		7.4	0.0	21.6	7.8	1.8		31.3	0.0	3.6	34.5	9.7		45.7	0.0	1.6	13.2	8.0	-	15.6	
PHF	0.000	0.750	0.750	0.917		0.841	0.000	0.711	0.750	0.450		0.813	0.000	0.500	0.768	0.559		0.803	0.000	0.500	0.611	0.500		0.650	0.891
Lights	0	3	12	22	-	37	0	105	39	6		153	0	18	169	38		225	0	7	62	3	-	72	487
% Lights		100.0	100.0	100.0		100.0		97.2	100.0	100.0		98.1		100.0	98.3	100.0		98.7	-	87.5	93.9	75.0	-	92.3	97.6
Buses	0	0	0	0		0	0	2	0	0		2	0	0	3	0		3	0	1	4	0		5	10
% Buses		0.0	0.0	0.0	-	0.0		1.9	0.0	0.0		1.3		0.0	1.7	0.0		1.3		12.5	6.1	0.0	-	6.4	2.0
Single-Unit Trucks	0	0	0	0		0	0	1	0	0		1	0	0	0	0		0	0	0	0	1	-	1	2
% Single-Unit Trucks		0.0	0.0	0.0		0.0		6.0	0.0	0.0		9.0		0.0	0.0	0.0		0.0		0.0	0.0	25.0		1.3	0.4
Articulated Trucks	0	0	0	0	-	0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	-	0	0
% Articulated Trucks		0.0	0.0	0.0		0.0		0.0	0.0	0.0		0.0		0.0	0.0	0.0		0.0		0.0	0.0	0.0		0.0	0.0
Bicycles on Road	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0
% Bicycles on Road		0.0	0.0	0.0		0.0		0.0	0.0	0.0		0.0		0.0	0.0	0.0		0.0		0.0	0.0	0.0	-	0.0	0.0
Pedestrians					3						4						3		-				1		
% Pedestrians					100.0						100.0						100.0						100.0		



Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Franklin+St+with+Forest+Ave TMC Site Code: Start Date: 08/20/2024 Page No: 4

								Turn	ing M	ovem	ent P	eak F	Turning Movement Peak Hour Data (5:00 PM)ata (5:00	(Mc									
			Franklin Street	Street					Franklin Street	Street				•	Forest Avenue	venue		-			Forest Avenue	enne			
			Eastbound	puno					Westboun	punc					Northbound	punc					Southbound	pun		-	
Start Time	U-Tum	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. In Total	Int. Total
5:00 PM	0	-	4	8	2	13	0	47	12	0	3	59	0	5	27	21	3	53	0	0	12	1	0	13	138
5:15 PM	0	3	5	14	0	22	0	39	8	1	1	48	0	5	31	13	2	49	0	1	10	0	0	11	130
5:30 PM	0	4	2	7	1	13	0	23	6	0	1	32	0	4	22	14	0	40	0	1	7	0	0	8	93
5:45 PM	0	3	3	14	0	20	0	39	6	0	0	48	0	2	21	7	2	33	0	0	15	0	0	15	116
Total	0	11	14	43	3	89	0	148	38	1	5	187	0	19	101	22	7	175	0	2	44	1	0	47	477
Approach %	0.0	16.2	20.6	63.2		1	0.0	79.1	20.3	0.5		,	0.0	10.9	57.7	31.4		-	0.0	4.3	93.6	2.1	-	-	
Total %	0.0	2.3	2.9	9.0		14.3	0.0	31.0	8.0	0.2		39.2	0.0	4.0	21.2	11.5	-	36.7	0.0	0.4	9.2	0.2	-	6.6	
PHF	0.000	0.688	0.700	0.768		0.773	0.000	0.787	0.792	0.250		0.792	0.000	0.950	0.815	0.655		0.825	0.000	0.500	0.733	0.250	-	0.783	0.864
Lights	0	11	12	43	-	99	0	147	38	1		186	0	19	66	52		170	0	2	43	1	-	46	468
% Lights		100.0	85.7	100.0		97.1		99.3	100.0	100.0		99.5		100.0	98.0	94.5		97.1		100.0	7.76	100.0		97.9	98.1
Buses	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	-	0	0
% Buses		0.0	0.0	0.0		0.0	-	0.0	0.0	0.0		0.0	•	0.0	0.0	0.0		0.0	i	0.0	0.0	0.0	-	0.0	0.0
Single-Unit Trucks	0	0	0	0		0	0	0	0	0		0	0	0	0	1		1	0	0	0	0		0	1
% Single-Unit Trucks	,	0.0	0.0	0.0	,	0.0		0.0	0:0	0.0	,	0:0		0.0	0.0	1.8	,	9.0		0.0	0.0	0.0		0:0	0.2
Articulated Trucks	0	0	0	0		0	0	0	0	0		0	0	0	0	0		0	0	0	0	0	-	0	0
% Articulated Trucks	,	0.0	0.0	0.0	,	0.0	,	0.0	0:0	0.0	,	0.0		0.0	0.0	0.0	,	0.0		0.0	0.0	0.0	,	0:0	0.0
Bicycles on Road	0	0	2	0	,	2	0	1	0	0		1	0	0	2	2		4	0	0	1	0		1	8
% Bicycles on Road		0.0	14.3	0.0		2.9		0.7	0.0	0.0		0.5		0.0	2.0	3.6		2.3		0.0	2.3	0.0		2.1	1.7
Pedestrians	-				3		-				5	-					7	-					0	-	
% Pedestrians					100.0						100.0						100.0								

Kenig Lindgren, O'Hara, Aboona, Inc. Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400 Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Forest Avenue with Access Drive TMC Site Code: Start Date: 08/20/2024 Page No: 1

			Int. Total	59	92	100	110	345	121	88	73	29	349		83	75	85	77	320	99	86	26	92	337	106	88	94	80	369	115	110	73	66	397	2117			2058
			App. Total	19	28	38	20	135	22	46	38	32	171		46	46	48	41	181	29	48	64	42	183	53	53	49	48	203	63	61	33	99	223	1096	1	51.8	1067
			Peds	0	0	0	0	0	0	0	0	0	0		0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	_	,	,	
	enne/	punc	Right	0	0	0	1	1	0	0	_	0	1		1	1	0	0	2	0	1	0	_	2	0	0	0	-	1	0	0	0	1	_	8	0.7	0.4	4
	Forest Avenue	Southbound	Thru	18	28	38	49	133	22	46	37	32	170		44	45	47	41	177	29	45	62	41	177	52	52	49	45	198	63	59	33	64	219	1074	0.86	20.7	1049
			Left	_	0	0	0	1	0	0	0	0	0		1	0	1	0	2	0	2	2	0	4	-	-	0	-	3	0	2	0	1	က	13	1.2	9.0	13
			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	_	1	0	0	0	0	0	_	0.1	0.0	-
			App. Total	40	48	09	58	206	65	42	33	34	174		35	28	35	34	132	36	49	33	32	150	51	36	4	32	163	51	49	39	33	172	266	•	47.1	696
			Peds	0	0	0	0	0	0	0	0	0	0	-	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	2	0	0	2	3	,	,	
	enne	pun	Right	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	-	0	_	_	3	0	0	0	0	0	3	0.3	0.1	က
	Forest Avenue	Northbound	Thru	40	47	09	58	205	64	42	33	34	173		35	27	35	34	131	36	48	33	32	149	50	36	43	30	159	51	49	39	32	171	988	99.1	46.7	961
ata			Left	0	1	0	0	1	1	0	0	0	1		0	0	0	0	0	0	-	0	0	1	0	0	0	_	1	0	0	0	1	-	5	0.5	0.2	4
ent Da			U-Tum	0	0	0	0	0	0	0	0	0	0		0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	_	0.1	0.0	-
Turning Movement Data			App. Total	0	0	1	1	2	1	0	2	0	3		1	1	1	2	5	1	0	0	-	2	2	0	-	0	3	1	0	0	0	-	16	-	9.0	16
ng Mc			Peds	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	3	,	,	
Turni	rive	pur	Right	0	0	1	0	1	1	0	-	0	2		1	1	0	0	2	1	0	0	_	2	-	0	_	0	2	1	0	0	0	_	10	62.5	0.5	10
	Access Drive	Westbound	Thru	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
			Left	0	0	0	1	1	0	0	_	0	1		0	0	1	2	3	0	0	0	0	0	-	0	0	0	1	0	0	0	0	0	9	37.5	0.3	9
			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
			App. Total	0	0	1	1	2	0	0	0	1	1		1	0	1	0	2	0	_	0	-	2	0	0	0	0	0	0	0	1	0	-	8	-	4.0	9
			Peds	1	8	2	0	11	1	9	8	0	10	-	1	1	3	4	6	1	4	4	2	11	7	9	00	25	46	9	4	2	4	16	103	,		
)rive	pur	Right	0	0	1	0	1	0	0	0	1	1		1	0	0	0	1	0	0	0	-	1	0	0	0	0	0	0	0	0	0	0	4	50.0	0.2	4
	Access Drive	Eastbound	Thru	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
			Left	0	0	0	1	1	0	0	0	0	0		0	0	1	0	1	0	_	0	0	1	0	0	0	0	0	0	0	1	0	_	4	50.0	0.2	2
			U-Tum	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
		į	Start Time	7:00 AM	7:15 AM	7:30 AM	7:45 AM	Hourly Total	8:00 AM	8:15 AM	8:30 AM	8:45 AM	Hourly Total	*** BREAK ***	2:00 PM	2:15 PM	2:30 PM	2:45 PM	Hourly Total	3:00 PM	3:15 PM	3:30 PM	3:45 PM	Hourly Total	4:00 PM	4:15 PM	4:30 PM	4:45 PM	Hourly Total	5:00 PM	5:15 PM	5:30 PM	5:45 PM	Hourly Total	Grand Total	Approach %	Total %	Lights

% Lights		50.0		100.0		75.0		100.0		100.0		100.0	100.0	80.0	97.3	100.0		97.2	100.0	100.0	7.76	50.0		97.4	97.2
Buses	0	1	0	0	-	1	0	0	0	0		0	0	0	9	0	-	9	0	0	10	1	-	11	18
% Buses		25.0		0.0	-	12.5	-	0.0		0.0		0.0	0.0	0.0	9.0	0.0	-	9.0	0.0	0.0	6.0	12.5	-	1.0	6.0
Single-Unit Trucks	0	-	0	0		-	0	0	0	0		0	0	-	8	0	,	6	0	0	9	-		7	17
% Single-Unit Trucks		25.0		0.0	,	12.5	,	0.0	,	0.0	,	0.0	0.0	20.0	8.0	0.0	,	6.0	0.0	0.0	9.0	12.5	,	9.0	8.0
Articulated Trucks	0	0	0	0		0	0	0	0	0		0	0	0	-	0		1	0	0	0	0		0	-
% Articulated Trucks		0.0	,	0.0	,	0.0	,	0.0	,	0.0	,	0.0	0.0	0.0	0.1	0.0	,	0.1	0.0	0.0	0.0	0.0	,	0.0	0.0
Bicycles on Road	0	0	0	0		0	0	0	0	0		0	0	0	12	0	-	12	0	0	6	2	-	11	23
% Bicycles on Road		0.0		0.0		0.0		0.0		0.0		0.0	0.0	0:0	1.2	0.0		1.2	0.0	0.0	0.8	25.0		1.0	1.1
Pedestrians			1		103						3				٠		3		ı		-		1		
Carcintock of 70					4000						4000						400						4000		



Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Forest Avenue with Access Drive TMC Site Code: Start Date: 08/20/2024 Page No: 3

_
7:30 AM)
Peak Hour Data
血
Movement
Turning

						-		5				5	יייי יייי שאם ישים יושים	֝֝֝֟֝֝֝֝֝֟֝֝֝֟֝֝֟֝֝֟֝֝֟֝֝֟֝֝֓֓֟֝֝֓֓֓֟֝֝֡֓֓֓֡֝		<u></u>		•							
			Acces	Access Drive					Access Driv	s Drive					Forest Avenue	venue		•			Forest Avenue	venue			
			East	Eastbound					Westbound	punoq					Northbound	pund		-			Southbound	puno			
Start Time	U-Tum	Left	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	U-Tum	Left	Thru	Right	Peds	App. Total	Int. Total
7:30 AM	0	0	0	1	2	1	0	0	0	1	0	1	0	0	09	0	0	09	0	0	38	0	0	38	100
7:45 AM	0	1	0	0	0	1	0	1	0	0	0	1	0	0	58	0	0	28	0	0	49	1	0	20	110
8:00 AM	0	0	0	0	1	0	0	0	0	-	0	1	0	1	64	0	0	65	0	0	22	0	0	22	121
8:15 AM	0	0	0	0	9	0	0	0	0	0	0	0	0	0	42	0	0	42	0	0	46	0	0	46	88
Total	0	1	0	1	6	2	0	1	0	2	0	3	0	1	224	0	0	225	0	0	188	1	0	189	419
Approach %	0.0	50.0	0.0	50.0		-	0.0	33.3	0.0	66.7	1		0.0	0.4	9.66	0.0		-	0.0	0.0	99.5	0.5	-		
Total %	0.0	0.2	0.0	0.2	-	0.5	0.0	0.2	0.0	0.5	,	0.7	0.0	0.2	53.5	0.0	-	53.7	0.0	0.0	44.9	0.2	-	45.1	
PHF	0.000	0.250	0.000	0.250		0.500	0.000	0.250	0.000	0.500		0.750	0.000	0.250	0.875	0.000		0.865	0.000	0.000	0.855	0.250	-	0.859	998.0
Lights	0	0	0	1		1	0	1	0	2		3	0	1	221	0		222	0	0	184	0	-	184	410
% Lights		0.0		100.0		50.0	•	100.0	•	100.0		100.0		100.0	98.7			98.7			6.76	0.0	-	97.4	97.9
Buses	0	-	0	0		1	0	0	0	0		0	0	0	2	0		2	0	0	3	1	-	4	7
% Buses	-	100.0	•	0.0	-	50.0	ı	0.0	•	0.0		0.0	•	0.0	6.0	•		6.0		•	1.6	100.0	-	2.1	1.7
Single-Unit Trucks	0	0	0	0		0	0	0	0	0		0	0	0	_	0		-	0	0	-	0		-	2
% Single-Unit Trucks		0.0		0.0		0.0		0.0		0.0	,	0.0		0.0	0.4			0.4			0.5	0:0		0.5	0.5
Articulated Trucks	0	0	0	0	,	0	0	0	0	0	,	0	0	0	0	0		0	0	0	0	0		0	0
% Articulated Trucks		0.0		0.0		0.0		0.0		0.0		0.0		0.0	0.0			0.0			0.0	0.0	,	0.0	0.0
Bicycles on Road	0	0	0	0		0	0	0	0	0	,	0	0	0	0	0	-	0	0	0	0	0	-	0	0
% Bicycles on Road		0.0		0.0		0.0		0.0		0.0	,	0.0		0.0	0.0	-		0.0			0.0	0.0	-	0.0	0.0
Pedestrians					6						0						0	-					0	-	
% Pedestrians					100.0																				



Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Forest Avenue with Access Drive TMC Site Code: Start Date: 08/20/2024 Page No: 4

			Int. Total	115	110	73	66	397		-	0.863	389	98.0	0	0.0	-	0.3	0	0.0	7	1.8	1	
			App. Total	63	61	33	99	223		56.2	0.845	221	99.1	0	0.0	0	0.0	0	0.0	2	6.0		
			Peds	0	0	0	0	0	-		-	-				٠				-		0	
	Forest Avenue	Southbound	Right	0	0	0	1	1	0.4	0.3	0.250	1	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
	Forest	South	Thru	63	29	33	64	219	98.2	55.2	0.855	217	99.1	0	0.0	0	0.0	0	0.0	2	6.0		
			Left	0	2	0	1	3	1.3	0.8	0.375	3	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
			U-Tum	0	0	0	0	0	0.0	0.0	0.000	0		0		0	•	0	•	0	•	•	
			App. Total	51	49	39	33	172		43.3	0.843	166	96.5	0	0.0	-	9.0	0	0.0	5	2.9		
			Peds	0	2	0	0	2			-	-	-									2	100.0
$\widehat{\mathbb{A}}$	Forest Avenue	Northbound	Right	0	0	0	0	0	0.0	0.0	0.000	0		0		0		0		0			
(5:00)	Forest	North	Thru	51	49	39	32	171	99.4	43.1	0.838	166	97.1	0	0.0	0	0.0	0	0.0	5	2.9		
Data			Left	0	0	0	1	1	9.0	0.3	0.250	0	0.0	0	0.0	-	100.0	0	0.0	0	0.0	٠	
Hour			U-Tum	0	0	0	0	0	0.0	0.0	0.000	0		0		0		0		0	•	•	
Turning Movement Peak Hour Data (5:00 PM)			App. Total	1	0	0	0	1	•	0.3	0.250	1	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
nent			Peds	0	0	0	0	0			-	-								-		0	
Mover	Access Drive	Westbound	Right	1	0	0	0	1	100.0	0.3	0.250	1	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
ning 1	Acce	Wes	Thru	0	0	0	0	0	0.0	0.0	0.000	0		0		0	•	0	•	0	٠		
Tur			Left	0	0	0	0	0	0.0	0.0	0.000	0		0		0		0	•	0	٠		
			U-Turn	0	0	0	0	0	0.0	0.0	0.000	0	•	0	•	0	•	0	•	0	•	•	
			App. Total	0	0	1	0	1	•	0.3	0.250	1	100.0	0	0.0	0	0.0	0	0.0	0	0.0	•	
			Peds	9	4	2	4	16			-	-	-			,				1	'	16	100.0
	Access Drive	Eastbound	Right	0	0	0	0	0	0.0	0.0	0.000	0		0	٠	0	•	0	•	0	•	٠	
	Acce	Eas	Thru	0	0	0	0	0	0.0	0.0	0.000	0		0	•	0	•	0	•	0	٠	•	
			Left	0	0	1	0	1	100.0	0.3	0.250	1	100.0	0	0.0	0	0.0	0	0.0	0	0.0	•	
			U-Tum	0	0	0	0	0	0.0	0.0	0.000	0	•	0	•	0	•	0	•	0	'	•	
			Start Time	5:00 PM	5:15 PM	5:30 PM	5:45 PM	Total	Approach %	Total %	PHF	Lights	% Lights	Buses	% Buses	Single-Unit Trucks	% Single-Unit Trucks	Articulated Trucks	% Articulated Trucks	Bicycles on Road	% Bicycles on Road	Pedestrians	% Pedestrians



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Count Name: Forest Avenue with Warren Avenue TMC Site Code: Start Date: 08/20/2024 Page No: 1

			Int. Total	91	143	172	178	584	173	152	123	115	563		132	134	141	133	540	129	170	183	166	648	199	171	166	184	720	183	179	154	174	069	3745			3647
•	•		App. Total	14	27	36	20	127	50	45	33	26	154	-	43	49	20	48	190	31	44	64	53	192	47	50	52	39	188	54	52	36	64	206	1057		28.2	1022
			Peds	2	3	2	2	6	0	9	1	3	10		4	2	1	2	6	5	1	9	9	18	3	9	9	3	18	2	5	1	2	10	74	1	-	
	enne/	pund	Right	9	9	10	15	37	14	9	5	3	28		10	10	11	5	36	7	5	6	9	27	8	4	8	9	26	2	2	2	5	17	171	16.2	4.6	164
	Forest Avenue	Southbound	Thru	7	20	26	32	85	36	36	25	22	119		31	33	38	40	142	23	38	53	40	154	38	46	43	31	158	49	46	32	26	183	841	9.62	22.5	815
			Left	1	1	0	3	5	0	3	3	1	7		2	9	1	3	12	1	1	2	7	11	1	0	_	2	4	0	1	2	3	9	45	4.3	1.2	43
			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
•			App. Total	46	22	88	78	569	72	62	47	46	227		25	48	44	42	188	22	65	51	53	224	83	61	48	80	272	52	22	58	45	212	1392	•	37.2	1360
			Peds	0	4	0	0	4	0	1	0	0	_	-	_	3	1	0	2	0	0	2	1	3	2	2	_	1	9	0	0	0	0	0	19		-	
	enne	pun	Right	6	9	11	6	35	4	6	7	6	29		9	11	8	8	33	10	10	11	7	38	11	10	6	14	44	2	1	8	8	19	198	14.2	5.3	196
	Forest Avenue	Northbound	Thru	29	34	50	44	157	49	39	29	25	142		24	23	20	29	96	25	35	25	23	108	38	28	28	29	123	37	37	33	29	136	762	54.7	20.3	741
ata			Left	8	17	27	25	77	19	13	11	12	55	-	24	14	16	5	29	20	20	15	23	78	34	23	11	37	105	13	19	17	8	22	431	31.0	11.5	422
ent D			U-Tum	0	0	0	0	0	0	1	0	0	_		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0.1	0.0	-
Turning Movement Data			App. Total	10	15	11	19	22	13	8	15	10	46	-	8	6	6	11	37	7	15	9	10	38	19	6	6	18	55	14	16	8	15	53	284	'	7.6	275
ng M			Peds	2	3	1	3	6	0	11	3	2	19		0	0	1	0	1	0	1	0	2	3	3	2	9	4	15	16	7	0	5	28	75	,	-	
Turni	enne/	pun	Right	2	4	1	5	12	_	1	2	3	7		_	3	2	2	8	0	4	0	2	9	4	1	_	7	13	_	2	0	1	4	20	17.6	1.3	45
	Warren Avenue	Westbound	Thru	1	4	6	6	23	5	5	8	2	20		4	2	1	5	12	4	4	3	3	14	7	5	9	8	56	9	5	2	8	21	116	40.8	3.1	113
			Left	7	7	1	5	20	7	2	5	5	19		3	4	9	4	17	3	7	3	5	18	8	3	2	3	16	7	6	9	9	28	118	41.5	3.2	117
			U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
			App. Total	21	44	37	31	133	38	37	28	33	136	-	27	28	38	32	125	36	46	62	20	194	20	51	22	47	205	63	54	52	20	219	1012	'	27.0	066
			Peds	0	6	0	2	11	0	3	1	0	4	-	_	2	3	3	6	2	5	5	0	12	9	5	16	16	43	7	1	2	4	14	93	,	-	
	enne/	pur	Right	10	21	18	20	69	14	22	20	22	78		17	16	23	21	22	26	25	48	40	139	37	36	40	31	144	45	34	34	32	145	652	64.4	17.4	639
	Warren Avenue	Eastbound	Thru	3	14	8	4	59	7	10	9	7	30		3	6	3	9	21	4	10	2	9	25	2	8	9	11	27	12	12	13	11	48	180	17.8	4.8	176
			Left	8	6	11	7	35	17	2	2	4	28		7	3	12	5	27	9	11	6	4	30	11	7	11	2	34	9	8	5	7	26	180	17.8	4.8	175
			U-Tum	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0
		į	Start Time	7:00 AM	7:15 AM	7:30 AM	7:45 AM	Hourly Total	8:00 AM	8:15 AM	8:30 AM	8:45 AM	Hourly Total	*** BREAK ***	2:00 PM	2:15 PM	2:30 PM	2:45 PM	Hourly Total	3:00 PM	3:15 PM	3:30 PM	3:45 PM	Hourly Total	4:00 PM	4:15 PM	4:30 PM	4:45 PM	Hourly Total	5:00 PM	5:15 PM	5:30 PM	5:45 PM	Hourly Total	Grand Total	Approach %	Total %	Lights

% Lights		97.2	97.8	98.0	,	97.8		99.2	97.4	90.0		96.8	100.0	97.9	97.2	99.0		97.7		92.6	6.96	95.9		96.7	97.4
Buses	0	0	0	2		2	0	0	1	0		1	0	2	7	-	-	10	0	0	12	2	-	14	27
% Buses	•	0.0	0.0	0.3		0.2		0.0	6.0	0.0		0.4	0.0	0.5	6.0	0.5	1	0.7	-	0.0	1.4	1.2	-	1.3	0.7
Single-Unit Trucks	0	3	4	-		8	0	0	~	2		3	0	3	4	0		7	0	0	2	2		10	28
% Single-Unit Trucks		1.7	2.2	0.2	,	0.8		0:0	6:0	4.0	,	1.1	0.0	2.0	0.5	0.0	,	0.5		0.0	9:0	2.9	,	6:0	0.7
Articulated Trucks	0	1	0	1		2	0	1	0	0		1	0	0	1	0	1	1	0	0	2	0	-	2	9
% Articulated Trucks		9.0	0.0	0.2	,	0.2		8:0	0.0	0.0	,	9.0	0.0	0.0	0.1	0.0	,	0.1		0.0	0.2	0.0		0.2	0.2
Bicycles on Road	0	-	0	6		10	0	0	1	3		4	0	4	6	1	-	14	0	2	7	0	-	6	37
% Bicycles on Road		9.0	0.0	4.1		1.0		0:0	6:0	0.9		4.1	0:0	6:0	1.2	0.5	,	1.0		4.4	8:0	0.0		6.0	1.0
Pedestrians					93	-					75			1			19	-					74		
% Dodoctrion					1000						1000						1000						1000		



Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Forest Avenue with Warren Avenue TMC Site Code: Start Date: 08/20/2024 Page No: 3

			Int. Total	172	178	173	152	675			0.948	657	97.3	10	1.5	3	9.0	-	0.1	4	9.0	.	
-	•		App. Total	36	20	20	45	181		26.8	0.905	171	94.5	5	2.8	2	1.1	1	9.0	2	1.1		-
			Peds	2	2	0	9	10	,		-	-	-	-	,	-					-	10	100.0
	venue	puno	Right	10	15	14	9	45	24.9	6.7	0.750	43	92.6	1	2.2	1	2.2	0	0.0	0	0.0		
	Forest Avenue	Southbound	Thru	56	32	36	36	130	71.8	19.3	0.903	122	93.8	4	3.1	1	8.0	1	0.8	2	1.5		
			Left	0	3	0	3	9	3.3	6.0	0.500	9	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
			U-Tum	0	0	0	0	0	0.0	0.0	0.000	0	-	0		0		0		0		-	
			App. Total	88	78	72	62	300		44.4	0.852	294	98.0	4	1.3	1	0.3	0	0.0	-	0.3	-	-
			Peds	0	0	0	1	1	٠		-	-									-	_	100.0
AM)	Forest Avenue	Northbound	Right	11	6	4	6	33	11.0	4.9	0.750	33	100.0	0	0.0	0	0.0	0	0.0	0	0.0		-
Turning Movement Peak Hour Data (7:30 AM)	Forest	North	Thru	20	44	49	39	182	60.7	27.0	0.910	179	98.4	2	1.1	1	0.5	0	0.0	0	0.0		
Data			Left	27	25	19	13	84	28.0	12.4	0.778	81	96.4	2	2.4	0	0.0	0	0.0	_	1.2		
Hour			U-Tum	0	0	0	1	1	0.3	0.1	0.250	1	100.0	0	0.0	0	0.0	0	0.0	0	0.0		'
Peak			App. Total	11	19	13	8	51		9.7	0.671	51	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
ment			Peds	1	3	0	11	15	,	1	1	-			,						-	15	100.0
Move	Warren Avenue	Westbound	Right	1	5	1	1	8	15.7	1.2	0.400	8	100.0	0	0.0	0	0.0	0	0.0	0	0.0		'
ning I	Warre	Wes	Thru	6	6	2	5	28	54.9	4.1	0.778	28	100.0	0	0.0	0	0.0	0	0.0	0	0.0		-
Ţ			Left	1	5	7	2	15	29.4	2.2	0.536	15	100.0	0	0.0	0	0.0	0	0.0	0	0.0		•
			U-Turn	0	0	0	0	0	0.0	0.0	0.000	0	•	0	•	0	•	0	•	0	-	•	
			App. Total	37	31	38	37	143	٠	21.2	0.941	141	98.6	1	0.7	0	0.0	0	0.0	-	0.7	•	
			Peds	0	2	0	3	5	•		-	-					•	-		1	-	5	100.0
	Warren Avenue	Eastbound	Right	18	20	14	22	74	51.7	11.0	0.841	72	97.3	1	1.4	0	0.0	0	0.0	-	1.4	•	•
	Warre	Ea	Thru	8	4	7	10	29	20.3	4.3	0.725	29	100.0	0	0.0	0	0.0	0	0.0	0	0.0	•	'
			n Left	11	7	17	5	40	28.0	5.9	0.588	40	100.0	0	0.0	0	0.0	0	0.0	0	0.0		'
			U-Tum	0	0	0	0	0	0.0	0.0	0.000	0	•	0	•	0 s	'	0 s	•	0	•	•	'
			Start Time	7:30 AM	7:45 AM	8:00 AM	8:15 AM	Total	Approach %	Total %	PHF	Lights	% Lights	Buses	% Buses	Single-Unit Trucks	% Single-Unit Trucks	Articulated Trucks	% Articulated Trucks	Bicycles on Road	% Bicycles on Road	Pedestrians	% Pedestrians



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Count Name: Forest Avenue with Warren Avenue TMC Site Code: Start Date: 08/20/2024 Page No: 4

			Int. Total	183	179	154	174	069			0.943	929	98.0	0	0.0	3	0.4	0	0.0	11	1.6		
			App. Total	54	52	36	64	206	,	29.9	0.805	201	9.76	0	0.0	2	1.0	0	0.0	3	1.5		
			Peds	2	5	1	2	10	-	-	-	-	-		,	,	-	,		-		10	100.0
	/enne	punc	Right	2	2	2	5	17	8.3	2.5	0.850	17	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
	Forest Avenue	Southbound	Thru	49	46	32	26	183	88.8	26.5	0.817	178	97.3	0	0.0	2	1.1	0	0.0	3	1.6		
			Left	0	1	2	3	9	2.9	6.0	0.500	9	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
			U-Turn	0	0	0	0	0	0.0	0.0	0.000	0		0		0		0		0			
			App. Total	25	22	28	45	212	-	30.7	0.914	205	96.7	0	0.0	-	0.5	0	0.0	9	2.8		-
			Peds	0	0	0	0	0	-	-	-	-	-	-		,	-			-		0	-
PM)	venue	puno	Right	2	1	8	8	19	9.0	2.8	0.594	18	94.7	0	0.0	0	0.0	0	0.0	1	5.3		
5:00	Forest Avenue	Northbound	Thru	37	37	33	29	136	64.2	19.7	0.919	131	96.3	0	0.0	-	0.7	0	0.0	4	2.9		
)ata (Left	13	19	17	8	22	26.9	8.3	0.750	26	98.2	0	0.0	0	0.0	0	0.0	1	1.8		
Juot			U-Tum	0	0	0	0	0	0.0	0.0	0.000	0		0		0		0		0			
Turning Movement Peak Hour Data (5:00 PM)			App. Total	14	16	8	15	53	-	7.7	0.828	52	98.1	0	0.0	0	0.0	0	0.0	1	1.9		
ent P			Peds	16	7	0	5	28	-	-	-	-					-				-	28	100.0
loven	Avenue	puno	Right	1	2	0	1	4	7.5	9.0	0.500	3	75.0	0	0.0	0	0.0	0	0.0	1	25.0		
ing M	Warren Avenue	Westbound	Thru	9	2	2	8	21	39.6	3.0	0.656	21	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
Turn			Left	7	6	9	9	28	52.8	4.1	0.778	28	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
			U-Turn	0	0	0	0	0	0.0	0.0	0.000	0		0		0		0		0			
·			App. Total	63	54	52	50	219	-	31.7	0.869	218	99.5	0	0.0	0	0.0	0	0.0	1	0.5		
			Peds	7	1	2	4	14	-	-	-	-	-	-	,	,	-	,				14	100.0
	Warren Avenue	Eastbound	Right	45	34	34	32	145	66.2	21.0	908.0	144	99.3	0	0.0	0	0.0	0	0.0	1	0.7		
	Warren	East	Thru	12	12	13	11	48	21.9	7.0	0.923	48	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
			Left	9	8	5	7	26	11.9	3.8	0.813	26	100.0	0	0.0	0	0.0	0	0.0	0	0.0		
			U-Tum	0	0	0	0	0	0.0	0.0	0.000	0		0		0	•	0	•	0	•	•	
			Start Time	5:00 PM	5:15 PM	5:30 PM	5:45 PM	Total	Approach %	Total %	PHF	Lights	% Lights	Buses	% Buses	Single-Unit Trucks	% Single-Unit Trucks	Articulated Trucks	% Articulated Trucks	Bicycles on Road	% Bicycles on Road	Pedestrians	% Pedestrians



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Count Name: Franklin Street with Access Drive TMC Site Code: Start Date: 08/20/2024 Page No: 1

Turning Movement Data

_		App. Total Int. Total	0 37	1 51	2 72		6 219	0 43	4 50	5 54	5 42	14 189		3 58	3 51		3 44	13 206	3 56			5 56	23 238		7 58		3 64		3 84		3 54		8 264	80 1357		5.9		100.0
		Peds A	0	4	2	4	10	4	1	2	1	8	-	1	0	2	3	9	3	2	0	1	9	4	3	3	4	14	2	2	3	2	6	53				,
Access Drive	Northbound	Right	0	1	2	1	4	0	0	3	3	9	•	1	2	3	1	7	3	3	4	2	12	0	3	3	1	7	3	0	2	0	5	41	51.3	3.0	41	100.0
		Left	0	0	0	2	2	0	4	2	2	8	-	2	1	1	2	9	0	2	9	3	11	1	4	2	2	6	0	1	1	1	3	39	48.8	2.9	39	100.0
		U-Turn	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0	
_		App. Total	17	27	48	45	137	31	31	36	24	122	-	41	34	34	25	134	34	34	42	38	148	38	41	38	46	163	56	49	33	49	187	891		65.7	870	97.6
		Peds	0	2	11	1	14	0	0	0	0	0	-	0	0	0	0	0	1	1	0	0	2	0	0	5	0	5	0	1	0	0	1	22				
Eranklin Street	Westbound	Thru	17	25	46	42	130	29	29	35	21	114		35	32	30	24	121	34	34	42	37	147	36	41	38	44	159	26	48	32	45	181	852	95.6	62.8	832	7.76
Franklin Street		Left	0	2	2	3	7	2	2	1	3	8	-	9	2	4	0	12	0	0	0	1	1	2	0	0	2	4	0	1	1	3	5	37	4.2	2.7	36	97.3
5		U-Turn	0	0	0	0	0	0	0	0	0	0	-	0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1	2	0.2	0.1	2	100.0
	•	App. Total	20	23	22	11	92	12	15	13	13	53	•	14	14	15	16	59	19	20	15	13	67	20	10	17	15	62	25	16	18	10	69	386		28.4	367	95.1
		Peds	0	0	0	0	0	0	0	0	0	0	-	0	0	0	0	0	1	_	0	0	2	0	0	1	0	1	0	1	0	0	1	4				
Franklin Stroot	Eastbound	Right	0	0	1	1	2	0	1	0	1	2	-	1	0	0	1	2	0	1	0	1	2	1	0	0	0	1	0	1	2	0	3	12	3.1	6.0	11	91.7
		Thru	20	23	21	10	74	12	14	13	12	51		13	14	15	15	22	19	19	15	12	92	19	10	17	15	61	25	15	16	10	99	374	6.96	27.6	356	95.2
_		U-Tum	0	0	0	0	0	0	0	0	0	0		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0.0	0.0	0	,
	į	Start Time	7:00 AM	7:15 AM	7:30 AM	7:45 AM	Hourly Total	8:00 AM	8:15 AM	8:30 AM	8:45 AM	Hourly Total	*** BREAK ***	2:00 PM	2:15 PM	2:30 PM	2:45 PM	Hourly Total	3:00 PM	3:15 PM	3:30 PM	3:45 PM	Hourly Total	4:00 PM	4:15 PM	4:30 PM	4:45 PM	Hourly Total	5:00 PM	5:15 PM	5:30 PM	5:45 PM	Hourly Total	Grand Total	Approach %	Total %	Lights	% Lights

Buses	0	2	0	-	2	0	0	5	-	5	0	0	0	-	0	7
% Buses	•	0.5	0.0	1	0.5	0.0	0.0	9.0	-	9.0	•	0.0	0.0		0.0	0.5
Single-Unit Trucks	0	8	0	-	8	0	0	7	-	7	0	0	0	-	0	15
% Single-Unit Trucks	-	2.1	0.0	1	2.1	0.0	0.0	8.0	1	0.8		0.0	0.0	-	0.0	1.1
Articulated Trucks	0	0	0	1	0	0	0	2	1	2	0	0	0		0	2
% Articulated Trucks	-	0.0	0.0	-	0.0	0.0	0.0	0.2	-	0.2	•	0.0	0.0	-	0.0	0.1
Bicycles on Road	0	8	7	1	6	0	7	9	1	7	0	0	0	-	0	16
% Bicycles on Road	-	2.1	8.3	1	2.3	0.0	2.7	0.7	1	0.8		0.0	0.0		0.0	1.2
Pedestrians	-	-		4	-	•	•	•	22	-	•	•	•	53	-	-
% Pedestrians				100.0					100.0					100.0		-



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Count Name: Franklin Street with Access Drive TMC Site Code: Start Date: 08/20/2024 Page No: 3

					lurning	urning Movement Peak Hour Data (7:30 AM	ent Pea	ik Hour I	Jata (/ :∖	30 AM)						
			Franklin Street			L .		Franklin Street	•	•			Access Drive			
Ë			Eastbound			_		Westbound					Northbound			
Start Time	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Left	Right	Peds	App. Total	Int. Total
7:30 AM	0	21	-	0	22	0	2	46	11	48	0	0	2	2	2	72
7:45 AM	0	10	_	0	11	0	3	42	1	45	0	2	1	4	3	59
8:00 AM	0	12	0	0	12	0	2	29	0	31	0	0	0	4	0	43
8:15 AM	0	14	-	0	15	0	2	29	0	31	0	4	0	1	4	20
Total	0	22	3	0	09	0	6	146	12	155	0	9	3	11	6	224
Approach %	0.0	95.0	5.0		•	0.0	5.8	94.2	-	-	0.0	2.99	33.3	-	-	-
Total %	0.0	25.4	1.3		26.8	0.0	4.0	65.2	-	69.2	0.0	2.7	1.3		4.0	
PHF	0.000	0.679	0.750		0.682	0.000	0.750	0.793	-	0.807	0.000	0.375	0.375	1	0.563	0.778
Lights	0	54	3		22	0	6	144	-	153	0	9	3	-	9	219
% Lights	•	94.7	100.0		95.0	-	100.0	98.6	-	7.86		100.0	100.0		100.0	8.76
Buses	0	_	0		1	0	0	_	-	1	0	0	0	1	0	2
% Buses	,	1.8	0.0		1.7		0.0	0.7	,	9.0	,	0.0	0.0	,	0.0	6:0
Single-Unit Trucks	0	2	0	-	2	0	0	-	-	-	0	0	0	-	0	3
% Single-Unit Trucks		3.5	0.0		3.3	-	0.0	0.7	-	9.0	•	0.0	0.0	1	0.0	1.3
Articulated Trucks	0	0	0		0	0	0	0		0	0	0	0	,	0	0
% Articulated Trucks	•	0.0	0.0		0.0		0.0	0.0		0.0	,	0.0	0.0		0.0	0.0
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0		0	0
% Bicycles on Road		0.0	0.0		0.0	-	0.0	0.0	-	0.0	•	0.0	0.0	-	0.0	0.0
Pedestrians		•	•	0	•	-	•	•	12	•	•	-	•	11	•	-
% Pedestrians					•				100.0					100.0		_



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Count Name: Franklin Street with Access Drive TMC Site Code: Start Date: 08/20/2024 Page No: 4

					Turning	Movem	nent Pea	Turning Movement Peak Hour Data (5:00 PM))ata (5:0	00 PM)						
			Franklin Street		•		-	Franklin Street		•			Access Drive			
i i i i i i i i i i i i i i i i i i i			Eastbound					Westbound					Northbound			
Start IIme	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Tum	Left	Right	Peds	App. Total	Int. Total
5:00 PM	0	25	0	0	25	0	0	56	0	56	0	0	3	2	3	84
5:15 PM	0	15	1	1	16	0	1	48	1	49	0	1	0	2	1	99
5:30 PM	0	16	2	0	18	0	1	32	0	33	0	1	2	3	3	54
5:45 PM	0	10	0	0	10	1	3	45	0	49	0	1	0	2	1	09
Total	0	99	3	1	69	1	5	181	1	187	0	3	5	6	8	264
Approach %	0.0	95.7	4.3	-	-	0.5	2.7	96.8		-	0.0	37.5	62.5	-	-	i
Total %	0.0	25.0	1.1	-	26.1	0.4	1.9	68.6		70.8	0.0	1.1	1.9	-	3.0	
PHF	0.000	0.660	0.375		0.690	0.250	0.417	0.808		0.835	0.000	0.750	0.417		0.667	0.786
Lights	0	09	3	1	63	1	5	178		184	0	3	5	1	8	255
% Lights	-	6.06	100.0	-	91.3	100.0	100.0	98.3		98.4	-	100.0	100.0	-	100.0	9.96
Buses	0	0	0	-	0	0	0	0		0	0	0	0	-	0	0
% Buses	-	0.0	0.0	-	0.0	0.0	0.0	0.0		0.0	-	0.0	0.0	-	0.0	0.0
Single-Unit Trucks	0	0	0	-	0	0	0	1	-	1	0	0	0	-	0	1
% Single-Unit Trucks	-	0.0	0.0	-	0.0	0.0	0.0	9.0		0.5	•	0.0	0.0	-	0.0	0.4
Articulated Trucks	0	0	0	1	0	0	0	0		0	0	0	0	-	0	0
% Articulated Trucks	-	0.0	0.0	-	0.0	0.0	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	0.0
Bicycles on Road	0	9	0	1	9	0	0	2	1	2	0	0	0	-	0	8
% Bicycles on Road		9.1	0.0	1	8.7	0.0	0.0	1.1		1.1		0.0	0.0	-	0.0	3.0
Pedestrians	-			1	-	-	-	-	1	-	-	-	-	6	-	
% Pedestrians	-		•	100.0	•	•	-	•	100.0	-	•		•	100.0		

Study Name Main Street with Access Drive TMC
Start Date Tuesday, August 20, 2024 7:00 AM
End Date Tuesday, August 20, 2024 6:00 PM
Site Code

Report Summary

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Peak 1	Specified Period 7:30 AM - 8:30 AM		One Hour Peak										- '	- '		- '						- '		



Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Main+St+with+Rogers+St TMC Site Code: Start Date: 08/20/2024 Page No: 1

		Int. Total	131	147	206	162	646	191	192	190	176	749		187	174	182	163	706	140	180	163	170	653	198	187	200	182	767	202	204	225	228	859	4380			4252	97.1
-	,	App. Total	37	44	61	22	197	61	85	09	52	258		91	83	87	84	345	61	93	82	94	330	106	98	98	97	399	91	106	117	116	430	1959	-	44.7	1895	2.96
		Peds	_	0	2	2	5	0	1	1	0	2	-	2	1	1	3	7	0	3	2	4	6	2	9	0	0	8	0	5	1	2	8	39	-			
	Main Street	Thru	30	37	22	51	175	49	72	57	45	223	-	85	73	79	78	315	50	06	78	93	311	102	83	79	89	353	79	95	109	101	384	1761	89.9	40.2	1701	9.96
		Left	7	9	4	4	21	12	12	3	7	34	-	9	10	8	5	29	11	3	4	1	19	4	15	19	8	46	12	11	8	15	46	195	10.0	4.5	191	97.9
		U-Tum	0	_	0	0	1	0	1	0	0	-		0	0	0	1	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0.2	0.1	3	100.0
-		App. Total	77	81	121	83	362	106	86	105	106	403	-	80	75	76	68	299	64	72	58	62	256	75	68	80	62	285	91	77	79	77	324	1929	-	44.0	1877	97.3
ata		Peds	0	0	2	2	4	0	0	0	0	0	-	2	1	0	3	9	5	3	2	3	13	0	1	0	4	5	2	0	0	0	2	30	-			
ement [Main Street	Right	4	9	8	3	21	3	3	4	8	13	-	2	4	7	5	18	2	2	_	9	11	9	8	9	5	25	7	4	3	7	21	109	5.7	2.5	104	95.4
Irning Movement Data		Thru	73	75	113	80	341	103	83	100	103	389	-	78	71	69	63	281	62	70	57	56	245	69	09	74	57	260	84	73	76	70	303	1819	94.3	41.5	1772	97.4
Turn		U-Turn	0	0	0	0	0	0	0	1	0	-		0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	-	0.1	0.0	_	100.0
-		App. Total	17	22	24	24	87	24	21	25	18	88	-	16	16	19	11	62	15	15	23	14	29	17	21	22	23	83	20	21	29	35	105	492	-	11.2	480	97.6
		Peds	_	1	3	8	13	5	3	1	_	10	-	0	2	3	1	9	2	3	2	2	6	3	3	33	7	46	3	9	5	1	15	66	-		1	
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-		Start Time	7:00 AM	7:15 AM	7:30 AM	7:45 AM	Hourly Total	8:00 AM	8:15 AM	8:30 AM	8:45 AM	Hourly Total	*** BREAK ***	2:00 PM	2:15 PM	2:30 PM	2:45 PM	Hourly Total	3:00 PM	3:15 PM	3:30 PM	3:45 PM	Hourly Total	4:00 PM	4:15 PM	4:30 PM	4:45 PM	Hourly Total	5:00 PM	5:15 PM	5:30 PM	5:45 PM	Hourly Total	Grand Total	Approach %	Total %	Lights	% Lights

Buses	0	0	2	,	2	0	23	-	,	24	0	2	17	,	19	48
% Buses	0.0	0.0	1.3	-	1.0	0.0	1.3	6.0	-	1.2	0.0	1.0	1.0	1	1.0	1.1
Single-Unit Trucks	0	2	3		5	0	20	2		22	0	2	33		35	62
% Single-Unit Trucks	0.0	2.1	0.8	-	1.0	0.0	1.1	1.8		1.1	0.0	1.0	1.9	1	1.8	1.4
Articulated Trucks	0	0	0		0	0	4	-	1	5	0	0	7		7	12
% Articulated Trucks	0.0	0.0	0.0	,	0.0	0.0	0.2	6.0	,	0.3	0.0	0.0	0.4	,	0.4	0.3
Bicycles on Road	0	0	2		2	0	0	-	,	-	0	0	က		3	9
% Bicycles on Road	0.0	0.0	0.5		0.4	0.0	0.0	6.0	,	0.1	0.0	0.0	0.2		0.2	0.1
Pedestrians			•	66		-	•	•	30		•	•	•	39	-	•
% Pedestrians				100 0					100 0					100 0		



Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Main+St+with+Rogers+St TMC Site Code: Start Date: 08/20/2024 Page No: 3

					Turning		nent Pea	Movement Peak Hour Data (7:30 AM))ata (7∷	30 AM)						
			Rogers Street					Main Street		•			Main Street			
i i i			Westbound					Northbound					Southbound			
Start Line	U-Tum	Left	Right	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	Int. Tota
7:30 AM	0	7	17	3	24	0	113	8	2	121	0	4	22	2	61	206
7:45 AM	0	9	18	8	24	0	80	3	2	83	0	4	51	2	55	162
8:00 AM	0	2	22	5	24	0	103	3	0	106	0	12	49	0	61	191
8:15 AM	0	2	19	3	21	0	83	3	0	86	1	12	72	1	85	192
Total	0	17	92	19	93	0	379	17	4	396	1	32	229	5	262	751
Approach %	0.0	18.3	81.7	-	-	0.0	95.7	4.3	-		0.4	12.2	87.4		-	•
Total %	0.0	2.3	10.1	-	12.4	0.0	50.5	2.3	-	52.7	0.1	4.3	30.5		34.9	•
PHF	0.000	0.607	0.864		0.969	0.000	0.838	0.531	-	0.818	0.250	0.667	0.795		0.771	0.911
Lights	0	17	72	,	89	0	369	17	,	386	1	31	217		249	724
% Lights	,	100.0	94.7		95.7		97.4	100.0	'	97.5	100.0	6.96	94.8		95.0	96.4
Buses	0	0	3	-	3	0	3	0	-	3	0	1	9	_	7	13
% Buses		0.0	3.9	,	3.2		0.8	0.0	,	0.8	0.0	3.1	2.6		2.7	1.7
Single-Unit Trucks	0	0	-	,	_	0	9	0	'	9	0	0	5		5	12
% Single-Unit Trucks	-	0.0	1.3	-	1.1		1.6	0.0	-	1.5	0.0	0.0	2.2	_	1.9	1.6
Articulated Trucks	0	0	0	,	0	0	_	0	,	1	0	0	1		1	2
% Articulated Trucks	-	0.0	0.0	-	0.0	-	0.3	0.0		0.3	0.0	0.0	0.4	_	0.4	0.3
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	0	0	_	0	0
% Bicycles on Road		0.0	0.0	-	0.0		0.0	0.0	,	0.0	0.0	0.0	0.0		0.0	0.0
Pedestrians	-	-	_	19	-	-	-	-	4	-		-	-	5	-	•
% Pedestrians	,	,	'	100.0					100.0	,				100.0		٠



Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Main+St+with+Rogers+St TMC Site Code: Start Date: 08/20/2024 Page No: 4

					Turning	Moven	Turning Movement Peak Hour Data (5:00 PM)	ik Hour [Data (5:	00 PM)						
			Rogers Street		,			Main Street	•				Main Street			
E tacto			Westbound					Northbound					Southbound			
Start Line	U-Turn	Left	Right	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	Int. Total
5:00 PM	0	3	17	3	20	0	84	7	2	91	0	12	79	0	91	202
5:15 PM	0	3	18	9	21	0	73	4	0	77	0	11	95	5	106	204
5:30 PM	2	5	22	5	29	0	92	3	0	79	0	8	109	1	117	225
5:45 PM	0	0	35	1	35	0	70	7	0	77	0	15	101	2	116	228
Total	2	11	92	15	105	0	303	21	2	324	0	46	384	8	430	859
Approach %	1.9	10.5	87.6	-	-	0.0	93.5	6.5		-	0.0	10.7	89.3	-	-	•
Total %	0.2	1.3	10.7	-	12.2	0.0	35.3	2.4	-	37.7	0.0	5.4	44.7	-	50.1	٠
PHF	0.250	0.550	0.657	1	0.750	0.000	0.902	0.750		0.890	0.000	0.767	0.881	-	0.919	0.942
Lights	2	11	92	-	105	0	302	21		323	0	46	378	-	424	852
% Lights	100.0	100.0	100.0	-	100.0		2.66	100.0		99.7	-	100.0	98.4	-	98.6	99.2
Buses	0	0	0		0	0	0	0	-	0	0	0	1	-	1	1
% Buses	0.0	0.0	0.0	-	0.0	•	0.0	0.0		0.0	-	0.0	0.3	-	0.2	0.1
Single-Unit Trucks	0	0	0	-	0	0	0	0		0	0	0	1	-	1	1
% Single-Unit Trucks	0.0	0.0	0.0	-	0.0	-	0.0	0.0		0.0	-	0.0	0.3	-	0.2	0.1
Articulated Trucks	0	0	0		0	0	1	0	-	1	0	0	_	-	1	2
% Articulated Trucks	0.0	0.0	0.0		0.0	-	0.3	0.0	-	0.3	-	0.0	0.3	-	0.2	0.2
Bicycles on Road	0	0	0	1	0	0	0	0	-	0	0	0	3	-	3	3
% Bicycles on Road	0.0	0.0	0.0	1	0.0		0.0	0.0		0.0		0.0	0.8	-	0.7	0.3
Pedestrians	-	-	-	15	-	-	-		2	-	-	-	-	8	-	
% Dodostriono				400 0					4000					100 0		

Kenig, Lindgren, O'Hara, Aboona, Inc. 9575 W. Higgins Rd., Suite 400

9575 W. Higgins Rd., Suite 400 Rosemont, Illinois, United States 60018 (847)518-9990 bmay@kloainc.com

Count Name: Public Alley South of Franklin - Full Site Code: Start Date: 08/20/2024 Page No: 1

Direction (Southbound)						
Start Time	Lights	Buses	Single-Unit Trucks	Articulated Trucks	Bicycles on Road	Total
08/20/2024 12:00 AM	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0
12:45 AM	0	0	0	0	0	0
1:00 AM	0	0	0	0	0	0
1:15 AM	0	0	0	0	0	0
1:30 AM	0	0	0	0	0	0
1:45 AM	0	0	0	0	0	0
2:00 AM	0	0	0	0	0	0
2:15 AM	0	0	0	0	0	0
2:30 AM	0	0	0	0	0	0
2:45 AM	0	0	0	0	0	0
3:00 AM	0	0	0	0	0	0
3:15 AM	0	0	0	0	0	0
3:30 AM	0	0	0	0	0	0
3:45 AM	0	0	0	0	0	0
4:00 AM	0	0	0	0	0	0
4:15 AM	0	0	0	0	0	0
4:30 AM	0	0	0	0	0	0
4:45 AM	0	0	0	0	0	0
5:00 AM	0	0	0	0	0	0
5:15 AM	0	0	0	0	0	0
5:30 AM	0	0	0	0	0	0
5:45 AM	0	0	0	0	0	0
6:00 AM	0	0	0	0	0	0
6:15 AM	0	0	0	0	0	0
6:30 AM	0	0	0	0	0	0
6:45 AM	1	0	1	0	0	2
7:00 AM	0	0	0	0	0	0
7:15 AM	2	0	0	0	0	2
7:30 AM	3	0	0	0	0	3
7:45 AM	4	0	0	0	0	4
8:00 AM	2	0	0	0	0	2
8:15 AM	3	0	0	0	0	3
8:30 AM	1	0	0	0	0	1
8:45 AM	4	0	0	0	0	4
9:00 AM	5	0	0	0	0	5
9:15 AM	4	0	0	0	0	4
9:30 AM	2	0	0	0	0	2

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10:00 AM 10:30 AM 10:30 AM 11:15 AM 11:

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0	0	0	0	0	1	1.0	6:00 AM	1	12:00 PM	C
0	0	0	0	0	0	0.0	12:00 AM	0	12:00 PM	C
0	0	0	0	0	93	6.96	8:30 AM	14	1:45 PM	73
10:45 PM	11:00 PM	11:15 PM	11:30 PM	11:45 PM	Total	Total %	AM Times	AM Peaks	PM Times	SASS MO

Rosemont, Illinois, United States 60018 (847)518-9990 bmay@kloainc.com Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Direction (Northbound)

Count Name: Public Alley South of Franklin - Full Site Code: Start Date: 08/20/2024 Page No: 4

rthbound)						
Start Time	Lights	Buses	Single-Unit Trucks	Articulated Trucks	Bicycles on Road	Total
08/20/2024 12:00 AM	0	0	0	0	0	0
12:15 AM	0	0	0	0	0	0
12:30 AM	0	0	0	0	0	0
12:45 AM	0	0	0	0	0	0
1:00 AM	0	0	0	0	0	0
1:15 AM	0	0	0	0	0	0
1:30 AM	0	0	0	0	0	0
1:45 AM	0	0	0	0	0	0
2:00 AM	0	0	0	0	0	0
2:15 AM	0	0	0	0	0	0
2:30 AM	0	0	0	0	0	0
2:45 AM	0	0	0	0	0	0
3:00 AM	0	0	0	0	0	0
3:15 AM	0	0	0	0	0	0
3:30 AM	0	0	0	0	0	0
3:45 AM	0	0	0	0	0	0
4:00 AM	0	0	0	0	0	0
4:15 AM	0	0	0	0	0	0
4:30 AM	0	0	0	0	0	0
4:45 AM	0	0	0	0	0	0
5:00 AM	0	0	0	0	0	0
5:15 AM	0	0	0	0	0	0
5:30 AM	0	0	0	0	0	0
5:45 AM	0	0	0	0	0	0
6:00 AM	0	0	0	0	0	0
6:15 AM	0	0	0	0	0	0
6:30 AM	0	0	0	0	0	0
6:45 AM	0	0	1	0	0	1
7:00 AM	0	0	0	0	0	0
7:15 AM	1	0	0	0	0	٢
7:30 AM	2	0	0	0	0	2
7:45 AM	က	0	0	0	0	3
8:00 AM	0	0	0	0	0	0
8:15 AM	4	0	0	0	0	4
8:30 AM	5	0	0	0	0	5
8:45 AM	5	0	0	0	0	5
9:00 AM	5	0	0	0	0	5
9:15 AM	4	0	0	0	0	4
9:30 AM	1	0	0	0	0	-

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9	0	0	0		9
3	0	0	0		3
3	0	0	0		3
4	0	0	0		4
4	0	0	0		4
4	0	0	0		4
4	0	0	0		4
4	0	0	0		4
9	0	0	0		9
5	0	0	0		5
9	0	0	0		9
4	0	0	0		4
-	0	-	0		2
2	0	0	0		2
4	0	0	0		4
4	0	0	0		4
4	0	0	0		4
4	0	0	0		4
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1	0	0	0		_
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0	0	0	0		0
1	0	0	0		1
1	0	0	0		1
0	0	0	0		0
1	0	0	0		1
0	0	0	0		0
1	0	0	0		1
0	0	0	0		0
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10:00 AM 10:30 AM 10:30 AM 11:15 AM 11:20 DM 12:15 PM 12:15 PM 13:15 PM 13:

10:45 PM	0	0	0	0	0	0	ı
11:00 PM	0	0	0	0	0	0	
11:15 PM	0	0	0	0	0	0	
11:30 PM	0	0	0	0	0	0	i
11:45 PM	0	0	0	0	0	0	
Total	183	0	2	0	0	185	
Total %	98.9	0.0	1.1	0.0	0.0	100.0	, ,
AM Times	8:30 AM	12:00 AM	6:00 AM	12:00 AM	12:00 AM	8:30 AM	
AM Peaks	19	0	1	0	0	19	
PM Times	1:45 PM	12:00 PM	12:00 PM	12:00 PM	3:00 PM	1:45 PM	i
Sylend Md	24	C	-	C	c	21	



Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Main St with Alley Site Code: Start Date: 05/17/2022 Page No: 1

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Main St with Alley Site Code: Start Date: 05/17/2022 Page No: 2

Turning Movement Peak Hour Data (7:30 AM)

	1				rannin	y IVIOVCII	ICHT I	ak i ioui	Data (1	.00 / ((1))	ı					ı
			Alley					Main St					Main St			
Ot and Time			Eastbound					Northbound					Southbound			
Start Time	U-Turn	Left	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	Int. Total
7:30 AM	0	0	0	1	0	0	2	114	0	116	0	72	0	0	72	188
7:45 AM	0	1	2	3	3	0	3	97	0	100	0	69	2	0	71	174
8:00 AM	0	0	0	1	0	0	1	98	0	99	0	68	1	0	69	168
8:15 AM	0	0	0	4	0	0	5	85	0	90	0	58	1	0	59	149
Total	0	1	2	9	3	0	11	394	0	405	0	267	4	0	271	679
Approach %	0.0	33.3	66.7	-	-	0.0	2.7	97.3	-	-	0.0	98.5	1.5	-	-	-
Total %	0.0	0.1	0.3	-	0.4	0.0	1.6	58.0	-	59.6	0.0	39.3	0.6	_	39.9	-
PHF	0.000	0.250	0.250	-	0.250	0.000	0.550	0.864	-	0.873	0.000	0.927	0.500	-	0.941	0.903
Lights	0	1	2	-	3	0	11	379	-	390	0	254	4	-	258	651
% Lights	-	100.0	100.0	-	100.0	-	100.0	96.2	-	96.3	-	95.1	100.0	-	95.2	95.9
Buses	0	0	0	-	0	0	0	3	-	3	0	3	0	-	3	6
% Buses	-	0.0	0.0	-	0.0	-	0.0	0.8	-	0.7	-	1.1	0.0	-	1.1	0.9
Single-Unit Trucks	0	0	0	-	0	0	0	7	-	7	0	9	0	-	9	16
% Single-Unit Trucks	-	0.0	0.0	-	0.0	-	0.0	1.8	-	1.7	-	3.4	0.0	-	3.3	2.4
Articulated Trucks	0	0	0	-	0	0	0	2	-	2	0	1	0	-	1	3
% Articulated Trucks	-	0.0	0.0	-	0.0	-	0.0	0.5	-	0.5	-	0.4	0.0	-	0.4	0.4
Bicycles on Road	0	0	0	-	0	0	0	3	-	3	0	0	0	-	0	3
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	8.0	-	0.7	-	0.0	0.0	-	0.0	0.4
Pedestrians	-	-		9		-			0		-	-		0		-
% Pedestrians	-	_	-	100.0	-	-	-	-	-	-	-	-	-	-	-	-

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Kenig Lindgren O'Hara Aboona, Inc. 9575 W. Higgins Rd., Suite 400

Rosemont, Illinois, United States 60018 (847)518-9990 kpachowicz@kloainc.com

Count Name: Main St with Alley Site Code: Start Date: 05/17/2022 Page No: 3

Turning Movement Peak Hour Data (4:45 PM)

					runni	a moven	HEIIL FE	ak noui	Dala (4.	45 FIVI)	1					1
			Alley					Main St					Main St			
Start Time			Eastbound					Northbound					Southbound			
Start Time	U-Turn	Left	Right	Peds	App. Total	U-Turn	Left	Thru	Peds	App. Total	U-Turn	Thru	Right	Peds	App. Total	Int. Total
4:45 PM	0	1	0	3	1	0	1	79	0	80	0	110	1	1	111	192
5:00 PM	0	1	2	0	3	0	3	108	0	111	0	97	0	0	97	211
5:15 PM	0	1	1	1	2	0	2	90	0	92	0	78	0	0	78	172
5:30 PM	0	1	1	0	2	0	1	92	1	93	0	113	4	0	117	212
Total	0	4	4	4	8	0	7	369	1	376	0	398	5	1	403	787
Approach %	0.0	50.0	50.0	-	-	0.0	1.9	98.1	-	-	0.0	98.8	1.2	-	-	-
Total %	0.0	0.5	0.5	-	1.0	0.0	0.9	46.9	-	47.8	0.0	50.6	0.6	-	51.2	-
PHF	0.000	1.000	0.500	-	0.667	0.000	0.583	0.854	-	0.847	0.000	0.881	0.313	-	0.861	0.928
Lights	0	4	4	-	8	0	7	360	-	367	0	389	5	-	394	769
% Lights	-	100.0	100.0	-	100.0	-	100.0	97.6	_	97.6	-	97.7	100.0	-	97.8	97.7
Buses	0	0	0	-	0	0	0	1	-	1	0	2	0	-	2	3
% Buses	-	0.0	0.0	-	0.0	-	0.0	0.3	-	0.3	-	0.5	0.0	-	0.5	0.4
Single-Unit Trucks	0	0	0	-	0	0	0	5	_	5	0	2	0	-	2	7
% Single-Unit Trucks	-	0.0	0.0	-	0.0	-	0.0	1.4	-	1.3	-	0.5	0.0	-	0.5	0.9
Articulated Trucks	0	0	0	-	0	0	0	3	-	3	0	2	0	-	2	5
% Articulated Trucks	-	0.0	0.0	-	0.0	-	0.0	8.0	_	8.0	-	0.5	0.0	-	0.5	0.6
Bicycles on Road	0	0	0	-	0	0	0	0	-	0	0	3	0	-	3	3
% Bicycles on Road	-	0.0	0.0	-	0.0	-	0.0	0.0	-	0.0	-	0.8	0.0	-	0.7	0.4
Pedestrians	-	-		4		-	_		1	-	-	-	-	1	_	-
% Pedestrians	-	-	-	100.0	-	-	-	-	100.0	-	-	-	-	100.0	-	-

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Site Plan

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ITE Trip Generation Sheets

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Multifamily Housing (Mid-Rise) Not Close to Rail Transit (221)

Vehicle Trip Ends vs: Dwelling Units On a: Weekday

Setting/Location: General Urban/Suburban

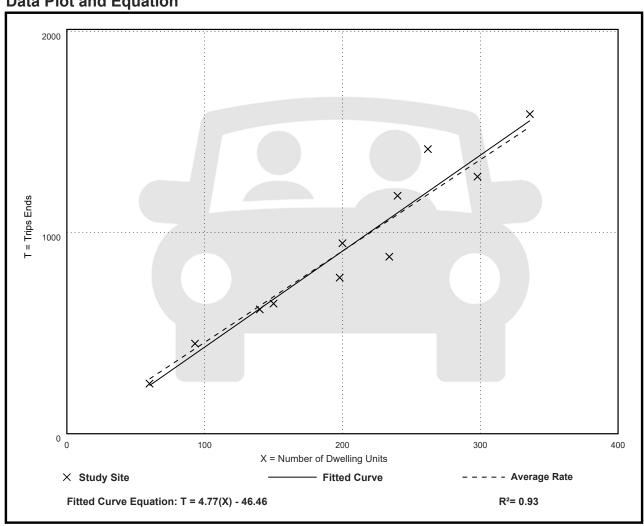
Number of Studies: 11 Avg. Num. of Dwelling Units: 201

Directional Distribution: 50% entering, 50% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
4.54	3.76 - 5.40	0.51

Data Plot and Equation





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Multifamily Housing (Mid-Rise) Not Close to Rail Transit (221)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 7 and 9 a.m.

Setting/Location: General Urban/Suburban

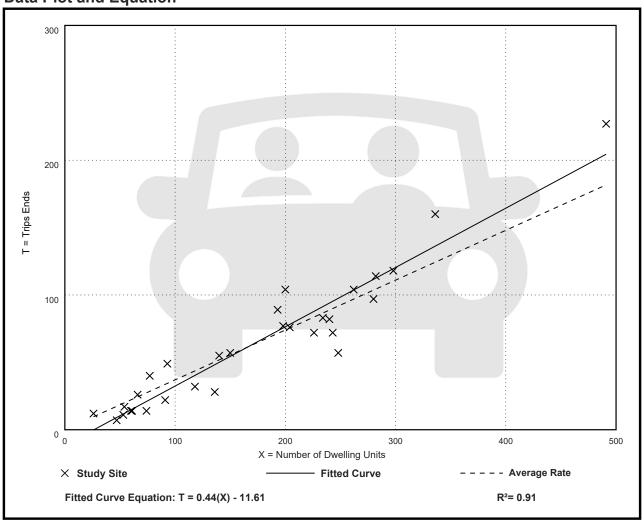
Number of Studies: 30 Avg. Num. of Dwelling Units: 173

Directional Distribution: 23% entering, 77% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.37	0.15 - 0.53	0.09

Data Plot and Equation





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Multifamily Housing (Mid-Rise) Not Close to Rail Transit (221)

Vehicle Trip Ends vs: Dwelling Units

On a: Weekday,

Peak Hour of Adjacent Street Traffic,

One Hour Between 4 and 6 p.m.

Setting/Location: General Urban/Suburban

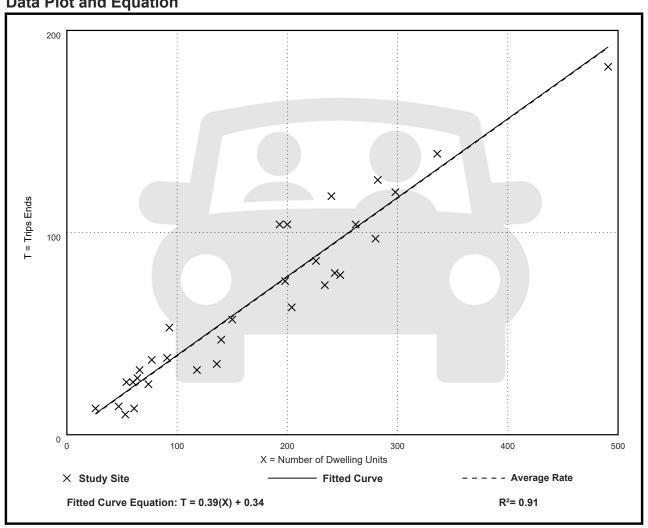
Number of Studies: 31 Avg. Num. of Dwelling Units: 169

Directional Distribution: 61% entering, 39% exiting

Vehicle Trip Generation per Dwelling Unit

Average Rate	Range of Rates	Standard Deviation
0.39	0.19 - 0.57	0.08

Data Plot and Equation





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CMAP 2050 Projections Letter

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433 West Van Buren Street, Suite 450 Chicago, IL 60607 cmap.illinois.gov | 312-454-0400

August 7, 2024

Ryan May Project Coordinator Kenig, Lindgren, O'Hara and Aboona, Inc. 9575 West Higgins Road Suite 400 Rosemont, IL 60018

Subject: Forest Ave, Warren Ave, Main St, Rogers St

IDOT

Dear Ms. May:

In response to a request made on your behalf and dated 8/2/2024, we have developed year 2050 average daily traffic (ADT) projections for the subject location.

ROAD SEGMENT	Current ADT	Year 2050 ADT
Forest Avenue north of Warren Avenue (west)	400	515
Forest Avenue south of Warren Avenue (west)	3,400	4,400
Warren Avenue (west) at Forest Avenue	1,550	2,000
Warren Avenue (east) at Forest Avenue	1,400	1,800
Forest Avenue south of Warren Avenue (east)	4,200	5,400
Main Street at Warren Avenue	6,800	7,800
Rogers Street at Main Street	1,400	1,600

Traffic projections are developed using existing ADT data provided in the request letter and the results from the June 2024 CMAP Travel Demand Analysis. The regional travel model uses CMAP 2050 socioeconomic projections and assumes the implementation of the ON TO 2050 Comprehensive Regional Plan for the Northeastern Illinois area. The provision of this data in support of your request does not constitute a CMAP endorsement of the proposed development or any subsequent developments.

If you have any questions, please call me at (312) 386-8806 or email me at jrodriguez@cmap.illinois.gov

Jose Rodriguez, PTP, AICP

Senior Planner, Research & Analysis

cc: Rios (IDOT)

 $2024_TrafficForecasts \\ \label{lowersGrove} \\ \label{lowersGroversGrove} \\ \label{lowersGrovers$

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Level of Service Criteria

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LEVEL OF SERVICE CRITERIA

Signalized	Intersections										
Level of Service	Interpretat	ion	Average Control Delay (seconds per vehicle)								
A	Favorable progression. Most ve green indication and travel throug stopping.	_	≤10								
В	Good progression, with more ve Level of Service A.	chicles stopping than for	> 10 - 20								
С	Individual cycle failures (i.e., one are not able to depart as a result during the cycle) may begin to apstopping is significant, although through the intersection without s	t of insufficient capacity pear. Number of vehicles many vehicles still pass	> 20 - 35								
D	The volume-to-capacity ratio is high and either progression is ineffective or the cycle length is too long. Many vehicles stop and individual cycle failures are noticeable.										
E	Progression is unfavorable. The volume-to-capacity ratio is high and the cycle length is long. Individual cycle failures are frequent.										
F	The volume-to-capacity ratio is very poor, and the cycle length is clear the queue.		> 80								
Unsignalize	ed Intersections										
	Level of Service	Average Total l	Delay (sec/veh)								
	A	0 -	10								
	В	> 10	- 15								
	С	> 15	- 25								
	D	> 25	- 35								
	E	> 35	- 50								
	F	> 5	50								
Source: High	way Capacity Manual, 6th Edition.										

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Capacity Analysis Summary Sheets
Existing Weekday Morning Peak Hour

Lanes, Volumes, Timings 1: Main Street & Franklin Street

08/23/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*		7	7	7>			र्स			^	7
Traffic Volume (vph)	42	0	18	5	15	16	33	421	0	0	236	97
Future Volume (vph)	42	0	18	5	15	16	33	421	0	0	236	97
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	35		0	0		0	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		1
Taper Length (ft)	25		•	25		•	25			25		•
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.75		0.93	0.94	0.86			1.00				0.96
Frt	0.10		0.850	0.01	0.923			1.00				0.850
Flt Protected	0.950		0.000	0.950	0.020			0.996				0.000
Satd. Flow (prot)	1770	0	1455	1504	1508	0	0	1837	0	0	1942	1583
Flt Permitted	0.736	0	1400	0.950	1000	U	0	0.967	· ·	0	1042	1000
Satd. Flow (perm)	1034	0	1347	1421	1508	0	0	1782	0	0	1942	1525
Right Turn on Red	1004	U	Yes	1721	1000	Yes	U	1702	Yes	U	1372	Yes
Satd. Flow (RTOR)			19		17	163			163			102
Link Speed (mph)		25	13		25			25			25	102
Link Distance (ft)		180			382			220			418	
Travel Time (s)		4.9			10.4			6.0			11.4	
Confl. Peds. (#/hr)	103	4.9	22	22	10.4	103	7	0.0	31	31	11.4	7
Confl. Bikes (#/hr)	103		22	22		103	I		31	31		I
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	0%	11%	20%	0%	0%	3%	3%	0%	0%	3%	2%
()	270	0%	0	20%	0%	0%	0	0	0%	0%	0	270
Bus Blockages (#/hr) Parking (#/hr)	U	U	U	U	U	U	U	U	U	U	U	U
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)		0 70			0 70			U 70			0 %	
()	44	0	19	5	33	0	0	478	0	0	248	102
Lane Group Flow (vph)		U		Perm	NA	U			U	U	NA	
Turn Type Protected Phases	Perm		Perm	Pelili			Perm	NA 2				Perm
	4		Л	0	8		2				6	G
Permitted Phases			7	8	•		2	0			_	6
Detector Phase	4		4	8	8		2	2			6	6
Switch Phase	F 0		۲.0	F 0	F 0		0.0	0.0			0.0	0.0
Minimum Initial (s)	5.0		5.0	5.0	5.0		8.0	8.0			8.0	8.0
Minimum Split (s)	22.5		22.5	22.5	22.5		22.5	22.5			22.5	22.5
Total Split (s)	30.0		30.0	30.0	30.0		60.0	60.0			60.0	60.0
Total Split (%)	33.3%		33.3%	33.3%	33.3%		66.7%	66.7%			66.7%	66.7%
Yellow Time (s)	3.5		3.5	3.0	3.0		3.0	3.0			3.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0			1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			0.0			0.0	0.0
Total Lost Time (s)	4.5		4.5	4.0	4.0			4.0			4.0	4.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None		None	None	None		C-Min	C-Min			C-Min	C-Min
Act Effct Green (s)	9.4		9.4	9.3	9.3			78.1			78.1	78.1
Actuated g/C Ratio	0.10		0.10	0.10	0.10			0.87			0.87	0.87

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Lanes, Volumes, Timings 1: Main Street & Franklin Street

08/23/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.41		0.12	0.03	0.19			0.31			0.15	0.08
Control Delay	47.9		16.4	33.8	24.3			2.6			2.2	0.7
Queue Delay	0.0		0.0	0.0	0.0			0.0			0.0	0.0
Total Delay	47.9		16.4	33.8	24.3			2.6			2.2	0.7
LOS	D		В	С	С			Α			Α	Α
Approach Delay		38.4			25.5			2.6			1.8	
Approach LOS		D			С			Α			Α	
Queue Length 50th (ft)	24		0	3	8			50			23	0
Queue Length 95th (ft)	55		20	12	34			90			52	10
Internal Link Dist (ft)		100			302			140			338	
Turn Bay Length (ft)	35											
Base Capacity (vph)	292		395	410	447			1546			1685	1336
Starvation Cap Reductn	0		0	0	0			0			0	0
Spillback Cap Reductn	0		0	0	0			0			0	0
Storage Cap Reductn	0		0	0	0			0			0	0
Reduced v/c Ratio	0.15		0.05	0.01	0.07			0.31			0.15	0.08
Intersection Summary												
71	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 0 (0%), Referenced	to phase 2:1	NBTL and	16:SBT, S	Start of G	reen							
Natural Cycle: 45												
Control Type: Actuated-Coo	ordinated											
Maximum v/c Ratio: 0.41												
Intersection Signal Delay: 5					tersection							
Intersection Capacity Utiliza	tion 63.7%			IC	U Level c	of Service	В					
Analysis Period (min) 15												
Splits and Phases: 1: Mai	in Street & F	- Franklin S	Street									
Ø2 (R)								₹ ø4				, I W
60 s							3	0 s			1	
Ø6 (R)								₩ Ø8				1.00
60 s							3	0 s				

Lanes, Volumes, Timings 2: Main Street & Warren Avenue

08/23/2024

	۶	→	*	•	+	•	1	†	~	/	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7		↑	7		^	7	7	f >	
Traffic Volume (vph)	19	30	11	0	30	7	0	369	2	9	220	14
Future Volume (vph)	19	30	11	0	30	7	0	369	2	9	220	14
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		60	0		55	0		0	60		0
Storage Lanes	0		1	0		1	0		1	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.97				0.89			0.91	0.97	1.00	
Frt			0.850			0.850			0.850		0.991	
Flt Protected		0.981								0.950		
Satd. Flow (prot)	0	1864	1615	0	2000	1615	0	1961	1615	1626	1809	0
Flt Permitted		0.858	1010	· ·	2000	1010		1001	1010	0.508	1000	
Satd. Flow (perm)	0	1575	1615	0	2000	1436	0	1961	1476	843	1809	0
Right Turn on Red		1010	Yes	· ·	2000	Yes		1001	Yes	0.10	1000	Yes
Satd. Flow (RTOR)			18			18			18		7	. 00
Link Speed (mph)		25			25	10		25	10		25	
Link Distance (ft)		405			420			116			254	
Travel Time (s)		11.0			11.5			3.2			6.9	
Confl. Peds. (#/hr)	37	11.0			11.0	37	28	0.2	27	27	0.0	28
Confl. Bikes (#/hr)	O1					O1	20					20
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	2%	0%	11%	4%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)		0 70			0 70			0 70			0 70	
Lane Group Flow (vph)	0	57	13	0	35	8	0	429	2	10	272	0
Turn Type	Perm	NA	Prot	U	NA	Perm	U	NA	Perm	Perm	NA	
Protected Phases	1 Cilli	4	4		8	1 Cilli		2	1 Cilli	1 Cilli	6	
Permitted Phases	1				- U	8			2	6	0	
Detector Phase	4	4	4		8	8		2	2	6	6	
Switch Phase					- U	0				0	0	
Minimum Initial (s)	5.0	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	
Minimum Split (s)	22.5	22.5	22.5		22.5	22.5		22.5	22.5	22.5	22.5	
Total Split (s)	30.0	30.0	30.0		30.0	30.0		60.0	60.0	60.0	60.0	
Total Split (%)	33.3%	33.3%	33.3%		33.3%	33.3%		66.7%	66.7%	66.7%	66.7%	
Yellow Time (s)	3.5	3.5	3.5		3.5	3.5		3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0		1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	1.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	
Total Lost Time (s)		4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	
()		4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	
Lead/Lag												
Lead-Lag Optimize?	None	None	Mona		None	Mona		C Min	C Min	C Min	C-Min	
Recall Mode	None	None	None		None	None		C-Min	C-Min	C-Min		
Act Effet Green (s)		8.5	8.5		8.5	8.5		75.4	75.4	75.4	75.4	
Actuated g/C Ratio		0.09	0.09		0.09	0.09		0.84	0.84	0.84	0.84	

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Lanes, Volumes, Timings

2: Main Street & Warren Avenue

08/23/2024

	•	-	*	1	•	*	1	†	1	-	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		0.39	0.08		0.19	0.05		0.26	0.00	0.01	0.18	
Control Delay		45.0	14.3		38.7	8.7		2.7	0.0	2.2	2.3	
Queue Delay		0.0	0.0		0.0	0.0		6.0	0.0	0.0	0.0	
Total Delay		45.0	14.3		38.7	8.7		8.7	0.0	2.2	2.3	
LOS		D	В		D	Α		Α	Α	Α	Α	
Approach Delay		39.3			33.1			8.7			2.3	
Approach LOS		D			С			Α			Α	
Queue Length 50th (ft)		31	0		19	0		44	0	1	24	
Queue Length 95th (ft)		63	13		43	7		79	0	4	46	
Internal Link Dist (ft)		325			340			36			174	
Turn Bay Length (ft)			60			55				60		
Base Capacity (vph)		446	470		566	419		1642	1239	706	1516	
Starvation Cap Reductn		0	0		0	0		1144	1131	0	0	
Spillback Cap Reductn		0	0		0	0		0	0	0	0	
Storage Cap Reductn		0	0		0	0		0	0	0	0	
Reduced v/c Ratio		0.13	0.03		0.06	0.02		0.86	0.02	0.01	0.18	
Intersection Summary												
Area Type:	Other											

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 45

Control Type: Actuated-Coordinated

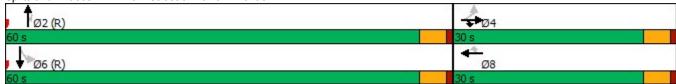
Maximum v/c Ratio: 0.39

Intersection Signal Delay: 10.3 Intersection Capacity Utilization 45.7%

Intersection LOS: B ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 2: Main Street & Warren Avenue



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Intersection Capacity Utilization 3: Forest Avenue & Franklin Street

	٠	→	•	•	•	*	4	†	1	-	↓	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	3	12	22	108	39	9	18	172	38	8	66	4
Pedestrians	1		3	3		1	3		4	4		3
Ped Button		Yes			Yes			Yes			Yes	
Pedestrian Timing (s)		16.0			16.0			16.0			16.0	
Free Right			No			No			No			No
Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Refr Cycle Length (s)	120	120	120	120	120	120	120	120	120	120	120	120
Volume Combined (vph)	0	37	0	0	156	0	0	228	0	0	78	0
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Factor (vph)	0.95	0.91	0.85	0.95	0.96	0.85	0.95	0.97	0.85	0.95	0.99	0.85
Saturated Flow (vph)	0	1724	0	0	1818	0	0	1845	0	0	1876	0
Ped Intf Time (s)	0.0	0.2	0.4	0.0	0.0	0.1	0.0	0.1	0.5	0.0	0.0	0.4
Pedestrian Frequency (%)		0.10			0.03			0.12			0.10	
Protected Option Allowed		No			No			No			No	
Reference Time (s)			0.0			0.0			0.0			0.0
Adj Reference Time (s)			0.0			0.0			0.0			0.0
Permitted Option												
Adj Saturation A (vph)	0	1756		0	372		0	1487		0	1330	
Reference Time A (s)	0.0	2.8		0.0	50.4		0.0	18.5		0.0	7.1	
Adj Saturation B (vph	0	0		0.0	0		0	0		NA	NA	
Reference Time B (s)	8.2	10.8		15.2	18.3		9.2	22.9		NA	NA	
Reference Time (s)	V. <u>_</u>	2.8			18.3		V	18.5			7.1	
Adj Reference Time (s)		9.1			22.3			22.5			11.9	
Split Option		U										
Ref Time Combined (s)	0.0	2.8		0.0	10.3		0.0	14.9		0.0	5.0	
Ref Time Seperate (s)	0.2	1.1		7.2	2.5		1.2	11.3		0.5	4.2	
Reference Time (s)	2.8	2.8		10.3	10.3		14.9	14.9		5.0	5.0	
Adj Reference Time (s)	9.1	9.1		14.5	14.5		19.0	19.0		10.1	10.1	
		0.1					10.0	10.0				
Summary	EB WB		NB SB	Со	mbined							
Protected Option (s)	NA		NA									
Permitted Option (s)	22.3		22.5									
Split Option (s)	23.6		29.1									
Minimum (s)	22.3		22.5		44.8							
Right Turns												
Adj Reference Time (s)												
Cross Thru Ref Time (s)												
Oncoming Left Ref Time (s)												
Combined (s)												
Intersection Summary												
Intersection Capacity Utilizat	ion		37.3%	IC	U Level	of Service			Α			
Reference Times and Phasir		do not re										

Intersection Capacity Utilization 5: Forest Avenue & Warren Avenue

	۶	•	1	1	ļ	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	*	7		4	1		
Volume (vph)	40	102	112	190	134	45	
Pedestrians	10		5			5	
Ped Button					Yes		
Pedestrian Timing (s)					16.0		
Free Right		No				No	
Ideal Flow	1900	1900	1900	1900	1900	1900	
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	
Refr Cycle Length (s)	120	120	120	120	120	120	
Volume Combined (vph)	40	102	0	302	179	0	
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Factor (vph)	0.95	0.85	0.95	0.98	0.96	0.85	
Saturated Flow (vph)	1805	1615	0.55	1865	1828	0.00	
Ped Intf Time (s)	0.0	0.0	0.0	0.0	0.2	0.6	
Pedestrian Frequency (%)	0.00	0.0	0.0	0.00	0.15	0.0	
Protected Option Allowed	No			No	No		
Reference Time (s)	NU	7.6		INU	NU	0.0	
Adj Reference Time (s)		11.6				0.0	
Permitted Option		11.0				0.0	
Adj Saturation A (vph)	120		0	276	1828		
	39.9		0.0	131.4	11.9		
Reference Time A (s)	39.9 NA		NA	131.4 NA	NA		
Adj Saturation B (vph Reference Time B (s)	NA		NA	NA NA	NA		
` ,	INA		INA	131.4	11.9		
Reference Time (s)				131.4	16.5		
Adj Reference Time (s)				133.4	10.5		
Split Option	0.7		0.0	10.4	11.0		
Ref Time Combined (s)	2.7		0.0	19.4	11.9		
Ref Time Seperate (s)	2.7		7.4	12.0	9.0		
Reference Time (s)	2.7		19.4	19.4	11.9		
Adj Reference Time (s)	8.0		23.4	23.4	16.5		
Summary	EB		NB SB	Со	mbined		
Protected Option (s)	NA		NA				
Permitted Option (s)	Err		135.4				
Split Option (s)	8.0		40.0				
Minimum (s)	8.0		40.0		48.0		
Right Turns	EBR						
Adj Reference Time (s)	11.6						
Cross Thru Ref Time (s)	16.5						
Oncoming Left Ref Time (s)	0.0						
Combined (s)	28.1						
. ,	20.1						
Intersection Summary			40.00/		111 - 2		
Intersection Capacity Utilization Reference Times and Phasing		do not re	40.0% epresent a			of Service plan.	•

4: Forest Avenue & AT&T Access Drive/Apartment Access Drive

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	1	1	0	2	1	224	0	0	188	1
Future Vol, veh/h	1	0	1	1	0	2	1	224	0	0	188	1
Conflicting Peds, #/hr	0	0	0	0	0	0	9	0	0	0	0	9
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	87	87	87	87	87	87	87	87	87	87	87	87
Heavy Vehicles, %	100	0	0	0	0	0	0	1	0	0	2	100
Mvmt Flow	1	0	1	1	0	2	1	257	0	0	216	1
Major/Minor N	/linor2		N	Minor1		ı	Major1		N	Major2		
Conflicting Flow All	486	485	226	476	485	257	226	0	0	257	0	0
Stage 1	226	226	-	259	259	-		-	-		-	-
Stage 2	260	259	_	217	226	_	_	_	_	_	_	_
Critical Hdwy	8.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	_
Critical Hdwy Stg 1	7.1	5.5	-	6.1	5.5	-	-	_	_	-	_	_
Critical Hdwy Stg 2	7.1	5.5	_	6.1	5.5	_	_	_	_	_	_	_
Follow-up Hdwy	4.4	4	3.3	3.5	4	3.3	2.2	_	_	2.2	-	-
Pot Cap-1 Maneuver	364	485	818	503	485	787	1354	_	_	1320	_	_
Stage 1	600	721	-	750	697		-	_	_		_	_
Stage 2	572	697	-	790	721	-	-	-	_	_	_	_
Platoon blocked, %	OIL	331		, 00	161			_	_		_	_
Mov Cap-1 Maneuver	360	480	811	502	480	787	1342	_	_	1320	_	_
Mov Cap-1 Maneuver	360	480	-	502	480		-	_	_	-	_	<u>-</u>
Stage 1	594	715	_	749	696	_	_	_	_	_	_	_
Stage 2	570	696	_	789	715	_	_	_	_	_	_	_
Jugo 2	510	330		, 00	, 10							
Annroach	EB			WB			NB			SB		
Approach												
HCM Control Delay, s	12.2			10.5			0			0		
HCM LOS	В			В								
Minor Lane/Major Mvmt		NBL	NBT	NRR I	EBLn1V	WRI n1	SBL	SBT	SBR			
Capacity (veh/h)		1342	-	-	499	662	1320	- 100	-			
HCM Lane V/C Ratio		0.001	<u>-</u>		0.005		1320	_	-			
HCM Control Delay (s)		7.7	0	-		10.5	0	-	-			
HCM Lane LOS		Α.	A	-	12.2 B	10.5 B	A	_	<u>-</u>			
HCM 95th %tile Q(veh)		0	- -	-	0	0	0	-	-			
		U	-	-	U	U	U	-	-			

6: Warren Avenue & Forest Avenue

Intersection							
Int Delay, s/veh	1.5						
Movement	WBL	WBR	NBT	NBR	SBL	SBT	
Lane Configurations	ሻ	7	↑	T T	ODL	41	
Traffic Vol, veh/h	15	36	266	33	35	201	
Future Vol, veh/h	15	36	266	33	35	201	
Conflicting Peds, #/hr	1	0	0	15	15	0	
Sign Control	Stop	Stop	Free	Free	Free	Free	
RT Channelized	Stop -	None		None	-	None	
Storage Length	0	0	_	0	_	NOHE -	
		-	0	-	-	0	
Veh in Median Storage	•						
Grade, %	0	-	0	-	-	0	
Peak Hour Factor	95	95	95	95	95	95	
Heavy Vehicles, %	0	0	2	0	0	3	
Mvmt Flow	16	38	280	35	37	212	
Major/Minor I	Minor1	N	Major1		Major2		
Conflicting Flow All	476	295	0	0	330	0	
Stage 1	295	-	-	_	-	-	
Stage 2	181	_	_	_	_	_	
Critical Hdwy	6.6	6.2	_	_	4.1	_	
Critical Hdwy Stg 1	5.4	- 0.2			7.1	_	
Critical Hdwy Stg 2	5.8	_	-	_	-		
, ,	3.5	3.3	-	-	2.2	-	
Follow-up Hdwy			-	-	1241		
Pot Cap-1 Maneuver	537	749	-	-	1241	-	
Stage 1	760	-	-	-	-	-	
Stage 2	838	-	-	-	-	-	
Platoon blocked, %			-	-		-	
Mov Cap-1 Maneuver	511	738	-	-	1223	-	
Mov Cap-2 Maneuver	511	-	-	-	-	-	
Stage 1	749	-	-	-	-	-	
Stage 2	809	-	-	-	-	-	
Approach	WB		NB		SB		
HCM Control Delay, s	10.7		0		1.3		
HCM LOS	В						
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1V	VBLn2	SBL	
Capacity (veh/h)		-	-	511	738	1223	
HCM Lane V/C Ratio		-	_	0.031		0.03	
HCM Control Delay (s)		_	_		10.1	8	
HCM Lane LOS		_	_	В	В	A	
HCM 95th %tile Q(veh))	_	_	0.1	0.2	0.1	
				J. 1	V.L	J. 1	

HCM 6th TWSC 8: N-S Alley & Franklin Street

Intersection						
Int Delay, s/veh	0.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1→			4	¥	
Traffic Vol, veh/h	57	3	9	146	6	3
Future Vol, veh/h	57	3	9	146	6	3
Conflicting Peds, #/hr	0	11	11	0	0	12
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	78	78	78	78	78	78
Heavy Vehicles, %	5	0	0	1	0	0
Mvmt Flow	73	4	12	187	8	4
Major/Minor	Major1		//oior?	N	Minor1	
	Major1		Major2			00
Conflicting Flow All	0	0	88	0	297	98
Stage 1	-	-	-	-	86	-
Stage 2	-	-	- 1 1	-	211	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1520	-	755	963
Stage 1	-	-	-	-	942	-
Stage 2	-	-	-	-	869	-
Platoon blocked, %	-	-	4504	-	1	040
Mov Cap-1 Maneuver	-	-	1504	-	741	942
Mov Cap-2 Maneuver	-	-	-	-	741	-
Stage 1	-	-	-	-	933	-
Stage 2	-	-	-	-	861	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		9.6	
HCM LOS	U		0.4		3.0 A	
TIOW EOO						
Minor Lane/Major Mvm	nt 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		798	-	-		-
HCM Lane V/C Ratio		0.014	-	-	0.008	-
HCM Control Delay (s)		9.6	-	-	7.4	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q(veh)		0			0	

9: Main Street & Oakley Access Drive

Intersection								
Int Delay, s/veh	0							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	¥	LDIN	HUL	4	- 1 <u>00</u> 1	אופט		
Traffic Vol, veh/h	0	0	0	450	253	0		
Future Vol, veh/h	0	0	0	450	253	0		
Conflicting Peds, #/hr	0	0	13	0	0	13		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	Stop -	None	-	None	-	None		
Storage Length	0	NOTIE	_	NOHE -	-	None -		
Storage Length Veh in Median Storage		-	-	0	0	-		
	e, # 0 0	-			0			
Grade, % Peak Hour Factor	93	93	93	93	93	93		
Heavy Vehicles, %	0	0	0	3	4	0		
Nvmt Flow	0	0	0	484	272	0		
Major/Minor	Minor2	<u> </u>	Major1	<u> </u>	/lajor2			
Conflicting Flow All	769	285	285	0	-	0		
Stage 1	285	-	-	-	-	-		
Stage 2	484	-	-	-	-	-		
Critical Hdwy	6.4	6.2	4.1	-	-	-		
Critical Hdwy Stg 1	5.4	-	-	-	-	-		
ritical Hdwy Stg 2	5.4	-	-	-	-	-		
ollow-up Hdwy	3.5	3.3	2.2	_	-	_		
ot Cap-1 Maneuver	*494	871	1321	_	_	_		
Stage 1	*827	-		-	-	-		
Stage 2	*676	-	-	-	-	-		
Platoon blocked, %	1	1	1	-	_	-		
Mov Cap-1 Maneuver		860	1304	-	_	-		
Mov Cap-2 Maneuver		-	-	_	_	_		
Stage 1	*817	_	_	_	_	-		
Stage 2	*668	_	_	_	_	_		
	300							
					0.5			
pproach	EB		NB		SB			
ICM Control Delay, s			0		0			
HCM LOS	Α							
//inor Lane/Major Mvn	nt	NBL	NBT I	EBLn1	SBT	SBR		
Capacity (veh/h)		1304						
ICM Lane V/C Ratio		-	_	_	_	_		
ICM Control Delay (s))	0	_	0	_	_		
ICM Lane LOS)	A	_	A		-		
HCM 95th %tile Q(veh	1)	0	-	- -	_	-		
`	1)	U	_	_	_	_		
Votes								
: Volume exceeds ca	pacity	\$: De	lay exc	eeds 30	00s	+: Comp	outation Not Defined	*: All major volume in platoon

10: Main Street & Funeral Home North Access Drive

Intersection									
Int Delay, s/veh	0								
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	W			4	1				
Traffic Vol, veh/h	0	1	0	450	253	0			
Future Vol, veh/h	0	1	0	450	253	0			
Conflicting Peds, #/hr	0	0	13	0	0	13			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	Slop -	None	-		-	None			
Storage Length	0	NOTIC	_			-			
				0	0				
Veh in Median Storage	e, # 0 0	-				-			
Grade, %	93	93	93	93	93	93			
Peak Hour Factor									
Heavy Vehicles, %	0	0	0	3	4	0			
Mvmt Flow	0	1	0	484	272	0			
Major/Minor	Minor2	<u> </u>	Major1	N	/lajor2				
Conflicting Flow All	769	285	285	0	-	0			
Stage 1	285	-	-	-	-	-			
Stage 2	484	-	-	-	-	-			
Critical Hdwy	6.4	6.2	4.1	-	-	-			
Critical Hdwy Stg 1	5.4	-	-	-	-	-			
Critical Hdwy Stg 2	5.4	-	-	-	-	-			
Follow-up Hdwy	3.5	3.3	2.2	_	-	_			
Pot Cap-1 Maneuver	*494	871	1321	-	-	_			
Stage 1	*827	-	-	_	_	_			
Stage 2	*676	_	_	_	_	_			
Platoon blocked, %	1	1	1	_	_	_			
Mov Cap-1 Maneuver	*482	860	1304	_	_	_			
Mov Cap-2 Maneuver	*482	-	-	_	_	_			
Stage 1	*817	_	-	_	_	_			
Stage 2	*668	_	_	_	_	_			
Olugo Z	000								
Approach	EB		NB		SB				
HCM Control Delay, s	9.2		0		0				
HCM LOS	Α								
Minor Lane/Major Mvn	nt	NBL	NRT I	EBLn1	SBT	SBR			
Capacity (veh/h)		1304	-	860	-	-			
HCM Lane V/C Ratio		-		0.001		_			
HCM Control Delay (s)		0	-	9.2	_	-			
HCM Lane LOS		A	_	9.2 A	_	-			
HCM 95th %tile Q(veh	1	0	_	0	-				
`	1	U	_	U	_	_			
Notes									
~: Volume exceeds ca	pacity	\$: De	lay exc	eeds 30	00s	+: Comp	outation Not Defined	*: All major volume in platoon	

HCM 6th TWSC 11: Main Street & Rogers Street

Intersection						
Int Delay, s/veh	1.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WBL	אטא	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	TIDIN	SDL N	<u>361</u>
Traffic Vol, veh/h	'T' 17	76	379	17	33	T 229
Future Vol, veh/h	17	76	379	17	33	229
Conflicting Peds, #/hr	4	5	0	19	19	229
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	60	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	0	5	3	0	3	5
Mvmt Flow	19	84	416	19	36	252
Major/Minor	Minor1	N	Major1		Major2	
						0
Conflicting Flow All	773	450	0	0	454	0
Stage 1	445	-	-	-	-	-
Stage 2	328	-	-	-	-	-
Critical Hdwy	6.4	6.25	-	-	4.13	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.345	-	-	2.227	-
Pot Cap-1 Maneuver	450	731	-	-	1117	-
Stage 1	713	-	-	-	-	-
Stage 2	784	-	-	-	-	-
Platoon blocked, %	1	1	_	_	1	_
Mov Cap-1 Maneuver		715	_	_	1097	-
Mov Cap-2 Maneuver	426	-	_	_	-	_
Stage 1	700	_	_		_	
•	755		-	-		
Stage 2	755	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	11.7		0		1.1	
HCM LOS	В		*			
110111 200						
Minor Lane/Major Mvr	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	636	1097	-
HCM Lane V/C Ratio		-	-	0.161	0.033	-
HCM Control Delay (s)	-	-	11.7	8.4	-
HCM Lane LOS		-	-	В	Α	-
HCM 95th %tile Q(veh	1)	-	-	0.6	0.1	-

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Capacity Analysis Summary Sheets
Existing Weekday Evening Peak Hour

Lanes, Volumes, Timings 1: Main Street & Franklin Street

	۶	→	•	•	←	•	1	†	~	1	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*		7	7	1>			ર્ન			^	7
Traffic Volume (vph)	49	0	21	3	8	17	37	356	0	0	403	147
Future Volume (vph)	49	0	21	3	8	17	37	356	0	0	403	147
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	35		0	0		0	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99		0.95	0.98	0.98			1.00				0.96
Frt			0.850		0.896							0.850
Flt Protected	0.950			0.950				0.995				
Satd. Flow (prot)	1805	0	1615	1805	1663	0	0	1890	0	0	1980	1615
Flt Permitted	0.740	-		0.950		-		0.941	-	-		10.10
Satd. Flow (perm)	1389	0	1537	1764	1663	0	0	1786	0	0	1980	1546
Right Turn on Red		-	Yes			Yes	_		Yes	-		Yes
Satd. Flow (RTOR)			22		18							152
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		180			382			220			418	
Travel Time (s)		4.9			10.4			6.0			11.4	
Confl. Peds. (#/hr)	5	1.0	9	9		5	9	0.0	9	9		9
Confl. Bikes (#/hr)			5			1			J	J		1
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)		0,0			0,0			0,0			0,0	
Lane Group Flow (vph)	51	0	22	3	26	0	0	405	0	0	415	152
Turn Type	Perm		Perm	Perm	NA		Perm	NA			NA	Perm
Protected Phases	1 01111		1 01111	1 01111	8		1 01111	2			6	1 01111
Permitted Phases	4		4	8			2	_				6
Detector Phase	4		4	8	8		2	2			6	6
Switch Phase	'		<u>'</u>									
Minimum Initial (s)	5.0		5.0	5.0	5.0		8.0	8.0			8.0	8.0
Minimum Split (s)	22.5		22.5	22.5	22.5		22.5	22.5			22.5	22.5
Total Split (s)	30.0		30.0	30.0	30.0		60.0	60.0			60.0	60.0
Total Split (%)	33.3%		33.3%	33.3%	33.3%		66.7%	66.7%			66.7%	66.7%
Yellow Time (s)	3.5		3.5	3.0	3.0		3.0	3.0			3.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0			1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0		1.0	0.0			0.0	0.0
Total Lost Time (s)	4.5		4.5	4.0	4.0			4.0			4.0	4.0
Lead/Lag	4.5		4.5	4.0	4.0			4.0			4.0	4.0
Lead-Lag Optimize?												
Recall Mode	None		None	None	None		C-Min	C-Min			C-Min	C-Min
	8.8		8.8	9.2	9.2		O-IVIII1	75.5			75.5	75.5
Act Effet Green (s)												
Actuated g/C Ratio	0.10		0.10	0.10	0.10			0.84			0.84	0.84

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Lanes, Volumes, Timings 1: Main Street & Franklin Street

	•	→	•	•	←	•	1	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.38		0.13	0.02	0.14			0.27			0.25	0.12
Control Delay	45.1		16.5	34.3	21.2			2.5			2.6	0.6
Queue Delay	0.0		0.0	0.0	0.0			0.0			0.0	0.0
Total Delay	45.1		16.5	34.3	21.2			2.5			2.6	0.6
LOS	D		В	С	С			Α			Α	Α
Approach Delay		36.5			22.6			2.5			2.1	
Approach LOS		D			С			Α			Α	
Queue Length 50th (ft)	28		0	2	4			39			41	0
Queue Length 95th (ft)	61		22	10	27			74			82	11
Internal Link Dist (ft)		100			302			140			338	
Turn Bay Length (ft)	35											
Base Capacity (vph)	393		451	509	493			1498			1661	1321
Starvation Cap Reductn	0		0	0	0			0			0	0
Spillback Cap Reductn	0		0	0	0			0			0	0
Storage Cap Reductn	0		0	0	0			0			0	0
Reduced v/c Ratio	0.13		0.05	0.01	0.05			0.27			0.25	0.12
Intersection Summary												
71	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 0 (0%), Referenced t	to phase 2:1	NBTL and	l 6:SBT, 8	Start of G	reen							
Natural Cycle: 45												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.38												
Intersection Signal Delay: 5.					tersection							
Intersection Capacity Utiliza	tion 61.3%			IC	U Level o	of Service	В					
Analysis Period (min) 15												
Splits and Phases: 1: Mai	n Street & F	ranklin S	Street									
↑ Ø2 (R)							350	₹ Ø4				35
60 s							3	0 s				
₩ Ø6 (R)								₹ Ø8				7.00

	۶	→	*	•	•	•	1	†	~	/	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7		↑	7			7	*	₽	-
Traffic Volume (vph)	21	51	7	0	45	17	0	286	16	4	386	15
Future Volume (vph)	21	51	7	0	45	17	0	286	16	4	386	15
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	-
Storage Length (ft)	0		60	0		55	0		0	60		0
Storage Lanes	0		1	0		1	0		1	1		0
Taper Length (ft)	25		-	25		•	25		•	25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.97			,,,,,,	0.88			0.86	0.93	1.00	
Frt			0.850			0.850			0.850		0.994	
Flt Protected		0.985								0.950		
Satd. Flow (prot)	0	1872	1615	0	2000	1615	0	1980	1615	1805	1867	0
Flt Permitted	· ·	0.883	1010	· ·	2000	1010		1000	1010	0.571	1001	J
Satd. Flow (perm)	0	1630	1615	0	2000	1417	0	1980	1393	1007	1867	0
Right Turn on Red	•		Yes	•		Yes			Yes			Yes
Satd. Flow (RTOR)			18			18			18		4	1 00
Link Speed (mph)		25			25	10		25	10		25	
Link Distance (ft)		405			420			116			254	
Travel Time (s)		11.0			11.5			3.2			6.9	
Confl. Peds. (#/hr)	41	11.0	4	4	11.0	41	27	0.2	48	48	0.0	27
Confl. Bikes (#/hr)	• •		2	•		2			10	10		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)		0 70			0 70			0 70			0 70	
Lane Group Flow (vph)	0	78	8	0	49	18	0	311	17	4	436	0
Turn Type	Perm	NA	Prot		NA	Perm	- U	NA	Perm	Perm	NA	J
Protected Phases	1 01111	4	4		8	1 01111		2	1 01111	1 01111	6	
Permitted Phases	4		- Т		J	8			2	6	U	
Detector Phase	4	4	4		8	8		2	2	6	6	
Switch Phase	<u> </u>		- Т		J	U					U	
Minimum Initial (s)	5.0	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	
Minimum Split (s)	22.5	22.5	22.5		22.5	22.5		22.5	22.5	22.5	22.5	
Total Split (s)	30.0	30.0	30.0		30.0	30.0		60.0	60.0	60.0	60.0	
Total Split (%)	33.3%	33.3%	33.3%		33.3%	33.3%		66.7%	66.7%	66.7%	66.7%	
Yellow Time (s)	3.5	3.5	3.5		3.5	3.5		3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0		1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	1.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	
Total Lost Time (s)		4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	
. ,		4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	
Lead/Lag												
Lead-Lag Optimize?	Nana	Mona	Mona		None	Mona		C Min	C Min	C Min	C-Min	
Recall Mode	None	None	None		None	None		C-Min	C-Min	C-Min		
Act Effct Green (s)		9.5	9.5		9.4	9.4		74.5	74.5	74.5	74.5	
Actuated g/C Ratio		0.11	0.11		0.10	0.10		0.83	0.83	0.83	0.83	

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Lanes, Volumes, Timings

2: Main Street & Warren Avenue

08/23/2024

	•	\rightarrow	*	1	•	*	1	Ť	1	1	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		0.45	0.04		0.23	0.11		0.19	0.01	0.00	0.28	
Control Delay		45.5	8.1		38.4	16.8		2.7	1.2	2.5	3.0	
Queue Delay		0.0	0.0		0.0	0.0		4.3	0.6	0.0	0.0	
Total Delay		45.5	8.1		38.4	16.8		7.0	1.8	2.5	3.0	
LOS		D	Α		D	В		Α	Α	Α	Α	
Approach Delay		42.0			32.6			6.7			3.0	
Approach LOS		D			С			Α			Α	
Queue Length 50th (ft)		42	0		26	0		32	0	1	49	
Queue Length 95th (ft)		83	7		57	19		65	4	m2	92	
Internal Link Dist (ft)		325			340			36			174	
Turn Bay Length (ft)			60			55				60		
Base Capacity (vph)		461	470		566	414		1638	1155	833	1545	
Starvation Cap Reductn		0	0		0	0		1235	1027	0	0	
Spillback Cap Reductn		0	0		0	0		0	0	0	0	
Storage Cap Reductn		0	0		0	0		0	0	0	0	
Reduced v/c Ratio		0.17	0.02		0.09	0.04		0.77	0.13	0.00	0.28	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 45

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.45

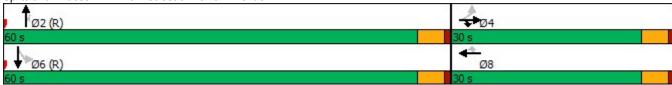
Intersection Signal Delay: 10.1 Intersection Capacity Utilization 44.1%

Intersection LOS: B
ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Main Street & Warren Avenue



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Intersection Capacity Utilization 3: Forest Avenue & Franklin Street

	۶	→	•	1	←	*	1	1	1	-	↓	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	11	12	43	147	38	1	19	99	53	2	43	1
Pedestrians			7	7			3		5	5		3
Ped Button		Yes						Yes			Yes	
Pedestrian Timing (s)		16.0						16.0			16.0	
Free Right			No			No			No			No
Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Refr Cycle Length (s)	120	120	120	120	120	120	120	120	120	120	120	120
Volume Combined (vph)	0	66	0	0	186	0	0	171	0	0	46	0
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Factor (vph)	0.95	0.89	0.85	0.95	0.96	0.85	0.95	0.95	0.85	0.95	0.99	0.85
Saturated Flow (vph)	0	1700	0	0	1823	0	0	1802	0	0	1890	0
Ped Intf Time (s)	0.0	0.6	0.9	0.0	0.0	0.0	0.0	0.2	0.6	0.0	0.0	0.4
Pedestrian Frequency (%)		0.21			0.00			0.15			0.10	
Protected Option Allowed		No			No			No			No	
Reference Time (s)			0.0			0.0			0.0			0.0
Adj Reference Time (s)			0.0			0.0			0.0			0.0
Permitted Option												
Adj Saturation A (vph)	0	1773		0	520		0	1062		0	1694	
Reference Time A (s)	0.0	5.0		0.0	43.0		0.0	19.5		0.0	3.3	
Adj Saturation B (vph	0	0		0	0		0	0		NA	NA	
Reference Time B (s)	8.7	13.2		17.8	20.2		9.3	19.6		NA	NA	
Reference Time (s)		5.0			20.2			19.5			3.3	
Adj Reference Time (s)		11.3			24.2			23.5			9.1	
Split Option												
Ref Time Combined (s)	0.0	5.2		0.0	12.2		0.0	11.6		0.0	2.9	
Ref Time Seperate (s)	0.7	1.4		9.8	2.4		1.3	6.8		0.1	2.7	
Reference Time (s)	5.2	5.2		12.2	12.2		11.6	11.6		2.9	2.9	
Adj Reference Time (s)	11.5	11.5		16.2	16.2		16.3	16.3		9.1	9.1	
Summary	EB WB		NB SB	Co	mbined							
Protected Option (s)	NA		NA									
Permitted Option (s)	24.2		23.5									
Split Option (s)	27.7		25.4									
Minimum (s)	24.2		23.5		47.8							
Right Turns												
Adj Reference Time (s)												
Cross Thru Ref Time (s)												
Oncoming Left Ref Time (s)												
Combined (s)												
Intersection Summary												
	ion		39.8%	10	U Level o	of Consiss			A			
Intersection Capacity Utilizat Reference Times and Phasir		do not re							А			

Intersection Capacity Utilization 5: Forest Avenue & Warren Avenue

	۶	•	4	†	ļ	1	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	*	7		4	1>		
Volume (vph)	26	192	77	135	186	17	
Pedestrians	10		14			14	
Ped Button					Yes		
Pedestrian Timing (s)					16.0		
Free Right		No				No	
Ideal Flow	1900	1900	1900	1900	1900	1900	
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	
Refr Cycle Length (s)	120	120	120	120	120	120	
Volume Combined (vph)	26	192	0	212	203	0	
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Factor (vph)	0.95	0.85	0.95	0.98	0.99	0.85	
Saturated Flow (vph)	1805	1615	0	1865	1876	0	
Ped Intf Time (s)	0.0	0.0	0.0	0.0	0.1	1.7	
Pedestrian Frequency (%)	0.00	0.0	0.0	0.00	0.37	1.7	
Protected Option Allowed	No			No	No		
Reference Time (s)	INO	14.3		NO	NU	0.0	
Adj Reference Time (s)		18.3				0.0	
Permitted Option		10.0				0.0	
Adj Saturation A (vph)	120		0	281	1876		
Reference Time A (s)	25.9		0.0	90.5	13.1		
Adj Saturation B (vph	NA		NA	NA	NA		
Reference Time B (s)	NA		NA	NA	NA		
Reference Time (s)	INA		INA	90.5	13.1		
Adj Reference Time (s)				94.5	18.2		
				34.3	10.2		
Split Option Ref Time Combined (s)	1.7		0.0	13.6	13.1		
()					12.0		
Ref Time Seperate (s) Reference Time (s)	1.7 1.7		5.1 13.6	8.5 13.6	13.1		
()	8.0		17.6	17.6	18.2		
Adj Reference Time (s)				0.11	10.2		
Summary	EB		NB SB	Col	mbined		
Protected Option (s)	NA		NA				
Permitted Option (s)	Err		94.5				
Split Option (s)	8.0		35.8				
Minimum (s)	8.0		35.8		43.8		
· /							
Right Turns	EBR						
Adj Reference Time (s)	18.3						
Cross Thru Ref Time (s)	18.2						
Oncoming Left Ref Time (s)	0.0						
Combined (s)	36.5						
Intersection Summary							
Intersection Capacity Utilization	on		36.5%	IC	U Level c	of Service	
Reference Times and Phasing	g Options	do not re	present a	n optimiz	ed timing	plan.	

4: Forest Avenue & AT&T Access Drive/Apartment Access Drive

Intersection												
Int Delay, s/veh	0.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	1	1	166	0	3	217	1
Future Vol, veh/h	1	0	0	0	0	1	1	166	0	3	217	1
Conflicting Peds, #/hr	0	0	2	2	0	0	16	0	0	0	0	16
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	0	0	0	0	0	100	0	0	0	0	0
Mvmt Flow	1	0	0	0	0	1	1	193	0	3	252	1
Major/Minor N	1inor2		<u> </u>	Minor1			Major1		<u> </u>	/lajor2		
Conflicting Flow All	471	470	271	456	470	193	269	0	0	193	0	0
Stage 1	275	275	-	195	195	-	-	-	-	-	-	-
Stage 2	196	195	-	261	275	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	5.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	3.1	-	-	2.2	-	-
Pot Cap-1 Maneuver	506	495	773	518	495	854	889	-	-	1392	-	-
Stage 1	736	686	-	811	743	-	-	-	-	-	-	-
Stage 2	810	743	-	748	686	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	496	486	760	515	486	854	875	-	-	1392	-	-
Mov Cap-2 Maneuver	496	486	-	515	486	-	-	-	-	-	-	-
Stage 1	724	674	-	810	742	-	-	-	-	-	-	-
Stage 2	808	742	-	744	674	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.3			9.2			0.1			0.1		
HCM LOS	В			A								
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		875	-	-	100	854	1392	-	-			
HCM Lane V/C Ratio		0.001	-	-	0.002		0.003	-	-			
HCM Control Delay (s)		9.1	0	-		9.2	7.6	0	-			
HCM Lane LOS		Α	A	-	В	Α	A	A	-			
HCM 95th %tile Q(veh)		0	-	-	0	0	0	-	-			

6: Warren Avenue & Forest Avenue

Intersection								
Int Delay, s/veh	1.6							
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	7	7	↑	7		414		
Traffic Vol, veh/h	28	24	188	18	54	324		
Future Vol, veh/h	28	24	188	18	54	324		
Conflicting Peds, #/hr	0	0	0	28	28	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	0	0	-	0	-	-		
Veh in Median Storage	e, # 0	-	0	-	-	0		
Grade, %	0	-	0	-	-	0		
Peak Hour Factor	94	94	94	94	94	94		
Heavy Vehicles, %	0	0	1	0	0	1		
Mvmt Flow	30	26	200	19	57	345		
Majar/Minar	\	N.	1-1-1		Maia#0		į	
	Minor1		Major1		Major2			
Conflicting Flow All	515	228	0	0	247	0		
Stage 1	228	-	-	-	-	-		
Stage 2	287	-	-	-	-	-		
Critical Hdwy	6.6	6.2	-	-	4.1	-		
Critical Hdwy Stg 1	5.4	-	-	-	-	-		
Critical Hdwy Stg 2	5.8	-	-	-	-	-		
Follow-up Hdwy	3.5	3.3	-	-	2.2	-		
Pot Cap-1 Maneuver	*767	*935	-	-	*1403	-		
Stage 1	*882	-	-	-	-	-		
Stage 2	*742	-	-	-	-	-		
Platoon blocked, %	1	1	-	-	1	-		
Mov Cap-1 Maneuver	*707	*910	-	-	*1366	-		
Mov Cap-2 Maneuver	*707	-	-	-	-	-		
Stage 1	*858	-	-	-	-	-		
Stage 2	*703	-	-	-	-	-		
Approach	WB		NB		SB		ĺ	
HCM Control Delay, s	9.7		0		1.3			
HCM LOS	9.7 A		U		1.0			
I IOW LOS								
Minor Lane/Major Mvm	it	NBT	NBRV	VBLn1V	VBLn2	SBL		SBT
Capacity (veh/h)		-	-	707	910	* 1366		-
HCM Lane V/C Ratio		-	-	0.042	0.028	0.042		-
HCM Control Delay (s)		-	-	10.3	9.1	7.8		0.2
HCM Lane LOS		-	-	В	Α	Α		Α
HCM 95th %tile Q(veh))	-	-	0.1	0.1	0.1		-
Notes							Į	
110100								utation Not Defined
~: Volume exceeds cap	anoity.	\$: De	101/01/0		1/10			

HCM 6th TWSC 8: N-S Alley & Franklin Street

Intersection						
Int Delay, s/veh	0.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1			4	W	
Traffic Vol, veh/h	60	3	6	179	3	5
Future Vol, veh/h	60	3	6	179	3	5
Conflicting Peds, #/hr	0	9	9	0	1	1
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	_	None	-		-	None
Storage Length	-	-	_	-	0	-
Veh in Median Storage	e,# 0	-	-	0	0	-
Grade, %	0	_	_	0	0	_
Peak Hour Factor	79	79	79	79	79	79
Heavy Vehicles, %	0	0	0	1	0	0
Mymt Flow	76	4	8	227	4	6
IVIVIII(I IOW	70	7	U	ZZI	7	U
Major/Minor	Major1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	89	0	331	88
Stage 1	-	-	-	-	87	-
Stage 2	-	-	-	-	244	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	-	-	-	-	5.4	-
Critical Hdwy Stg 2	-	-	-	-	5.4	_
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1519	-	716	976
Stage 1	_	-	-	_	941	-
Stage 2	_	_	_	_	835	_
Platoon blocked, %	_	_		_	1	
Mov Cap-1 Maneuver	_	_	1506	_	704	967
Mov Cap-2 Maneuver	-	_	-	_	704	-
Stage 1	_	_	_	_	933	_
Stage 2	_	_	_	<u> </u>	829	_
Stage 2	_		_		023	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		9.3	
HCM LOS					Α	
Minor Lane/Major Mvn	ot I	NBLn1	EBT	EBR	WBL	WBT
	IL I					WDI
Capacity (veh/h)		848	-	-		-
		0.012	-		0.005	-
HCM Lane V/C Ratio				_	7.4	0
HCM Control Delay (s)		9.3	-			
		9.3 A 0	- -	- -	A 0	A

9: Main Street & Oakley Access Drive

ntersection								
nt Delay, s/veh	0							
Novement	EBL	EBR	NBL	NBT	SBT	SBR		
ane Configurations	W	LDIN	INDL	4	\$	ODIN		
offic Vol, veh/h	1	0	0	392	432	0		
ure Vol, veh/h	1	0	0	392	432	0		
licting Peds, #/hr	1	0	12	0	0	12		
Control	Stop	Stop	Free	Free	Free	Free		
Channelized	Stop -	None		None	-	None		
rage Length	0	-	_	-	_	NOHE		
n in Median Storage,		_	_	0	0			
ide, %	, # 0	_	_	0	0	_		
ak Hour Factor	90	90	90	90	90	90		
vy Vehicles, %	0	90	90	90	90	90		
nt Flow	1	0	0	436	480	0		
III, I-IOW	I	U	U	430	400	U		
(b.t.)	4: 0	_			4			
	Minor2		Major1		/lajor2			
flicting Flow All	929	492	492	0	-	0		
Stage 1	492	-	-	-	-	-		
Stage 2	437	-	-	-	-	-		
cal Hdwy	6.4	6.2	4.1	-	-	-		
cal Hdwy Stg 1	5.4	-	-	-	-	-		
cal Hdwy Stg 2	5.4	-	-	-	-	-		
ow-up Hdwy	3.5	3.3	2.2	-	-	-		
Cap-1 Maneuver	*349	*730	*1095	-	-	-		
Stage 1	*689	-	-	-	-	-		
Stage 2	*722	-	-	-	-	-		
toon blocked, %	1	1	1	-	-	-		
v Cap-1 Maneuver	*341	*722	*1083	-	-	-		
/ Cap-2 Maneuver	*341	-	-	-	-	-		
Stage 1	*681	-	-	-	-	-		
Stage 2	*714	-	-	-	-	-		
roach	EB		NB		SB			
M Control Delay, s	15.6		0		0			
CM LOS	С							
nor Lane/Major Mvm	t	NBL	NRT	EBLn1	SBT	SBR		
pacity (veh/h)		* 1083		341	-	-		
M Lane V/C Ratio		1005	_	0.003	_	_		
M Control Delay (s)		0		15.6	_	_		
A Lane LOS		A	_	13.0 C	_	_		
M 95th %tile Q(veh)		0	_	0	-	_		
· ´		U		U				
es								
olume exceeds cap	acity	\$: De	elay exc	eeds 30)0s	+: Comp	outation Not Defined	*: All major volume in platoon

10: Main Street & Funeral Home North Access Drive

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	W	LDI	HUL	4	- 1 <u>00</u> 1	ODIN
Traffic Vol, veh/h	0	0	0	391	430	0
Future Vol, veh/h	0	0	0	391	430	0
Conflicting Peds, #/hr	0	0	13	0	430	13
Sign Control				Free	Free	Free
	Stop	Stop	Free			
RT Channelized	-	None	-		-	
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	0	1	0
Mvmt Flow	0	0	0	434	478	0
Major/Minor	Minor2		Major1	N	Major2	
Conflicting Flow All	925	491	491	0	-	0
Stage 1	491	491	491			
	434			-	-	-
Stage 2		-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	*314	*730	*1095	-	-	-
Stage 1	*689	-	-	-	-	-
Stage 2	*722	-	-	-	-	-
Platoon blocked, %	1	1	1	-	-	-
Mov Cap-1 Maneuver	*306	*721	*1082	-	-	-
Mov Cap-2 Maneuver	*306	-	-	-	-	-
Stage 1	*680	_	-	-	_	-
Stage 2	*713	_	_	_	_	_
o tago _						
Approach	EB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	Α					
Minor Lane/Major Mvm	- +	NBL	NDT	EBLn1	SBT	SBR
				LDLIII		SDR
		* 1082	-	-	-	-
Capacity (veh/h)		-	-	-	-	-
HCM Lane V/C Ratio				0	-	-
HCM Lane V/C Ratio HCM Control Delay (s)		0	-			
HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS		Α	-	Α	-	-
HCM Lane V/C Ratio HCM Control Delay (s)					-	-
HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh		Α	-	Α		
HCM Lane V/C Ratio HCM Control Delay (s) HCM Lane LOS)	A 0	-	A -	-	

HCM 6th TWSC 11: Main Street & Rogers Street

Movement	Intersection									
ane Configurations in affice Vol., vehith 13 92 303 21 46 381 authors vehith 14 381 authors vehith 15 0 authors vehicles, which is a vehicle vehith 15 0 authors vehicles vehicles vehicles vehicles, which is a vehicle vehicle vehicles, which is a vehicle vehicle vehicle vehicle vehicles, which is a vehicle vehicle vehicle vehicles, which is a vehicle vehicle vehicle vehicle vehicles, which is a vehicle vehicle vehicle vehicle vehicles, which is a vehicle veh	Int Delay, s/veh	1.8								
rraffic Vol. veh/h 13 92 303 21 46 381 vonflicting Pols, #/hr 2 8 0 15 15 0 sign Control Stop Stop Free Free Free Free TT Channelized - None - None vorage Length 0 60 0 reak Hour Factor 94 94 94 94 94 leavy Vehicles, % 0 0 0 0 0 0 1 rwm Flow 14 98 322 22 49 405 Alajor/Minor Minor1 Major1 Major2 vonflicting Flow All 853 356 0 0 359 0 Stage 1 348	Movement	WBL	WBR	NBT	NBR	SBL	SBT			
rraffic Vol. veh/h 13 92 303 21 46 381 vonflicting Pols, #/hr 2 8 0 15 15 0 sign Control Stop Stop Free Free Free Free TT Channelized - None - None vorage Length 0 60 0 reak Hour Factor 94 94 94 94 94 leavy Vehicles, % 0 0 0 0 0 0 1 rwm Flow 14 98 322 22 49 405 Alajor/Minor Minor1 Major1 Major2 vonflicting Flow All 853 356 0 0 359 0 Stage 1 348	Lane Configurations	W		1		*	*			
ruture Vol, veh/h	Traffic Vol, veh/h		92		21					
Conflicting Peds, #hr 2 8 0 15 15 0	Future Vol, veh/h									
Stop Stop Free	· · · · · · · · · · · · · · · · · · ·									
None	Sign Control	Stop	Stop	Free	Free	Free	Free			
Storage Length	RT Channelized									
Veh in Median Storage, # 0		0		_		60				
Grade, % 0 - 0 - 0 - 0 0 - 0 0 0 0 0 0 0 1 0 0 0 1 0 0 0 0			-	0	_		0			
Peak Hour Factor 94 94 94 94 94 94 94 94 94 94 94 94 94	ŭ	,	_		_	_				
Reavy Vehicles, %		-	94	-	94	94				
Major Majo				-			-			
Major/Minor Minor1 Major1 Major2			-							
Stage 1 348 -	MATERIAL TOWN	17	30	ULL	LL	73	700			
Stage 1 348 -										
Stage 1 348 - - - - -										
Stage 2 505 - - - - - -	Conflicting Flow All			0	0	359	0			
Critical Hdwy Stg 1 5.4 4.1 - Critical Hdwy Stg 1 5.4	ŭ		-	-	-	-	-			
Critical Hdwy Stg 1 5.4				-	-		-			
Critical Hdwy Stg 2 5.4	Critical Hdwy		6.2	-	-	4.1	-			
Sollow-up Hdwy	Critical Hdwy Stg 1	5.4	-	-	-	-	-			
Stage 1	Critical Hdwy Stg 2	5.4	-	-	-	-	-			
Stage 1 *778 Stage 2 *669	Follow-up Hdwy	3.5	3.3	-	-	2.2	-			
Stage 2 *669	Pot Cap-1 Maneuver	*429	*825	-	-	*1238	-			
Platoon blocked, % 1 1 1 - Mov Cap-1 Maneuver *405 *807 *1220 - Mov Cap-2 Maneuver *405 Stage 1 *767 Stage 2 *641 Stage 2 *641 Stage 3 *641 Nopproach WB NB SB ICM Control Delay, s 10.9 0 0.9 ICM LOS B Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT Capacity (veh/h) - 719 *1220 - ICM Lane V/C Ratio - 0.155 0.04 - ICM Control Delay (s) - 10.9 8.1 - ICM Control Delay (s) - B A - ICM Lane LOS - B A - ICM 95th %tile Q(veh) - 0.5 0.1 - Notes	Stage 1	*778	-	-	-	-	-			
Alatoon blocked, % 1 1 1 1 1	Stage 2	*669	-	-	-	-	-			
Mov Cap-2 Maneuver *405 -	Platoon blocked, %	1	1	-	-	1	-			
Mov Cap-2 Maneuver *405 -	-	*405	*807	-	-	*1220	-			
Stage 1 *767 -	Mov Cap-2 Maneuver		-	-	-	-	-			
Stage 2 *641			-	-	-	-	-			
Approach WB NB SB SB SC SC SC SC SC S			-	_	-	-	-			
ACM Control Delay, s 10.9	<u> </u>									
ACM Control Delay, s 10.9	Annroach	\\/D		ND		QD.				
Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT Capacity (veh/h) - 719 * 1220 - HCM Lane V/C Ratio - 0.155 0.04 - HCM Control Delay (s) - 10.9 8.1 - HCM Lane LOS - B A - HCM 95th %tile Q(veh) - 0.5 0.1 -										
Minor Lane/Major Mvmt NBT NBRWBLn1 SBL SBT Capacity (veh/h) - 719 * 1220 - HCM Lane V/C Ratio - 0.155 0.04 - HCM Control Delay (s) - 10.9 8.1 - HCM Lane LOS - B A - HCM 95th %tile Q(veh) - 0.5 0.1 -				U		0.9				
Capacity (veh/h) 719 * 1220 - HCM Lane V/C Ratio - 0.155 0.04 - HCM Control Delay (s) - 10.9 8.1 - HCM Lane LOS - B A - HCM 95th %tile Q(veh) - 0.5 0.1 -	HOM LOS	В								
Capacity (veh/h) 719 * 1220 - HCM Lane V/C Ratio - 0.155 0.04 - HCM Control Delay (s) - 10.9 8.1 - HCM Lane LOS - B A - HCM 95th %tile Q(veh) - 0.5 0.1 -										
ICM Lane V/C Ratio 0.155 0.04 - ICM Control Delay (s) 10.9 8.1 - ICM Lane LOS - B A - ICM 95th %tile Q(veh) - 0.5 0.1 -	Minor Lane/Major Mvm	ıt	NBT	NBRV			SBT			
ICM Control Delay (s) 10.9 8.1 - ICM Lane LOS B A - ICM 95th %tile Q(veh) 0.5 0.1 - Iotes	Capacity (veh/h)		-	-	719 '	1220	-			
ICM Lane LOS B A - ICM 95th %tile Q(veh) 0.5 0.1 - Iotes	HCM Lane V/C Ratio		-	-	0.155	0.04	-			
ICM Lane LOS B A - ICM 95th %tile Q(veh) 0.5 0.1 - Iotes	HCM Control Delay (s)		-	-	10.9	8.1	-			
ICM 95th %tile Q(veh) 0.5 0.1 - lotes	HCM Lane LOS		-	-			-			
lotes	HCM 95th %tile Q(veh)		-	-			-			
*: volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in platoon			¢. D	lave are	d - 00	10-	0	outstian Nat Daties I	*. All marion values a la salat	
	~: volume exceeds cap	bacity	⊅: De	ay exc	eeas 30	JUS	+: Comp	Dutation Not Defined	: All major volume in platoon	

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<u>Capacity Analysis Summary Sheets</u> Year 2030 No-Build Weekday Morning Peak Hour

Lanes, Volumes, Timings 1: Main Street & Franklin Street

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*		7	*	₽			र्स			^	7
Traffic Volume (vph)	44	0	22	5	16	17	35	478	0	0	289	102
Future Volume (vph)	44	0	22	5	16	17	35	478	0	0	289	102
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	35		0	0		0	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		1
Taper Length (ft)	25		•	25		•	25			25		•
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.75	1.00	0.93	0.94	0.86	1.00	1.00	1.00	1.00	1.00	1.00	0.96
Frt	0.10		0.850	0.01	0.923			1.00				0.850
Flt Protected	0.950		0.000	0.950	0.020			0.997				0.000
Satd. Flow (prot)	1770	0	1455	1504	1508	0	0	1839	0	0	1942	1583
Flt Permitted	0.734	0	1400	0.950	1000	0	0	0.965	· ·	0	1042	1000
Satd. Flow (perm)	1032	0	1347	1421	1508	0	0	1779	0	0	1942	1525
Right Turn on Red	1002	U	Yes	1721	1000	Yes	U	1773	Yes	U	1372	Yes
Satd. Flow (RTOR)			23		18	163			163			107
Link Speed (mph)		25	20		25			25			25	101
Link Distance (ft)		180			382			220			418	
Travel Time (s)		4.9			10.4			6.0			11.4	
. ,	103	4.9	22	22	10.4	103	7	0.0	31	31	11.4	7
Confl. Peds. (#/hr) Confl. Bikes (#/hr)	103		22	22		103	I		31	31		I
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
	2%	0%	11%	20%	0%	0%		3%		0%	3%	2%
Heavy Vehicles (%)							3%		0%			
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)		0%			0%			0%			0%	
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)	40	0	22	_	25	^	0	E40	^	^	204	107
Lane Group Flow (vph)	46	0	23	5	35	0	0	540	0	0	304	107
Turn Type	Perm		Perm	Perm	NA		Perm	NA			NA	Perm
Protected Phases	4		Λ	0	8		^	2			6	C
Permitted Phases	4		7	8			2				_	6
Detector Phase	4		4	8	8		2	2			6	6
Switch Phase	5 0		5 0		5 0		0.0	0.0			0.0	0.0
Minimum Initial (s)	5.0		5.0	5.0	5.0		8.0	8.0			8.0	8.0
Minimum Split (s)	22.5		22.5	22.5	22.5		22.5	22.5			22.5	22.5
Total Split (s)	30.0		30.0	30.0	30.0		60.0	60.0			60.0	60.0
Total Split (%)	33.3%		33.3%	33.3%	33.3%		66.7%	66.7%			66.7%	66.7%
Yellow Time (s)	3.5		3.5	3.0	3.0		3.0	3.0			3.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0			1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			0.0			0.0	0.0
Total Lost Time (s)	4.5		4.5	4.0	4.0			4.0			4.0	4.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None		None	None	None		C-Min	C-Min			C-Min	C-Min
Act Effct Green (s)	9.5		9.5	9.9	9.9			74.8			74.8	74.8
Actuated g/C Ratio	0.11		0.11	0.11	0.11			0.83			0.83	0.83

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Lanes, Volumes, Timings 1: Main Street & Franklin Street

	۶	→	*	1	•	*	1	†	1	1	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.42		0.14	0.03	0.19			0.37			0.19	0.08
Control Delay	48.2		15.9	33.6	23.9			3.2			2.7	0.7
Queue Delay	0.0		0.0	0.0	0.0			0.0			0.0	0.0
Total Delay	48.2		15.9	33.6	23.9			3.2			2.7	0.7
LOS	D		В	С	С			Α			Α	А
Approach Delay		37.4			25.1			3.2			2.2	
Approach LOS		D			С			Α			Α	
Queue Length 50th (ft)	25		0	3	9			61			30	0
Queue Length 95th (ft)	57		22	12	35			98			65	11
Internal Link Dist (ft)		100			302			140			338	
Turn Bay Length (ft)	35											
Base Capacity (vph)	292		398	410	448			1477			1613	1284
Starvation Cap Reductn	0		0	0	0			0			0	0
Spillback Cap Reductn	0		0	0	0			0			0	0
Storage Cap Reductn	0		0	0	0			0			0	0
Reduced v/c Ratio	0.16		0.06	0.01	0.08			0.37			0.19	0.08
Intersection Summary												
, , , , , , , , , , , , , , , , , , ,	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 0 (0%), Referenced t	o phase 2:I	NBTL and	l 6:SBT, S	Start of G	reen							
Natural Cycle: 50												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.42												
Intersection Signal Delay: 5.					tersection							
Intersection Capacity Utiliza	tion 66.8%			IC	CU Level of	of Service	С					
Analysis Period (min) 15												
Splits and Phases: 1: Mai	n Street & I	- Franklin S	Street									
Ø2 (R)							200	₹ ø4			11.1	- 50
60 s							3	0 s			- 1	
Ø6 (R)								Ø8				- 35

	۶	→	•	•	•	•	1	†	~	-	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7		†	7		^	7	7	7>	
Traffic Volume (vph)	20	32	12	0	32	7	0	419	2	9	260	17
Future Volume (vph)	20	32	12	0	32	7	0	419	2	9	260	17
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		60	0		55	0		0	60		0
Storage Lanes	0		1	0		1	0		1	1		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.97				0.89			0.91	0.97	1.00	
Frt			0.850			0.850			0.850		0.991	
Flt Protected		0.981								0.950		
Satd. Flow (prot)	0	1864	1615	0	2000	1615	0	1961	1615	1626	1809	0
Flt Permitted		0.858								0.475		
Satd. Flow (perm)	0	1575	1615	0	2000	1436	0	1961	1476	792	1809	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)			18			18			18		7	
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		405			420			116			254	
Travel Time (s)		11.0			11.5			3.2			6.9	
Confl. Peds. (#/hr)	37					37	28		27	27		28
Confl. Bikes (#/hr)												
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	2%	0%	11%	4%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	60	14	0	37	8	0	487	2	10	322	0
Turn Type	Perm	NA	Prot		NA	Perm		NA	Perm	Perm	NA	
Protected Phases		4	4		8			2			6	
Permitted Phases	4					8			2	6		
Detector Phase	4	4	4		8	8		2	2	6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	
Minimum Split (s)	22.5	22.5	22.5		22.5	22.5		22.5	22.5	22.5	22.5	
Total Split (s)	30.0	30.0	30.0		30.0	30.0		60.0	60.0	60.0	60.0	
Total Split (%)	33.3%	33.3%	33.3%		33.3%	33.3%		66.7%	66.7%	66.7%	66.7%	
Yellow Time (s)	3.5	3.5	3.5		3.5	3.5		3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0		1.0	1.0	1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	
Total Lost Time (s)		4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None		None	None		C-Min	C-Min	C-Min	C-Min	
Act Effct Green (s)		8.6	8.6		8.6	8.6		75.3	75.3	75.3	75.3	
Actuated g/C Ratio		0.10	0.10		0.10	0.10		0.84	0.84	0.84	0.84	

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Lanes, Volumes, Timings 2: Main Street & Warren Avenue

08/23/2024

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		0.40	0.08		0.19	0.05		0.30	0.00	0.02	0.21	
Control Delay		45.2	14.7		38.6	8.6		2.9	0.0	2.2	2.4	
Queue Delay		0.0	0.0		0.0	0.0		6.9	0.0	0.0	0.0	
Total Delay		45.2	14.7		38.6	8.6		9.8	0.0	2.2	2.4	
LOS		D	В		D	Α		Α	Α	Α	Α	
Approach Delay		39.4			33.3			9.7			2.4	
Approach LOS		D			С			Α			Α	
Queue Length 50th (ft)		33	0		20	0		52	0	1	30	
Queue Length 95th (ft)		66	14		45	7		93	0	4	55	
Internal Link Dist (ft)		325			340			36			174	
Turn Bay Length (ft)			60			55				60		
Base Capacity (vph)		446	470		566	419		1640	1237	662	1513	
Starvation Cap Reductn		0	0		0	0		1093	1128	0	0	
Spillback Cap Reductn		0	0		0	0		0	0	0	0	
Storage Cap Reductn		0	0		0	0		0	0	0	0	
Reduced v/c Ratio		0.13	0.03		0.07	0.02		0.89	0.02	0.02	0.21	
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 0 (0%), Referenced	d to phase 2:	NBT and	6:SBTL, S	Start of G	reen							
Natural Cycle: 45												

Control Type: Actuated-Coordinated

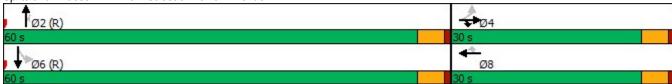
Maximum v/c Ratio: 0.40

Intersection Signal Delay: 10.6 Intersection Capacity Utilization 48.2%

Intersection LOS: B ICU Level of Service A

Analysis Period (min) 15

Splits and Phases: 2: Main Street & Warren Avenue



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Intersection Capacity Utilization 3: Forest Avenue & Franklin Street

	۶	→	•	•	•	•	4	†	-	-	ţ	4
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	3	13	23	113	41	9	19	181	43	8	69	4
Pedestrians	1		3	3		1	3		4	4		3
Ped Button		Yes			Yes			Yes			Yes	
Pedestrian Timing (s)		16.0			16.0			16.0			16.0	
Free Right			No			No			No			No
Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Refr Cycle Length (s)	120	120	120	120	120	120	120	120	120	120	120	120
Volume Combined (vph)	0	39	0	0	163	0	0	243	0	0	81	0
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Factor (vph)	0.95	0.91	0.85	0.95	0.96	0.85	0.95	0.97	0.85	0.95	0.99	0.85
Saturated Flow (vph)	0	1725	0	0	1819	0	0	1842	0	0	1877	0
Ped Intf Time (s)	0.0	0.2	0.4	0.0	0.0	0.1	0.0	0.1	0.5	0.0	0.0	0.4
Pedestrian Frequency (%)		0.10			0.03			0.12			0.10	
Protected Option Allowed		No			No			No			No	
Reference Time (s)			0.0			0.0			0.0			0.0
Adj Reference Time (s)			0.0			0.0			0.0			0.0
Permitted Option												
Adj Saturation A (vph)	0	1757		0	361		0	1477		0	1344	
Reference Time A (s)	0.0	2.9		0.0	54.2		0.0	19.8		0.0	7.3	
Adj Saturation B (vph	0	0		0	0		0	0		NA	NA	
Reference Time B (s)	8.2	10.9		15.5	18.8		9.3	23.9		NA	NA	
Reference Time (s)		2.9			18.8			19.8			7.3	
Adj Reference Time (s)		9.1			22.8			23.8			12.1	
Split Option		-			-							
Ref Time Combined (s)	0.0	2.9		0.0	10.8		0.0	15.9		0.0	5.2	
Ref Time Seperate (s)	0.2	1.1		7.5	2.7		1.3	11.9		0.5	4.4	
Reference Time (s)	2.9	2.9		10.8	10.8		15.9	15.9		5.2	5.2	
Adj Reference Time (s)	9.1	9.1		14.9	14.9		19.9	19.9		10.2	10.2	
			ND OD									
Summary	EB WB		NB SB	Co	mbined							
Protected Option (s)	NA		NA									
Permitted Option (s)	22.8		23.8									
Split Option (s)	24.1		30.2		40.0							
Minimum (s)	22.8		23.8		46.6							
Right Turns												
Adj Reference Time (s)												
Cross Thru Ref Time (s)												
Oncoming Left Ref Time (s)												
Combined (s)												
Intersection Summary												
Intersection Capacity Utiliza	tion		38.8%	IC	U Level o	of Service			А			
Reference Times and Phasi		do not re	present a	ın optimiz	ed timing	plan.						

Intersection Capacity Utilization 5: Forest Avenue & Warren Avenue

	۶	•	1	†	ļ	4	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	*	7		4	1>	02.1	
Volume (vph)	45	107	120	200	141	47	
Pedestrians	10	101	5	200		5	
Ped Button					Yes		
Pedestrian Timing (s)					16.0		
Free Right		No				No	
Ideal Flow	1900	1900	1900	1900	1900	1900	
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	
Refr Cycle Length (s)	120	120	120	120	120	120	
Volume Combined (vph)	45	107	0	320	188	0	
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Factor (vph)	0.95	0.85	0.95	0.98	0.96	0.85	
Saturated Flow (vph)	1805	1615	0	1864	1829	0	
Ped Intf Time (s)	0.0	0.0	0.0	0.0	0.2	0.6	
Pedestrian Frequency (%)	0.00			0.00	0.15		
Protected Option Allowed	No			No	No		
Reference Time (s)		8.0				0.0	
Adj Reference Time (s)		12.0				0.0	
Permitted Option							
Adj Saturation A (vph)	120		0	273	1829		
Reference Time A (s)	44.9		0.0	140.6	12.5		
Adj Saturation B (vph	NA		NA	NA	NA		
Reference Time B (s)	NA		NA	NA	NA		
Reference Time (s)				140.6	12.5		
Adj Reference Time (s)				144.6	17.0		
Split Option							
Ref Time Combined (s)	3.0		0.0	20.6	12.5		
Ref Time Seperate (s)	3.0		8.0	12.6	9.4		
Reference Time (s)	3.0		20.6	20.6	12.5		
Adj Reference Time (s)	8.0		24.6	24.6	17.0		
Summary	EB		NB SB	Co	mbined		
				CO	ilibilieu		
Protected Option (s)	NA		NA 144.6				
Permitted Option (s) Split Option (s)	Err 8.0		144.6 41.6				
Minimum (s)	8.0		41.6		49.6		
iviiriiIIIuIII (S)	0.0		41.0		49.0		
Right Turns	EBR						
Adj Reference Time (s)	12.0						
Cross Thru Ref Time (s)	17.0						
Oncoming Left Ref Time (s)	0.0						
Combined (s)	29.0						
Intersection Summary							
Intersection Capacity Utilization	n		41.4%	IC	U Level c	of Service	
Reference Times and Phasing		do not re					

4: Forest Avenue & AT&T Access Drive/Apartment Access Drive

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	1	1	0	2	1	238	0	0	197	1
Future Vol, veh/h	1	0	1	1	0	2	1	238	0	0	197	1
Conflicting Peds, #/hr	0	0	0	0	0	0	9	0	0	0	0	9
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	87	87	87	87	87	87	87	87	87	87	87	87
Heavy Vehicles, %	100	0	0	0	0	0	0	1	0	0	2	100
Mvmt Flow	1	0	1	1	0	2	1	274	0	0	226	1
Major/Minor N	1inor2		1	Minor1		N	Major1		N	Major2		
Conflicting Flow All	513	512	236	503	512	274	236	0	0	274	0	0
Stage 1	236	236	-	276	276	-	-	-	-	-	-	-
Stage 2	277	276	-	227	236	-	-	-	-	-	-	-
Critical Hdwy	8.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	-	-
Critical Hdwy Stg 1	7.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	7.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	4.4	4	3.3	3.5	4	3.3	2.2	-	-	2.2	-	-
Pot Cap-1 Maneuver	347	468	808	482	468	770	1343	-	-	1301	-	-
Stage 1	591	713	-	735	685	-	-	-	-	-	-	-
Stage 2	559	685	-	780	713	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	343	463	801	481	463	770	1331	-	-	1301	-	-
Mov Cap-2 Maneuver	343	463	-	481	463	-	-	-	-	-	-	-
Stage 1	585	707	-	734	684	-	-	-	-	-	-	-
Stage 2	557	684	-	779	707	-	-	-	-	-	-	-
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.5			10.6			0			0		
HCM LOS	В			В								
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1331	-	-	400	642	1301	-	-			
HCM Lane V/C Ratio		0.001	-	-	0.005		-	-	-			
HCM Control Delay (s)		7.7	0	-		10.6	0	-	-			
HCM Lane LOS		Α	Α	-	В	В	Α	-	-			
HCM 95th %tile Q(veh)		0	-	-	0	0	0	-	-			
,												

6: Warren Avenue & Forest Avenue

Intersection						
Int Delay, s/veh	1.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*	7	↑	7		41
Traffic Vol, veh/h	16	40	279	35	37	211
Future Vol, veh/h	16	40	279	35	37	211
Conflicting Peds, #/hr	1	0	0	15	15	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None		None	-	None
Storage Length	0	0	_	0	_	-
Veh in Median Storage		-	0	-	_	0
Grade, %	0	<u>-</u>	0	_	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	2	0	0	3
	17	42	294	37	39	222
Mvmt Flow	17	42	294	31	39	222
Major/Minor	Minor1	N	Major1	l	Major2	
Conflicting Flow All	499	309	0	0	346	0
Stage 1	309	_	_	-	_	-
Stage 2	190	-	-	_	_	_
Critical Hdwy	6.6	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.4	-	_	_		_
Critical Hdwy Stg 2	5.8	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	_	-	2.2	_
		736	-	_	1224	
Pot Cap-1 Maneuver	520		-	-	1224	-
Stage 1	749	-	-	-	-	-
Stage 2	829	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	493	725	-	-	1207	-
Mov Cap-2 Maneuver	493	-	-	-	-	-
Stage 1	739	-	-	-	-	-
Stage 2	797	-	-	-	-	-
Annyanah	WD		ND		CD	
Approach	WB		NB		SB	
HCM Control Delay, s	11		0		1.3	
HCM LOS	В					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1V	VBLn2	SBL
Capacity (veh/h)		-	-			1207
HCM Lane V/C Ratio		_			0.058	
HCM Control Delay (s)		<u>-</u>	_		10.3	8.1
HCM Lane LOS		-		12.0 B	10.3 B	
	\	-	-	0.1	0.2	0.1
HCM 95th %tile Q(veh		-	-	0.1	0.2	U. I

HCM 6th TWSC 8: N-S Alley & Franklin Street

Intersection						
Int Delay, s/veh	0.6					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	4			4	¥	
Traffic Vol, veh/h	63	3	9	153	6	3
Future Vol, veh/h	63	3	9	153	6	3
Conflicting Peds, #/hr		11	11	0	0	12
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None		None	-	None
Storage Length	_	-	_	-	0	-
Veh in Median Storag		_	_	0	0	_
Grade, %	0	_	_	0	0	<u>-</u>
Peak Hour Factor	78	78	78	78	78	78
	5	0	0	1		
Heavy Vehicles, %					0	0
Mvmt Flow	81	4	12	196	8	4
Major/Minor	Major1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	96	0	314	106
Stage 1	_	_	-	_	94	-
Stage 2	_	_	_	_	220	_
Critical Hdwy	_	_	4.1	_	6.4	6.2
Critical Hdwy Stg 1	_	_	4.1	_	5.4	0.2
		-	_		5.4	
Critical Hdwy Stg 2	-	-	-	-		-
Follow-up Hdwy	-	-	2.2	-	3.5	3.3
Pot Cap-1 Maneuver	-	-	1510	-	737	954
Stage 1	-	-	-	-	935	-
Stage 2	-	-	-	-	860	-
Platoon blocked, %	-	-		-	1	
Mov Cap-1 Maneuver	-	-	1494	-	723	933
Mov Cap-2 Maneuver		-	-	-	723	-
Stage 1	_	-	-	_	926	-
Stage 2	_	-	-	-	852	-
-						
			\4/D			
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		9.7	
HCM LOS					Α	
Minor Lane/Major Mv	mt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)	1110 1	782	-		1494	
						-
HCM Lane V/C Ratio	. \	0.015	-		0.008	-
HCM Control Delay (s	5)	9.7	-	-		0
HCM Lane LOS		A	-	-	A	Α
HCM 95th %tile Q(vel	n)	0	-	-	0	-

HCM 6th TWSC 9: Main Street & Oakley Access Drive

Intersection								
Int Delay, s/veh	0							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	¥	רטוג	HUL	4	- 1dC	אופט		
Traffic Vol, veh/h	0	0	0	494	289	0		
Future Vol, veh/h	0	0	0	494	289	0		
Conflicting Peds, #/hr		0	13	0	0	13		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None		None		None		
Storage Length	0	-	-	-	-	-		
Veh in Median Storag	e,# 0	-	-	0	0	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	93	93	93	93	93	93		
Heavy Vehicles, %	0	0	0	3	4	0		
Mvmt Flow	0	0	0	531	311	0		
Major/Minor	Minor2	ı	Major1	N	//ajor2			
Conflicting Flow All	855	324	324	0	- viajuiz	0		
Stage 1	324	324	324	-	-	-		
Stage 2	531	-	-	-	-	-		
Critical Hdwy	6.4	6.2	4.1		_			
Critical Hdwy Stg 1	5.4	0.2	- 7 . 1	-	-	_		
Critical Hdwy Stg 2	5.4	_	_	_	_	_		
Follow-up Hdwy	3.5	3.3	2.2	_	_	_		
Pot Cap-1 Maneuver	*426	849	1280	_	_	_		
Stage 1	*803	-	00	_	_	_		
Stage 2	*642	-	_	-	_	-		
Platoon blocked, %	1	1	1	_	-	_		
Mov Cap-1 Maneuver		839	1264	-	-	-		
Mov Cap-2 Maneuver		-		-	-	_		
Stage 1	*793	-	-	-	-	-		
Stage 2	*634	-	-	-	-	-		
y -								
Annuari	ED		ND		OD			
Approach	EB		NB		SB			
HCM Control Delay, s			0		0			
HCM LOS	A							
Minor Lane/Major Mvi	mt	NBL	NBT	EBLn1	SBT	SBR		
Capacity (veh/h)		1264	_	-	-	-		
HCM Lane V/C Ratio		-	-	-	-	-		
HCM Control Delay (s	s)	0	-	0	-	-		
HCM Lane LOS	,	A	-	A	-	-		
HCM 95th %tile Q(veh	1)	0	-	-	-	-		
,								
Notes	nnoit.	¢. D.	lov, exc	and of)() ₀	Cara	utation Not Defined	
~: Volume exceeds ca	apacity	\$: De	lay exc	eeds 30	JUS	+: Com	outation Not Defined	

10: Main Street & Funeral Home North Access Drive

Intersection									
Int Delay, s/veh	0								
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	W	LDIT	HUL	4	1	ODIT			
Traffic Vol, veh/h	0	1	0	517	302	0			
Future Vol, veh/h	0	1	0	517	302	0			
Conflicting Peds, #/hr	0	0	13	0	0	13			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	Stop -	None	-		-	None			
Storage Length	0	INOHE -	_			-			
			-	0	0				
Veh in Median Storage		-							
Grade, %	0	-	-	0	0	-			
Peak Hour Factor	93	93	93	93	93	93			
Heavy Vehicles, %	0	0	0	3	4	0			
Mvmt Flow	0	1	0	556	325	0			
Major/Minor	Minor2		Major1	N	/lajor2				
Conflicting Flow All	894	338	338	0	-	0			
Stage 1	338	-	-	-	-	-			
Stage 2	556	-	-	-	-	-			
Critical Hdwy	6.4	6.2	4.1	-	_	_			
Critical Hdwy Stg 1	5.4	-	-	_	-	_			
Critical Hdwy Stg 2	5.4	-	-	-	-	_			
Follow-up Hdwy	3.5	3.3	2.2	_	-	_			
Pot Cap-1 Maneuver	*411	*825	*1238	_	_	_			
Stage 1	*778	-	-	_	_	_			
Stage 2	*608	_	_	_	_	_			
Platoon blocked, %	1	1	1	_	_	_			
Mov Cap-1 Maneuver			*1223	_	_	_			
Mov Cap-2 Maneuver		-	-	_	_	_			
Stage 1	*769	_	_	_	_	_			
Stage 2	*601		_			_			
Glaye Z	001	_	_	_	-	_			
Approach	EB		NB		SB				
HCM Control Delay, s	9.4		0		0				
HCM LOS	Α								
Minor Lane/Major Mvr	nt	NBL	NRT	EBLn1	SBT	SBR			
Capacity (veh/h)		* 1223	-	815	-	יופט			
HCM Lane V/C Ratio		1225		0.001	_	_			
HCM Control Delay (s)	0	-	9.4	_	-			
HCM Lane LOS)	A	_	9.4 A	_	-			
HCM 95th %tile Q(veh	1)	0	-	0	-				
	IJ	U	•	U	-	-			
Notes									
~: Volume exceeds ca	pacity	\$: De	elay exc	eeds 30	00s	+: Comp	outation Not Defined	*: All major volume in platoon	

HCM 6th TWSC 11: Main Street & Rogers Street

Intersection								
nt Delay, s/veh	1.8							
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
ane Configurations	**	אפוזי	13€	TIDIT)	<u> </u>		
raffic Vol, veh/h	18	80	430	18	35	271		
uture Vol, veh/h	18	80	430	18	35	271		
onflicting Peds, #/hr	4	5	0	19	19	0		
gn Control	Stop	Stop	Free	Free	Free	Free		
T Channelized	Stop -	None		None		None		
orage Length	0	-	-	NOHE	60	None		
eh in Median Storage		-	0	-	-	0		
ade, %	0		0			0		
	91	- 01	91	- 01	- 01	91		
eak Hour Factor		91		91	91			
eavy Vehicles, %	0	5	472	0	3	5		
mt Flow	20	88	473	20	38	298		
ajor/Minor I	Minor1	N	Major1		Major2			
nflicting Flow All	880	507	0	0	512	0		
Stage 1	502	-	-	-	-	-		
Stage 2	378	-	-	-	-	-		
ritical Hdwy	6.4	6.25	-	-	4.13	-		
itical Hdwy Stg 1	5.4	-	-	-	-	-		
itical Hdwy Stg 2	5.4	-	-	-	-	-		
llow-up Hdwy	3.5	3.345	-	-	2.227	-		
ot Cap-1 Maneuver	*365	*707	-	-	*1062	-		
Stage 1	*676	-	-	-	-	-		
Stage 2	*749	-	-	-	-	-		
atoon blocked, %	1	1	-	-	1	-		
ov Cap-1 Maneuver	*344	*691	-	-	*1043	-		
ov Cap-2 Maneuver	*344	-	-	-	-	-		
Stage 1	*664	-	-	-	-	-		
Stage 2	*719	-	-	-	-	-		
proach	WB		NB		SB			
CM Control Delay, s	12.6		0		1			
CM LOS	12.0 B		U		1			
DIVI LUS	В							
inor Lane/Major Mvm	ıt	NBT	NBRV	VBLn1	SBL	SBT		
pacity (veh/h)		-	-		* 1043	-		
CM Lane V/C Ratio		-	-	0.185		-		
CM Control Delay (s)		-	-	12.6	8.6	-		
CM Lane LOS		-	-	В	Α	-		
CM 95th %tile Q(veh)		-	-	0.7	0.1	-		
otes								
Volume exceeds car	nacity	\$· De	lay exc	eeds 3	00s	+· Comr	outation Not Defined	*: All major volume in platoon
Volume exceeds cap	Judity	ψ. De	ay cat	ccus o	003	·. Com	Jalation Not Defined	. All major volume in platoon

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<u>Capacity Analysis Summary Sheets</u> Year 2030 No-Build Weekday Evening Peak Hour

Lanes, Volumes, Timings 1: Main Street & Franklin Street

	۶	→	*	•	←	•	4	†	~	/	Ţ	✓
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*		7	7	f)			र्स			^	7
Traffic Volume (vph)	51	0	24	3	8	18	39	412	0	0	472	154
Future Volume (vph)	51	0	24	3	8	18	39	412	0	0	472	154
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	35	• 70	0	0	• 70	0	0	• 70	0	0	0,0	0
Storage Lanes	1		1	1		0	0		0	0		1
Taper Length (ft)	25		•	25		•	25			25		•
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99	1.00	0.95	0.98	0.98	1.00	1.00	1.00	1.00	1.00	1.00	0.96
Frt	0.00		0.850	0.00	0.894			1.00				0.850
Flt Protected	0.950		0.000	0.950	0.001			0.996				0.000
Satd. Flow (prot)	1805	0	1615	1805	1659	0	0	1892	0	0	1980	1615
Flt Permitted	0.740	· ·	1010	0.950	1000	•		0.938		•	1000	1010
Satd. Flow (perm)	1389	0	1537	1764	1659	0	0	1781	0	0	1980	1546
Right Turn on Red	1003	U	Yes	1704	1003	Yes	0	1701	Yes	U	1500	Yes
Satd. Flow (RTOR)			25		19	103			103			159
Link Speed (mph)		25	20		25			25			25	100
Link Distance (ft)		180			382			220			418	
Travel Time (s)		4.9			10.4			6.0			11.4	
Confl. Peds. (#/hr)	5	4.3	9	9	10.4	5	9	0.0	9	9	11.4	9
Confl. Bikes (#/hr)	J		5	9		1	9		9	9		1
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%
• ,	0 %	0 %	0 /0	0 /8	0 %	0 /8	0 /0	0 %	0 %	0 /8	0	0 %
Bus Blockages (#/hr) Parking (#/hr)	U	U	U	U	U	U	U	U	U	U	U	U
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)		0 /0			0 /0			0 /0			0 /0	
	53	0	25	3	27	0	0	465	0	0	487	159
Lane Group Flow (vph)	Perm	U	Perm	Perm	NA	U	Perm	NA	U	U	NA	
Turn Type Protected Phases	Pellii		Pellii	reiiii	8		Pellii	2				Perm
	1		1	0	0		2				6	G
Permitted Phases	4		4	8	0		2	2			6	6
Detector Phase	4		4	8	8		2	2			6	6
Switch Phase	F 0		F 0	F 0	F 0		0.0	0.0			0.0	0.0
Minimum Initial (s)	5.0		5.0	5.0	5.0		8.0	8.0			8.0	8.0
Minimum Split (s)	22.5		22.5	22.5	22.5		22.5	22.5			22.5	22.5
Total Split (s)	30.0		30.0	30.0	30.0		60.0	60.0			60.0	60.0
Total Split (%)	33.3%		33.3%	33.3%	33.3%		66.7%	66.7%			66.7%	66.7%
Yellow Time (s)	3.5		3.5	3.0	3.0		3.0	3.0			3.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0			1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			0.0			0.0	0.0
Total Lost Time (s)	4.5		4.5	4.0	4.0			4.0			4.0	4.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None		None	None	None		C-Min	C-Min			C-Min	C-Min
Act Effct Green (s)	8.9		8.9	9.3	9.3			75.4			75.4	75.4
Actuated g/C Ratio	0.10		0.10	0.10	0.10			0.84			0.84	0.84

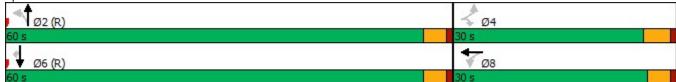
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Lanes, Volumes, Timings 1: Main Street & Franklin Street

08/23/2024

	•	→	*	1	←	*	4	†	1	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.39		0.14	0.02	0.14			0.31			0.29	0.12
Control Delay	45.3		16.0	34.3	20.7			2.8			2.8	0.6
Queue Delay	0.0		0.0	0.0	0.0			0.0			0.0	0.0
Total Delay	45.3		16.0	34.3	20.7			2.8			2.8	0.6
LOS	D		В	С	С			Α			Α	Α
Approach Delay		35.9			22.1			2.8			2.3	
Approach LOS		D			С			Α			Α	
Queue Length 50th (ft)	29		0	2	4			47			51	0
Queue Length 95th (ft)	63		23	9	28			90			101	11
Internal Link Dist (ft)		100			302			140			338	
Turn Bay Length (ft)	35											
Base Capacity (vph)	393		453	509	492			1492			1658	1321
Starvation Cap Reductn	0		0	0	0			0			0	0
Spillback Cap Reductn	0		0	0	0			0			0	0
Storage Cap Reductn	0		0	0	0			0			0	0
Reduced v/c Ratio	0.13		0.06	0.01	0.05			0.31			0.29	0.12
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 0 (0%), Referenced	to phase 2:1	NBTL and	16:SBT, 8	Start of G	reen							
Natural Cycle: 45												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.39												
Intersection Signal Delay: 5					tersection							
Intersection Capacity Utiliza	tion 67.9%			IC	U Level o	of Service	С					
Analysis Period (min) 15												

Splits and Phases: 1: Main Street & Franklin Street



	۶	→	*	•	•	4	4	†	~	/	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7		↑	7		^	7	*	₽	
Traffic Volume (vph)	22	54	7	0	47	18	0	339	17	4	436	19
Future Volume (vph)	22	54	7	0	47	18	0	339	17	4	436	19
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	-
Storage Length (ft)	0		60	0		55	0		0	60		0
Storage Lanes	0		1	0		1	0		1	1		0
Taper Length (ft)	25		-	25		-	25		•	25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.97			,,,,,,	0.88			0.86	0.94	1.00	
Frt			0.850			0.850			0.850		0.994	
Flt Protected		0.986								0.950		
Satd. Flow (prot)	0	1873	1615	0	2000	1615	0	1980	1615	1805	1867	0
Flt Permitted	· ·	0.886	1010		2000	1010	· ·	1000	1010	0.542	1001	
Satd. Flow (perm)	0	1637	1615	0	2000	1417	0	1980	1393	965	1867	0
Right Turn on Red	•	1001	Yes		2000	Yes	· ·	1000	Yes	000	1001	Yes
Satd. Flow (RTOR)			18			20			18		5	. 00
Link Speed (mph)		25			25			25	10		25	
Link Distance (ft)		405			420			116			254	
Travel Time (s)		11.0			11.5			3.2			6.9	
Confl. Peds. (#/hr)	41	11.0	4	4	11.0	41	27	0.2	48	48	0.0	27
Confl. Bikes (#/hr)	• •		2	•		2	- 1		10	10		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)		0 70			070			0 70			070	
Lane Group Flow (vph)	0	83	8	0	51	20	0	368	18	4	495	0
Turn Type	Perm	NA	Prot		NA	Perm		NA	Perm	Perm	NA	
Protected Phases	1 01111	4	4		8	1 01111		2	1 01111	1 01111	6	
Permitted Phases	4		- Т		J	8			2	6	U	
Detector Phase	4	4	4		8	8		2	2	6	6	
Switch Phase	<u> </u>		- Т		J						U	
Minimum Initial (s)	5.0	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	
Minimum Split (s)	22.5	22.5	22.5		22.5	22.5		22.5	22.5	22.5	22.5	
Total Split (s)	30.0	30.0	30.0		30.0	30.0		60.0	60.0	60.0	60.0	
Total Split (%)	33.3%	33.3%	33.3%		33.3%	33.3%		66.7%	66.7%	66.7%	66.7%	
Yellow Time (s)	3.5	3.5	3.5		3.5	3.5		3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0		1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	1.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	
Total Lost Time (s)		4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	
. ,		4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	
Lead/Lag												
Lead-Lag Optimize?	Nana	Mono	Mona		None	Mona		C Min	C Min	C Min	C-Min	
Recall Mode	None	None	None		None	None		C-Min	C-Min	C-Min		
Act Effct Green (s)		9.7	9.7		9.7	9.7		74.2	74.2	74.2	74.2	
Actuated g/C Ratio		0.11	0.11		0.11	0.11		0.82	0.82	0.82	0.82	

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Lanes, Volumes, Timings

2: Main Street & Warren Avenue

08/23/2024

	•	\rightarrow	*	1	•	*	1	Ť	1	1	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		0.47	0.04		0.24	0.12		0.23	0.02	0.01	0.32	
Control Delay		45.6	8.0		38.1	16.2		2.9	1.2	2.5	3.3	
Queue Delay		0.0	0.0		0.0	0.0		5.0	0.6	0.0	0.0	
Total Delay		45.6	8.0		38.1	16.2		7.9	1.9	2.5	3.3	
LOS		D	Α		D	В		Α	Α	Α	Α	
Approach Delay		42.3			32.0			7.7			3.3	
Approach LOS		D			С			Α			Α	
Queue Length 50th (ft)		45	0		27	0		40	0	1	59	
Queue Length 95th (ft)		87	7		59	20		79	5	m2	110	
Internal Link Dist (ft)		325			340			36			174	
Turn Bay Length (ft)			60			55				60		
Base Capacity (vph)		463	470		566	415		1633	1152	796	1541	
Starvation Cap Reductn		0	0		0	0		1184	1022	0	0	
Spillback Cap Reductn		0	0		0	0		0	0	0	0	
Storage Cap Reductn		0	0		0	0		0	0	0	0	
Reduced v/c Ratio		0.18	0.02		0.09	0.05		0.82	0.14	0.01	0.32	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 45

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.47

Intersection Signal Delay: 10.2
Intersection Capacity Utilization 46.1%

Intersection LOS: B
ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Main Street & Warren Avenue



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Intersection Capacity Utilization 3: Forest Avenue & Franklin Street

	۶	-	•	•	•	•	1	†	-	-	ţ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	12	13	45	154	40	1	20	104	58	2	45	1
Pedestrians			7	7			3		5	5		3
Ped Button		Yes						Yes			Yes	
Pedestrian Timing (s)		16.0						16.0			16.0	
Free Right			No			No			No			No
Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Refr Cycle Length (s)	120	120	120	120	120	120	120	120	120	120	120	120
Volume Combined (vph)	0	70	0	0	195	0	0	182	0	0	48	0
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Factor (vph)	0.95	0.90	0.85	0.95	0.96	0.85	0.95	0.95	0.85	0.95	0.99	0.85
Saturated Flow (vph)	0	1702	0	0	1824	0	0	1799	0	0	1890	0
Ped Intf Time (s)	0.0	0.6	0.9	0.0	0.0	0.0	0.0	0.2	0.6	0.0	0.0	0.4
Pedestrian Frequency (%)		0.21			0.00			0.15			0.10	
Protected Option Allowed		No			No			No			No	
Reference Time (s)			0.0			0.0			0.0			0.0
Adj Reference Time (s)			0.0			0.0			0.0			0.0
Permitted Option												
Adj Saturation A (vph)	0	1776		0	530		0	1054		0	1700	
Reference Time A (s)	0.0	5.3		0.0	44.1		0.0	20.9		0.0	3.4	
Adj Saturation B (vph	0	0		0	0		0	0		NA	NA	
Reference Time B (s)	8.8	13.5		18.2	20.8		9.3	20.3		NA	NA	
Reference Time (s)		5.3			20.8			20.3			3.4	
Adj Reference Time (s)		11.5			24.8			24.3			9.1	
Split Option												
Ref Time Combined (s)	0.0	5.5		0.0	12.8		0.0	12.3		0.0	3.1	
Ref Time Seperate (s)	0.8	1.5		10.2	2.5		1.3	7.1		0.1	2.9	
Reference Time (s)	5.5	5.5		12.8	12.8		12.3	12.3		3.1	3.1	
Adj Reference Time (s)	11.7	11.7		16.8	16.8		16.9	16.9		9.1	9.1	
Summary	EB WB		NB SB	Co	mbined							
Protected Option (s)	NA		NA	- 00	mbined							
Permitted Option (s)	24.8		24.3									
Split Option (s)	28.5		26.0									
Minimum (s)	24.8		24.3		49.2							
. ,	24.0		24.0		43.2							
Right Turns												
Adj Reference Time (s)												
Cross Thru Ref Time (s)												
Oncoming Left Ref Time (s)												
Combined (s)												
Intersection Summary												
Intersection Capacity Utilizat	tion		41.0%	IC	U Level	of Service			Α			
Reference Times and Phasir	ng Options	do not re	epresent a	an optimiz	ed timing	plan.						

Intersection Capacity Utilization 5: Forest Avenue & Warren Avenue

	۶	•	4	1	ļ	1	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	*	7		र्स	7		
Volume (vph)	29	202	84	142	195	18	
edestrians	10		14			14	
ed Button					Yes		
edestrian Timing (s)					16.0		
ree Right		No				No	
eal Flow	1900	1900	1900	1900	1900	1900	
ost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	
inimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	
efr Cycle Length (s)	120	120	120	120	120	120	
olume Combined (vph)	29	202	0	226	213	0	
ne Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	
urning Factor (vph)	0.95	0.85	0.95	0.98	0.99	0.85	
aturated Flow (vph)	1805	1615	0.93	1865	1876	0.03	
ed Intf Time (s)	0.0	0.0	0.0	0.0	0.1	1.7	
edestrian Frequency (%)	0.00	0.0	0.0	0.00	0.1	1.1	
rotected Option Allowed	No			No	No		
•	INO	1E 0		INO	NO	0.0	
eference Time (s)		15.0				0.0	
j Reference Time (s)		19.0				0.0	
rmitted Option	400			075	4070		
j Saturation A (vph)	120		0	275	1876		
ference Time A (s)	28.9		0.0	98.5	13.8		
Saturation B (vph	NA		NA	NA	NA		
eference Time B (s)	NA		NA	NA	NA		
eference Time (s)				98.5	13.8		
dj Reference Time (s)				102.5	18.6		
lit Option							
ef Time Combined (s)	1.9		0.0	14.5	13.8		
ef Time Seperate (s)	1.9		5.6	9.0	12.6		
eference Time (s)	1.9		14.5	14.5	13.8		
lj Reference Time (s)	8.0		18.5	18.5	18.6		
mmary	EB		NB SB	Co	mbined		
otected Option (s)	NA		NA				
ermitted Option (s)	Err		102.5				
olit Option (s)	8.0		37.1				
nimum (s)	8.0		37.1		45.1		
ht Turns	EBR						
dj Reference Time (s)	19.0						
ross Thru Ref Time (s)	18.6						
ncoming Left Ref Time (s)	0.0						
ombined (s)	37.6						
	J1.U						
ersection Summary			07.00			10	
tersection Capacity Utilization		ala as i	37.6%			of Service	Α
eference Times and Phasing	g Options	ao not re	present a	an optimiz	ed timing	pian.	

4: Forest Avenue & AT&T Access Drive/Apartment Access Drive

Intersection												
Int Delay, s/veh	0.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol., veh/h	1	0	0	0	0	1	1	176	0	3	228	1
Future Vol, veh/h	1	0	0	0	0	1	1	176	0	3	228	1
Conflicting Peds, #/hr	0	0	2	2	0	0	16	0	0	0	0	16
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	_	_	None	_	-	None	_	_	None	-	_	None
Storage Length	-	-	_	-	-	-	_	-	-	-	-	_
Veh in Median Storage,	# -	0	-	_	0	-	_	0	-	-	0	_
Grade, %	_	0	_	-	0	-	_	0	_	-	0	_
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	0	0	0	0	0	100	0	0	0	0	0
Mvmt Flow	1	0	0	0	0	1	1	205	0	3	265	1
Major/Minor N	/linor2		ľ	Minor1			Major1		N	Major2		
Conflicting Flow All	496	495	284	481	495	205	282	0	0	205	0	0
Stage 1	288	288	-	207	207	-	-	-	-	-	-	-
Stage 2	208	207	-	274	288	-	-	-	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	5.1	-	-	4.1	-	-
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	-	-	-	-	-	-
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	3.1	-	-	2.2	-	-
Pot Cap-1 Maneuver	487	479	760	499	479	841	877	-	-	1378	-	-
Stage 1	724	677	-	800	734	-	-	-	-	-	-	-
Stage 2	799	734	-	736	677	-	-	-	-	-	-	-
Platoon blocked, %								-	_		-	-
Mov Cap-1 Maneuver	477	470	747	497	470	841	864	-	-	1378	-	-
Mov Cap-2 Maneuver	477	470	-	497	470	-	-	-	-	-	-	-
Stage 1	712	665	-	799	733	-	-	-	-	-	-	-
Stage 2	797	733	_	732	665	-	-	-	-	-	-	-
Ŭ '												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.6			9.3			0.1			0.1		
HCM LOS	В			Α								
Minor Lane/Major Mvmt		NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		864	-	-	477	841	1378	-	-			
HCM Lane V/C Ratio		0.001	-	-		0.001	0.003	-	-			
HCM Control Delay (s)		9.2	0	-	12.6	9.3	7.6	0	-			
HCM Lane LOS		Α	Α	-	В	Α	Α	Α	-			
HCM 95th %tile Q(veh)		0	_	_	0	0	0	_	_			

6: Warren Avenue & Forest Avenue

Intersection								
Int Delay, s/veh	1.6						•	
Movement	WBL	WBR	NBT	NBR	SBL	SBT	Į	
Lane Configurations	NDL N	7	<u>↑</u>	7	ODL	4₽	1	
Traffic Vol, veh/h	29	28	197	19	57	340		
Future Vol, veh/h	29	28	197	19	57	340		
Conflicting Peds, #/hr	0	0	0	28	28	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-			
Storage Length	0	0	<u>-</u>	0	_	-		
Veh in Median Storage		-	0	-	_	0		
Grade, %	σ, π 0	_	0	_	_	0		
Peak Hour Factor	94	94	94	94	94	94		
Heavy Vehicles, %	94	0	1	94	0	1		
Mvmt Flow	31	30	210	20	61	362		
IVIVIIIL FIOW	31	30	210	20	01	302		
Major/Minor N	Minor1	N	Major1	N	Major2			
Conflicting Flow All	541	238	0	0	258	0		
Stage 1	238	-	-	-	-	-		
Stage 2	303	_	_	_	-	_		
Critical Hdwy	6.6	6.2	_	_	4.1	-		
Critical Hdwy Stg 1	5.4	-	_	_	-	_		
Critical Hdwy Stg 2	5.8	_	_	_	-	_		
Follow-up Hdwy	3.5	3.3	_	_	2.2	_		
Pot Cap-1 Maneuver	*753	*928	_		*1392	_		
Stage 1	*875	-	<u>-</u>	_	1002	_		
Stage 1	*729			_	_	_		
Platoon blocked, %	123	1	-	_	1	_		
	*692	*903			*1355			
Mov Cap-1 Maneuver			-	-		-		
Mov Cap-2 Maneuver	*692	-	-		-	-		
Stage 1	*852	-	-	-	-	-		
Stage 2	*688	-	-	-	-			
Approach	WB		NB		SB			
HCM Control Delay, s	9.8		0		1.3			
HCM LOS	Α		U		1.0			
TIOW LOO								
Minor Lane/Major Mvm	it	NBT	NBRV	VBLn1V	VBLn2	SBL		SBT
Capacity (veh/h)		-	-	692	903	* 1355		-
HCM Lane V/C Ratio		-	-	0.045	0.033	0.045		-
HCM Control Delay (s)		-	-	10.4	9.1	7.8		0.2
HOW CONTION DETAY (3)				В	Α	Α		Α
HCM Lane LOS		-	-	ט				
		-	-	0.1	0.1	0.1		-
HCM Lane LOS HCM 95th %tile Q(veh)		-	-			0.1		-
HCM Lane LOS)	-	-		0.1			utation Not Defined

HCM 6th TWSC 8: N-S Alley & Franklin Street

Intersection						
Int Delay, s/veh	0.4					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	¥	
Traffic Vol, veh/h	65	3	6	188	3	5
Future Vol, veh/h	65	3	6	188	3	5
Conflicting Peds, #/hr	0	9	9	0	1	1
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	_	-	_	-	0	-
Veh in Median Storage	e, # 0	_	_	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	79	79	79	79	79	79
Heavy Vehicles, %	0	0	0	1	0	0
Mymt Flow	82	4	8	238	4	6
IVIVIIIL FIOW	02	4	O	230	4	U
Major/Minor	Major1	N	Major2	N	Minor1	
Conflicting Flow All	0	0	95	0	348	94
Stage 1	-	-	-	-	93	-
Stage 2	-	-	-	-	255	-
Critical Hdwy	-	-	4.1	-	6.4	6.2
Critical Hdwy Stg 1	_	_	_	_	5.4	-
Critical Hdwy Stg 2	_	_	_	_	5.4	_
Follow-up Hdwy	_	_	2.2	_	3.5	3.3
Pot Cap-1 Maneuver	_	_	1512	_	717	968
Stage 1	<u>-</u>	<u>-</u>	1012	<u>-</u>	936	-
Stage 2	_		_		837	_
Platoon blocked, %		-	-		1	-
· · · · · · · · · · · · · · · · · · ·	-	-	1400	-		050
Mov Cap-1 Maneuver	-	-	1499	-	705	959
Mov Cap-2 Maneuver	-	-	-	-	705	-
Stage 1	-	-	-	-	928	-
Stage 2	-	-	-	-	832	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.2		9.3	
HCM LOS	U		0.2		9.5 A	
TIGIVI LOS						
Minor Lane/Major Mvn	nt 1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		845	-	-		-
HCM Lane V/C Ratio		0.012	-	-	0.005	-
HCM Control Delay (s)		9.3	-	-	7.4	0
HCM Lane LOS		Α	-	-	Α	Α
HCM 95th %tile Q(veh)	0	-	-	0	-

9: Main Street & Oakley Access Drive

Intersection								
Int Delay, s/veh	0							
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	 ₩	LDR	NDL			אמט		
Traffic Vol, veh/h		0	0	4 435	1 → 478	0		
Future Vol, veh/h	1	0	0	435	478	0		
Conflicting Peds, #/hr	1	0	12	435	4/0	12		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	Stop	None		None		None		
Storage Length	0	None -		none -	-	None -		
			-	0	0			
Veh in Median Storage	•	-						
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	90	90	90	90	90	90		
Heavy Vehicles, %	0	0	0	1	0	0		
Mvmt Flow	1	0	0	483	531	0		
Major/Minor	Minor2		Major1	N	/lajor2			J
Conflicting Flow All	1027	543	543	0	-	0		
Stage 1	543	-	-	-	_	-		
Stage 2	484	_	_	_	_	_		
Critical Hdwy	6.4	6.2	4.1	_	_	_		
Critical Hdwy Stg 1	5.4	0.2	T. I	_	_	_		
Critical Hdwy Stg 2	5.4	_	_	_	_			
Follow-up Hdwy	3.5	3.3	2.2		_	_		
Pot Cap-1 Maneuver	*269		*1043	-	_			
Stage 1	*656	- 095	1043	-	-	-		
Stage 2	*689	-	-		-			
Platoon blocked, %	1	1	- 1	-		-		
· · · · · · · · · · · · · · · · · · ·			•	-	-	-		
Mov Cap-1 Maneuver				-	-	-		
Mov Cap-2 Maneuver	*264	-	-	-	-	-		
Stage 1	*649	-	-	-	-	-		
Stage 2	*681	-	-	-	-	-		
Approach	EB		NB		SB			
HCM Control Delay, s			0		0			
HCM LOS	C							
	J							
Minor Lane/Major Mvn		NBL	NBT	EBLn1	SBT	SBR		
Capacity (veh/h)		* 1031	-	264	-	-		
HCM Lane V/C Ratio		-	-	0.004	-	-		
HCM Control Delay (s))	0	-	18.7	-	-		
HCM Lane LOS		Α	-	С	-	-		
HCM 95th %tile Q(veh	ı)	0	-	0	-	-		
,								
Notes	!1	Φ. D.	Jan	00	١٥-	0	outstan Nat D. C I	
~: Volume exceeds ca	pacity	\$: De	elay exc	eeds 30	JUS ·	+: Comp	outation Not Defined	,

10: Main Street & Funeral Home North Access Drive

Intersection						
Int Delay, s/veh	0					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	**	בטונ	TIDE	4	\$	ODIN
Traffic Vol, veh/h	0	0	0	454	499	0
Future Vol, veh/h		0		454	499	0
	0		0			
Conflicting Peds, #/hr	0	0	13	0	0	13
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	0	0	0	0	1	0
Mvmt Flow	0	0	0	504	554	0
	•					
Major/Minor M	/linor2	N	Major1	Λ	//ajor2	
Conflicting Flow All	1071	567	567	0	-	0
Stage 1	567	-	-	-	-	-
Stage 2	504	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	_	_	_
Critical Hdwy Stg 1	5.4	-	-	_	_	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	2.2	_	_	_
				-	_	
Pot Cap-1 Maneuver	220	663	1012	-	-	-
Stage 1	632	-	-	-	-	-
Stage 2	680	-	-	-	-	-
Platoon blocked, %	1	1	1	-	-	-
Mov Cap-1 Maneuver	215	655	999	-	-	-
Mov Cap-2 Maneuver	215	-	-	-	-	-
Stage 1	624	-	_	-	-	-
Stage 2	672	_	_	_	_	_
Olago Z	012					
Approach	EB		NB		SB	
HCM Control Delay, s	0		0		0	
HCM LOS	A					
	· ·					
				,	055	05-
Minor Lane/Major Mvmt		NBL	NBT I	EBLn1	SBT	SBR
Capacity (veh/h)		999	-	-	-	-
HCM Lane V/C Ratio		-	-	-	-	-
HCM Control Delay (s)		0	-	0	-	-
TOWN CONTROL BOILD, (C)				A	-	_
		А	-	$\overline{}$		
HCM Lane LOS HCM 95th %tile Q(veh)		A 0	-	-	_	_

HCM 6th TWSC 11: Main Street & Rogers Street

Intersection								
Int Delay, s/veh	1.7						_	
Movement	WBL	WBR	NBT	NBR	SBL	SBT		
Lane Configurations	W	11511	4	11511	ሻ	<u> </u>		
Traffic Vol, veh/h	14	97	357	22	48	434		
Future Vol, veh/h	14	97	357	22	48	434		
Conflicting Peds, #/hr	2	8	0	15	15	0		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	Slop -	None	-			None		
					60			
Storage Length	0	-	-	-		-		
Veh in Median Storage	-	-	0	-	-	0		
Grade, %	0	-	0	-	-	0		
Peak Hour Factor	94	94	94	94	94	94		
Heavy Vehicles, %	0	0	0	0	0	1		
Mvmt Flow	15	103	380	23	51	462		
M = : = :/N A::= = ::	NA:1		1-:1		4-:0			
	Minor1		Major1		Major2			
Conflicting Flow All	973	415	0	0	418	0		
Stage 1	407	-	-	-	-	-		
Stage 2	566	-	-	-	-	-		
Critical Hdwy	6.4	6.2	-	-	4.1	-		
Critical Hdwy Stg 1	5.4	-	-	-	-	-		
Critical Hdwy Stg 2	5.4	-	-	-	-	-		
Follow-up Hdwy	3.5	3.3	_	_	2.2	_		
Pot Cap-1 Maneuver	*336	788	_	_	1179	_		
Stage 1	*744	-	_		- 1175	_		
Stage 2	*624	_			_			
				-				
Platoon blocked, %	1	1	-	-	1	-		
Mov Cap-1 Maneuver		771	-	-	1163	-		
Mov Cap-2 Maneuver	*317	-	-	-	-	-		
Stage 1	*734	-	-	-	-	-		
Stage 2	*595	-	-	-	-	-		
A	MD		ND		OD.			
Approach	WB		NB		SB			
HCM Control Delay, s	11.7		0		8.0			
HCM LOS	В							
Minor Lane/Major Mvn	nt	NBT	NRRV	VBLn1	SBL	SBT		
			145111	653	1163			
Capacity (veh/h)		-	-			-		
HCM Lane V/C Ratio		-		0.181		-		
HCM Control Delay (s)		-	-	11.7	8.2	-		
HCM Lane LOS		-	-	В	Α	-		
HCM 95th %tile Q(veh)	-	-	0.7	0.1	-		
Notes								
	nacity	¢. Da	lov ovo	oodo 20)/\c	L. Came		utation Not Defined
~: Volume exceeds ca		a. De	lav exc	eeas su	JUS ·	+. Comu)	utation Not Defined
HCM 95th %tile Q(veh Notes			-)) 0.7) 0.7 0.1) 0.7 0.1 -) 0.7 0.1 -

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<u>Capacity Analysis Summary Sheets</u> Year 2030 Total Projected Weekday Morning Peak Hour

Lanes, Volumes, Timings 1: Main Street & Franklin Street

	۶	→	•	•	←	•	4	†	~	/	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*		7	7	₽			र्स			^	7
Traffic Volume (vph)	51	0	26	5	16	17	36	478	0	0	289	104
Future Volume (vph)	51	0	26	5	16	17	36	478	0	0	289	104
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	35	• , ,	0	0	• 70	0	0	• 70	0	0	0,0	0
Storage Lanes	1		1	1		0	0		0	0		1
Taper Length (ft)	25		•	25		•	25			25		•
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.75	1.00	0.93	0.94	0.86	1.00	1.00	1.00	1.00	1.00	1.00	0.96
Frt	0.70		0.850	0.01	0.923			1.00				0.850
Flt Protected	0.950		0.000	0.950	0.020			0.997				0.000
Satd. Flow (prot)	1770	0	1455	1504	1508	0	0	1839	0	0	1942	1583
Flt Permitted	0.734	U	1700	0.950	1000	U	U	0.964	U	U	1372	1000
Satd. Flow (perm)	1032	0	1347	1421	1508	0	0	1777	0	0	1942	1525
Right Turn on Red	1032	U	Yes	1421	1500	Yes	U	1777	Yes	U	1342	Yes
Satd. Flow (RTOR)			27		18	163			163			109
Link Speed (mph)		25	21		25			25			25	109
Link Distance (ft)		180			382			220			418	
()		4.9			10.4			6.0			11.4	
Travel Time (s)	103	4.9	22	22	10.4	103	7	0.0	31	31	11.4	7
Confl. Peds. (#/hr)	103		22	22		103	1		31	٥١		1
Confl. Bikes (#/hr)	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05	0.05
Peak Hour Factor	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	2%	0%	11%	20%	0%	0%	3%	3%	0%	0%	3%	2%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)		00/			00/			00/			00/	
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)	5 4	•	07	_	0.5		•	511			004	400
Lane Group Flow (vph)	54	0	27	5	35	0	0	541	0	0	304	109
Turn Type	Perm		Perm	Perm	NA		Perm	NA			NA	Perm
Protected Phases					8			2			6	
Permitted Phases	4		4	8			2				_	6
Detector Phase	4		4	8	8		2	2			6	6
Switch Phase												
Minimum Initial (s)	5.0		5.0	5.0	5.0		8.0	8.0			8.0	8.0
Minimum Split (s)	22.5		22.5	22.5	22.5		22.5	22.5			22.5	22.5
Total Split (s)	30.0		30.0	30.0	30.0		60.0	60.0			60.0	60.0
Total Split (%)	33.3%		33.3%	33.3%	33.3%		66.7%	66.7%			66.7%	66.7%
Yellow Time (s)	3.5		3.5	3.0	3.0		3.0	3.0			3.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0			1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0			0.0			0.0	0.0
Total Lost Time (s)	4.5		4.5	4.0	4.0			4.0			4.0	4.0
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None		None	None	None		C-Min	C-Min			C-Min	C-Min
Act Effct Green (s)	10.2		10.2	10.6	10.6			74.1			74.1	74.1
Actuated g/C Ratio	0.11		0.11	0.12	0.12			0.82			0.82	0.82

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Lanes, Volumes, Timings 1: Main Street & Franklin Street

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Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.47		0.15	0.03	0.18			0.37			0.19	0.09
Control Delay	49.0		14.7	32.8	23.1			3.4			2.9	8.0
Queue Delay	0.0		0.0	0.0	0.0			0.0			0.0	0.0
Total Delay	49.0		14.7	32.8	23.1			3.4			2.9	8.0
LOS	D		В	С	С			Α			Α	Α
Approach Delay		37.6			24.3			3.4			2.3	
Approach LOS		D			С			Α			Α	
Queue Length 50th (ft)	29		0	3	9			65			32	0
Queue Length 95th (ft)	63		23	12	34			103			69	11
Internal Link Dist (ft)		100			302			140			338	
Turn Bay Length (ft)	35											
Base Capacity (vph)	292		401	410	448			1463			1599	1275
Starvation Cap Reductn	0		0	0	0			0			0	0
Spillback Cap Reductn	0		0	0	0			0			0	0
Storage Cap Reductn	0		0	0	0			0			0	0
Reduced v/c Ratio	0.18		0.07	0.01	0.08			0.37			0.19	0.09
Intersection Summary												
Area Type:	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 0 (0%), Referenced	to phase 2:1	NBTL and	l 6:SBT, S	Start of G	reen							
Natural Cycle: 50												
Control Type: Actuated-Co	ordinated											
Maximum v/c Ratio: 0.47												
Intersection Signal Delay: 6					tersection							
Intersection Capacity Utiliza	ation 66.8%			IC	U Level c	of Service	С					
Analysis Period (min) 15												
Splits and Phases: 1: Ma	ain Street & F	ranklin S	Street									
Ø2 (R)							335-53	₹ ø4				35
60 s							3	⊕ Ø4 0s				
4 25 (0)								- an				1 90
▼ Ø6 (R)							2	₩ Ø8				

	۶	→	*	1	+	1	1	†	~	/	Ţ	-√
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		र्स	7		↑	7		^	7	ሻ	1>	
Traffic Volume (vph)	20	32	12	0	32	7	0	420	2	9	264	17
Future Volume (vph)	20	32	12	0	32	7	0	420	2	9	264	17
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%	14	1,5	0%		1,5	0%			0%	12
Storage Length (ft)	0	0,0	60	0	0 70	55	0	0,0	0	60	0 70	0
Storage Lanes	0		1	0		1	0		1	1		0
Taper Length (ft)	25		•	25		•	25		•	25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	1.00	0.97	1.00	1.00	1.00	0.89	1.00	1.00	0.91	0.97	1.00	1.00
Frt		0.51	0.850			0.850			0.850	0.51	0.991	
Flt Protected		0.981	0.000			0.000			0.000	0.950	0.551	
Satd. Flow (prot)	0	1864	1615	0	2000	1615	0	1961	1615	1626	1809	0
Flt Permitted	U	0.858	1013	U	2000	1013	U	1301	1013	0.475	1003	U
Satd. Flow (perm)	0	1575	1615	0	2000	1436	0	1961	1476	792	1809	0
Right Turn on Red	U	1070	Yes	U	2000	Yes	U	1301	Yes	132	1003	Yes
Satd. Flow (RTOR)			18			18			18		7	163
,		25	10		25	10		25	10		25	
Link Speed (mph)		405						116				
Link Distance (ft)					420						254	
Travel Time (s)	27	11.0			11.5	27	00	3.2	07	07	6.9	00
Confl. Peds. (#/hr)	37					37	28		27	27		28
Confl. Bikes (#/hr)	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00	0.00
Peak Hour Factor	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86	0.86
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	2%	0%	11%	4%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)		•••			•••			•••				
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)	_			_	_	_	_		_			
Lane Group Flow (vph)	0	60	14	0	37	8	0	488	2	10	327	0
Turn Type	Perm	NA	Prot		NA	Perm		NA	Perm	Perm	NA	
Protected Phases		4	4		8			2			6	
Permitted Phases	4					8			2	6		
Detector Phase	4	4	4		8	8		2	2	6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	
Minimum Split (s)	22.5	22.5	22.5		22.5	22.5		22.5	22.5	22.5	22.5	
Total Split (s)	30.0	30.0	30.0		30.0	30.0		60.0	60.0	60.0	60.0	
Total Split (%)	33.3%	33.3%	33.3%		33.3%	33.3%		66.7%	66.7%	66.7%	66.7%	
Yellow Time (s)	3.5	3.5	3.5		3.5	3.5		3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0		1.0	1.0	1.0	1.0	
Lost Time Adjust (s)		0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	
Total Lost Time (s)		4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	
Lead/Lag												
Lead-Lag Optimize?												
Recall Mode	None	None	None		None	None		C-Min	C-Min	C-Min	C-Min	
Act Effct Green (s)		8.6	8.6		8.6	8.6		75.3	75.3	75.3	75.3	
Actuated g/C Ratio		0.10	0.10		0.10	0.10		0.84	0.84	0.84	0.84	
, waatoa gi o i tallo		0.10	0.10		0.10	0.10		0.07	0.04	0.07	0.07	

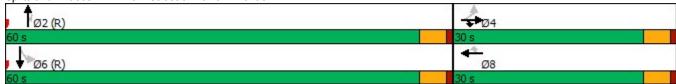
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Lanes, Volumes, Timings 2: Main Street & Warren Avenue

08/26/2024

	۶	→	•	1	•	*	4	†	1	-	ļ	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		0.40	0.08		0.19	0.05		0.30	0.00	0.02	0.22	
Control Delay		45.2	14.7		38.6	8.6		2.9	0.0	2.2	2.4	
Queue Delay		0.0	0.0		0.0	0.0		7.0	0.0	0.0	0.0	
Total Delay		45.2	14.7		38.6	8.6		9.9	0.0	2.2	2.4	
LOS		D	В		D	Α		Α	Α	Α	Α	
Approach Delay		39.4			33.3			9.8			2.4	
Approach LOS		D			С			Α			Α	
Queue Length 50th (ft)		33	0		20	0		52	0	1	31	
Queue Length 95th (ft)		66	14		45	7		93	0	4	56	
Internal Link Dist (ft)		325			340			36			174	
Turn Bay Length (ft)			60			55				60		
Base Capacity (vph)		446	470		566	419		1640	1237	662	1513	
Starvation Cap Reductn		0	0		0	0		1093	1128	0	0	
Spillback Cap Reductn		0	0		0	0		0	0	0	0	
Storage Cap Reductn		0	0		0	0		0	0	0	0	
Reduced v/c Ratio		0.13	0.03		0.07	0.02		0.89	0.02	0.02	0.22	
Intersection Summary												
71	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 0 (0%), Referenced t	o phase 2:I	NBT and	6:SBTL, S	Start of G	reen							
Natural Cycle: 45												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.40												
Intersection Signal Delay: 10					tersection							
Intersection Capacity Utilizat	tion 48.3%			IC	CU Level o	of Service	Α					
Analysis Period (min) 15												
Solits and Phases: 2: Mai	n Street & V	Narren A	venue									

Splits and Phases: 2: Main Street & Warren Avenue



Queuing and Blocking Report Year 2030 Total Weekday Morning Peak Hour

08/27/2024

Intersection: 2: Main Street & Warren Avenue

Movement	EB	EB	WB	WB	NB	NB	SB	SB	
Directions Served	LT	R	Т	R	T	R	L	TR	
Maximum Queue (ft)	64	30	60	43	65	8	48	150	
Average Queue (ft)	29	8	16	4	42	0	6	50	
95th Queue (ft)	58	29	45	22	68	4	29	114	
Link Distance (ft)	320		373		45	45		178	
Upstream Blk Time (%)					10				
Queuing Penalty (veh)					20				
Storage Bay Dist (ft)		60		55			60		
Storage Blk Time (%)	1		1	0			0	4	
Queuing Penalty (veh)	0		0	0			0	0	

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Intersection Capacity Utilization 3: Forest Avenue & Franklin Street

	۶	-	•	•	•	•	1	†	/	-	ţ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	3	13	23	117	41	10	19	181	49	8	69	4
Pedestrians	1		3	3		1	3		4	4		3
Ped Button		Yes			Yes			Yes			Yes	
Pedestrian Timing (s)		16.0			16.0			16.0			16.0	
Free Right			No			No			No			No
Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Refr Cycle Length (s)	120	120	120	120	120	120	120	120	120	120	120	120
Volume Combined (vph)	0	39	0	0	168	0	0	249	0	0	81	0
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Factor (vph)	0.95	0.91	0.85	0.95	0.96	0.85	0.95	0.97	0.85	0.95	0.99	0.85
Saturated Flow (vph)	0	1725	0	0	1817	0	0	1837	0	0	1877	0
Ped Intf Time (s)	0.0	0.2	0.4	0.0	0.0	0.1	0.0	0.1	0.5	0.0	0.0	0.4
Pedestrian Frequency (%)		0.10			0.03			0.12			0.10	
Protected Option Allowed		No			No			No			No	
Reference Time (s)			0.0			0.0			0.0			0.0
Adj Reference Time (s)			0.0			0.0			0.0			0.0
Permitted Option												
Adj Saturation A (vph)	0	1757		0	359		0	1481		0	1336	
Reference Time A (s)	0.0	2.9		0.0	56.1		0.0	20.3		0.0	7.3	
Adj Saturation B (vph	0	0		0	0		0	0		NA	NA	
Reference Time B (s)	8.2	10.9		15.8	19.1		9.3	24.4		NA	NA	
Reference Time (s)		2.9			19.1			20.3			7.3	
Adj Reference Time (s)		9.1			23.1			24.3			12.1	
Split Option		-			-			-				
Ref Time Combined (s)	0.0	2.9		0.0	11.1		0.0	16.4		0.0	5.2	
Ref Time Seperate (s)	0.2	1.1		7.8	2.7		1.3	11.9		0.5	4.4	
Reference Time (s)	2.9	2.9		11.1	11.1		16.4	16.4		5.2	5.2	
Adj Reference Time (s)	9.1	9.1		15.3	15.3		20.4	20.4		10.2	10.2	
		• • •	ND OD				•					
Summary	EB WB		NB SB	Co	mbined							
Protected Option (s)	NA		NA									
Permitted Option (s)	23.1		24.3									
Split Option (s)	24.4		30.6									
Minimum (s)	23.1		24.3		47.4							
Right Turns												
Adj Reference Time (s)												
Cross Thru Ref Time (s)												
Oncoming Left Ref Time (s)												
Combined (s)												
Intersection Summary												
	tion		30 50/	10	'III ovol a	of Convice			Λ			
Intersection Capacity Utilization Reference Times and Phasis		do not ro	39.5%			of Service			Α			
Meleterice Times and Filasi	ng Options	40 1101 16	بابا وعطالا و	ιιι ομιιτιίΖ	.eu ummg	piall.						

Intersection Capacity Utilization 5: Forest Avenue & Warren Avenue

	۶	•	4	†	ļ	1	
Movement	EBL	EBR	NBL	NBT	SBT	SBR	
Lane Configurations	*	7		4	1		
Volume (vph)	46	107	120	201	143	50	
Pedestrians	10		5			5	
Ped Button					Yes	-	
Pedestrian Timing (s)					16.0		
Free Right		No				No	
Ideal Flow	1900	1900	1900	1900	1900	1900	
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	
Refr Cycle Length (s)	120	120	120	120	120	120	
Volume Combined (vph)	46	107	0	321	193	0	
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	
Turning Factor (vph)	0.95	0.85	0.95	0.98	0.96	0.85	
Saturated Flow (vph)	1805	1615	0	1864	1826	0	
Ped Intf Time (s)	0.0	0.0	0.0	0.0	0.2	0.6	
Pedestrian Frequency (%)	0.00			0.00	0.15		
Protected Option Allowed	No			No	No		
Reference Time (s)		8.0				0.0	
Adj Reference Time (s)		12.0				0.0	
Permitted Option							
Adj Saturation A (vph)	120		0	274	1826		
Reference Time A (s)	45.9		0.0	140.6	12.8		
Adj Saturation B (vph	NA		NA	NA	NA		
Reference Time B (s)	NA		NA	NA	NA		
Reference Time (s)				140.6	12.8		
Adj Reference Time (s)				144.6	17.3		
Split Option							
Ref Time Combined (s)	3.1		0.0	20.7	12.8		
Ref Time Seperate (s)	3.1		8.0	12.7	9.6		
Reference Time (s)	3.1		20.7	20.7	12.8		
Adj Reference Time (s)	8.0		24.7	24.7	17.3		
Summary	EB		NB SB	Col	mbined		
Protected Option (s)	NA		NA NA	001	moniou		
Permitted Option (s)	Err		144.6				
Split Option (s)	8.0		42.0				
Minimum (s)	8.0		42.0		50.0		
,			42.0		50.0		
Right Turns	EBR						
Adj Reference Time (s)	12.0						
Cross Thru Ref Time (s)	17.3						
Oncoming Left Ref Time (s)	0.0						
Combined (s)	29.3						
Intersection Summary							
Intersection Capacity Utilization	on		41.7%	IC	U Level o	of Service	
Reference Times and Phasing		do not re					

4: Forest Avenue & AT&T Access Drive/Apartment Access Drive

Intersection												
Int Delay, s/veh	0.1											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	1	1	0	2	1	240	0	0	202	1
Future Vol, veh/h	1	0	1	1	0	2	1	240	0	0	202	1
Conflicting Peds, #/hr	0	0	0	0	0	0	9	0	0	0	0	9
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	87	87	87	87	87	87	87	87	87	87	87	87
Heavy Vehicles, %	100	0	0	0	0	0	0	1	0	0	2	100
Mvmt Flow	1	0	1	1	0	2	1	276	0	0	232	1
Major/Minor N	linor2		ı	Minor1		ı	Major1			Major2		
Conflicting Flow All	521	520	242	511	520	276	242	0	0	276	0	0
Stage 1	242	242		278	278	-	- '-	-	-		-	-
Stage 2	279	278	_	233	242	_	_	_	_	_	_	_
Critical Hdwy	8.1	6.5	6.2	7.1	6.5	6.2	4.1	-	-	4.1	_	_
Critical Hdwy Stg 1	7.1	5.5	-	6.1	5.5	-	-	_	_	-	_	_
Critical Hdwy Stg 2	7.1	5.5	_	6.1	5.5	_	_	-	-	-	_	_
Follow-up Hdwy	4.4	4	3.3	3.5	4	3.3	2.2	_	_	2.2	_	_
Pot Cap-1 Maneuver	343	463	802	476	463	768	1336	-	-	1299	-	_
Stage 1	586	709	-	733	684	-	-	_	_	-	_	_
Stage 2	557	684	-	775	709	-	-	-	-	-	-	-
Platoon blocked, %								_	_		-	_
Mov Cap-1 Maneuver	339	458	795	475	458	768	1325	_	_	1299	_	-
Mov Cap-2 Maneuver	339	458	-	475	458		-	_	-	-	-	_
Stage 1	580	703	_	732	683	-	-	-	_	_	-	-
Stage 2	555	683	_	774	703	_	_	_	_	_	_	_
5												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.6			10.7			0			0		
HCM LOS	В			В								
200												
Minor Lane/Major Mvmt		NBL	NBT	NBR I	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1325	-	-		637	1299	-	-			
HCM Lane V/C Ratio		0.001	_		0.005		-	_	_			
HCM Control Delay (s)		7.7	0	-	12.6	10.7	0	-	-			
HCM Lane LOS		A	A	_	В	В	A	_	_			
HCM 95th %tile Q(veh)		0	-	-	0	0	0	-	-			
(·•··)												

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6: Warren Avenue & Forest Avenue

Intersection						
Int Delay, s/veh	1.5					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	ሻ	7	<u>↑</u>	TVDIC	ODL	41
Traffic Vol, veh/h	16	40	280	35	37	213
Future Vol, veh/h	16	40	280	35	37	213
Conflicting Peds, #/hr	10	0	0	15	15	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Slop -	None		None	-	None
Storage Length	0	0	_	0	_	-
Veh in Median Storage		-	0	-	-	0
	, # 0	-				0
Grade, %			0	-	-	
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	2	0	0	3
Mvmt Flow	17	42	295	37	39	224
Major/Minor N	/linor1	N	Major1		Major2	
Conflicting Flow All	501	310	0	0	347	0
Stage 1	310	_	_	-	-	_
Stage 2	191	-	-	-	-	_
Critical Hdwy	6.6	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.4	-	_	_	-	_
Critical Hdwy Stg 2	5.8	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	_	_	2.2	_
Pot Cap-1 Maneuver	519	735	_	_	1223	_
Stage 1	748	-	_	_	1220	_
Stage 2	828	_	_		_	_
Platoon blocked, %	020	_	_	_	-	_
	402	705		_	1000	
Mov Cap-1 Maneuver	493	725	-	-	1206	-
Mov Cap-2 Maneuver	493	-	-	-	-	-
Stage 1	738	-	-	-	-	-
Stage 2	797	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	11		0		1.3	
HCM LOS	В		U		1.0	
HOW LOO						
Minor Lane/Major Mvm	t	NBT	NBRV	VBLn1V		SBL
Capacity (veh/h)		-	-			1206
HCM Lane V/C Ratio		-		0.034	0.058	0.032
HCM Control Delay (s)		-	-	12.6	10.3	8.1
HCM Lane LOS		-	-	В	В	Α
HCM 95th %tile Q(veh)		-	-	0.1	0.2	0.1
2111 22 21 70 21 2 (1011)				<u> </u>		

HCM 6th TWSC 8: N-S Alley & Franklin Street

Intersection						
Int Delay, s/veh	1.1					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1>			4	W	
Traffic Vol, veh/h	68	4	11	154	11	9
Future Vol, veh/h	68	4	11	154	11	9
Conflicting Peds, #/hr		11	11	0	0	12
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-		-		-	None
Storage Length	_	-	_	-	0	-
Veh in Median Storag	e,# 0	_	_	0	0	-
Grade, %	0, 11	_	_	0	0	_
Peak Hour Factor	78	78	78	78	78	78
Heavy Vehicles, %	5	0	0	1	0	0
Mvmt Flow	87	5	14	197	14	12
IVIVITIT FIOW	0/	5	14	197	14	12
Major/Minor	Major1	N	Major2	1	Minor1	
Conflicting Flow All	0	0	103	0	326	113
Stage 1	_	_	-	_	101	_
Stage 2	_	_	-	_	225	_
Critical Hdwy	_	_	4.1	_	6.4	6.2
Critical Hdwy Stg 1	_	_		_	5.4	-
Critical Hdwy Stg 2	_		_	_	5.4	_
, ,	_	_	2.2	_	3.5	3.3
Follow-up Hdwy		-	1502		724	945
Pot Cap-1 Maneuver	-	-	1502	-		
Stage 1	-	-	-	-	928	-
Stage 2	-	-	-	-	855	-
Platoon blocked, %	-	-		-	1	
Mov Cap-1 Maneuver		-	1486	-	709	924
Mov Cap-2 Maneuver	-	-	-	-	709	-
Stage 1	-	-	-	-	919	-
Stage 2	-	-	-	-	845	-
A mara a a b	ED		WD		ND	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.5		9.7	
HCM LOS					Α	
Minor Lane/Major Mvi	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		792	-		1486	-
HCM Lane V/C Ratio		0.032	_		0.009	<u> </u>
HCM Control Delay (s	Λ	9.7		_		
)		-			0
HCM Lane LOS	.\	Α	-	-	A	Α
HCM 95th %tile Q(vel	1)	0.1	-	-	0	-

9: Main Street & Oakley Access Drive

Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr	0 EBL	EBR	NBL					
Movement E Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr	EBL	EBR	NIRI					
Lane Configurations Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr		EDK		NDT	CDT	SBR		
Traffic Vol, veh/h Future Vol, veh/h Conflicting Peds, #/hr			NDL	NBT	SBT	SDK		
Future Vol, veh/h Conflicting Peds, #/hr	_	٥	٥	વ	1	0		
Conflicting Peds, #/hr	0	0	0	518 518	306	0		
	0	0	13	0 0	0	13		
Ciara Caratral C					Free	Free		
Sign Control Sign Channelized	Stop	Stop	Free	Free None		None		
	0	NOTIE	-		-	None -		
Storage Length				-	0			
Veh in Median Storage, #		-	-	0	_	-		
Grade, %	93	93	93	93	93	93		
Peak Hour Factor			93	93				
Heavy Vehicles, %	0	0			220	0		
Mvmt Flow	0	0	0	557	329	0		
Major/Minor Mine	or2	N	Major1	N	/lajor2			
Conflicting Flow All 8	899	342	342	0	-	0		
	342	-	-	-	-	-		
	557	-	-	-	-	-		
	6.4	6.2	4.1	-	-	-		
Critical Hdwy Stg 1	5.4	-	-	-	-	-		
Critical Hdwy Stg 2	5.4	-	-	-	-	-		
Follow-up Hdwy	3.5	3.3	2.2	-	-	-		
	358	*825	*1238	-	-	-		
	778	-	-	-	-	-		
	608	-	-	-	-	-		
Platoon blocked, %	1	1	1	-	-	-		
	349	*815	*1223	-	-	-		
	349	-	-	-	-	-		
•	769	-	-	-	-	-		
Stage 2 *6	601	-	-	-	-	-		
Approach	ЕВ		NB		SB			
HCM Control Delay, s	0		0		0			
HCM LOS	A		U		U			
I IOWI LOU								
Minor Lane/Major Mvmt		NBL	NBT I	EBLn1	SBT	SBR		
Capacity (veh/h)	*	1223	-	-	-	-		
HCM Lane V/C Ratio		-	-	-	-	-		
HCM Control Delay (s)		0	-	0	-	-		
HCM Lane LOS		Α	-	Α	-	-		
HCM 95th %tile Q(veh)		0	-	-	-	-		
Notes								
Notes ~: Volume exceeds capaci	itv	\$: De	elav exc	eeds 30	00s	+: Comr	outation Not Defined	*: All major volume in platoon

10: Main Street & Funeral Home North Access Drive

Peak Hour Factor 93 98 98 Mostage 2 \$507 2 42 2 2 - - - - - - - - - - - - - - - -	Intersection								
Movement	Int Delay, s/veh	0							
Lane Configurations			EDD	ND	NDT	CDT	CDD		
Traffic Vol, veh/h			FRK	NRL			SRK		
Future Vol, veh/h Conflicting Peds, #hr Stop Stop Free Free RT Channelized None RT None None None None None None None None									
Conflicting Peds, #/hr Sign Control Stop Stop Free Free Free Free Free Free Free Fre									
Sign Control Stop RT Channelized Stop None Free RT Channelized Free RT Channelized Free RT Channelized None None									
RT Channelized - None - None - None Storage Length 0									
Storage Length		Stop		Free		Free			
Veh in Median Storage, # 0 - - 0 0 - Grade, % 0 - - 0 0 - Good of the pack Hour Factor 93 94 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 1 <t< td=""><td></td><td>-</td><td>None</td><td>-</td><td>None</td><td>-</td><td>None</td><td></td><td></td></t<>		-	None	-	None	-	None		
Grade, % 0 0 0 Peak Hour Factor 93 93 93 93 93 93 93 93 93 93 93 93 93			-	-		-	-		
Peak Hour Factor 93 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94 94	Veh in Median Storage	e, # 0	-	-	0	0	-		
Heavy Vehicles, %	Grade, %								
Mymt Flow 0 0 0 557 329 0 Major/Minor Minor2 Major1 Major2 Conflicting Flow All 899 342 342 0 0 Stage 1 342 - - - - Stage 2 557 - - - - Critical Hdwy 64 6.2 4.1 - - - Critical Hdwy Stg 1 5.4 - - - - - Critical Hdwy Stg 2 5.4 - - - - - Follow-up Hdwy 3.5 3.3 2.2 - - - Follow-up Hdwy 3.5 3.3 2.2 - - - Follow-up Hdwy 3.5 3.3 2.2 - - - Stage 1 *778 - - - - - Stage 2 *608 - - - -	Peak Hour Factor	93	93	93	93	93	93		
Major/Minor Minor2 Major1 Major2 Conflicting Flow All 899 342 342 0 0 Stage 1 342 - - - - Stage 2 557 - - - - Critical Hdwy Stg 1 5.4 - - - - Critical Hdwy Stg 2 5.4 - - - - - Follow-up Hdwy 3.5 3.3 2.2 - <t< td=""><td>Heavy Vehicles, %</td><td>0</td><td>0</td><td>0</td><td>3</td><td>4</td><td>0</td><td></td><td></td></t<>	Heavy Vehicles, %	0	0	0	3	4	0		
Conflicting Flow All 899 342 342 0 - 0 Stage 1 342 Stage 2 557	Mvmt Flow	0	0	0	557	329	0		
Conflicting Flow All 899 342 342 0 - 0 Stage 1 342 Stage 2 557									
Conflicting Flow All 899 342 342 0 - 0 Stage 1 342 Stage 2 557 Critical Howy 6.4 6.2 4.1 Critical Howy Stg 1 5.4 Critical Howy Stg 2 5.4 Critical Howy Stg 2 5.4 Follow-up Howy 3.5 3.3 2.2 Follow-up Howy 3.5 3.3 2.2 Stage 1 *778 Stage 2 *608 Stage 2 *608 Platoon blocked, 1 1 1 Mov Cap-1 Maneuver *401 *815 *1223 Stage 1 *769 Stage 1 *769 Stage 2 *601 Stage 2 *601 Approach EB NB SB HCM Control Delay, s 0 0 0 HCM LOS A Minor Lane/Major Mvmt NBL NBT EBLn1 SBT SBR Capacity (veh/h) *1223 HCM Lane V/C Ratio HCM Lane LOS A - A - A HCM S5th %tile Q(veh) 0	Major/Minor	Minor		Majort		/oicr0			
Stage 1 342 -									
Stage 2 557									
Critical Hdwy 6.4 6.2 4.1 -	9								
Critical Hdwy Stg 1 5.4 -									
Critical Hdwy Stg 2 5.4 -									
Follow-up Hdwy 3.5 3.3 2.2 Pot Cap-1 Maneuver *411 *825 *1238 Stage 1 *778									
Pot Cap-1 Maneuver *411 *825 *1238 - <td< td=""><td></td><td></td><td></td><td></td><td>-</td><td>-</td><td>-</td><td></td><td></td></td<>					-	-	-		
Stage 1 *778 - - - Stage 2 *608 - - - Platoon blocked, % 1 1 1 - - Mov Cap-1 Maneuver *401 *815 *1223 - - Mov Cap-2 Maneuver *401 - - - - Stage 1 *769 - - - - Stage 2 *601 - - - - Stage 2 *601 - - - - Approach EB NB SB HCM Control Delay, s 0 0 0 HCM LOS A **Minor Lane/Major Mvmt **NBL NBT EBLn1 SBT SBR **Capacity (veh/h) **1223					-				
Stage 2			*825	*1238	-	-	-		
Platoon blocked, % 1 1 1 - - - Mov Cap-1 Maneuver *401 *815 *1223 - - - - - - Mov Cap-2 Maneuver *401 -<				-	-	-			
Mov Cap-1 Maneuver *401 *815 *1223 -					-	-	-		
Mov Cap-2 Maneuver *401 - - - - Stage 1 *769 - - - - Stage 2 *601 - - - - Approach EB NB SB HCM Control Delay, s 0 0 0 HCM LOS A Minor Lane/Major Mvmt NBL NBT EBLn1 SBT SBR Capacity (veh/h) * 1223 - - - HCM Lane V/C Ratio - - - - HCM Control Delay (s) 0 - 0 - - HCM Lane LOS A - A - - HCM 95th %tile Q(veh) 0 - - - - Notes					-	-	-		
Stage 1 *769 -	Mov Cap-1 Maneuver		*815	*1223	-	-	-		
Stage 2 *601 -			-	-	-	-	-		
Approach EB NB SB HCM Control Delay, s 0 0 0 HCM LOS A A A Minor Lane/Major Mvmt NBL NBT EBLn1 SBT SBR Capacity (veh/h) * 1223 - - - HCM Lane V/C Ratio - - - - HCM Control Delay (s) 0 - 0 - - HCM Lane LOS A - A - - - HCM 95th %tile Q(veh) 0 - - - - -	_		-	-	-	-	-		
HCM Control Delay, s 0 0 0 HCM LOS A Minor Lane/Major Mvmt NBL NBT EBLn1 SBT SBR Capacity (veh/h) * 1223 HCM Lane V/C Ratio HCM Control Delay (s) 0 - 0 HCM Lane LOS A - A HCM 95th %tile Q(veh) 0	Stage 2	*601	-	-	-	-	-		
HCM Control Delay, s 0 0 0 HCM LOS A Minor Lane/Major Mvmt NBL NBT EBLn1 SBT SBR Capacity (veh/h) * 1223 HCM Lane V/C Ratio HCM Control Delay (s) 0 - 0 HCM Lane LOS A - A HCM 95th %tile Q(veh) 0									
HCM Control Delay, s 0 0 0 HCM LOS A Minor Lane/Major Mvmt NBL NBT EBLn1 SBT SBR Capacity (veh/h) * 1223 HCM Lane V/C Ratio HCM Control Delay (s) 0 - 0 HCM Lane LOS A - A HCM 95th %tile Q(veh) 0	Annroach	ED		NID		CD			
Minor Lane/Major Mvmt NBL NBT EBLn1 SBT SBR Capacity (veh/h) * 1223 - - - HCM Lane V/C Ratio - - - - HCM Control Delay (s) 0 - 0 - HCM Lane LOS A - A - HCM 95th %tile Q(veh) 0 - - - Notes									
Minor Lane/Major Mvmt NBL NBT EBLn1 SBR Capacity (veh/h) * 1223 - - - HCM Lane V/C Ratio - - - - HCM Control Delay (s) 0 - 0 - - HCM Lane LOS A - A - - - HCM 95th %tile Q(veh) 0 - - - - - Notes - - - - - - -				U		U			
Capacity (veh/h) * 1223	HOIVI LUS	А							
Capacity (veh/h) * 1223									
Capacity (veh/h) * 1223	Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR		
HCM Lane V/C Ratio - - - - - HCM Control Delay (s) 0 - 0 - - HCM Lane LOS A - A - HCM 95th %tile Q(veh) 0 - - - Notes				_	-	-	-		
HCM Control Delay (s) 0 - 0 HCM Lane LOS A - A HCM 95th %tile Q(veh) 0 Notes				_	_	_	_		
HCM Lane LOS A - A HCM 95th %tile Q(veh) 0 Notes									
HCM 95th %tile Q(veh) 0 Notes							_		
Notes)		_		_	_		
	,	7	J						
~: Volume exceeds capacity \$: Delay exceeds 300s +: Computation Not Defined *: All major volume in pla									
	~: Volume exceeds ca	pacity	\$: De	elay exc	eeds 30	00s	+: Comp	utation Not Defined	*: All major volume in plate

11: Main Street & Rogers Street

Intersection						
Int Delay, s/veh	1.8					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y	WDIX	13	NUN)	<u> </u>
Traffic Vol, veh/h	18	80	431	18	35	T 275
Future Vol, veh/h	18	80	431	18	35	275
<u> </u>	4	5	431	19	19	0
Conflicting Peds, #/hr		_				
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	60	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	91	91	91	91	91	91
Heavy Vehicles, %	0	5	3	0	3	5
Mvmt Flow	20	88	474	20	38	302
Major/Minor N	Minort		Anior1		Majara	
	Minor1		Major1		Major2	
Conflicting Flow All	885	508	0	0	513	0
Stage 1	503	-	-	-	-	-
Stage 2	382	-	-	-	-	-
Critical Hdwy	6.4	6.25	-	-	4.13	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.345	-	-	2.227	-
Pot Cap-1 Maneuver	*359	*707	_	-	*1062	-
Stage 1	*676	-	-	-	-	-
Stage 2	*745	-	-	-	_	_
Platoon blocked, %	1	1	-	_	1	_
Mov Cap-1 Maneuver	*339	*691	_		*1043	_
Mov Cap-2 Maneuver	*339	-	_	_	-	_
Stage 1	*664	_	_		_	_
				-		-
Stage 2	*715	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	12.6		0		1	
HCM LOS	В				•	
TIOW LOO						
Minor Lane/Major Mvm	ıt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	580	* 1043	-
HCM Lane V/C Ratio		-	-	0.186	0.037	-
HCM Control Delay (s)		-	_	12.6	8.6	_
HCM Lane LOS		-	-	В	Α	_
HCM 95th %tile Q(veh)		-	_	0.7	0.1	_
` '						
Notes						
~: Volume exceeds cap	pacity	\$: De	lay exc	eeds 30	00s	+: Comp
	,		•			

12: Forest Avenue & Garage Access Drive

Intersection						
Int Delay, s/veh	0.2					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥		1>			4
Traffic Vol, veh/h	2	5	242	1	1	201
Future Vol, veh/h	2	5	242	1	1	201
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	_	0	_	-	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	1	0	0	2
Mvmt Flow	2	5	255	1	1	212
WWW.CT IOW	_		200		•	
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	470	256	0	0	256	0
Stage 1	256	-	-	-	-	-
Stage 2	214	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	556	788	-	-	1321	_
Stage 1	791	-	-	-	-	-
Stage 2	826	_	-	-	-	-
Platoon blocked, %	0_0		_	_		_
Mov Cap-1 Maneuver	555	788	_	_	1321	_
Mov Cap-2 Maneuver	555	-	_	_	-	_
Stage 1	791	_	_	_	_	_
Stage 2	825	_	_	_	_	_
Olage 2	023					-
Approach	WB		NB		SB	
HCM Control Delay, s	10.2		0		0	
HCM LOS	В					
Minor Lane/Major Mvn	o t	NBT	NIDDV	VBLn1	SBL	SBT
	IL	NDI	INDIN			SDI
Capacity (veh/h)		-	-	704	1321	-
HCM Lane V/C Ratio		-	-		0.001	-
HCM Control Delay (s)		-	-	10.2	7.7	0
HCM Lane LOS	,	-	-	В	A	Α
HCM 95th %tile Q(veh)	-	-	0	0	-

HCM 6th TWSC 13: N-S Alley & Garage Access Drive

Intersection						
Int Delay, s/veh	3.8					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			4	₽	
Traffic Vol, veh/h	11	0	0	5	6	3
Future Vol, veh/h	11	0	0	5	6	3
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	_	-	_	-
Veh in Median Storage		_	_	0	0	_
Grade, %	0	<u>-</u>	_	0	0	<u>-</u>
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	0	0	0
Mymt Flow	12	0	0	5	6	3
INIVITIL FIOW	12	U	U	ວ	U	J
Major/Minor N	Minor2	N	Major1	N	//ajor2	
Conflicting Flow All	13	8	9	0	-	0
Stage 1	8	_	-	-	-	-
Stage 2	5	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	_	-
Pot Cap-1 Maneuver	1011	1080	1624	-	-	_
Stage 1	1020	-	-	_	_	_
Stage 2	1023	_	_	_	_	_
Platoon blocked, %	1020			_	_	_
Mov Cap-1 Maneuver	1011	1080	1624	_	_	_
Mov Cap-1 Maneuver	1011	-	1024	_	_	_
Stage 1	1020			_		
•				-		
Stage 2	1023	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.6		0		0	
HCM LOS	Α					
		N.D.	NET		007	000
Minor Lane/Major Mvm	<u>it</u>	NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1624		1011	-	-
HCM Lane V/C Ratio		-		0.011	-	-
HCM Control Delay (s)		0	-	8.6	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-

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<u>Capacity Analysis Summary Sheets</u> Year 2030 Total Projected Weekday Evening Peak Hour

Lanes, Volumes, Timings 1: Main Street & Franklin Street

	۶	→	•	•	←	•	1	†	~	1	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations	*		7	*	1>			र्स			^	7
Traffic Volume (vph)	55	0	26	3	8	18	42	412	0	0	472	160
Future Volume (vph)	55	0	26	3	8	18	42	412	0	0	472	160
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	2000	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	35		0	0		0	0		0	0		0
Storage Lanes	1		1	1		0	0		0	0		1
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor	0.99		0.95	0.98	0.98			1.00				0.96
Frt			0.850		0.894							0.850
Flt Protected	0.950		0.000	0.950	0.00			0.995				0.000
Satd. Flow (prot)	1805	0	1615	1805	1659	0	0	1890	0	0	1980	1615
Flt Permitted	0.740	, and the second	1010	0.950	1000			0.932			1000	1010
Satd. Flow (perm)	1389	0	1537	1764	1659	0	0	1769	0	0	1980	1546
Right Turn on Red	1000	, and the second	Yes	1101	1000	Yes	· ·	1100	Yes		1000	Yes
Satd. Flow (RTOR)			27		19	. 00			. 00			165
Link Speed (mph)		25			25			25			25	100
Link Distance (ft)		180			382			220			418	
Travel Time (s)		4.9			10.4			6.0			11.4	
Confl. Peds. (#/hr)	5	1.0	9	9	10.1	5	9	0.0	9	9		9
Confl. Bikes (#/hr)	•		5	J		1	J			J		1
Peak Hour Factor	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97	0.97
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	0%	0%	0%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)		0 70			0 70			070			0 70	
Lane Group Flow (vph)	57	0	27	3	27	0	0	468	0	0	487	165
Turn Type	Perm	U	Perm	Perm	NA	U	Perm	NA	J	0	NA	Perm
Protected Phases	1 Cilli		1 Cilli	1 Cilli	8		1 Cilli	2			6	1 Cilli
Permitted Phases	4		1	8	J		2				U	6
Detector Phase	4		4	8	8		2	2			6	6
Switch Phase			7	U U	U						U	U
Minimum Initial (s)	5.0		5.0	5.0	5.0		8.0	8.0			8.0	8.0
Minimum Split (s)	22.5		22.5	22.5	22.5		22.5	22.5			22.5	22.5
Total Split (s)	30.0		30.0	30.0	30.0		60.0	60.0			60.0	60.0
Total Split (%)	33.3%		33.3%	33.3%	33.3%		66.7%	66.7%			66.7%	66.7%
Yellow Time (s)	3.5		3.5	3.0	3.0		3.0	3.0			3.0	3.0
All-Red Time (s)	1.0		1.0	1.0	1.0		1.0	1.0			1.0	1.0
Lost Time Adjust (s)	0.0		0.0	0.0	0.0		1.0	0.0			0.0	0.0
Total Lost Time (s)	4.5		4.5	4.0	4.0			4.0			4.0	4.0
、 ,	4.5		4.5	4.0	4.0			4.0			4.0	4.0
Lead/Lag												
Lead-Lag Optimize?	Mana		Mana	Mana	Mana		C Min	C Min			C 14:-	C Main
Recall Mode	None		None	None	None		C-Min	C-Min			C-Min	C-Min
Act Effet Green (s)	9.1		9.1	9.5	9.5			75.2			75.2	75.2
Actuated g/C Ratio	0.10		0.10	0.11	0.11			0.84			0.84	0.84

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Lanes, Volumes, Timings 1: Main Street & Franklin Street

	1	→	*	•	•	•	1	†	-	-	↓	1
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio	0.41		0.15	0.02	0.14			0.32			0.29	0.13
Control Delay	45.5		15.3	34.0	20.5			2.9			2.9	0.7
Queue Delay	0.0		0.0	0.0	0.0			0.0			0.0	0.0
Total Delay	45.5		15.3	34.0	20.5			2.9			2.9	0.7
LOS	D		В	С	С			Α			Α	Α
Approach Delay		35.8			21.8			2.9			2.3	
Approach LOS		D			С			Α			Α	
Queue Length 50th (ft)	31		0	2	4			49			52	0
Queue Length 95th (ft)	66		23	9	27			92			103	12
Internal Link Dist (ft)		100			302			140			338	
Turn Bay Length (ft)	35											
Base Capacity (vph)	393		454	509	492			1477			1653	1318
Starvation Cap Reductn	0		0	0	0			0			0	0
Spillback Cap Reductn	0		0	0	0			0			0	0
Storage Cap Reductn	0		0	0	0			0			0	0
Reduced v/c Ratio	0.15		0.06	0.01	0.05			0.32			0.29	0.13
Intersection Summary												
71	Other											
Cycle Length: 90												
Actuated Cycle Length: 90												
Offset: 0 (0%), Referenced t	to phase 2:I	NBTL and	l 6:SBT, 9	Start of G	reen							
Natural Cycle: 45												
Control Type: Actuated-Coo	rdinated											
Maximum v/c Ratio: 0.41												
Intersection Signal Delay: 5.					tersection							
Intersection Capacity Utiliza	tion 68.3%			IC	U Level o	of Service	C					
Analysis Period (min) 15												
Splits and Phases: 1: Mai	n Street & I	- -ranklin S	Street									
Ø2 (R)							355-50	₹ ø4				35
60 s							3	0 s			- 16	
4 (0)								100				7.55
▼ Ø6 (R)								♥ Ø8				

Lanes, Volumes, Timings 2: Main Street & Warren Avenue

	۶	→	*	•	•	•	4	†	~	/	Ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		ર્ન	7		↑	7		^	7	*	₽	
Traffic Volume (vph)	22	54	7	0	47	18	0	342	17	4	438	19
Future Volume (vph)	22	54	7	0	47	18	0	342	17	4	438	19
Ideal Flow (vphpl)	1900	1900	1900	1900	2000	1900	1900	2000	1900	1900	1900	1900
Lane Width (ft)	12	12	12	12	12	12	12	12	12	12	12	12
Grade (%)		0%			0%			0%			0%	
Storage Length (ft)	0		60	0		55	0		0	60		0
Storage Lanes	0		1	0		1	0		1	1		0
Taper Length (ft)	25		-	25		•	25		-	25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Ped Bike Factor		0.97				0.88			0.86	0.94	1.00	,,,,,,
Frt			0.850			0.850			0.850		0.994	
Flt Protected		0.986								0.950		
Satd. Flow (prot)	0	1873	1615	0	2000	1615	0	1980	1615	1805	1867	0
Flt Permitted	· ·	0.886	1010	· ·	2000	1010	· ·	1000	1010	0.540	1001	
Satd. Flow (perm)	0	1637	1615	0	2000	1417	0	1980	1393	962	1867	0
Right Turn on Red	· ·	1001	Yes	· ·	2000	Yes	· ·	1000	Yes	002	1001	Yes
Satd. Flow (RTOR)			18			20			18		5	. 00
Link Speed (mph)		25			25			25			25	
Link Distance (ft)		405			420			116			254	
Travel Time (s)		11.0			11.5			3.2			6.9	
Confl. Peds. (#/hr)	41	11.0	4	4	11.0	41	27	0.2	48	48	0.0	27
Confl. Bikes (#/hr)	• •		2	•		2	- 1		10	10		1
Peak Hour Factor	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92
Growth Factor	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%	100%
Heavy Vehicles (%)	0%	0%	0%	0%	0%	0%	0%	1%	0%	0%	1%	0%
Bus Blockages (#/hr)	0	0	0	0	0	0	0	0	0	0	0	0
Parking (#/hr)												
Mid-Block Traffic (%)		0%			0%			0%			0%	
Shared Lane Traffic (%)		0 70			0 70			0 70			0 70	
Lane Group Flow (vph)	0	83	8	0	51	20	0	372	18	4	497	0
Turn Type	Perm	NA	Prot		NA	Perm	<u> </u>	NA	Perm	Perm	NA	
Protected Phases	1 01111	4	4		8	1 01111		2	1 01111	1 01111	6	
Permitted Phases	4		- Т		J	8			2	6	U	
Detector Phase	4	4	4		8	8		2	2	6	6	
Switch Phase	<u> </u>		- Т		J	U					U	
Minimum Initial (s)	5.0	5.0	5.0		5.0	5.0		5.0	5.0	5.0	5.0	
Minimum Split (s)	22.5	22.5	22.5		22.5	22.5		22.5	22.5	22.5	22.5	
Total Split (s)	30.0	30.0	30.0		30.0	30.0		60.0	60.0	60.0	60.0	
Total Split (%)	33.3%	33.3%	33.3%		33.3%	33.3%		66.7%	66.7%	66.7%	66.7%	
Yellow Time (s)	3.5	3.5	3.5		3.5	3.5		3.5	3.5	3.5	3.5	
All-Red Time (s)	1.0	1.0	1.0		1.0	1.0		1.0	1.0	1.0	1.0	
Lost Time Adjust (s)	1.0	0.0	0.0		0.0	0.0		0.0	0.0	0.0	0.0	
Total Lost Time (s)		4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	
. ,		4.5	4.5		4.5	4.5		4.5	4.5	4.5	4.5	
Lead/Lag												
Lead-Lag Optimize?	Nana	Mona	Mona		None	Mona		C Min	C Min	C Min	C-Min	
Recall Mode	None	None	None		None	None		C-Min	C-Min	C-Min		
Act Effct Green (s)		9.7	9.7		9.7	9.7		74.2	74.2	74.2	74.2	
Actuated g/C Ratio		0.11	0.11		0.11	0.11		0.82	0.82	0.82	0.82	

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Lanes, Volumes, Timings

2: Main Street & Warren Avenue

08/26/2024

	•	\rightarrow	*	1	←	•	1	1	1	-	ţ	4
Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
v/c Ratio		0.47	0.04		0.24	0.12		0.23	0.02	0.01	0.32	
Control Delay		45.6	8.0		38.1	16.2		2.9	1.2	2.5	3.3	
Queue Delay		0.0	0.0		0.0	0.0		5.1	0.6	0.0	0.0	
Total Delay		45.6	8.0		38.1	16.2		8.0	1.9	2.5	3.3	
LOS		D	Α		D	В		Α	Α	Α	Α	
Approach Delay		42.3			32.0			7.7			3.3	
Approach LOS		D			С			Α			Α	
Queue Length 50th (ft)		45	0		27	0		40	0	1	59	
Queue Length 95th (ft)		87	7		59	20		80	5	m2	111	
Internal Link Dist (ft)		325			340			36			174	
Turn Bay Length (ft)			60			55				60		
Base Capacity (vph)		463	470		566	415		1633	1152	793	1541	
Starvation Cap Reductn		0	0		0	0		1181	1022	0	0	
Spillback Cap Reductn		0	0		0	0		0	0	0	0	
Storage Cap Reductn		0	0		0	0		0	0	0	0	
Reduced v/c Ratio		0.18	0.02		0.09	0.05		0.82	0.14	0.01	0.32	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 0 (0%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 45

Control Type: Actuated-Coordinated

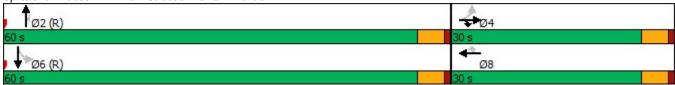
Maximum v/c Ratio: 0.47

Intersection Signal Delay: 10.2 Intersection LOS: B
Intersection Capacity Utilization 46.2% ICU Level of Service A

Analysis Period (min) 15

m Volume for 95th percentile queue is metered by upstream signal.

Splits and Phases: 2: Main Street & Warren Avenue



Queuing and Blocking Report Year 2030 Total Weekday Evening Peak Hour

08/27/2024

Intersection: 2: Main Street & Warren Avenue

Movement	EB	EB	WB	WB	NB	NB	SB	SB	
Directions Served	LT	R	Т	R	T	R	L	TR	
Maximum Queue (ft)	106	44	75	47	67	32	44	184	
Average Queue (ft)	46	6	23	10	43	5	4	91	
95th Queue (ft)	89	27	56	33	68	21	24	157	
Link Distance (ft)	320		373		45	45		178	
Upstream Blk Time (%)					9	0		1	
Queuing Penalty (veh)					16	0		2	
Storage Bay Dist (ft)		60		55			60		
Storage Blk Time (%)	6	0	1	0				10	
Queuing Penalty (veh)	0	0	0	0				0	

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Intersection Capacity Utilization 3: Forest Avenue & Franklin Street

	۶	-	•	•	•	•	1	†	1	-	Ţ	1
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Volume (vph)	12	13	45	160	40	2	20	104	64	3	45	1
Pedestrians			7	7			3		5	5		3
Ped Button		Yes						Yes			Yes	
Pedestrian Timing (s)		16.0						16.0			16.0	
Free Right			No			No			No			No
Ideal Flow	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Lost Time (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Minimum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0
Refr Cycle Length (s)	120	120	120	120	120	120	120	120	120	120	120	120
Volume Combined (vph)	0	70	0	0	202	0	0	188	0	0	49	0
Lane Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Factor (vph)	0.95	0.90	0.85	0.95	0.96	0.85	0.95	0.94	0.85	0.95	0.99	0.85
Saturated Flow (vph)	0	1702	0	0	1822	0	0	1793	0	0	1888	0
Ped Intf Time (s)	0.0	0.6	0.9	0.0	0.0	0.0	0.0	0.2	0.6	0.0	0.0	0.4
Pedestrian Frequency (%)		0.21			0.00			0.15			0.10	
Protected Option Allowed		No			No			No			No	
Reference Time (s)			0.0			0.0			0.0			0.0
Adj Reference Time (s)			0.0			0.0			0.0			0.0
Permitted Option												
Adj Saturation A (vph)	0	1776		0	529		0	1176		0	1607	
Reference Time A (s)	0.0	5.3		0.0	45.9		0.0	19.4		0.0	3.7	
Adj Saturation B (vph	0	0		0	0		0	0		NA	NA	
Reference Time B (s)	8.8	13.5		18.6	21.3		9.3	20.8		NA	NA	
Reference Time (s)		5.3			21.3			19.4			3.7	
Adj Reference Time (s)		11.5			25.3			23.4			9.1	
Split Option												
Ref Time Combined (s)	0.0	5.5		0.0	13.3		0.0	12.8		0.0	3.1	
Ref Time Seperate (s)	0.8	1.5		10.6	2.5		1.3	7.2		0.2	2.9	
Reference Time (s)	5.5	5.5		13.3	13.3		12.8	12.8		3.1	3.1	
Adj Reference Time (s)	11.7	11.7		17.3	17.3		17.3	17.3		9.1	9.1	
Summary	EB WB		NB SB	Co	mbined							
Protected Option (s)	NA		NA									
Permitted Option (s)	25.3		23.4									
Split Option (s)	29.0		26.4									
Minimum (s)	25.3		23.4		48.7							
Right Turns												
Adj Reference Time (s)												
Cross Thru Ref Time (s)												
Oncoming Left Ref Time (s)												
Combined (s)												
Intersection Summary												
Intersection Capacity Utilizat			40.6%			of Service			Α			
Reference Times and Phasir	ng Options	do not re	present a	ın optimiz	ed timing	plan.						

Intersection Capacity Utilization 5: Forest Avenue & Warren Avenue

	۶	•	4	†	ļ	1		
ovement	EBL	EBR	NBL	NBT	SBT	SBR		
ne Configurations	*	#		4	1>			
lume (vph)	32	202	84	144	196	20		
destrians	10		14			14		
Button					Yes			
estrian Timing (s)					16.0			
e Right		No				No		
al Flow	1900	1900	1900	1900	1900	1900		
t Time (s)	4.0	4.0	4.0	4.0	4.0	4.0		
mum Green (s)	4.0	4.0	4.0	4.0	4.0	4.0		
Cycle Length (s)	120	120	120	120	120	120		
me Combined (vph)	32	202	0	228	216	0		
e Utilization Factor	1.00	1.00	1.00	1.00	1.00	1.00		
ing Factor (vph)	0.95	0.85	0.95	0.98	0.99	0.85		
rated Flow (vph)	1805	1615	0.00	1865	1874	0.00		
I Intf Time (s)	0.0	0.0	0.0	0.0	0.2	1.7		
estrian Frequency (%)	0.00	3.0	3.0	0.00	0.37			
tected Option Allowed	No			No	No			
erence Time (s)	110	15.0		140	110	0.0		
Reference Time (s)		19.0				0.0		
nitted Option		13.0				0.0		
Saturation A (vph)	120		0	277	1874			
erence Time A (s)	31.9		0.0	98.6	14.0			
Saturation B (vph	NA		NA	NA	NA			
erence Time B (s)	NA		NA	NA	NA			
erence Time (s)	INA		INA	98.6	14.0			
Reference Time (s)				102.6	18.7			
				102.0	10.7			
t Option	2.1		0.0	14.7	14.0			
Time Combined (s)	2.1		5.6		14.0			
Time Seperate (s)	2.1			9.1				
ference Time (s)			14.7	14.7	14.0			
Reference Time (s)	8.0		18.7	18.7	18.7			
ımary	EB		NB SB	Col	mbined			
ected Option (s)	NA		NA					
nitted Option (s)	Err		102.6					
Option (s)	8.0		37.4					
mum (s)	8.0		37.4		45.4			
nt Turns	EBR							
Reference Time (s)	19.0							
ss Thru Ref Time (s)	18.7							
oming Left Ref Time (s)	0.0							
bined (s)	37.7							
. ,	07.1							
section Summary			07.00/	10				
section Capacity Utilization Tence Times and Phasing		1	37.8%			of Service	Α	

HCM 6th TWSC

4: Forest Avenue & AT&T Access Drive/Apartment Access Drive

Intersection												
Int Delay, s/veh	0.2											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	0	0	0	0	1	1	181	0	3	231	1
Future Vol, veh/h	1	0	0	0	0	1	1	181	0	3	231	1
Conflicting Peds, #/hr	0	0	2	2	0	0	16	0	0	0	0	16
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage,	# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	86	86	86	86	86	86	86	86	86	86	86	86
Heavy Vehicles, %	0	0	0	0	0	0	100	0	0	0	0	0
Mvmt Flow	1	0	0	0	0	1	1	210	0	3	269	1
Major/Minor N	/linor2		N	Minor1			Major1		N	Major2		
Conflicting Flow All	505	504	288	490	504	210	286	0	0	210	0	0
Stage 1	292	292	-	212	212		-	_	-	-	-	-
Stage 2	213	212	_	278	292	-	-	_	-	-	-	-
Critical Hdwy	7.1	6.5	6.2	7.1	6.5	6.2	5.1	-	-	4.1	-	_
Critical Hdwy Stg 1	6.1	5.5	-	6.1	5.5	-		_	_		_	-
Critical Hdwy Stg 2	6.1	5.5	-	6.1	5.5	-	_	_	_	_	_	_
Follow-up Hdwy	3.5	4	3.3	3.5	4	3.3	3.1	_	_	2.2	_	_
Pot Cap-1 Maneuver	481	473	756	492	473	835	874	-	-	1373	-	_
Stage 1	720	675	-	795	731	-		_	_	-	-	-
Stage 2	794	731	-	733	675	-	-	-	-	-	-	_
Platoon blocked, %	. • .	. • 1		. 00	3, 3			_	_		_	_
Mov Cap-1 Maneuver	471	464	743	490	464	835	861	_	-	1373	-	_
Mov Cap-2 Maneuver	471	464	-	490	464	-	-	_	_	-	_	_
Stage 1	708	663	_	794	730	-	-	_	-	_	_	_
Stage 2	792	730	_	729	663	_	_	_	_	_	_	_
		. 00			300							
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.7			9.3			0.1			0.1		
HCM LOS	12.7 B			9.3 A			0.1			0.1		
TIOWI LOS	D			А								
Minor Lane/Major Mvmt		NBL	NBT	NRD	EBLn1V	WRI n1	SBL	SBT	SBR			
Capacity (veh/h)		861	IND I	NDK I	471	835	1373	- 201	JDK			
HCM Lane V/C Ratio		0.001				0.001			-			
			-					-	-			
HCM Lang LOS		9.2	0	-	12.7	9.3	7.6	0	-			
HCM 05th %(tile O(yeh)		A	Α	-	В	A	A	Α	-			
HCM 95th %tile Q(veh)		0	-	-	0	0	0	-	-			

HCM 6th TWSC

6: Warren Avenue & Forest Avenue

Intersection						
Int Delay, s/veh	1.6					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	Y T	7	1	7	ODL	41
Traffic Vol, veh/h	29	28	199	19	57	341
Future Vol, veh/h	29	28	199	19	57	341
Conflicting Peds, #/hr	0	0	0	28	28	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	Stop -	None	-	None	-	
Storage Length	0	0	_	0	_	-
Veh in Median Storage		-	0	-	-	0
Grade, %	s, # 0 0	-	0			
	-		-	- 04	- 04	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	0	1	0	0	1
Mvmt Flow	31	30	212	20	61	363
Major/Minor I	Minor1	N	/lajor1	ľ	Major2	
Conflicting Flow All	544	240	0	0	260	0
Stage 1	240	-	_	_	-	-
Stage 2	304	_	_	_	_	_
Critical Hdwy	6.6	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.4	-	_	_	- '	_
Critical Hdwy Stg 2	5.8	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	<u>-</u>	_	2.2	_
Pot Cap-1 Maneuver	*750	*928	_		*1392	
•	*875	920	_	-	1392	_
Stage 1	*728			-		
Stage 2		-	-	-	-	-
Platoon blocked, %	1	1	-	-	1	-
Mov Cap-1 Maneuver	*689	*903	-	-	*1355	-
Mov Cap-2 Maneuver	*689	-	-	-	-	-
Stage 1	*852	-	-	-	-	-
Stage 2	*687	-	-	-	-	-
Approach	WB		NB		SB	
	9.8				1.3	
HCM Control Delay, s			0		1.3	
HCM LOS	Α					
Minor Lane/Major Mvm	nt	NBT	NBRV	VBLn1V	VBLn2	SBL
Capacity (veh/h)		_	_	689		* 1355
HCM Lane V/C Ratio		_	_	0.045		
HCM Control Delay (s)		_	_	10.5	9.1	7.8
HCM Lane LOS		_	_	В	A	Α.
HCM 95th %tile Q(veh))	_	_	0.1	0.1	0.1
	1			0.1	0.1	0.1
Notes						
~: Volume exceeds cap	pacity	\$: De	lay exc	eeds 30	00s	+: Comp

HCM 6th TWSC 8: N-S Alley & Franklin Street

Intersection						
Int Delay, s/veh	0.7					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	₽			र्स	W	
Traffic Vol, veh/h	68	7	11	192	6	8
Future Vol, veh/h	68	7	11	192	6	8
Conflicting Peds, #/hr	0	9	9	0	1	1
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-		-	None
Storage Length	_	-	_	-	0	-
Veh in Median Storage	e,# 0	_	_	0	0	_
Grade, %	0	<u>-</u>	_	0	0	_
Peak Hour Factor	79	79	79	79	79	79
	0	0	0	19	0	0
Heavy Vehicles, %	86		14	•	8	
Mvmt Flow	80	9	14	243	ð	10
Major/Minor	Major1	N	Major2	I	Minor1	
Conflicting Flow All	0	0	104	0	372	101
Stage 1	-	-	_	_	100	-
Stage 2	_	_	-	_	272	_
Critical Hdwy	_	_	4.1	_	6.4	6.2
Critical Hdwy Stg 1	_	_	- '	_	5.4	-
Critical Hdwy Stg 2	_	_	_	_	5.4	_
Follow-up Hdwy		_	2.2	_	3.5	3.3
	-	-	1500		691	960
Pot Cap-1 Maneuver	-	-		-		
Stage 1	-	-	-	-	929	-
Stage 2	-	-	-	-	821	-
Platoon blocked, %	-	-		-	1	
Mov Cap-1 Maneuver	-	-	1487	-	676	951
Mov Cap-2 Maneuver	-	-	-	-	676	-
Stage 1	-	-	-	-	921	-
Stage 2	-	-	-	-	811	-
, and the second						
Ammunah	ED		WD		ND	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.4		9.5	
HCM LOS					Α	
Minor Lane/Major Mvn	nt I	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		810	_	_		_
HCM Lane V/C Ratio		0.022	_		0.009	_
HCM Control Delay (s)	\	9.5	_	_	7.4	0
HCM Lane LOS		9.5 A	_	_	7.4 A	A
		_	-	-	Α.	Α.
HCM 95th %tile Q(veh	١	0.1	_	_	0	_

HCM 6th TWSC

9: Main Street & Oakley Access Drive

Intersection								
Int Delay, s/veh	0							
		EDD	NDI	NDT	CDT	CDD		
Movement	EBL	EBR	NBL	NBT	SBT	SBR		
Lane Configurations	Y			4	1			
Traffic Vol, veh/h	1	0	0	458	502	0		
Future Vol, veh/h	1	0	0	458	502	0		
Conflicting Peds, #/hr	1	0	12	0	0	12		
Sign Control	Stop	Stop	Free	Free	Free	Free		
RT Channelized	-	None	-	None	-	None		
Storage Length	0	-	-	-	-	-		
Veh in Median Storage	e, # 0	-	-	0	0	-		
Grade, %	0	-	-	0	0	-		
Peak Hour Factor	90	90	90	90	90	90		
Heavy Vehicles, %	0	0	0	1	0	0		
Mvmt Flow	1	0	0	509	558	0		
Major/Mirar	Minaro		10:1		10i-c-0			
	Minor2		Major1		/lajor2			
Conflicting Flow All	1080	570	570	0	-	0		
Stage 1	570	-	-	-	-	-		
Stage 2	510	-	-	-	-	-		
Critical Hdwy	6.4	6.2	4.1	-	-	-		
Critical Hdwy Stg 1	5.4	-	-	-	-	-		
Critical Hdwy Stg 2	5.4	-	-	-	-	-		
Follow-up Hdwy	3.5	3.3	2.2	-	-	-		
Pot Cap-1 Maneuver	*227	*660	*991	-	-	-		
Stage 1	*623	-	-	-	-	-		
Stage 2	*673	-	-	-	-	-		
Platoon blocked, %	1	1	1	-	-	-		
Mov Cap-1 Maneuver	*222	*653	*979	-	-	-		
Mov Cap-2 Maneuver	*222	-	-	-	-	-		
Stage 1	*616	-	-	-	-	-		
Stage 2	*666	-	-	-	-	-		
Annroach	ED		ND		CD			
Approach	EB		NB		SB			
HCM Control Delay, s	21.3		0		0			
HCM LOS	С							
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR		
Capacity (veh/h)		* 979		222	<u> </u>	<u> </u>		
HCM Lane V/C Ratio		919		0.005	-	-		
HCM Control Delay (s)		0	_	21.3	-			
HCM Lane LOS		A		21.3 C				
HCM 95th %tile Q(veh	١		-	0	-	-		
HOW SOUT WITH Q(Ven)	0	-	U		-		
Notes								
~: Volume exceeds ca	pacity	\$: De	lay exc	eeds 30	00s	+: Com	outation Not Defined	*: All major volume in platod
			,					,

HCM 6th TWSC

10: Main Street & Funeral Home North Access Drive

Intersection									
Int Delay, s/veh	0								
Movement	EBL	EBR	NBL	NBT	SBT	SBR			
Lane Configurations	W			4	₽	_			
Traffic Vol, veh/h	0	0	0	457	501	0			
Future Vol, veh/h	0	0	0	457	501	0			
Conflicting Peds, #/hr	0	0	13	0	0	13			
Sign Control	Stop	Stop	Free	Free	Free	Free			
RT Channelized	Slop -	None	-			None			
Storage Length	0	NOHE -	-		-	NOHE -			
				-	-				
Veh in Median Storage		-	-	0	0	-			
Grade, %	0	-	-	0	0	-			
Peak Hour Factor	90	90	90	90	90	90			
Heavy Vehicles, %	0	0	0	0	1	0			
Mvmt Flow	0	0	0	508	557	0			
Major/Minor I	Minor2	N	/lajor1	N	lajor2				
Conflicting Flow All	1078	570	570	0		0			
Stage 1	570	-	-	-	-	-			
Stage 2	508	_	-	_	-	_			
Critical Hdwy	6.4	6.2	4.1	_	_	_			
Critical Hdwy Stg 1	5.4	-	_	_	_	_			
Critical Hdwy Stg 2	5.4	_	_	_	_	_			
Follow-up Hdwy	3.5	3.3	2.2	_	_	_			
Pot Cap-1 Maneuver	*216	*660	*991	_	_	_			
Stage 1	*623	-	-	_	_	_			
Stage 2	*676	_	_	_	_	_			
Platoon blocked, %	1	1	1	_	_				
Mov Cap-1 Maneuver	*211	*652	*978	_	_	_			
Mov Cap-1 Maneuver	*211	-	310		_				
Stage 1	*615	_	-						
	*668	-	-	-	-	-			
Stage 2	000	_	-	-	-	_			
Approach	EB		NB		SB				
HCM Control Delay, s	0		0		0				
HCM LOS	A								
Minor Lane/Major Mvm	nt.	NBL	NDT	EBLn1	SBT	SBR			
	It		INDI	LOLIII	JDI	אמט			
Capacity (veh/h)		* 978	-	-	-	-			
HCM Cantrol Dalay (a)		-	-	-	-	-			
HCM Control Delay (s)		0	-	0	-	-			
HCM Lane LOS		A	-	Α	-	-			
HCM 95th %tile Q(veh)		0	-	-	-	-			
Notes									
~: Volume exceeds cap	oacity	\$: De	lay exc	eeds 30	0s	+: Comp	outation Not Defined	*: All major volume in platoon	

HCM 6th TWSC 11: Main Street & Rogers Street

Intersection						
Int Delay, s/veh	1.7					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	¥	.,,,,,,,,	1>		ሻ	<u> </u>
Traffic Vol, veh/h	14	97	360	22	48	436
Future Vol, veh/h	14	97	360	22	48	436
Conflicting Peds, #/hr		8	0	15	15	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	- -		-		-	None
Storage Length	0	-	<u>-</u>	-	60	-
Veh in Median Storage		_	0	_	-	0
Grade, %	0, # 0	<u>-</u>	0	<u>-</u>	_	0
Peak Hour Factor	94	94	94	94	94	94
Heavy Vehicles, %	0	0	0	0	0	1
Mymt Flow	15	103	383	23	51	464
IVIVIIIL FIOW	15	103	J0J	23	וכ	404
Major/Minor	Minor1	N	Major1	N	Major2	
Conflicting Flow All	978	418	0	0	421	0
Stage 1	410	-	-	-	_	-
Stage 2	568	-	-	_	_	-
Critical Hdwy	6.4	6.2	_	_	4.1	_
Critical Hdwy Stg 1	5.4	-	_	_	-	_
Critical Hdwy Stg 2	5.4	_	_	_	_	_
Follow-up Hdwy	3.5	3.3	_	_	2.2	_
Pot Cap-1 Maneuver	*331	784	_	_	1175	_
Stage 1	*744	- 104	_	<u>-</u>	1170	_
Stage 2	*622	_	_	_	_	_
Platoon blocked, %	1	1	_	_	1	-
		767			1158	
Mov Cap-1 Maneuver		-	-	-		-
Mov Cap-2 Maneuver		-	-	-	-	-
Stage 1	*734	-	-	-	-	-
Stage 2	*593	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s			0		0.8	
HCM LOS	В		U		0.0	
TICIVI LOG	U					
Minor Lane/Major Mvr	nt	NBT	NBRV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	647	1158	-
HCM Lane V/C Ratio		-	-	0.183	0.044	-
HCM Control Delay (s	s)	_	-	11.8	8.3	-
HCM Lane LOS	,	-	-	В	Α	-
HCM 95th %tile Q(veh	1)	-	-	0.7	0.1	-
Notes						_
~: Volume exceeds ca	pacity	\$: De	lay exc	eeds 30)0s ·	+: Comp

HCM 6th TWSC

12: Forest Avenue & Garage Access Drive

Intersection						
Int Delay, s/veh	0.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	**		₽			4
Traffic Vol, veh/h	1	3	181	2	4	234
Future Vol, veh/h	1	3	181	2	4	234
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	_	None	_	None	-	None
Storage Length	0	_	_	_	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	_	0	-	_	0
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	0	0	0
Mymt Flow	1	3	191	2	4	246
IVIVIII(I IOW		0	101		7	240
Major/Minor I	Minor1	N	Major1		Major2	
Conflicting Flow All	446	192	0	0	193	0
Stage 1	192	-	-	-	-	-
Stage 2	254	-	-	-	-	-
Critical Hdwy	6.4	6.2	-	-	4.1	-
Critical Hdwy Stg 1	5.4	-	-	-	-	-
Critical Hdwy Stg 2	5.4	_	-	-	-	_
Follow-up Hdwy	3.5	3.3	-	-	2.2	-
Pot Cap-1 Maneuver	574	855	-	-	1392	-
Stage 1	845	-	-	-	-	-
Stage 2	793	_	-	-	_	_
Platoon blocked, %			_	_		_
Mov Cap-1 Maneuver	572	855	_	_	1392	_
Mov Cap-2 Maneuver	572	-	_	_	-	_
Stage 1	845	_	_	_	_	_
Stage 2	791	_	_		_	_
Stage 2	791	-	-	-	-	-
Approach	WB		NB		SB	
HCM Control Delay, s	9.8		0		0.1	
HCM LOS	Α					
Minor Long/Major Mym	.1	NDT	NDDV	VDI 51	CDI	CDT
Minor Lane/Major Mvm	II	NBT	INBKV	VBLn1	SBL	SBT
Capacity (veh/h)		-	-	761	1392	-
HCM Lane V/C Ratio		-	-	0.006		-
HCM Control Delay (s)		-	-	9.8	7.6	0
HCM Lane LOS		-	-	A	A	Α
HCM 95th %tile Q(veh)		-	-	0	0	-

HCM 6th TWSC 13: N-S Alley & Garage Access Drive

Intersection						
Int Delay, s/veh	2.2					
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	TOL.	LDI	NDL	ND I		JUC
Traffic Vol, veh/h	6	0	0	4	1	9
Future Vol, veh/h	6	0	0	4	5	9
	0	0	0	0	0	0
Conflicting Peds, #/hr						
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-		-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	95	95	95	95	95	95
Heavy Vehicles, %	0	0	0	0	0	0
Mvmt Flow	6	0	0	4	5	9
Major/Minor N	/linor2	n	Major1	N	//ajor2	
Conflicting Flow All	14	10	14	0	- najoiz	0
Stage 1	10					
		-	-	-	-	-
Stage 2	4	-	-	-	-	-
Critical Hdwy	6.4	6.2	4.1	-	-	-
Critical Hdwy Stg 1	5.4	-		_	-	-
Critical Hdwy Stg 2	5.4	-	-	-	-	-
Follow-up Hdwy	3.5	3.3	2.2	-	-	-
Pot Cap-1 Maneuver	1010	1077	1617	-	-	-
Stage 1	1018	-	-	-	-	-
Stage 2	1024	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	1010	1077	1617	_	_	_
Mov Cap-2 Maneuver	1010	-	-	_	_	_
Stage 1	1018	_	_	_	_	_
Stage 2	1010	_	_		_	
Stage 2	1024	-	_	_	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	8.6		0		0	
HCM LOS	Α					
Minantana/Minantana		NDI	NDT	EDL 4	ODT	000
Minor Lane/Major Mvm	[NBL		EBLn1	SBT	SBR
Capacity (veh/h)		1617		1010	-	-
HCM Lane V/C Ratio		-	-	0.006	-	-
HCM Control Delay (s)		0	-	0.0	-	-
HCM Lane LOS		Α	-	Α	-	-
HCM 95th %tile Q(veh)		0	-	0	-	-
TOTAL COURT FOUND CELLOTIS						

1/21 , 2024

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Chairman Rickard
Village of Downers Grove Civic Center
Planning and Zoning Commission
890 Curtiss Avenue
Downers Grove, IL 60515
Attn: Jason Zawila, jzawila@downers.us

Re:

ORD 2024-10610

Letter of Support for 4Corners, LLC Redevelopment of 4919 Forest Avenue Request for Zoning Map Amendment, Special Use, PUD & Site Plan Approval

Dear Chairman Rickard,

As the owner of All Creatives Great, a local business and neighbor of the property located at 4919 Forest Avenue (the "Property"), I am writing to express my support for 4Corners, LLC (the "Applicant") and its application for a zoning map amendment, planned unit development designation, special use permit, and site plan approval for the redevelopment of the Property..

The Applicant proposes to construct a seven-story multi-family residential building at the Property, containing 62 rental residential units, 89 vehicular parking spaces, and no commercial space. Currently, the Property is zoned as part of the Downtown Business District and consists of three existing lots that are improved with two aging, increasingly obsolete two-story buildings and a surface parking lot. These buildings have been substantially vacant for the last six years.

I have reviewed the plans for the development and believe that the proposal is appropriate for the surrounding neighborhood. As a local business owner, I believe that activating a vacant and underutilized lot with a high quality multi-family residential building is in the interest of the public convenience and will not have any adverse impact on the general welfare of the neighborhood. The proposed development will increase the diversity and availability of housing options in the neighborhood which in turn will contribute to the downtown's vitality. For these reasons, I strongly support the proposed development and requested zoning change.

Sincerely,

Cc (vie e-mail):

November 19, 2024

Page 226 of 243

Chairman Rickard
Village of Downers Grove Civic Center
Planning and Zoning Commission
890 Curtiss Avenue
Downers Grove, IL 60515
Attn: Jason Zawila, jzawila@downers.us

Re: Letter of Support for 4Corners, LLC

Redevelopment of 4919 Forest Avenue

Request for Zoning Map Amendment, Special Use, PUD & Site Plan Approval

Dear Chairman Rickard,

ORD 2024-10610

As the owner of the property located at 4920 Main St. and a neighbor of the property located at 4919 Forest Avenue (the "Property"), I am writing to express my strong support for 4Corners, LLC (the "Applicant") and its application for a zoning map amendment, planned unit development, special use permit, and site plan approval for the redevelopment of the Property.

The Applicant proposes to construct a high-quality, seven-story multi-family residential building at the Property. The development will contain 62 rental residential units, 89 vehicular parking spaces, and no commercial space. Currently, the Property contains two aging, increasingly obsolete two-story buildings and a surface parking lot which have remained substantially vacant for the last six years.

I have reviewed the plans for the proposed development and believe the project is well-suited to the character of the surrounding community. As a local business owner, I support activating this vacant and underutilized site with a high-quality multi-family residential building that will contribute to the neighborhood's growth. In addition to providing more housing options and bringing more residents to the area, this redevelopment will increase the local tax base by reactivating a long-vacant site and enhancing surrounding property values, further supporting the downtown's vitality. For these reasons, I strongly support the proposed development and requested zoning change.

Sincerely,

Cc (vie e-mail):

11/18, 2024

Chairman Rickard Village of Downers Grove Civic Center Planning and Zoning Commission 890 Curtiss Avenue Downers Grove, IL 60515 Attn: Jason Zawila, jzawila@downers.us

Re: Letter of Support for 4Corners, LLC Redevelopment of 4919 Forest Avenue

Request for Zoning Map Amendment, Special Use, PUD & Site Plan Approval

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pend llo

Cc (vie e-mail):

November 19, 2024

Page 228 of 243

Chairman Rickard
Village of Downers Grove Civic Center
Planning and Zoning Commission
890 Curtiss Avenue
Downers Grove, IL 60515
Attn: Jason Zawila, jzawila@downers.us

Re: Letter of Support for 4Corners, LLC

Redevelopment of 4919 Forest Avenue

Request for Zoning Map Amendment, Special Use, PUD & Site Plan Approval

Dear Chairman Rickard,

As the owner of Cappetta's Funeral Home, a local business and neighbor of the property located at 4919 Forest Avenue (the "Property"), I am writing to express my support for 4Corners, LLC (the "Applicant") and its application for a zoning map amendment, planned unit development designation, special use permit, and site plan approval for the redevelopment of the Property.

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Sincerely

Cc (vie e-mail):

_____, 2024

Chairman Rickard
Village of Downers Grove Civic Center
Planning and Zoning Commission
890 Curtiss Avenue
Downers Grove, IL 60515

Attn: Jason Zawila, jzawila@downers.us

Re: Letter of Support for 4Corners, LLC
Redevelopment of 4919 Forest Avenue
Request for Zoning Map Amendment, Special Use, PUD & Site Plan Approval

Dear Chairman Rickard,

As a representative of the Moose Lodge, local business and neighbor of the property located at 4919 Forest Avenue (the "**Property**"), I am writing to express my support for 4Corners, LLC (the "**Applicant**") and its application for a zoning map amendment, planned unit development designation, special use permit, and site plan approval for the redevelopment of the Property..

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Sincerely,

DocuSigned by:

Stew Lytle

2011/2024

Cc (vie e-mail): Liz Butler, Taft Stettinius & Hollister LLP (LButler@taftlaw.com) 1/21 , 2024

Page 230 of 243

Chairman Rickard
Village of Downers Grove Civic Center
Planning and Zoning Commission
890 Curtiss Avenue
Downers Grove, IL 60515
Attn: Jason Zawila, jzawila@downers.us

Re:

ORD 2024-10610

Letter of Support for 4Corners, LLC Redevelopment of 4919 Forest Avenue Request for Zoning Map Amendment, Special Use, PUD & Site Plan Approval

Dear Chairman Rickard,

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Sincerely,

Cc (vie e-mail):

November 19, 2024

Page 231 of 243

Chairman Rickard
Village of Downers Grove Civic Center
Planning and Zoning Commission
890 Curtiss Avenue
Downers Grove, IL 60515
Attn: Jason Zawila, jzawila@downers.us

Re: Letter of Support for 4Corners, LLC

Redevelopment of 4919 Forest Avenue

Request for Zoning Map Amendment, Special Use, PUD & Site Plan Approval

Dear Chairman Rickard,

ORD 2024-10610

As the owner of the property located at 4920 Main St. and a neighbor of the property located at 4919 Forest Avenue (the "Property"), I am writing to express my strong support for 4Corners, LLC (the "Applicant") and its application for a zoning map amendment, planned unit development, special use permit, and site plan approval for the redevelopment of the Property.

The Applicant proposes to construct a high-quality, seven-story multi-family residential building at the Property. The development will contain 62 rental residential units, 89 vehicular parking spaces, and no commercial space. Currently, the Property contains two aging, increasingly obsolete two-story buildings and a surface parking lot which have remained substantially vacant for the last six years.

I have reviewed the plans for the proposed development and believe the project is well-suited to the character of the surrounding community. As a local business owner, I support activating this vacant and underutilized site with a high-quality multi-family residential building that will contribute to the neighborhood's growth. In addition to providing more housing options and bringing more residents to the area, this redevelopment will increase the local tax base by reactivating a long-vacant site and enhancing surrounding property values, further supporting the downtown's vitality. For these reasons, I strongly support the proposed development and requested zoning change.

Sincerely,

Cc (vie e-mail):

11/18, 2024

Chairman Rickard Village of Downers Grove Civic Center Planning and Zoning Commission 890 Curtiss Avenue Downers Grove, IL 60515 Attn: Jason Zawila, jzawila@downers.us

Re: Letter of Support for 4Corners, LLC
Redevelopment of 4919 Forest Avenue

Request for Zoning Map Amendment, Special Use, PUD & Site Plan Approval

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Sincerely,

pend llo

Cc (vie e-mail):

November 19, 2024

Page 233 of 243

Chairman Rickard
Village of Downers Grove Civic Center
Planning and Zoning Commission
890 Curtiss Avenue
Downers Grove, IL 60515
Attn: Jason Zawila, jzawila@downers.us

Re: Letter of Support for 4Corners, LLC

Redevelopment of 4919 Forest Avenue

Request for Zoning Map Amendment, Special Use, PUD & Site Plan Approval

Dear Chairman Rickard,

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I have reviewed the plans for the development and believe that the proposal is appropriate for the surrounding neighborhood. As a local business owner, I believe that activating a vacant and underutilized lot with a high quality multi-family residential building is in the interest of the public convenience and will not have any adverse impact on the general welfare of the neighborhood. The proposed development will increase the diversity and availability of housing options in the neighborhood which in turn will contribute to the downtown's vitality. For these reasons, I strongly support the proposed development and requested zoning change.

Sincerely

Cc (vie e-mail):

_____, 2024

Chairman Rickard
Village of Downers Grove Civic Center
Planning and Zoning Commission
890 Curtiss Avenue
Downers Grove, IL 60515
Attn: Jacon Zawila izawila@downers.us

Attn: Jason Zawila, jzawila@downers.us

Re: Letter of Support for 4Corners, LLC
Redevelopment of 4919 Forest Avenue
Request for Zoning Map Amendment, Special Use, PUD & Site Plan Approval

Dear Chairman Rickard,

As a representative of the Moose Lodge, local business and neighbor of the property located at 4919 Forest Avenue (the "**Property**"), I am writing to express my support for 4Corners, LLC (the "**Applicant**") and its application for a zoning map amendment, planned unit development designation, special use permit, and site plan approval for the redevelopment of the Property..

The Applicant proposes to construct a seven-story multi-family residential building at the Property, containing 62 rental residential units, 89 vehicular parking spaces, and no commercial space. Currently, the Property is zoned as part of the Downtown Business District and consists of three existing lots that are improved with two aging, increasingly obsolete two-story buildings and a surface parking lot. These buildings have been substantially vacant for the last six years.

I have reviewed the plans for the development and believe that the proposal is appropriate for the surrounding neighborhood. As a local business owner, I believe that activating a vacant and underutilized lot with a high quality multi-family residential building is in the interest of the public convenience and will not have any adverse impact on the general welfare of the neighborhood. The proposed development will increase the diversity and availability of housing options in the neighborhood which in turn will contribute to the downtown's vitality. For these reasons, I strongly support the proposed development and requested zoning change.

Sincerely,

Docusigned by:

Stew Lyte

11/21/2024

Cc (vie e-mail): Liz Butler, Taft Stettinius & Hollister LLP (LButler@taftlaw.com) ORD 2024-10610 Page 235 of 243



November 26, 2024

Chairman Rickard

Village of Downers Grove Civic Center

Planning and Zoning Commission

890 Curtis Avenue

Downers Grove, IL 60515

Attn: Jason Zawila

Dear Chairman Rickard,

I write representing Community Bank of Downers Grove located at 1111 Warren Avenue and in support of the redevelopment located at 4919 Forest Avenue by 4Corners LLC. The proposed multifamily rental property containing sixty-two rental units will increase the availability and diversity of rental options in downtown Downers Grove and continue to enhance the vibrancy of the community with minimal, if any, adverse impact.

I have personally reviewed the plans for the development and believe it is appropriate for the surrounding community and generally consistent with the Vision Statement of the Downers Grove Comprehensive Plan as well as the land use map contained in the "Guiding DG" documents, albeit requiring a zoning map amendment. As an aside, it is heartening to see private development dollars and economic development flowing to the downtown area north of the railroad tracks.

Take note that Hinsdale Bank & Trust, the parent organization of Community Bank of Downers Grove, has a long standing and mutually beneficial relationship with Michael Gatto, a member of ownership of the subject development, and would look favorably on financing this development if requested to do so.

Respectfully

Dennis J. Jones

Chairman

Hinsdale Bank & Trust Co.

ORD 2024-10610 Page 236 of 243



Downers Grove Economic Development Corporation

5159 Mochel Downers Grove, IL 60515 630.729.0380 www.dgedc.com

November 22, 2024

Chairman Rickard Village of Downers Grove Civic Center Planning and Zoning Commission 890 Curtiss Avenue Downers Grove, IL 60515

Attn: Jason Zawila, jzawila@downers.us

Re: Letter of Support for 4Corners, LLC
4919 Forest Avenue
Request for Zoning Map Amendment, Special Use, PUD & Site Plan Approval

Dear Chairman Rickard,

On behalf of the Downers Grove Economic Development Corporation, I am writing to express our strong support for 4Corners, LLC (the "Applicant") and its application for a zoning map amendment, planned unit development designation, special use permit, and site plan approval for the redevelopment of the property located at 4919 Forest Avenue (the "Property").

The Applicant proposes to construct a high-quality, seven-story multi-family residential building at the Property. The development will contain 62 rental residential units, 89 vehicular parking spaces, and no commercial space. Currently, the Property consists of two aging, increasingly obsolete two-story buildings and a surface parking lot that have remained substantially vacant for the last six years.

We have reviewed the plans for the proposed development and believe the project is well-suited to the character of the surrounding community, aligning with the Village's Comprehensive Plan in terms of use, density, scale, and design. Approval of this application will allow for the revitalization of an underutilized property with a high-quality multi-family residential building, increasing the diversity and availability of housing options, supporting the local tax base, and contributing positively to downtown's vitality. For these reasons, the Downers Grove Economic Development Corporation strongly supports the proposed development and requested zoning change.

Please do not hesitate to contact us at bryan@dgedc.com or 630.729.0380 with any questions.

Sincerely,

Bryan Gay

President & CEO

Downers Grove Economic Development Corporation

Cc (vie e-mail):

ORD 2024-10610 Page 237 of 243

VILLAGE OF DOWNERS GROVE PLAN COMMISSION MEETING

December 2, 2024, 7:00 P.M.

FILE 24-PCE-0034: A PETITION SEEKING APPROVAL FOR A PLANNED UNIT DEVELOPMENT AMENDMENT. THE PROPERTY IS CURRENTLY ZONED DOWNTOWN BUSINESS/PLANNED UNIT DEVELOPMENT 61, DB/PD #61. THE PROPERTY IS LOCATED AT THE NORTHEAST CORNER OF THE INTERSECTION OF WASHINGTON STREET AND WARREN AVENUE, COMMONLY KNOWN AS 844 WARREN AVENUE, DOWNERS GROVE, IL (PINS: 09-08-125-004). ERIC SYTER, PETITIONER AND TIMOTHY CANNING OWNER

Chairman Rickard stated they received a memo from staff asking to continue this public hearing to the January 6 meeting.

Motion to continue by Commissioner Toth, seconded by Commissioner Rutledge.

FILE 24-PCE-0029: A PETITION SEEKING APPROVAL FOR A PLANNED UNIT DEVELOPMENT, MAP AMENDMENT AND A SPECIAL USE TO ALLOW FOR AN APARTMENT BUILDING. THE PROPERTY IS CURRENTLY ZONED DB, DOWNTOWN BUSINESS. THE PROPERTY IS LOCATED 175 FEET FROM THE INTERSECTION OF FRANKLIN STREET AND FOREST AVENUE, COMMONLY KNOWN AS 4919 FOREST AVENUE, DOWNERS GROVE, IL (PINS: 09-08-116-004, 09-08-116-006). LIZ BUTLER, PETITIONER AND DUNELAND MANAGEMENT ONE LLC, OWNER.

Liz Butler, attorney at Taft, Stettinius & Hollister, stated that the petitioner, 4Corners Development, LLC, is a real estate development and construction firm that enhances communities while prioritizing integrity, transparency, and collaboration with the community. She gave a brief overview of the proposed development, which involves the demolition of two existing structures, consolidation of three lots into a single, and construction of a new second story all residential building. The development will have 62 rental residential units, 89 vehicular parking spaces with 26 of those being tandem spaces, high quality building materials, appropriate bulk, height, and massing articulation, and will complement the existing character of the area and will adhere to the Village's downtown design guidelines. She stated they are seeking favorable recommendation of a planned unit development, zoning map amendment from downtown business to downtown business PUD, and special use approval for the multiunit residential building. Ms. Butler went over the key factors they wish the commission to consider with the project. She expressed that the 62 new residential units will address the critical housing need in Downers Grove and the project will revitalize a site that has been underutilized. She gave an overview of the site context and location, existing conditions on the site, and changes that resulted from the community engagement process, including operations in the alley that abuts the property.

Ben Kennedy, Kennedy Mann Architecture, explained how they designed the building to adhere to the downtown design guidelines, community, and planning. He discussed curb cuts, the three foot dedication off of the alley side, and loading zone at the front of the building. He went over details

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of the first floor, including the main entry, lobby, amenity space, internal package room, water room, electric room, generator room, bike room, rear drive, maintenance room, and internal trash room. He then discussed some details of the second floor, which include a lobby, storage room, domestic hot water tanks, and dog wash area. Mr. Kennedy went over parking, which includes 42 parking on the first floor and 47 on the second floor. He added there will be 62 regular spaces and 25 tandem, with 2 internal guest parking spots. He stated there will be a variety of unit types, with the average one bedroom unit being 868 square feet and, the two-bedroom at 983 square feet, and three-bedroom at 1,545 square feet. He noted that the third floor is stepped back, which result in terraces for a lot of the third floor units. On floors 4 through 6, the terraces will become balconies. There are 10 larger units on the 7th floor, with 1 one-bedroom, 4 two-bedroom, and 5 three-bedroom units. Mr. Kennedy went over the building elevation design, building materials and colors, utility service rooms, egress and ingress, and mentioned the revisions that came through the process of community engagement and discussing the project with staff. This included the dedication of three feet of the alley, revisions to window and balcony patterns on the building facade, and requiring left turns only out of the garage entrance on the alley.

Ms. Butler stated the project conforms with the recommendations of the comprehensive plan in a number of ways. She talked about the project benefits, including creating 50 to 75 jobs during construction, increase in property tax revenue, and transformation of the underutilized vacant site into a vibrant residential development. She said they worked closely with staff on the review of the proposed development and thanked them for their guidance. She noted the proposed development complies with all the standards of the zoning ordinance, and it is designed to be an amenity and provide benefits to the community.

Chairman Rickard asked for any questions for the petitioner.

Chairman Rickard asked Ms. Butler to talk about the improvement in the alley. Ms. Butler responded that the alley will be repaved.

Commissioner Boyle asked about the impervious area decreasing. Claudia Welp, Civil Engineer for the project, stated the total amount of impervious area is not decreasing, but not increasing enough to require stormwater management. She said they were provided more than 20,000 square feet of impervious area and had to provide detention and additional storage prior to releasing water to the city sewer, but since they are not increasing the restriction of 20,000 square feet, they only have to provide post-construction and best management practices.

Commissioner Frankovic asked if the alley was currently one way traffic going north. Ms. Butler expressed the project will install signage to direct people from the proposed development using the alley. Jason Zawila, Planning Manager, added there is traffic in both directions in the alley.

Commissioner Rutledge asked what the door in the renderings for the north elevation was for. Mr. Kennedy explained that door is for the water room.

Chairman Rickard asked for public input.

Scott Richards, shared his disappointed in the seventh floor and the lack of greenery on the project. He stated the front of the building is so close to the sidewalk that there is not any room to put any trees or bushes and greenery adds beauty to the buildings. He wished there was more of a setback

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in regard to the entry way on the west side so they could have something of interest as the entry way and wished the building was more of a natural transition. Mr. Richards added that parking on Forest Avenue during the day is very full.

Judy Donofrio, stated that the team did a great job with meeting the community and went over the issues came up at meeting and in conversations, which included flooding issues, additional traffic on the street, parking, and the bad shape of the alley.

Zach Frazier, talked about the north setback being 4 inches directly from his property line. He voiced concerns on how they are going to erect a wall and build the building 4 inches from his property line without disrupting his property. He discussed the report stating the building is 70 feet high, but all elevations are showing it going above 70 feet. He shared the downtown area has an 800 square foot minimum lot area per dwelling unit but 342 square feet per dwelling unit is proposed, and wanted to know why they are being allowed a deviation from the standard.

Barbara Koran reiterated the comment about the traffic flow. She explained that with Main Steet going down to one lane, and bringing in more units will cause more issues with traffic. She was also concerned that 1.44 parking spaces with having three and two bedroom apartments is not going to be enough, and only two guest parking spaces for 62 units is not enough.

Peggy Spiegel talked about the beauty of the front of the building. She said Downers Grove has beautiful front lawns, and they should setback the building a little more to add some bushes and a few trees.

Jenna Winningham echoed the concerns on parking and traffic, specifically when they mentioned if they are over capacity, they can park in paid lots. She stated she was not sure which paid lots they were talking about and 1.4 spaces per unit is not enough. She added that it is not fair for the taxpayers to have to support supplying parking because they do not have enough parking.

Charles Data (ph) also addressed the traffic and parking. He said it is clear there is not enough parking unless 62 units have no friends or family. He said there is a lot of traffic on Forest that they cannot even get out of their building at times.

Ron Welfler stated that they live on the southwest corner of their building, where they can see and hear the constant traffic. He said that between 4 and 6 p.m., when there is a train, it is common for traffic turning off of Main Street to use Forest Avenue and it backs up traffic on Forest and Franklin. He shared that with this additional traffic, they will not be able to get out of their building and access for emergency vehicles will be a problem.

Chairman Rickard then asked for the staff report.

Emily Hepworth, Development Planner, explained the petition was for a special unit and a planned unit development and rezoning at 4919 Forest Avenue. She stated that a mailing notice was provided to all property owners within 250 feet, a legal notice was posted in the Daily Herald, and a public notice sign was posted along Forest Avenue. The petitioner held two neighborhood meetings and staff received one public comment in opposition and seven support letters. She expressed that the project is a seven story multifamily residential building with a total of 62 units, with parking on the first and second floor and a total of 89 parking stalls, and a restriction on all right turns out of

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the second floor parking deck into the alley. Ms. Hepworth shared that the petition requested a deviation from the required minimum lot area per dwelling unit, and staff found the proposed level of density to be appropriate. She also discussed the front elevation, windows, building materials, and articulation. She talked about design guidelines, comprehensive plan elements for the development, and reviewed development criteria. Staff found they met the criteria for approval standards for a planned unit development, rezoning, and a special use.

Chairman Rickard asked for clarification that on a build-to-zone and that they are required to build within a certain distance of the sidewalk for a certain percentage of frontage. He said they are encouraging them to put the building along the sidewalk, which eliminates landscape space. He asked if there are any landscape requirements for a PUD like this or if it just based on landscape space available. Ms. Hepworth responded that the build-to-zone requires 80% of the primary façade to be within 0 to 10 feet of the property line, so they would encourage that build-to regulation to be met and any landscaping they can fit would be additional.

Chairman Rickard asked who was responsible for maintaining any improvements in the alley. Mr. Zawila answered that the Village is responsible for that.

Chairman Rickard asked if the building being 4 inches off the north property line was a building department and engineering issue. Mr. Zawila stated that would be reviewed during the permit review process.

Commissioner Frankovic asked what the distance was from the current structure on the north property to the property line. Mr. Kennedy stated it appears to be at least 10 feet.

Commissioner Boyle asked for clarification of a door on the north side of the building, particularly if it would house utilities. Mr. Kennedy responded yes.

Commissioner Boyle asked if there were no requirements for rear and side setbacks. Ms. Hepworth answered that was correct.

Commissioner Boyle asked for a clarification on the 70 feet building max height. Ms. Hepworth explained that they measure height from the established grade and take two spot elevations from the front corners of the building, and then take the average and measure from the top of the roof structure.

Chairman Rickard noted there are certain allowed extensions to go above that. Commissioner asked Ms. Hepworth for any thoughts on the seven floors versus six in terms of the design guidelines.

Ms. Hepworth deferred to the petitioner on why they chose that, but it does meet regulation. Commissioner Frankovic asked if there were any plans for the Village to add more public parking in the future. Mr. Zawila responded they are currently going through the comprehensive plan project which will look at the recommendations for the downtown area, which will review parking.

Chairman Rickard stated 1.4 parking ratio does meet the parking ordinance and other properties that have been built in the last few years have had less than that. He asked if there has been any known history of issues, problems, or complaints from lack of parking for residents in those buildings.

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Mr. Zawila answered that the only time they hear it is in meetings like this, but they do have an understanding that the buildings approved and constructed have sufficient parking.

Chairman Rickard said the standards they have are based on data and types of amenities in the neighborhood and is standard in other towns as well. Mr. Zawila expressed reduced parking is intentional for zoning districts close to the train station.

Commissioner Rutledge asked about concerns on flooding and how that was being addressed. Ms. Hepworth responded that any future development would have to adhere to the stormwater ordinance and not cause any extra burden or affects to neighboring properties. Mr. Zawila added that if approved, it will follow all building, fire, and stormwater ordinance codes.

Chairman Rickard asked for the petitioner to come back and address any issues or comments.

Ms. Butler invited the traffic engineer to come up and address traffic concerns. Javier Midlan, traffic engineer, expressed the traffic study followed all guidelines and the Village of Downers Grove requirements. He stated that due to the location of the site being in close proximity to the train station, it is anticipated that estimated traffic will be reduced and it was indicated that 21% of the Downers Grove population work from home, 9% use transit, and 2% either walk or drive to work. He said data translated to one additional vehicle every 8 minutes potentially with this development. He shared that the average peak parking demand for the development of this size is 1.23 spaces per dwelling unit, which translates into a parking demand of 76 spaces.

Commissioner Toth asked how tandem parking spots work. Mr. Midlan explained tandem parking sports and added they work here because there tends to be less traffic and parking demand near a train station for transit.

Commissioner Boyle asked if the intention for the development to assign parking for each tenant. Mr. Midlan responded that is typically how it works for apartment, by being assigned to one unit.

Commissioner Patel asked if the apartment will plan to assign parking spots by unit. Mr. Midlan explained that parking demand is typically controlled by the leasing company assigning spaces.

Ms. Butler asked the civil engineer to come to the podium to address questions related to stormwater. Ms. Welp discussed the stormwater approach the project is taking. She said they are required to follow the county and Village requirements and described the stormwater storage structure that will be built as a part of this development.

Ms. Butler reminded the Commission that the proposed development meets all the standards of the zoning ordinance in respect to height, parking, and setbacks, and the only deviation being requested is with the respect to residential density.

Ben Kennedy, project architect, discussed how they arrived at the proposed density and how the building will be constructed 4 inches off of the property line. He noted they are 4 inches at one end but 8.4 at the other end. He shared that when the finalized the engineering design of the building, they will determine how they have to support soil from their property to the other property, typically this is done with some type of shoring. In regards to the density, he stated that the average of the 62 units is about a 1040 square foot average overall and they wanted more family sized, larger units.

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Commissioner Toth asked about the shoring that would be between the property lines. Mr. Kennedy said they are usually about 4 to 6 inches in width and it will done in a way that it can be monitored to make sure there is no disturbances to the adjacent properties. J. P. Bartley, member of the ownership group, talked about how parking is allocated during the leasing process. He said the spots are going to be assigned on a tenant by tenant basis and the tenant will have the option to rent a space or two, and they will carefully monitor the parking count to make sure parking is available to every unit. He added that once it is fully leased up, they usually are left with spots leftover that could be used for guests or could be rented to other apartments that have higher demand, but is it first come, first serve.

Ms. Butler added that access is controlled to the parking facility so not anyone can just come in and park. She also said they received a number of letters in support for the proposed development and the Economic Development Commission is in support and a number of surrounding business owners and members of the community.

Chairman Rickard asked for discussion from the commissioners.

Chairman Rickard stated the building has the potential to be one of nicest buildings they have downtown and all design standards were met and likely exceeded and they are trying to increase density around downtown. He noted some people have issues with the parking, but the parking standards here have been met. He shared that he lives a couple blocks north of the proposed development and it does get busy with traffic during rush hours, but he knew that when he bought the property and that comes with living near a downtown area. Chairman Rickard expressed this is exactly what the comprehensive plan is telling developers and builders that we want to see in that area and the standards had been met.

Commissioner Toth expressed that he appreciated the traffic explanation and analysis and the parking explanation also made sense and was helpful. He stated it was a good project and was in support.

Chairman Rickard said a lot of residents are looking for smaller scale, more affordable housing, and these smaller units allowed younger people to get into town and rent, because currently there are not a whole lot of smaller units for younger professional types, singles, or empty nesters.

Commissioner Frankovic voiced the standard had been met for approval. She said the design was very well thought out and all of the main concerns they had were addressed and this would be a great asset to the area. She added a lot of the traffic issues can be addressed as part of the updated Comprehensive Plan. She was in support.

Commissioner Rutledge stated the standards have been met and exceeded in some cases. She appreciated the innovation of the design and is alignment with the plan in Downers Grove. She also liked the mixed dwellings and variety of room options. She was in support.

Chairman Rickard asked if anyone wanted to make a motion.

Commissioner Toth made the motion.

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WITH RESPECT TO FILE 24-PCE-0029 AND BASED ON THE PETITIONER'S SUBMITTAL, THE STAFF REPORT, AND THE TESTIMONY PRESENTED, IT IS FOUNDED THE PETITIONER HAS MET THE STANDARDS OF APPROVAL FOR THE ZONING MAP AMENDMENT, PLANNED UNIT DEVELOPMENT AMENDMENT AND SPECIAL USE AS REQUIRED BY THE VILLAGE OF DOWNERS GROVE ZONING ORDINANCE AND IS IN THE PUBLIC INTEREST, AND THEREFORE, COMMISSIONER TOTH MADE A MOTION THAT THE PLANNING AND ZONING COMMISSION RECOMMEND TO THE VILLAGE COUNCIL APPROVAL OF FILE 24-PCE-0029, SUBJECT TO THE CONDITIONS AS LISTED.

SECOND BY COMMISSIONER V. PATEL

ROLL CALL:

AYE: TOTH, V. PATEL, BOYLE, FRANKOVIC, K. PATEL, RUTLEDGE, CHAIRMAN RICKARD

NAY: NONE

MOTION APPROVED. VOTE: 7-0

/s/ Celeste K. Weilandt
Recording Secretary

(As transcribed by Ditto Transcripts)